Fort Bend County, Texas Invitation for Bid



Construction of Stella Road from Cottonwood School Road to W. Fairgrounds Road for Fort Bend County Mobility Bond Project No. 20116

BID 24-076

SUBMIT BIDS TO:

Fort Bend County Purchasing Department Travis Annex 301 Jackson, Suite 201 Richmond, TX 77469

Note: All correspondence must include the term "Purchasing Department" in address to assist in proper delivery

SUBMIT NO LATER THAN:

Tuesday, August 20, 2024 2:00 PM (Central)

LABEL ENVELOPE:

BID 24-076 Construction of Stella Road

ALL BIDS MUST BE RECEIVED IN AND TIME/DATE STAMPED BY THE PURCHASING OFFICE OF FORT BEND COUNTY ON OR BEFORE THE SPECIFIED TIME/DATE STATED ABOVE.

BIDS RECEIVED AS REQUIRED WILL THEN BE OPENED AND PUBLICLY READ.

BIDS RECEIVED AFTER THE SPECIFIED TIME, WILL BE RETURNED UNOPENED.

Results will not be given by phone. Results will be provided to bidder in writing after Commissioners Court award. Requests for information must be in writing and directed to: LeAnn Cernoch Senior Buyer

<u>LeAnn.Cernoch@fortbendcountytx.gov</u>

Vendor Responsibilities:

- Download and complete any addendums. (Addendums will be posted on the Fort Bend County website no Later than 48 hours prior to bid opening)
- > Submit response in accordance with requirements stated on the cover of this document.
- DO NOT submit responses via email or fax.

Prepared: 7/25/24 Issued: 7/28/24



COUNTY PURCHASING AGENT Fort Bend County, Texas

Vendor Information

Jaime Kovar
Purchasing Agent

Office (281) 341-8640

i dichasing rigent					,
Legal Company Name					
(top line of W9) Business Name					
(if different from legal name)					
(ir uniterent from regar name	Corporation/LLC		Partnership	Age in Bu	isiness?
Type of Business	Sole Proprietor/Inc	dividual	Tax Exempt		
Federal ID # or S.S. #			SAM.gov Unique Entity ID #		
SAM.gov CAGE / NCAGE					
Publicly Traded Business	No Yo	es Ticker Sy	mbol		
Remittance Address					
City/State/Zip					
Physical Address					
City/State/Zip					
Phone Number					
E-mail					
Contact Person					
Check all that apply to the company listed above and provide certification number.	DBE-Disadvantaged Business Enterp SBE-Small Business Enterprise HUB-Texas Historically Underutiliz WBE-Women's Business Enterprise	ed Business	Certification # Certification # Certification #		Exp Date
Company's gross annual	<\$500,000	\$500,000	0-\$4,999,999		•
receipts	\$5,000,000-\$16,999,999	\$17,000,	000-\$22,399,999	>\$22,400,0	00
NAICs codes (Please enter all that apply)					
Signature of Authorized Representative					
Printed Name					
Title					
Date					

1.0 GENERAL REQUIREMENTS:

- 1.1 Read this entire document carefully. Follow all instructions. You are responsible for fulfilling all requirements and specifications. Be sure you understand them.
- 1.2 General Requirements apply to all advertised bids; however, these may be superseded, whole or in part, by the scope, special requirements, specifications, special specifications or other data contained herein.
- 1.3 Governing Law: Bidder is advised that these requirements shall be fully governed by the laws of the State of Texas and that Fort Bend County may request and rely on advice, decisions and opinions of the Attorney General of Texas and the County Attorney concerning any portion of these requirements.
- 1.4 Bid Form Completion: Fill out, sign, and return to the Fort Bend County Purchasing Department one (1) complete bid form. An authorized representative of the bidder must sign the Contract Sheet. The Contract will be binding only when signed by the County Judge, Fort Bend County and a purchase order authorizing the item(s) desired has been issued. The use of corrective fluid is not acceptable and may result in the disqualification of bid. If an error is made, the bidder must draw a line through error and initial each change.
- 1.5 Bid Returns: Bidders must return all completed bids to the Fort Bend County Purchasing Department at 301 Jackson, Suite 201 Richmond Texas no later than 2:00 P.M. on the date specified. Late bids will not be accepted. Bids must be submitted in a sealed envelope, addressed as follows: Fort Bend County Purchasing Agent, Travis Annex, 301 Jackson, Suite 201 Richmond, Texas 77469.
- Addenda: No interpretation of the meaning of the drawings, specifications or 1.6 other bid documents will be made to any bidder orally. All requests for such interpretations must be made in writing addressed to LeAnn Cernoch, Senior Buyer, 301, Jackson, Suite 201, Richmond, Texas, 77469, E-mail: LeAnn.Cernoch@fortbendcountytx.gov. Any and all interpretations and any supplemental instructions will be in the form of written addenda to the contract documents which will be posted on Fort Bend County's website. Addenda will **ONLY** be issued by the Fort Bend County Purchasing Agent. It is the sole responsibility of each bidder to insure receipt of any and all addenda. addenda issued will become part of the contract documents. Bidders must sign and include it in the returned bid package. Deadline for submission of questions and/or clarification is no later than Tuesday, August 13, 2024 at 9:30AM (central) Requests received after the deadline will not be responded to due to the time constraints of this bid process.
- 1.7 References: All bidders must submit, **WITH BID**, at least three (3) references from clients for whom a project similar to that specified herein has been

- successfully accomplished. References must include clients name, contact person and telephone number.
- 1.8 Bid Bond: All bidders must submit, **WITH BID**, a cashier's check or certified check for at least five percent (5%) of the total bid price, payable to the order of Fort Bend County, or a Bid Bond in the same amount issued by a surety, acceptable to Fort Bend County, authorized to do business in the State of Texas, as a guarantee that the Bidder will do the work described herein at the rates stated herein. Unsuccessful bidder's Cashier's Check or Certified Check will be returned only after a written request to do so have been received in the Office of the Fort Bend County Purchasing Agent.
- 1.9 Material Safety Data Sheets: Under the "Hazardous Communication Act", commonly known as the "Texas Right to Know Act", a bidder must provide to Fort Bend County and using departments, with each delivery, material safety data sheets, which are, applicable to hazardous substances defined in the Act. Bidders are obligated to maintain a current, updated file in the Fort Bend County Purchasing Department. Failure of the bidder to maintain such a file will be cause to reject any bid applying thereto.
- 1.10 Pricing: Prices for all goods and/or services shall be firm for the duration of this Contract and shall be stated on the bid sheet. Prices shall be all inclusive. No price changes, additions, or subsequent qualifications will be honored during the course of the Contract. All prices must be written in ink or typewritten. If there are any additional charges of any kind, other than those mentioned above, specified or unspecified, bidder MUST indicate the items required and attendant costs or forfeit the right to payment for such items.
- 1.11 Term Contracts: If the Contract is intended to cover a specific time period, said time will be given in the specifications under scope.
- 1.12 Recycled Materials: Fort Bend County encourages the use of products made of recycled materials and shall give preference in purchasing to products made of recycled materials if the products meet applicable specifications as to quantity and quality. Fort Bend County will be the sole judge in determining product preference application.
- 1.13 Evaluation: Evaluation shall be used as a determinant as to which bid items or services are the most efficient and/or most economical for Fort Bend County. It shall be based on all factors which have a bearing on price and performance of the items in the user environment. All bids are subject to tabulation by the Fort Bend County Purchasing Department and recommendation to Fort Bend County Commissioners Court. Compliance with all bid requirements, delivery and needs of the using department are considerations in evaluating bids. Pricing is NOT the only criteria for making a recommendation. The Fort Bend County Purchasing Department reserves the right to contact any bidder, at any time, to clarify, verify or request information with regard to any bid.

- 1.14 Disqualification of Bidder: Upon signing this bid document, a bidder offering to sell supplies, materials, services, or equipment to Fort Bend County certifies that the bidder has not violated the antitrust laws of this state codified in section 15.01, et seq., Business & Commerce Code, or the federal antitrust laws, and has not communicated directly or indirectly the bid made to any competitor or any other person engaged in such line of business. Any or all bids may be rejected if Fort Bend County believes that collusion exists among the bidders. Bids in which the prices are obviously unbalanced may be rejected. If multiple bids are submitted by a bidder and after the bids are opened, one of the bids is withdrawn, the result will be that all of the bids submitted by that bidder will be withdrawn; however, nothing herein prohibits a vendor from submitting multiple bids for different products or services.
- 1.15 Awards: Fort Bend County reserves the right to award this Contract on the basis of lowest and best bid in accordance with the laws of the State of Texas, to waive any formality or irregularity, to make awards to more than one bidder, to reject any or all bids. In the event the lowest dollar bidder meeting specifications is not awarded a contract, the bidder may appear before the Commissioners Court and present evidence concerning its responsibility.
- 1.16 Contract Obligation: Fort Bend County Commissioners Court must award the Contract and the County Judge or other person authorized by the Fort Bend County Commissioners Court must sign the Contract before it becomes binding on Fort Bend County or the bidders. Department heads are not authorized to sign agreements for Fort Bend County. Binding agreements shall remain in effect until all products and/or services covered by this purchase have been satisfactorily delivered and accepted.

2.0 SCOPE:

It is the intent of Fort Bend County to contract with one (1) vendor for all materials, supplies, equipment, tools, services, labor and supervision necessary to complete the Construction of Stella Road from Cottonwood School Road to W. Fairgrounds Road, hereinafter referred to as the "Project," as specified herein.

2.1 Work means the procurement, delivery and proper construction and/or installation of all materials and facilities and associated appurtenances necessary to fulfill the winning bidder's obligations (hereinafter the "Contractor") under the Contract as awarded for the Project specified herein, including the coordination and administration of all services necessary for Contractor, and/or its agents and/or subcontractors, to fulfill Contractor's obligations under the Contract.

3.0 PRE-BID CONFERENCE:

A pre-bid conference will be conducted on Tuesday, August 6, 2024 at 9:30 AM (CST). The

pre-bid conference will be held at the Fort Bend County Purchasing Department located in the Travis Annex at 301 Jackson, Suite 201, Richmond, Texas 77469. All bidders are encouraged to attend.

4.0 LIQUIDATED DAMAGES:

The County and the Contractor recognize the delays, expense, and difficulties involved in proving in a legal or arbitration proceeding the actual loss suffered by the County if the work is not complete on time. Accordingly, instead of requiring any such proof, the County and the Contractor agree that as liquidated damages for delay (but not as a penalty) the Contractor shall pay the County \$1,500.00 for each day that expires after the time specified herein for completion until the Work is complete, unless contract time has been adjusted by extension of time approved by Commissioner's Court.

The Contractor will be placed on one (1) year probation if liquidated damages are accrued. During the probation period, if the Contractor accrues liquidated damages on another project, they will be disqualified from being awarded any County work for two (2) years.

5.0 COMPLETION TIME & PAYMENT:

- 5.1 Fort Bend County shall pay the Contractor in current funds for the Contractor's performance of the Contract the contract sum, as stated herein, after receipt of notice to proceed and a purchase order issued by the Fort Bend County Purchasing Agent.
- 5.2 Based upon Applications for payment submitted to the County Auditor, Fort Bend County shall make progress payments on account of the contract sum to the Contractor as provided below and elsewhere in the contract documents.
 - 5.2.1 The period covered by each application for payment shall be one calendar month ending on the last day of the month.
 - 5.2.2 Provided a customary, accurate and complete application for payment is received by the County Auditor not later than the 15th day of a month, Fort Bend County shall make payment of all undisputed amounts to the Contractor not later than the 15th day of the next month. If an application for payment is received by the County Auditor after the application deadline fixed above, payment shall be made by Fort Bend County not later than 30 days after the County Auditor receives the application for payment.
 - 5.2.3 Application for payment shall indicate the percentage of completion of each portion of the Project as of the end of the period covered by the application for payment.
 - 5.2.4 Subject to the provisions of the contract documents, the amount of each progress payment shall be computed as follows:

- 5.2.4.1 Take that portion of the contract sum properly allocable to completed Project less retainage of ten percent (10%).
- 5.2.4.2 Add that portion of the contract sum properly allocable to materials and equipment delivered and suitably stored at the site for subsequent incorporation in the completed construction (or, if approved by Fort Bend County, suitably stored off the site at a location agreed upon in writing), less retainage of ten percent (10%).
- 5.2.4.3 Subtract the aggregate of previous payments made by Fort Bend County.
- 5.2.4.4 The progress payment amount as determined in above shall be further modified under the following circumstances:

Upon substantial completion of the Project, add a sum sufficient to increase the total payments to one hundred percent (100%) of the contract sum, less such amounts as Fort Bend County shall determine should be deducted for incomplete work and unsettled claims.

- 5.2.4.5 Final payment, constituting the entire unpaid undisputed balance of the contract sum, shall be made by Fort Bend County to the Contractor when Fort Bend County and the Contractor agree that the Contract has been fully performed by the Contractor.
- 5.3 Before the first application for payment, the Contractor shall submit to the Engineering Department a schedule of values allocated to various portions of the work, prepared in such form and supported by such data to substantiate its accuracy as the Engineering Department may require. This schedule, unless objected to by the Engineering Department shall be used as a basis for reviewing the Contractor's application for payment.
- 5.4 Contractor must provide with each application for payment a contractor's affidavit certifying bills against the Contractor for labor, material and expendable equipment employed in the performance of Contractor have been paid in full prior to acceptance of final payment from Fort Bend County.
- 5.5 The Contractor will permit Fort Bend County, or any duly authorized agent of Fort Bend County, to inspect and examine the books and records of the Contractor for the purpose of verifying the amount of work performed under the Contract. Fort Bend County's right to inspect survives the termination of the Contract for a period of five years.

Initials of Bidder:	
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6.0 LIMIT OF APPROPRIATION:

Prior to the execution of this Contract, Contractor has been advised by County, and Contractor clearly understands and agrees, such understanding and agreement being of the absolute essence to this Contract, that County shall have available only those funds specifically allocated in this Contract to fully discharge any and all liabilities which may be incurred by County in bringing this Project to an absolute conclusion, resulting in a complete, fully furnished, fully equipped and fully usable facility, and that the total of any and all basic construction costs, costs of providing the required services and materials, all fees and compensation of any sort to the Contractor, and any and all costs for any and all things or purposes coming inuring under or out of this Contract, irrespective of the nature thereof, shall not exceed said specifically allocated sum, notwithstanding any word, statement or thing contained in or inferred from the preceding provision of this Contract which might in any light by any person be interpreted to the contrary.

7.0 RIGHT TO ASSURANCE:

Whenever Fort Bend County in good faith has reason to question the Contractor's intent or ability to perform, Fort Bend County may demand that the Contractor give written assurance of its intent to perform and its plan to properly continue performance, including a reasonably detailed timeline. In the event that a demand is made and no assurance is given within five (5) business days, Fort Bend County may treat this failure as an anticipatory repudiation of the Contract.

8.0 PERFORMANCE & PAYMENT BONDS:

Performance and Payment Bonds: In the event the total accepted bid price exceeds \$25,000 the Contractor must provide to the Office of the County Purchasing Agent, a performance bond and a payment bond, each in the amount of 100% of the total contract sum within ten (10) calendar days after receipt of notification of bid award. Such bonds shall be executed by a corporate surety duly authorized and admitted to do business in the State of Texas and licensed in the State of Texas to issue surety bonds with a Best Rating of "A" or better. Fort Bend County reserves the right to accept or reject any surety company proposed by the Contractor. In the event Fort Bend County rejects, the proposed surety company, the Contractor will be afforded five (5) additional days to submit the required bonds issued by a surety company acceptable to Fort Bend County.

9.0 POWER OF ATTORNEY:

An attorney-in-fact who signs a bid bond, performance bond or payment bond must file with each bond a certified and effectively dated copy of his or her power of attorney.

10.0 INSURANCE:

10.1 All respondents shall submit, with response, a <u>current</u> certificate of insurance indicating coverage in the amounts stated below. In lieu of submitting a certificate of insurance, respondents may submit, with response, a notarized

- statement from an Insurance company, authorized to conduct business in the State of Texas, and acceptable to Fort Bend County, guaranteeing the issuance of an insurance policy, with the coverage stated below, to the firm named therein, if successful, upon award of this Contract.
- 10.2 At contract execution, contractor shall furnish County with properly executed certificates of insurance which shall evidence all insurance required and provide that such insurance shall not be canceled, except on 30 days prior written notice to County. Contractor shall provide certified copies of insurance endorsements and/or policies if requested by County. Contractor shall maintain such insurance coverage from the time Services commence until Services are completed and provide replacement certificates, policies and/or endorsements for any such insurance expiring prior to completion of Services. Contractor shall obtain such insurance written on an Occurrence form (or a Claims Made form for Professional Liability insurance) from such companies having Best's rating of A/VII or better, licensed or approved to transact business in the State of Texas, and shall obtain such insurance of the following types and minimum limits:
 - 10.2.1 Workers' Compensation insurance. Substitutes to genuine Workers' Compensation Insurance will not be allowed.
 - 10.2.2 Employers' Liability insurance with limits of not less than \$1,000,000 per injury by accident, \$1,000,000 per injury by disease, and \$1,000,000 per bodily injury by disease.
 - 10.2.3 Commercial general liability insurance with a limit of not less than \$1,000,000 each occurrence and \$2,000,000 in the annual aggregate. Policy shall cover liability for bodily injury, personal injury, and property damage and products/completed operations arising out of the business operations of the policyholder.
 - 10.2.4 Business Automobile Liability coverage with a combined Bodily Injury/Property Damage limit of not less than \$1,000,000 each accident. The policy shall cover liability arising from the operation of licensed vehicles by policyholder.
- 10.3 County and the members of Commissioners Court shall be named as additional insured to all required coverage except for Workers' Compensation and Professional Liability (if required). All Liability policies including Workers' Compensation written on behalf of contractor, excluding Professional Liability, shall contain a waiver of subrogation in favor of County and members of Commissioners Court.
- 10.4 If required coverage is written on a claims-made basis, contractor warrants that any retroactive date applicable to coverage under the policy precedes the effective date of the contract; and that continuous coverage will be maintained or an

- extended discovery period will be exercised for a period of two (2) years beginning from the time that work under the agreement is completed.
- 10.5 Contractor shall not commence any portion of the work under this Contract until it has obtained the insurance required herein and certificates of such insurance have been filed with and approved by Fort Bend County.
- 10.6 No cancellation of or changes to the certificates, or the policies, may be made without sixty (60) days prior, written notification to Fort Bend County.
- 10.7 Approval of the insurance by Fort Bend County shall not relieve or decrease the liability of the Contractor.

11.0 INDEMNIFICATION:

Respondent shall save harmless County from and against all claims, liability, and expenses, including reasonable attorney's fees, arising from activities of respondent, its agents, servants or employees, performed under this agreement that result from the negligent act, error, or omission of respondent or any of respondent's agents, servants or employees.

- 11.1 Respondent shall timely report all such matters to Fort Bend County and shall, upon the receipt of any such claim, demand, suit, action, proceeding, lien or judgment, not later than the fifteenth day of each month; provide Fort Bend County with a written report on each such matter, setting forth the status of each matter, the schedule or planned proceedings with respect to each matter and the cooperation or assistance, if any, of Fort Bend County required by Respondent in the defense of each matter.
- 11.2 Respondent's duty to defend, indemnify and hold Fort Bend County harmless shall be absolute. It shall not abate or end by reason of the expiration or termination of any contract unless otherwise agreed by Fort Bend County in writing. The provisions of this section shall survive the termination of the contract and shall remain in full force and effect with respect to all such matters no matter when they arise.
- In the event of any dispute between the parties as to whether a claim, demand, suit, action, proceeding, lien or judgment appears to have been caused by or appears to have arisen out of or in connection with acts or omissions of Respondent, Respondent shall never-the-less fully defend such claim, demand, suit, action, proceeding, lien or judgment until and unless there is a determination by a court of competent jurisdiction that the acts and omissions of Respondent are not at issue in the matter.
- 11.4 Respondent's indemnification shall cover, and Respondent agrees to indemnify Fort Bend County, in the event Fort Bend County is found to have been negligent for having selected Respondent to perform the work described in this request.

Initials of Bidder:	
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- 11.5 The provision by Respondent of insurance shall not limit the liability of Respondent under an agreement.
- 11.6 Respondent shall cause all trade contractors and any other contractor who may have a contract to perform construction or installation work in the area where work will be performed under this request, to agree to indemnify Fort Bend County and to hold it harmless from all claims for bodily injury and property damage that may arise from said Respondent's operations. Such provisions shall be in form satisfactory to Fort Bend County.
- 11.7 Loss Deduction Clause Fort Bend County shall be exempt from, and in no way liable for, any sums of money which may represent a deductible in any insurance policy. The payment of deductibles shall be the sole responsibility of Respondent and/or trade contractor providing such insurance.

12.0 PREVAILING WAGES:

This project is subject to the prevailing wage rate requirements of Chapter 2258 of the Government Code. All persons employed by Contractor shall be compensated at not less than the rates shown below. Contractor shall keep detailed records of each of its workers and said records shall be made available to County for inspection at all reasonable times. The Contractor shall pay Fort Bend County sixty dollars (\$60.00) for each worker employed by the Contractor for the provision of services described herein for each calendar day or part of the day that the worker is paid less than the below stated rates. Contractors may also visit www.wdol.gov/dba.aspx.

General Decision Number: TX20240038 01/05/2024 Superseded General Decision Number: TX20230038

State: Texas

Construction Type: Highway

Counties: Austin, Brazoria, Chambers, Fort Bend, Galveston, Hardin, Harris, Jefferson, Liberty, Montgomery, Orange, San Jacinto and Waller Counties in Texas.

HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects & railroad construction; bascule, suspension & spandrel arch bridges designed for commercial navigation, bridges involving marine construction; and other major bridges).

Note: Contracts subject to the Davis-Bacon Act are generally required to pay at least the applicable minimum wage rate required under Executive Order 14026 or Executive Order 13658. Please note that these Executive Orders apply to covered contracts entered into by the federal government that are subject to the Davis-Bacon Act itself, but do not apply to contracts subject only to the Davis-Bacon Related Acts, including those set forth at 29 CFR 5.1(a)(2)-(60).

If the contract is entered into on or after January 30, 2022, or the contract is renewed or extended (e.g., an option is exercised) on or after January 30, 2022, Executive Order 14026 generally

applies to the contract. The contractor must pay all covered workers at least \$17.20 per hour (or the applicable wage rate listed on this wage determination, if it is higher) for all hours spent performing on that contract in 2024.

If the contract was awarded on or between January 1, 2015 and January 29, 2022, and the contract is not renewed or extended on or after January 30, 2022, Executive Order 13658 generally applies to the contract. The contractor must pay all covered workers at least \$12.90 per hour (or the applicable wage rate listed on this wage determination, if it is higher) for all hours spent performing on that contract in 2024.

The applicable Executive Order minimum wage rate will be adjusted annually. If this contract is covered by one of the Executive Orders and a classification considered necessary for performance of work on the contract does not appear on this wage determination, the contractor must still submit a conformance request.

Additional information on contractor requirements and worker protections under the Executive Orders is available at www.dol.gov/whd/govcontracts.

Modification Number Publication Date 0 01/05/2024

POWER EQUIPMENT OPERATOR:

SUTX2011-013 08/10/2011

	Rates	Fringes
CEMENT MASON/CONCRETE FINISHER (Paving and		
Structures)	\$ 12.98 **	
ELECTRICIAN	\$ 27.11	
FORM BUILDER/FORM SETTER		
Paving & Curb	\$ 12.34 **	:
Structures	\$ 12.23 **	:
LABORER		
Asphalt Raker	\$ 12.36 **	:
Flagger	\$ 10.33 **	
Laborer, Common	\$ 11.02 **	
Laborer, Utility	\$ 11.73 **	
Pipelayer	\$ 12.12 **	
Work Zone Barricade Servicer	\$ 11.67 **	
PAINTER (Structures)	\$ 18.62	

Asphalt Distributor Asphalt Paving Machine Broom or Sweeper Concrete Pavement Finishing Machine Concrete Pavement Finishing Machine Concrete Paving, Curing, Float, Texturing Machine Concrete Saw Crane, Hydraulic 80 Tons or less Crane, Lattice boom 80 tons or less Crane, Lattice boom over 80 Tons Crawler Tractor Excavator, 50,000 pounds or less Excavator, 50,000 pounds Foundation Drill, Crawler Mounted Foundation Drill, Crawler Mounted Front End Loader 3 CY or Less Front End Loader, Over 3 CY Loader/Backhoe Mechanic Milling Machine Motor Grader, Fine Grade Motor Grader, Rough Off Road Hauler Pavement Marking Machine Piledriver Roller, Asphalt Roller, Other Scraper Spreader Box Servicer Structural Steel TRUCK DRIVER Low Boy Float Single Axle Single or Tandem Axle Dump Tradem Axle Tractor w/Semi Trailer Structural Steel Structural Steel Stractor w/Semi Trailer		
Broom or Sweeper	Asphalt Distributor	
Concrete Paving, Curing, Float, Texturing Machine \$ 13.07 ** Concrete Paving, Curing, Float, Texturing Machine \$ 11.71 ** Concrete Saw \$ 13.99 ** Crane, Hydraulic 80 Tons or less \$ 14.97 ** Crane, Lattice boom 80 tons or less \$ 14.97 ** Crane, Lattice boom over 80 Tons \$ 15.80 ** Crawler Tractor \$ 13.68 ** Excavator, 50,000 pounds or less \$ 12.71 ** Excavator, Over 50,000 pounds \$ 14.53 ** Foundation Drill, Crawler Mounted \$ 17.43 Foundation Drill, Crawler Mounted \$ 15.89 ** Front End Loader 3 CY or Less \$ 13.32 ** Front End Loader, Over 3 CY \$ 13.17 ** Loader/Backhoe \$ 14.29 ** Mechanic \$ 16.96 ** Motor Grader, Fine Grade \$ 15.69 ** Motor Grader, Fine Grade \$ 15.69 ** Motor Grader, Rough \$ 14.23 ** Off Road Hauler \$ 14.60 ** Pavement Marking Machine \$ 11.18 ** Piledriver \$ 14.95 ** Roller, Asphalt \$ 11.57 ** Scraper \$ 13.47 **<	Asphalt Paving Machine	
Concrete Paving, Curing, Float, Texturing Machine \$ 11.71 ** Concrete Saw \$ 13.99 ** Crane, Hydraulic 80 Tons or less \$ 13.86 ** Crane, Lattice boom 80 tons or less \$ 14.97 ** Crane, Lattice boom over 80 Tons \$ 15.80 ** Crawler Tractor \$ 13.68 ** Excavator, 50,000 pounds or less \$ 12.71 ** Excavator, Over 50,000 pounds \$ 14.53 ** Foundation Drill, Crawler Mounted \$ 17.43 Foundation Drill, Truck Mounted \$ 15.89 ** Front End Loader 3 CY or Less \$ 13.32 ** Front End Loader, Over 3 CY \$ 13.17 ** Loader/Backhoe \$ 14.29 ** Mechanic \$ 16.96 ** Milling Machine \$ 15.69 ** Motor Grader, Fine Grade \$ 15.69 ** Motor Grader, Rough \$ 14.23 ** Off Road Hauler \$ 14.60 ** Pavement Marking Machine \$ 11.18 ** Piledriver \$ 14.95 ** Roller, Other \$ 11.57 ** Scraper \$ 13.47 ** Spreader Box \$ 13.58 ** Servicer		
Concrete Saw Crane, Hydraulic 80 Tons or less Crane, Lattice boom 80 tons or less Crane, Lattice boom 80 tons or less Crane, Lattice boom over 80 Tons Crawler Tractor Excavator, 50,000 pounds or less Excavator, Over 50,000 pounds Foundation Drill, Crawler Mounted Foundation Drill, Truck Mounted Front End Loader 3 CY or Less Front End Loader, Over 3 CY Loader/Backhoe Mechanic Milling Machine Motor Grader, Fine Grade Motor Grader, Rough Off Road Hauler Pavement Marking Machine Piledriver Roller, Asphalt Roller, Other Scraper Spreader Box Servicer Steel Worker Reinforcing Steel Structural Steel Welder Structural Steel TRUCK DRIVER Low Boy Float Single Axle Single or Tandem Axle Dump \$ 11.48 **	Concrete Pavement Finishing Machine	
Crane, Hydraulic 80 Tons or less \$ 13.86 ** Crane, Lattice boom 80 tons or less \$ 14.97 ** Crane, Lattice boom over 80 Tons \$ 15.80 ** Crawler Tractor \$ 13.68 ** Excavator, 50,000 pounds or less \$ 12.71 ** Excavator, Over 50,000 pounds \$ 14.53 ** Foundation Drill, Crawler Mounted \$ 17.43 Foundation Drill, Truck Mounted \$ 15.89 ** Front End Loader 3 CY or Less \$ 13.32 ** Front End Loader, Over 3 CY \$ 13.17 ** Loader/Backhoe \$ 14.29 ** Mechanic \$ 16.96 ** Motor Grader, Fine Grade \$ 15.69 ** Motor Grader, Fine Grade \$ 15.69 ** Motor Grader, Rough \$ 14.23 ** Off Road Hauler \$ 14.60 ** Pavement Marking Machine \$ 11.18 ** Piledriver \$ 14.95 ** Roller, Asphalt \$ 11.57 ** Scraper \$ 13.47 ** Spreader Box \$ 13.47 ** Servicer \$ 13.97 ** Steel Worker \$ 12.85 ** Reinforcing Steel \$ 15.15 ** Structural Steel \$ 14.39 **	Concrete Paving, Curing, Float, Texturing Machine	\$ 11.71 **
Crane, Lattice boom 80 tons or less \$ 14.97 ** Crane, Lattice boom over 80 Tons \$ 15.80 ** Crawler Tractor \$ 13.68 ** Excavator, 50,000 pounds or less \$ 12.71 ** Excavator, Over 50,000 pounds \$ 14.53 ** Foundation Drill, Crawler Mounted \$ 17.43 Foundation Drill, Truck Mounted \$ 15.89 ** Front End Loader 3 CY or Less \$ 13.32 ** Front End Loader, Over 3 CY \$ 13.17 ** Loader/Backhoe \$ 14.29 ** Mechanic \$ 16.96 ** Milling Machine \$ 13.53 ** Motor Grader, Fine Grade \$ 15.69 ** Motor Grader, Rough \$ 14.23 ** Off Road Hauler \$ 14.60 ** Pavement Marking Machine \$ 11.18 ** Piledriver \$ 14.95 ** Roller, Asphalt \$ 11.95 ** Roller, Other \$ 11.57 ** Scraper \$ 13.47 ** Spreader Box \$ 13.58 ** Servicer \$ 13.97 ** Steel Worker \$ 12.85 ** Structural Steel \$ 14.39 ** TRUCK DRIVER \$ 14.00 ** Low Bo	Concrete Saw	\$ 13.99 **
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Single or Tandem Axle Dump \$ 11.48 **		
	•	
	Tandem Axle Tractor w/Semi Trailer	\$ 12.27 **

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

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** Workers in this classification may be entitled to a higher minimum wage under Executive Order 14026 (\$17.20) or 13658 (\$12.90). Please see the Note at the top of the wage determination for more information. Please also note that the minimum wage requirements of Executive Order 14026 are not currently being enforced as to any contract or subcontract to which the states of Texas, Louisiana, or Mississippi, including their agencies, are a party.

Note: Executive Order (EO) 13706, Establishing Paid Sick Leave for Federal Contractors applies to all contracts subject to the Davis-Bacon Act for which the contract is awarded (and any solicitation was issued) on or after January 1, 2017. If this contract is covered by the EO, the contractor must provide employees with 1 hour of paid sick leave for every 30 hours they work, up to 56 hours of paid sick leave each year. Employees must be permitted to use paid sick leave for their own illness, injury or other health-related needs, including preventive care; to assist a family member (or person who is like family to the employee) who is ill, injured, or has other health-related needs, including preventive care; or for reasons resulting from, or to assist a family member (or person who is like family to the employee) who is a victim of, domestic violence, sexual assault, or stalking. Additional information on contractor requirements and worker protections under the EO is available at www.dol.gov/whd/govcontracts.

Unlisted classifications needed for work not included within the scope of the classifications listed may be added after award only as provided in the labor standards contract clauses (29CFR 5.5 (a) (1) (iii)).

The body of each wage determination lists the classification and wage rates that have been found to be prevailing for the cited type(s) of construction in the area covered by the wage determination. The classifications are listed in alphabetical order of "identifiers" that indicate whether the particular rate is a union rate (current union negotiated rate for local), a survey rate (weighted average rate) or a union average rate (weighted union average rate).

Union Rate Identifiers

A four letter classification abbreviation identifier enclosed in dotted lines beginning with characters other than "SU" or "UAVG" denotes that the union classification and rate were prevailing for that classification in the survey. Example: PLUM0198-005 07/01/2014. PLUM is an abbreviation identifier of the union which prevailed in the survey for this classification, which in this example would be Plumbers. 0198 indicates the local union number or district council number where applicable, i.e., Plumbers Local 0198. The next number, 005 in the example, is an internal number used in processing the wage determination. 07/01/2014 is the effective date of the most current negotiated rate, which in this example is July 1, 2014.

Union prevailing wage rates are updated to reflect all rate changes in the collective bargaining agreement (CBA) governing this classification and rate.

Survey Rate Identifiers

Classifications listed under the "SU" identifier indicate that no one rate prevailed for this classification in the survey and the published rate is derived by computing a weighted average

rate based on all the rates reported in the survey for that classification. As this weighted average rate includes all rates reported in the survey, it may include both union and non-union rates. Example: SULA2012-007 5/13/2014. SU indicates the rates are survey rates based on a weighted average calculation of rates and are not majority rates. LA indicates the State of Louisiana. 2012 is the year of survey on which these classifications and rates are based. The next number, 007 in the example, is an internal number used in producing the wage determination. 5/13/2014 indicates the survey completion date for the classifications and rates under that identifier.

Survey wage rates are not updated and remain in effect until a new survey is conducted.

Union Average Rate Identifiers

Classification(s) listed under the UAVG identifier indicate that no single majority rate prevailed for those classifications; however, 100% of the data reported for the classifications was union data. EXAMPLE: UAVG-OH-0010 08/29/2014. UAVG indicates that the rate is a weighted union average rate. OH indicates the state. The next number, 0010 in the example, is an internal number used in producing the wage determination. 08/29/2014 indicates the survey completion date for the classifications and rates under that identifier.

A UAVG rate will be updated once a year, usually in January of each year, to reflect a weighted average of the current negotiated/CBA rate of the union locals from which the rate is based.

WAGE DETERMINATION APPEALS PROCESS

- 1.) Has there been an initial decision in the matter? This can be:
- * an existing published wage determination
- * a survey underlying a wage determination
- * a Wage and Hour Division letter setting forth a position on a wage determination matter
- * a conformance (additional classification and rate) ruling

On survey related matters, initial contact, including requests for summaries of surveys, should be with the Wage and Hour Regional Office for the area in which the survey was conducted because those Regional Offices have responsibility for the Davis-Bacon survey program. If the response from this initial contact is not satisfactory, then the process described in 2.) and 3.) should be followed.

With regard to any other matter not yet ripe for the formal process described here, initial contact should be with the Branch of Construction Wage Determinations. Write to:

Branch of Construction Wage Determinations Wage and Hour Division U.S. Department of Labor 200 Constitution Avenue, N.W. Washington, DC 20210

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2.) If the answer to the question in 1.) is yes, then an interested party (those affected by the action) can request review and reconsideration from the Wage and Hour Administrator (See 29 CFR Part 1.8 and 29 CFR Part 7). Write to:

Wage and Hour Administrator U.S. Department of Labor 200 Constitution Avenue, N.W. Washington, DC 20210

The request should be accompanied by a full statement of the interested party's position and by any information (wage payment data, project description, area practice material, etc.) that the requestor considers relevant to the issue.

3.) If the decision of the Administrator is not favorable, an interested party may appeal directly to the Administrative Review Board (formerly the Wage Appeals Board). Write to:

Administrative Review Board U.S. Department of Labor 200 Constitution Avenue, N.W. Washington, DC 20210

4.) All decisions by the Administrative Review Board are final.

13.0 PERMITS:

It shall be the sole responsibility of the successful bidder to obtain all required permits in the name of Fort Bend County.

14.0 CONTRACTOR'S RESPONSIBILITY FOR WORK:

- 14.1 <u>Preconstruction Work</u>. Contractor shall do (or cause to be done) the following as preconstruction work:
 - 14.1.1 On written demand as requested by Fort Bend County, cause the Contractor's personnel to meet with Fort Bend County and the Engineer to discuss the status of the Project.
 - 14.1.2 On written demand as requested by Fort Bend County, review drawings and specifications with the Engineer to permit the Contractor and the Engineer to determine the compliance of the proposed facility with applicable building codes.
- 14.2 <u>Construction Work</u>. Contractor shall do (or cause to be done) the following as construction work:
 - 14.2.1 Perform (or cause to be performed) all preparatory work at the construction site required herein, including (without limitation) soil and

- concrete testing and demolition of improvements existing at the construction site and all actions necessary for compliance with all laws and regulations as to actions to be taken by owners or contractors before construction begins, including without limitation those in regard to archaeological and environmental requirements.
- 14.2.2 Construct and install (or cause to be constructed and installed) the Project on the construction site in accordance with this Contract and the drawings and specifications approved by Fort Bend County.
- 14.2.3 Furnish (or cause to be furnished) all materials, supplies, equipment, tools, labor, supervision, utilities, transportation, and other materials and services necessary to complete the Project described herein.
- 14.2.4 Materials testing necessary for the Project and required by laws and regulations, construction industry standards as approved by Fort Bend County and this Contract; the frequency of testing shall be approved by Fort Bend County. It is the contractor's responsibility to engage a material testing laboratory to perform testing on the structural concrete to be used for foundation work in this project. The cost of testing shall be incidental to bid item for drill shaft foundation. Testing of concrete shall comply with current TXDOT criteria. Contractor has to submit the name of the testing laboratory, intended to be used by the contractor for this project, for County's approval.
- Standards for Review and Approval. Fort Bend County acknowledges that in 14.3 order to meet the deadlines for the completion of the Project, and in order to accomplish the efficient completion of the Project, the Contractor may submit matters to Fort Bend County in stages for approval or consent. Upon receipt of any matter submitted by the Contractor for review and approval, Fort Bend County shall review the same and shall diligently and promptly (but in any event within 14 calendar days for any such matter, other than a proposed change order, and within 28 calendar days for a proposed change order) give the Contractor notice of Fort Bend County's approval or disapproval, setting forth in detail all reasons for any disapproval. Fort Bend County's right to disapprove any such matter submitted (other than a proposed change order) shall be limited to the elements thereof (a) which do not conform substantially to matters previously approved, (b) which are new elements not previously presented and approved and the Contractor is unable to demonstrate that such new element is reasonably necessary for completion of the Project, or (c) which depict matters that are violations of this Contract or applicable laws and regulations.
 - 14.3.1 If Fort Bend County disapproves of a particular matter or Proposed Change Order, the Contractor shall have the right to resubmit such matter or Proposed Change Order to Fort Bend County, altered to satisfy Fort Bend County's basis for disapproval. Any resubmission shall be subject to review and approval by Fort Bend County.

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- 14.3.2 Fort Bend County and the Contractor shall attempt in good faith to resolve any disputes concerning the approval of any aspect of the Project expeditiously, so as not to delay the completion of the Project in accordance with this Contract.
- 14.3.3 Expedited Approvals. Fort Bend County recognizes the importance of expeditious action upon all matters submitted to Fort Bend County for review and approval and of expeditious response to those aspects of the Project requiring approval by governmental authorities having jurisdiction there over. Fort Bend County agrees to exercise its rights of review and approval hereunder with due diligence, reasonableness, and good faith. Fort Bend County shall use its reasonable efforts to expedite any required review of the Project or other matters by any governmental authority.

14.4 Changes.

- 14.4.1 General. Fort Bend County may make changes to the Project by altering, adding to, or deducting from the Project. All changes in the Project which (a) require an adjustment in the contract sum or an adjustment in the final completion date or (b) involve a material change in the overall scope or function of the Project shall be requested and authorized before commencing such changes by use of written change order notices, Proposed Change Orders and Change Orders, which change order procedure shall be the exclusive means to effect such changes in the Project.
- 14.4.2 Change Order Procedure. If at any time Fort Bend County desires to make any change in the Project requiring the issuance of a Change Order, Fort Bend County shall so advise the Contractor in writing by delivery to the Contractor of a written notice describing the change. Upon receipt of such notice initiated by Fort Bend County, the Contractor shall within a reasonable period of time advise Fort Bend County of the Contractor's proposal for the adjustments, if any, in the contract sum, the schedule of values, and the final completion date attributable to such change by delivering a written notice thereof (the "Proposed Change Order") to Fort Bend County. Such Proposed Change Order shall contain a description of the proposed change and shall set forth the Contractor's estimate of the increase or decrease, if any, in the contract sum and the change, if any, in the schedule of values and the final completion date attributable to such change. If the Contractor desires to make a change in the Project requiring the issuance of a change order, the Contractor shall deliver to Fort Bend County a Proposed Change Order. Upon execution by Fort Bend County, a Proposed Change Order shall constitute (and be defined herein as) a "Change Order" for purposes of this Contract. The Contractor shall forthwith perform the work as changed in accordance with such Change Order. All work performed pursuant to a Change Order shall be performed in accordance with the terms of this Contract. All Proposed Change Orders

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shall be submitted for approval by Fort Bend County. No action, acquiescence or inaction by Fort Bend County or any representative of Fort Bend County shall be construed to be a waiver of requirements set forth in this Contract in regard to Change Orders or ratification of a violation of such requirements, and all acts in violation of this provision shall be considered void.

- 14.4.3 <u>Change Order Authorization</u>. Each Change Order shall be signed by Fort Bend County and an authorized representative of the Contractor.
- 14.4.4 Contract Sum Adjustments. The contract sum and the schedule of values shall be adjusted only as a result of a Change Order requiring such adjustment. Any extra work performed without a proper Change Order shall be considered voluntary and not subject to additional compensation. The Contractor shall not be entitled to an adjustment in the contract sum (or a Change Order permitting such adjustment) or to damages as a result of any delays in the Project caused by the acts or omissions of Fort Bend County, provided that this sentence is not applicable to delays that constitute more than 90 days in any 365-day period or cause the Project to be interrupted for a continuous period of 45 days through no fault of the Contractor.
- 14.4.5 When Fort Bend County and the Contractor agree upon the adjustments in the contract sum, the schedule of values, and the final completion date attributable to such adjustment, such agreement will be documented by preparation and if approved by the Fort Bend County Commissioners Court, execution of an appropriate Change Order.
- 14.5 <u>Site Access</u>. Prior to the transfer date, Fort Bend County and the Contractor shall have uninterrupted access to the construction site. Subsequent to the transfer date, Fort Bend County will permit the Contractor, the Engineer, and their representatives and subcontractors to enter upon the Project at times reasonably necessary to complete the punch list items.
- 14.6 <u>Applicable Laws and Regulations</u>. Contractor shall in its performance of the Project comply with all applicable laws and regulations. Any delays in the prosecution of the Project caused by any changes in the laws and regulations or the application or enforcement of the laws and regulations may entitle the Contractor to an extension of time.
- 14.7 <u>Familiarity with Project</u>. The Contractor represents and accepts that it has: (a) visited the property(ies), (b) taken such other steps as may be necessary to ascertain the nature and location of the Project and the general and local conditions which affect the Project or the cost thereof, (c) investigated the labor situation as regards to the Project, (d) examined the property(ies), the obstacles which may be encountered and all other observable conditions having a bearing upon the performance of the Project, the superintendence of the Project, the time of completion and all other relevant matters, and (e) reported to Fort Bend County

- the results of all of the foregoing. The Contractor represents that it is familiar with all phases of the Project and the matters that may affect the Project or its prosecution under this Contract.
- 14.8 <u>Standard of Performance</u>. The Contractor shall prosecute (or cause to be prosecuted) the Project in accordance with the best efforts for the construction and development of projects similar to the Project in the State of Texas, using qualified, careful, and efficient contractors and workers and in conformity with the provisions of this Contract. The Contractor shall perform the work in a good and workmanlike manner.
- 14.9 Warranty of Contractor. The Contractor warrants to Fort Bend County that: (i) the Contractor possesses the skill and knowledge ordinarily possessed by wellinformed members of its trade or profession and the Contractor will use its best efforts to ensure that the services provided under this Contract will be performed, delivered, and conducted in accordance with the best professional standards and in accordance with industry standards, and (ii) the Contractor is fully experienced and properly qualified to perform the class of work provided for herein, and that it is properly equipped, organized and financed to perform such work, and (iii) following the date of acceptance of this Contract, the services provided by the Contractor to Fort Bend County will conform to the representations contained in this Contract, including all attachments, schedules and exhibits. All warranties provided by the Contractor in this Contract shall be cumulative, shall be deemed consistent and not in conflict, are intended to be given full force and effect and to be interpreted expansively to give the broadest warranty protection to Fort Bend County.
- 14.10 Contractor's Personnel. Contractor shall employ only competent, skilled personnel for the Project. Prior to the final completion date, the Contractor shall maintain a superintendent who shall be authorized to act on behalf of the Contractor and with whom Fort Bend County may consult at all reasonable times. The superintendent shall not be transferred from the Project without Fort Bend County's consent (which shall not be unreasonably withheld or delayed); provided, however, the superintendent shall not be assigned solely to the Project and shall be entitled to spend reasonable time working on matters unrelated to the Project so long as such work on other matters does not render the superintendent unavailable to the Project or unavailable to Fort Bend County. However, such obligation to furnish the superintendent and such staff personnel shall not be construed (a) to preclude the promotion within the Contractor's organization of any person assigned to the Project or (b) to give rise to any liability of the Contractor if any person assigned to the Project (including, without limitation, the superintendent) leaves the Contractor's employment. If the superintendent is transferred from the Project, Fort Bend County shall have the right to approve the replacement superintendent (which approval will not be unreasonably withheld or delayed). The Contractor, the Architect, and the other subcontractors shall comply with all applicable health, safety, and loss prevention rules of applicable governmental authorities. The

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Contractor shall, at its own expense, remove from the Project any person who fails to comply with such rules and instructions. The Contractor shall at all times enforce strict discipline and good order among its employees and shall not employ on the Project any unfit person or anyone not skilled in the work assigned to him. Fort Bend County may, upon written notice to the Contractor, require the Contractor to remove an individual immediately from providing services for the following reasons: violation of the terms and conditions of this Contract; violation of Fort Bend County's or the Contractor's work rules and regulations; criminal activity; or violation of state, federal, or municipal statutes. Fort Bend County may, upon thirty (30) days written notice to the Contractor, require the removal of any individual from providing services without cause.

- 14.11 <u>Inspection</u>. The Project and all parts thereof shall be subject to inspection from time to time by inspectors designated by Fort Bend County. No such inspections shall relieve The Contractor of any of its obligations hereunder. Neither failure to inspect nor failure to discover or reject any of the work as not in accordance with the drawings and specifications or any provision of this Contract shall be construed to imply an acceptance of such work or to relieve the Contractor of any of its obligations hereunder. Fort Bend County agrees that its right of inspection shall be used reasonably and in a timely manner so as not to delay orderly completion of the Project.
- 14.12 Protection Against Risks. The Contractor shall take all precautions which are necessary and adequate, against conditions created during the progress of the Project which involve a risk of bodily harm to persons or a risk of damage or loss to any property. The Contractor shall regularly inspect all work, materials and equipment to discover and determine any such conditions and shall be responsible for discovery, determination, and correction of any such conditions. The Contractor shall comply with all federal, state, and local occupational hazard and safety standards, codes and regulations applicable in the jurisdiction where the Project is being performed. The Contractor shall include the substance of this clause in its entirety in all subcontracts for any work to be performed at the construction site.
- 14.13 Equipment. Except as expressly provided herein to the contrary, the Contractor shall furnish (or cause to be furnished) all construction, transportation, installation, tools, and other equipment and facilities required for the performance of the Project within the times specified herein. Such equipment and facilities shall be serviceable and kept fit for the uses intended. Defective items shall be removed from the construction site promptly and at the Contractor's cost. The Contractor shall schedule (or cause to be scheduled) its other operations so as to not interfere with its duty to timely furnish the necessary equipment and facilities and personnel to operate the same at the times necessary for the orderly completion of the Project.

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- 14.14 <u>Materials</u>. Except as may be specifically provided otherwise in the Contract or approved in advance by Fort Bend County, the Contractor shall provide Fort Bend County with copies of material testing reports and to cause all materials, equipment, and fabricated items incorporated in the Project to be new and of a suitable grade of their respective kinds for their intended use.
- 14.15 <u>Delay, Disruption or Hindrance Damages</u>. Contractor and the County contemplate that Contractor's performance may be delayed, disrupted or interfered with by unanticipated causes including but not limited to the following:
 - a) Severe and unavoidable natural disasters such as fires, floods, epidemics and earthquakes;
 - b) Abnormal weather conditions;
 - c) Acts or failures to act of the County, third party utility owners or other third party entities; and
 - d) Acts of war or terrorism.

Contractor and the County agree and stipulate that an extension of the Contract Time shall be the sole remedy of Contractor for delays in performance of the Work, whether or not such delays are foreseeable, except for delays caused solely by acts of the County that constitute fraud, intentional misrepresentation, gross negligence, intentional arbitrary or capricious acts and/or omissions or intentional interference with Contractor's performance of the Work and then only to the extent such acts continue after Contractor notifies Owner in writing of such conduct. For delays caused by any act(s) other than fraud, intentional misrepresentation, gross negligence, intentional arbitrary or capricious acts and/or omissions or intentional interference with Contractor's performance of the Work Contractor shall not be entitled to any compensation or recovery of any damages including, without limitation, those damages prohibited or limited in Sections 14.15.1 – 14.15.8 below. The County's exercise of any of its rights or remedies under the Contract including, without limitation, ordering changes in the Work or directing suspension, rescheduling, or correction of the Work, in response to any breach or failure by the Contractor to comply with the terms of the Contract Documents or the Contractor's obligations arising therefrom, shall not be construed as intentional interference with Contractor's performance of the Work regardless of the extent or frequency of the County's exercise of such rights or remedies.

Without limiting the foregoing, except as otherwise expressly provided in this Agreement in calculating the amount of any claim recoverable by Contractor, the following limitations on the recovery of damages shall apply:

- 14.15.1 No indirect or consequential damages will be allowed.
- 14.15.2 No recovery shall be based on a comparison of planned expenditures to

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- total actual expenditures, or on estimated losses of labor efficiency, or on a comparison of planned manloading to actual manloading, or any other analysis that is used to show damages indirectly.
- 14.15.3 Damages, to the extent recoverable, are limited to the additional, actual costs specifically shown to have been directly incurred by the Contractor and solely caused by the proven wrong.
- 14.15.4 No damages will be allowed for home office overhead or other home office charges.
- 14.15.5 No exemplary damages or unjust enrichment damages shall be recoverable.
- 14.15.6 No recovery of attorney's fees shall be recoverable except as expressly permitted under the Agreement.
- 14.15.7 No profit will be allowed on any damage claim, except as expressly recoverable under the Agreement as Fee on Cost of the Work incurred.
- 14.15.8 Notwithstanding any other damage limitation herein the County and the Contractor recognize the delays, expense, and difficulties involved in proving in a legal or arbitration proceeding the actual loss suffered by the Contractor if the County is found to have intentionally interfered with Contractor's performance of the Work by fraud, misrepresentation, gross negligence, or intentional arbitrary or capricious acts and/or omissions. Accordingly, instead of requiring any such proof, the County and the Contractor agree that as liquidated damages (in lieu of any other remedy or damages) for delay, disruption or hindrance (but not as a penalty) the County shall pay the Contractor \$1,500.00 for each day that a court of competent jurisdiction finds the County's conduct referenced in Section14.15 (above) is the sole cause of Contractor's delay in completing the Work.

15.0 TERMINATION:

- 15.1 Fort Bend County may terminate the Contract for cause if the Contractor:
 - 15.1.1 Persistently or repeatedly refuses or fails to supply enough properly skilled workers or proper materials.
 - 15.1.2 Fails to make payment to Subcontractors for materials or labor in accordance with the respective agreements between the Contractor and the Subcontractor.

Initials	of Bidder:	

- 15.1.3 Persistently disregards laws, ordinances, or rules, regulations or orders of a public authority having jurisdiction.
- 15.1.4 Otherwise commits substantial breach of a provision of the Contract Documents.
- 15.2 When any of the above reasons exists, Fort Bend County may, without prejudice to any other rights or remedies of Fort Bend County and after giving the Contractor and the Contractor's surety, if any, seven days' written notice, terminate employment of the Contractor and may, subject to any prior rights of the surety:
 - 15.2.1 Take possession of the site and of all materials, equipment, tools, and construction equipment and machinery thereon owned by the Contractor.
 - 15.2.2 Finish the Project by whatever reasonable method Fort Bend County may deem expedient.
 - 15.2.3 When Fort Bend County terminates the Contract for one of the reasons stated in this section, the Contractor shall not be entitled to receive further payment until the Project is finished. Therefore, the Contractor shall be promptly paid for all work actually and satisfactorily completed.
- 15.3 Termination for Convenience of Fort Bend County

Fort Bend county reserves the right, without breach, to terminate the Contract prior to, or during the performance of the Work, for any reason. Upon such an occurrence, the following shall apply.

- 15.3.1 The County will notify Contractor in writing of the county's determination to terminate the contract for convenience and the effective date of the Contract termination. The notice may also contain instructions necessary for the protection, storage or decommissioning of incomplete work or systems, and for safety.
- 15.3.2 Upon receipt of the notice of termination, Contractor shall immediately proceed with the following obligations, regardless of any dispute in determining or adjusting any amounts due at that point in the Contract:
 - 15.3.2.1 Stop all work.
 - 15.3.2.2 Place no further subcontracts or orders for materials or services.
 - 15.3.2.3 Terminate all subcontracts for convenience.
 - 15.3.2.4 Cancel all materials and equipment orders as applicable.

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- 15.3.2.5 Take appropriate action that is necessary to protect and preserve all property related to the Contract which is in the possession of Contractor.
- 15.3.2.6 When the Contract is terminated for Owner's convenience, Contractor may recover from Owner payment for all Work executed. Contractor may not claim lost profits or lost business opportunities.
- 15.4 <u>Settlement on Termination.</u> When the Contract is terminated by the County under 15.3, at any time prior to one hundred eighty (180) days after the effective date of termination, Contractor shall submit a final termination settlement proposal to the County based upon recoverable costs as provided under the Contract. If Contractor fails to submit the proposal within the time allowed, the County may unilaterally determine the amount due to Contractor because of the termination and pay the determined amount to Contractor.

16.0 COMPLETION, TRANSFER, & ACCEPTANCE:

- 16.1 <u>Final Completion</u>. Upon the occurrence of the final completion date, the punch list items shall be promptly commenced and thereafter completed within thirty (30) days after final completion.
- 16.2 <u>Transfer and Acceptance</u>. Upon the occurrence of final completion, care, custody and control of the Project shall pass to Fort Bend County. As referenced herein, the "<u>Transfer Date</u>" shall mean the date on which the care, custody and control of the Project passes to Fort Bend County. Subsequent to the Transfer Date all risk of loss with respect to the Project shall be by Fort Bend County and the Contractor shall be thereafter obligated to cover the Project with their Insurance.

17.0 SUSPENSION BY FORT BEND COUNTY FOR CONVENIENCE:

- 17.1 Fort Bend County may, without cause, order the Contractor in writing to suspend, delay or interrupt the Project in whole or in part for such period of time as Fort Bend County may determine.
- 17.2 An adjustment shall be made for increase in the cost of performance, caused by suspension, delay or interruption. No adjustment shall be made to the extent:
 - 17.2.1 That performance is, was or would have been so suspended, delayed or interrupted by another cause for which the Contractor is responsible.
 - 17.2.2 That an equitable adjustment is made or denied under another provision of this Contract.

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17.3 Adjustments made in the cost of performance may have a mutually agreed fixed or percentage fee.

18.0 INDEPENDENT CONTRACTOR:

The Contractor shall be an independent contractor and any provisions of this Contract that may appear to give Fort Bend County the right to direct the Contractor as to the details of the manner of doing the Project shall be deemed to mean that the Contractor shall follow the desires of Fort Bend County in the results of the Project only and not in the means whereby the Project is to be accomplished. The Contractor shall be responsible as to the details of completing the Project. Neither the agents, representatives, nor employees of the Contractor, shall be deemed to be the agents, representatives, or employees of Fort Bend County. The Contractor further represents that it accepts a fiduciary role and responsibility with respect to Fort Bend County and will, to its best abilities, act in the best interests of Fort Bend County and the timely completion of the Project. The Contractor agrees and understands that neither it nor any of its agents or employees may act in the name of Fort Bend County except and unless specifically authorized in writing by Fort Bend County to do so. The Contractor shall furnish construction administration and management services and use the Contractor's best efforts to complete the Project in an expeditious and economical manner consistent with the interests of Fort Bend County.

19.0 NOTICE

- 19.1 All written notices, demands, and other papers or documents to be delivered to Fort Bend County under this Contract shall be delivered to the Engineering Department, 301 Jackson, Richmond, Texas 77469, or at such other place or places as Fort Bend County may from time to time designate by written notice delivered to the Contractor. For purposes of notice under this Contract, a copy of any notice or communication hereunder shall also be forwarded to the following address: Fort Bend County, 301 Jackson Street, Richmond, Texas 77469, Attention: County Judge.
- 19.2 All written notices, demands, and other papers or documents to be delivered to the Contractor under this Contract shall be delivered to the Authorized Representative identified in the Contract documents or such other place or places as the Contractor may designate by written notice delivered to Fort Bend County.

20.0 RECORDS:

- 20.1 Fort Bend County shall be the absolute and unqualified owner of all drawings, preliminary layouts, record drawings, sketches and other documents prepared pursuant to the Contract by Contractor.
- 20.2 The Contractor agrees to maintain and preserve for a period of at least five years after the earlier of the expiration of the defects period or termination of this Contract, accurate and complete records relating to the performance of the

Project. The Contractor agrees to, upon request, provide Fort Bend County with such records.

21.0 SUCCESSORS & ASSIGNS:

- 21.1 Fort Bend County and the Contractor bind themselves and their successors, executors, administrators and assigns to the other party of this Contract and to the successors, executors, administrators and assigns of such other party, in respect to all covenants of this Contract.
- 21.2 Neither Fort Bend County nor the Contractor shall assign, sublet or transfer its interest in this Contract without the prior written consent of the other.
- 21.3 Nothing herein shall be construed as creating any personal liability on the part of any officer or agent of any public and/or governmental body that may be a party hereto.

22.0 PUBLIC CONTACT:

Contact with the news media, citizens of Fort Bend County or governmental agencies shall be the sole responsibility of Fort Bend County. Under no circumstances, whatsoever, shall Contractor release any material or information developed in the performance of its services hereunder without the express written permission of Fort Bend County, except where required to do so by law.

23.0 MODIFICATIONS:

This instrument contains the entire Contract between the parties relating to the rights herein granted and obligations herein assumed. Any oral or written representations or modifications concerning this instrument shall be of no force and effect excepting a subsequent written modification signed by both parties hereto.

24.0 SILENCE OF SPECIFICATIONS:

The apparent silence of specifications as to any detail, or the apparent omission from it of a detailed description concerning any point, shall be regarded as meaning that only the best commercial practice is to prevail and that only material and workmanship of the finest quality are to be used. All interpretations of specifications shall be made on the basis of this statement. The items furnished under this contract shall be new, unused of the latest product in production to commercial trade and shall be of the highest quality as to materials used and workmanship. Manufacturer furnishing these items shall be experienced in design and construction of such items and shall be an established supplier of the item bid.

25.0 SEVERABILITY:

In the event one or more of the provisions contained in these requirements or the specifications shall for any reason be held to be invalid, illegal or unenforceable in any respect, such invalidity,

illegality, or unenforceability shall not affect any other provision hereof and these requirements or the specifications shall be construed as if such invalid, illegal, or unenforceable provision had never been contained herein.

26.0 **GOVERNING FORMS:**

In the event of any conflict between the terms and provisions of these requirements and the specifications, the specifications shall govern. In the event of any conflict of interpretation of any part of this overall document, Fort Bend County's interpretation shall govern.

27.0 TAX EXEMPT:

Fort Bend County is exempt from state and local sales and use taxes under Section 151.309 of the Texas Tax Code. This Contract is deemed to be a separate contract for Texas tax purposes, and as such, Fort Bend County hereby issues its Texas Exemption for the purchase of any items qualifying for exemption under this Contract. Contractor is to issue its Texas Resale Certificate to vendors and subcontractors for such items qualifying for this exemption, and further, contractor should state these items at cost.

28.0 **ENTIRE AGREEMENT:**

The Parties agree that this Contract contains all of the terms and conditions of the understanding of the parties relating to the subject matter hereof. All prior negotiations, discussions, correspondence and preliminary understandings between the parties and others relating hereto are superseded by this Contract. By entering into this Contract, the parties do not intend to create any obligations, express or implied, other than those specifically set out in this Contract.

29.0 APPLICABLE LAW & VENUE

This Contract shall be construed under and in accord with the laws of the State of Texas, and all obligations of the parties created hereunder are performable in Fort Bend County, Texas, and that venue for any litigation arising out of or related to this Contract shall lie solely in the court of appropriate jurisdiction located in Fort Bend County, Texas.

30.0 **ENCLOSURE:**

The following being incorporated herein by reference for all purposes as though fully set forth herein word for word.

Enclosure #1 – Specifications and Plans

31.0 **PRICING:** Complete excel unit pricing form.

32.0 **PROJECT DURATION:**

Bidder agrees, if awarded the contract, to complete all work required by the contract documents within ____ calendar days (maximum 365 days) after issuance of a purchase order by the Initials of Bidder: _____

County Purchasing Agent and notice to proceed by the Engineering Department.

33.0 AWARD:

This contract will be awarded to the overall lowest and best bid.

34.0 TEXAS ETHICS COMMISSION FORM 1295:

34.1 Effective January 1, 2016 all contracts executed by Commissioners Court, regardless of the dollar amount, will require completion of Form 1295 "Certificate of Interested Parties", per the new Government Code Statute §2252.908. All vendors submitting a response to a formal Bid, RFP, SOQ or any contracts, contract amendments, renewals or change orders are required to complete the Form 1295 online through the State of Texas Ethics Commission website. Please visit: https://www.ethics.state.tx.us/filinginfo/1295/

34.2 On-line instructions:

- 34.2.1 Name of governmental entity is to read: Fort Bend County
- 34.2.2 Identification number used by the governmental entity is: <u>B24-076</u>
- 34.2.3 Description is the title of the solicitation: <u>Construction of Stella Road from Cottonwood School Road to W. Fairgrounds Road</u>.
- 34.3 Apparent low bidder(s) will be required to provide the Form 1295 within three (3) calendar days from notification; however, if your company is publicly traded you are not required to complete this form.

35.0 STATE LAW REQUIREMENTS FOR CONTRACTS:

The contents of this section are required by Texas Law and are included by County regardless of content.

- 35.1 Agreement to Not Boycott Israel Chapter 2271 Texas Government Code: Contractor verifies that if Contractor employs ten (10) or more full-time employees and this Agreement has a value of \$100,000 or more, Contractor does not boycott Israel and will not boycott Israel during the term of this Agreement.
- 35.2 Texas Government Code Section 2251.152 Acknowledgment: By signature on vendor form, Contractor represents pursuant to Section 2252.152 of the Texas Government Code, that Contractor is not listed on the website of the Comptroller of the State of Texas concerning the listing of companies that are identified under Section 806.051, Section 807.051 or Section 2253.153.

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initials of Bidder:	

36.0 HUMAN TRAFFICKING:

By acceptance of this contract, Contractor acknowledges that Fort Bend County is opposed to human trafficking and that no County funds will be used in support of services or activities that violate human trafficking laws

37.0 INDEMNITY FOR BODILY INJURY OR DEATH CLAIMS

Indemnity for certain bodily injury or death claims. To the fullest extent permitted by law, contractor shall indemnify, defend and hold harmless the county from and against all claims, losses, expenses, costs, demands, suits, causes of action, and damages, including without limitation, attorneys' fees and expenses, for bodily injury or death of any employee of contractor, its agents, or its subcontractors of every tier, even if the bodily injury or death is caused by or alleged to have been caused by the sole or partial negligence, fault or strict liability of any indemnitee.

Indemnity for all other claims. For all claims not addressed in the preceding section or section 11.0 above, including, without limitation, claims for damage to or loss of use of property and claims for bodily injury to or death of any person other than that addressed in the immediately preceding section, to the fullest extent permitted by law, contractor shall indemnify, defend and hold harmless the county from and against all claims, losses, expenses, costs, demands, suits, causes of action, and damages, including without limitation, attorneys' fees and expenses, of any nature whatsoever arising out of or related to this contract or the work to be performed under this contract, but only to the extent of the negligence or other fault of the contractor, its agents, representatives, employees or subcontractors of any tier.

38.0 AGREEMENT TO ARBITRATE UNDER THE FEDERAL ARBITRATION ACT

To the maximum extent allowed by law, any controversy or claim arising out of or relating to this contract, or the breach thereof, shall be settled by arbitration under the Federal Arbitration Act, 9 U.S.C. § 1, et seq. administered by the American Arbitration Association under its Construction Industry Arbitration Rules, and judgment on the award rendered by the arbitrator(s) may be entered in any court having jurisdiction thereof. For cases in which the amount in controversy is less than \$250,000, there shall be no discovery other than an expeditious and complete exchange of documents relative to the dispute. For cases in which the amount in controversy is between \$250,000 and \$1,000,000, there shall be no discovery except for an expeditious and complete exchange of such documentary information and up to three (3) depositions per side (including expert depositions, if any). For cases in which the amount in controversy exceeds \$1,000,000, there shall be no discovery except for an expeditious and complete exchange of such documentary information up to five (5) depositions per side (including expert depositions, if any). No formal interrogatories, request for admissions or formal request for production of documents shall be allowed in the arbitration process. The hearing on the merits will be completed no later than ninety (90) days after the initial demand for arbitration is made for disputes involving amounts in controversy of up to \$250,000; no later than no later than one hundred twenty (120) days after the initial demand for arbitration is made for disputes involving amounts in controversy of between \$250,000 and \$1,000,000; and, no later than three hundred sixty five (365) days after the initial demand for arbitration is made for disputes involving amounts in controversy of over \$1,000,000.

39.0 ADDITIONAL REQUIRED FORMS:

All vendors submitting are required to complete and return with submission

- 39.1 Vendor Form
- 39.2 W9 Form
- 39.3 Tax Form/Debt/Residence Certification
- 39.4 Contractor Acknowledgement of Stormwater Management Program

Contract Sheet Bid 24-076

THE STATE OF TEXAS COUNTY OF FORT BEND

This memorandum of agreement made	e and entered into on	the	day of		, 20,
by and between Fort Bend County ir	n the State of Texas	(hereinafter	designated	County), acting	g herein by
County Judge KP George, by virtu	ue of an order of	Fort Bend	County (Commissioners	Court, and
		(hereinafter	designated	Contractor).	
(company name)					
WITNESSETH:					
The Contractor and the County agree to	hat the bid and specifi	cations for	the Constru	action of Stella	Road from
Cottonwood School Road to W. Fairg	grounds Road for For	t Bend Cou	nty Mobili	ty Bond Project	No. 20116
which are hereto attached and made a pa	art hereof, together with	h this instrui	nent and the	e bond (when rec	quired) shall
constitute the full agreement and contra	act between parties and	for furnish	ing the item	ns set out and de	scribed; the
County agrees to pay the prices stipulated	d in the accepted bid.				
It is further agreed that this contract sha	all not become binding	or effective	until signe	d by the parties l	nereto and a
purchase order authorizing the items desi	ired has been issued.				
Executed at Richmond, Texas this	day of			20)
				Fort Bend Co	unty, Texas
	By:				
				County Judge,	KP George
	By:				
				Signature of	Contractor
	By:				
				Printed Nan	ne and Title



Request for Taxpayer Identification Number and Certification

Give Form to the requester. Do not send to the IRS.

intornar	OVING COLVID			
	1 Name (as shown on your income tax return). Name is required on this line; do not leave this line blank.			
page 2.	2 Business name/disregarded entity name, if different from above			
s on	3 Check appropriate box for federal tax classification; check only one of the following seven boxes: Individual/sole proprietor C Corporation S Corporation Partnership single-member LLC	4 Exemptions (codes apply only to certain entities, not individuals; see instructions on page 3): Exempt payee code (if any)		
ĕĕ	Limited liability company. Enter the tax classification (C=C corporation, S=S corporation, P=partners	Exemption from FATCA reporting		
Print or type Instruction	Note. For a single-member LLC that is disregarded, do not check LLC; check the appropriate box in the tax classification of the single-member owner.	code (if any)		
무급	Under (see instructions) ▶		(Applies to accounts maintained outside the U.S.)	
pecifi	6 Address (number, street, and apt. or suite no.)	Requester's name a	and address (optional)	
See S	6 City, state, and ZIP code			
	7 List account number(s) here (optional)			
Part	Taxpayer Identification Number (TIN)			
	our TIN in the appropriate box. The TIN provided must match the name given on line 1 to av		curity number	
resider entities	withholding. For individuals, this is generally your social security number (SSN). However, ft alien, sole proprietor, or disregarded entity, see the Part I instructions on page 3. For other, it is your employer identification number (EIN). If you do not have a number, see <i>How to ge</i>	r		
TIN on	page 3.	or		
	the account is in more than one name, see the instructions for line 1 and the chart on page	4 for Employer	identification number	
guideli	nes on whose number to enter.		-	
Part	Certification			
Under	penalties of perjury, I certify that:			
1. The	number shown on this form is my correct taxpayer identification number (or I am waiting for	a number to be is	sued to me); and	
Ser	not subject to backup withholding because: (a) I am exempt from backup withholding, or (brice (IRS) that I am subject to backup withholding as a result of a failure to report all interest onger subject to backup withholding; and			
3. I an	a U.S. citizen or other U.S. person (defined below); and			
4. The	FATCA code(s) entered on this form (if any) indicating that I am exempt from FATCA reporting	ng is correct.		
becaus interes genera	cation instructions. You must cross out item 2 above if you have been notified by the IRS the you have failed to report all interest and dividends on your tax return. For real estate trans paid, acquisition or abandonment of secured property, cancellation of debt, contributions the ly, payments other than interest and dividends, you are not required to sign the certification ions on page 3.	actions, item 2 doe o an individual reti	es not apply. For mortgage rement arrangement (IRA), and	
Sign Here	Signature of U.S. person ▶ Da	ate ▶		

General Instructions

Section references are to the Internal Revenue Code unless otherwise noted.

Future developments. Information about developments affecting Form W-9 (such as legislation enacted after we release it) is at www.irs.gov/fw9.

Purpose of Form

An individual or entity (Form W-9 requester) who is required to file an information return with the IRS must obtain your correct taxpayer identification number (TIN) which may be your social security number (SSN), individual taxpayer identification number (ITIN), adoption taxpayer identification number (ATIN), or employer identification number (EIN), to report on an information return the amount paid to you, or other amount reportable on an information return. Examples of information returns include, but are not limited to, the following:

- Form 1099-INT (interest earned or paid)
- Form 1099-DIV (dividends, including those from stocks or mutual funds)
- Form 1099-MISC (various types of income, prizes, awards, or gross proceeds)
- Form 1099-B (stock or mutual fund sales and certain other transactions by brokers)
- Form 1099-S (proceeds from real estate transactions)
- Form 1099-K (merchant card and third party network transactions)

- Form 1098 (home mortgage interest), 1098-E (student loan interest), 1098-T (tuition)
- Form 1099-C (canceled debt)
- Form 1099-A (acquisition or abandonment of secured property)

Use Form W-9 only if you are a U.S. person (including a resident alien), to provide your correct TIN.

If you do not return Form W-9 to the requester with a TIN, you might be subject to backup withholding. See What is backup withholding? on page 2.

By signing the filled-out form, you:

- 1. Certify that the TIN you are giving is correct (or you are waiting for a number to be issued),
 - 2. Certify that you are not subject to backup withholding, or
- 3. Claim exemption from backup withholding if you are a U.S. exempt payee. If applicable, you are also certifying that as a U.S. person, your allocable share of any partnership income from a U.S. trade or business is not subject to the withholding tax on foreign partners' share of effectively connected income, and
- 4. Certify that FATCA code(s) entered on this form (if any) indicating that you are exempt from the FATCA reporting, is correct. See *What is FATCA reporting?* on page 2 for further information.

Form W-9 (Rev. 12-2014) Page **2**

Note. If you are a U.S. person and a requester gives you a form other than Form W-9 to request your TIN, you must use the requester's form if it is substantially similar to this Form W-9.

Definition of a U.S. person. For federal tax purposes, you are considered a U.S. person if you are:

- An individual who is a U.S. citizen or U.S. resident alien;
- A partnership, corporation, company, or association created or organized in the United States or under the laws of the United States;
- · An estate (other than a foreign estate); or
- A domestic trust (as defined in Regulations section 301.7701-7).

Special rules for partnerships. Partnerships that conduct a trade or business in the United States are generally required to pay a withholding tax under section 1446 on any foreign partners' share of effectively connected taxable income from such business. Further, in certain cases where a Form W-9 has not been received, the rules under section 1446 require a partnership to presume that a partner is a foreign person, and pay the section 1446 withholding tax. Therefore, if you are a U.S. person that is a partner in a partnership conducting a trade or business in the United States, provide Form W-9 to the partnership to establish your U.S. status and avoid section 1446 withholding on your share of partnership income.

In the cases below, the following person must give Form W-9 to the partnership for purposes of establishing its U.S. status and avoiding withholding on its allocable share of net income from the partnership conducting a trade or business in the United States:

- In the case of a disregarded entity with a U.S. owner, the U.S. owner of the disregarded entity and not the entity;
- In the case of a grantor trust with a U.S. grantor or other U.S. owner, generally, the U.S. grantor or other U.S. owner of the grantor trust and not the trust; and
- In the case of a U.S. trust (other than a grantor trust), the U.S. trust (other than a grantor trust) and not the beneficiaries of the trust.

Foreign person. If you are a foreign person or the U.S. branch of a foreign bank that has elected to be treated as a U.S. person, do not use Form W-9. Instead, use the appropriate Form W-8 or Form 8233 (see Publication 515, Withholding of Tax on Nonresident Aliens and Foreign Entities).

Nonresident alien who becomes a resident alien. Generally, only a nonresident alien individual may use the terms of a tax treaty to reduce or eliminate U.S. tax on certain types of income. However, most tax treaties contain a provision known as a "saving clause." Exceptions specified in the saving clause may permit an exemption from tax to continue for certain types of income even after the payee has otherwise become a U.S. resident alien for tax purposes.

If you are a U.S. resident alien who is relying on an exception contained in the saving clause of a tax treaty to claim an exemption from U.S. tax on certain types of income, you must attach a statement to Form W-9 that specifies the following five items:

- 1. The treaty country. Generally, this must be the same treaty under which you claimed exemption from tax as a nonresident alien.
 - 2. The treaty article addressing the income
- 3. The article number (or location) in the tax treaty that contains the saving clause and its exceptions.
 - 4. The type and amount of income that qualifies for the exemption from tax.
- 5. Sufficient facts to justify the exemption from tax under the terms of the treaty article.

Example. Article 20 of the U.S.-China income tax treaty allows an exemption from tax for scholarship income received by a Chinese student temporarily present in the United States. Under U.S. law, this student will become a resident alien for tax purposes if his or her stay in the United States exceeds 5 calendar years. However, paragraph 2 of the first Protocol to the U.S.-China treaty (dated April 30, 1984) allows the provisions of Article 20 to continue to apply even after the Chinese student becomes a resident alien of the United States. A Chinese student who qualifies for this exception (under paragraph 2 of the first protocol) and is relying on this exception to claim an exemption from tax on his or her scholarship or fellowship income would attach to Form W-9 a statement that includes the information described above to support that exemption.

If you are a nonresident alien or a foreign entity, give the requester the appropriate completed Form W-8 or Form 8233.

Backup Withholding

What is backup withholding? Persons making certain payments to you must under certain conditions withhold and pay to the IRS 28% of such payments. This is called "backup withholding." Payments that may be subject to backup withholding include interest, tax-exempt interest, dividends, broker and barter exchange transactions, rents, royalties, nonemployee pay, payments made in settlement of payment card and third party network transactions, and certain payments from fishing boat operators. Real estate transactions are not subject to backup withholding.

You will not be subject to backup withholding on payments you receive if you give the requester your correct TIN, make the proper certifications, and report all your taxable interest and dividends on your tax return.

Payments you receive will be subject to backup withholding if:

- 1. You do not furnish your TIN to the requester,
- 2. You do not certify your TIN when required (see the Part II instructions on page 3 for details), $\,$

- 3. The IRS tells the requester that you furnished an incorrect TIN.
- 4. The IRS tells you that you are subject to backup withholding because you did not report all your interest and dividends on your tax return (for reportable interest and dividends only), or
- 5. You do not certify to the requester that you are not subject to backup withholding under 4 above (for reportable interest and dividend accounts opened after 1983 only).

Certain payees and payments are exempt from backup withholding. See *Exempt payee code* on page 3 and the separate Instructions for the Requester of Form W-9 for more information.

Also see Special rules for partnerships above.

What is FATCA reporting?

The Foreign Account Tax Compliance Act (FATCA) requires a participating foreign financial institution to report all United States account holders that are specified United States persons. Certain payees are exempt from FATCA reporting. See Exemption from FATCA reporting code on page 3 and the Instructions for the Requester of Form W-9 for more information.

Updating Your Information

You must provide updated information to any person to whom you claimed to be an exempt payee if you are no longer an exempt payee and anticipate receiving reportable payments in the future from this person. For example, you may need to provide updated information if you are a C corporation that elects to be an S corporation, or if you no longer are tax exempt. In addition, you must furnish a new Form W-9 if the name or TIN changes for the account; for example, if the grantor of a grantor trust dies.

Penalties

Failure to furnish TIN. If you fail to furnish your correct TIN to a requester, you are subject to a penalty of \$50 for each such failure unless your failure is due to reasonable cause and not to willful neglect.

Civil penalty for false information with respect to withholding. If you make a false statement with no reasonable basis that results in no backup withholding, you are subject to a \$500 penalty.

Criminal penalty for falsifying information. Willfully falsifying certifications or affirmations may subject you to criminal penalties including fines and/or imprisonment.

Misuse of TINs. If the requester discloses or uses TINs in violation of federal law, the requester may be subject to civil and criminal penalties.

Specific Instructions

Line 1

You must enter one of the following on this line; **do not** leave this line blank. The name should match the name on your tax return.

If this Form W-9 is for a joint account, list first, and then circle, the name of the person or entity whose number you entered in Part I of Form W-9.

a. **Individual.** Generally, enter the name shown on your tax return. If you have changed your last name without informing the Social Security Administration (SSA) of the name change, enter your first name, the last name as shown on your social security card, and your new last name.

Note. ITIN applicant: Enter your individual name as it was entered on your Form W-7 application, line 1a. This should also be the same as the name you entered on the Form 1040/1040A/1040EZ you filed with your application.

- b. **Sole proprietor or single-member LLC.** Enter your individual name as shown on your 1040/1040A/1040EZ on line 1. You may enter your business, trade, or "doing business as" (DBA) name on line 2.
- c. Partnership, LLC that is not a single-member LLC, C Corporation, or S Corporation. Enter the entity's name as shown on the entity's tax return on line 1 and any business, trade, or DBA name on line 2.
- d. Other entities. Enter your name as shown on required U.S. federal tax documents on line 1. This name should match the name shown on the charter or other legal document creating the entity. You may enter any business, trade, or DBA name on line 2.
- e. **Disregarded entity.** For U.S. federal tax purposes, an entity that is disregarded as an entity separate from its owner is treated as a "disregarded entity." See Regulations section 301.7701-2(c)(2)(iii). Enter the owner's name on line 1. The name of the entity entered on line 1 should never be a disregarded entity. The name on line 1 should be the name shown on the income tax return on which the income should be reported. For example, if a foreign LLC that is treated as a disregarded entity for U.S. federal tax purposes has a single owner that is a U.S. person, the U.S. owner's name is required to be provided on line 1. If the direct owner of the entity is also a disregarded entity, enter the first owner that is not disregarded for federal tax purposes. Enter the disregarded entity's name on line 2, "Business name/disregarded entity name." If the owner of the disregarded entity is a foreign person, the owner must complete an appropriate Form W-8 instead of a Form W-9. This is the case even if the foreign person has a U.S. TIN.

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Line 2

If you have a business name, trade name, DBA name, or disregarded entity name, you may enter it on line 2.

Line 3

Check the appropriate box in line 3 for the U.S. federal tax classification of the person whose name is entered on line 1. Check only one box in line 3.

Limited Liability Company (LLC). If the name on line 1 is an LLC treated as a partnership for U.S. federal tax purposes, check the "Limited Liability Company" box and enter "P" in the space provided. If the LLC has filed Form 8832 or 2553 to be taxed as a corporation, check the "Limited Liability Company" box and in the space provided enter "C" for C corporation or "S" for S corporation. If it is a single-member LLC that is a disregarded entity, do not check the "Limited Liability Company" box; instead check the first box in line 3 "Individual/sole proprietor or single-member LLC."

Line 4, Exemptions

If you are exempt from backup withholding and/or FATCA reporting, enter in the appropriate space in line 4 any code(s) that may apply to you.

Exempt payee code.

- Generally, individuals (including sole proprietors) are not exempt from backup withholding.
- Except as provided below, corporations are exempt from backup withholding for certain payments, including interest and dividends.
- Corporations are not exempt from backup withholding for payments made in settlement of payment card or third party network transactions.
- Corporations are not exempt from backup withholding with respect to attorneys' fees or gross proceeds paid to attorneys, and corporations that provide medical or health care services are not exempt with respect to payments reportable on Form 1099-MISC.

The following codes identify payees that are exempt from backup withholding. Enter the appropriate code in the space in line 4.

- $1-\!$ An organization exempt from tax under section 501(a), any IRA, or a custodial account under section 403(b)(7) if the account satisfies the requirements of section 401(f)(2)
 - 2-The United States or any of its agencies or instrumentalities
- $3-\!A$ state, the District of Columbia, a U.S. commonwealth or possession, or any of their political subdivisions or instrumentalities
- $4-\!\mbox{A}$ foreign government or any of its political subdivisions, agencies, or instrumentalities
 - 5-A corporation
- 6—A dealer in securities or commodities required to register in the United States, the District of Columbia, or a U.S. commonwealth or possession
- $7\!-\!\mathrm{A}$ futures commission merchant registered with the Commodity Futures Trading Commission
 - 8-A real estate investment trust
- $9-\!$ An entity registered at all times during the tax year under the Investment Company Act of 1940
 - 10-A common trust fund operated by a bank under section 584(a)
 - 11-A financial institution
- $12\!-\!A$ middleman known in the investment community as a nominee or custodian
 - 13-A trust exempt from tax under section 664 or described in section 4947

The following chart shows types of payments that may be exempt from backup withholding. The chart applies to the exempt payees listed above, 1 through 13.

IF the payment is for	THEN the payment is exempt for		
Interest and dividend payments	All exempt payees except for 7		
Broker transactions	Exempt payees 1 through 4 and 6 through 11 and all C corporations. S corporations must not enter an exempt payee code because they are exempt only for sales of noncovered securities acquired prior to 2012.		
Barter exchange transactions and patronage dividends	Exempt payees 1 through 4		
Payments over \$600 required to be reported and direct sales over \$5,000 ¹	Generally, exempt payees 1 through 5 ²		
Payments made in settlement of payment card or third party network transactions	Exempt payees 1 through 4		

¹See Form 1099-MISC, Miscellaneous Income, and its instructions.

² However, the following payments made to a corporation and reportable on Form 1099-MISC are not exempt from backup withholding: medical and health care payments, attorneys' fees, gross proceeds paid to an attorney reportable under section 6045(f), and payments for services paid by a federal executive agency.

Exemption from FATCA reporting code. The following codes identify payees that are exempt from reporting under FATCA. These codes apply to persons submitting this form for accounts maintained outside of the United States by certain foreign financial institutions. Therefore, if you are only submitting this form for an account you hold in the United States, you may leave this field blank. Consult with the person requesting this form if you are uncertain if the financial institution is subject to these requirements. A requester may indicate that a code is not required by providing you with a Form W-9 with "Not Applicable" (or any similar indication) written or printed on the line for a FATCA exemption code.

- A—An organization exempt from tax under section 501(a) or any individual retirement plan as defined in section 7701(a)(37)
- B-The United States or any of its agencies or instrumentalities
- C—A state, the District of Columbia, a U.S. commonwealth or possession, or any of their political subdivisions or instrumentalities
- D—A corporation the stock of which is regularly traded on one or more established securities markets, as described in Regulations section 1.1472-1(c)(1)(i)
- E—A corporation that is a member of the same expanded affiliated group as a corporation described in Regulations section 1.1472-1(c)(1)(i)
- F—A dealer in securities, commodities, or derivative financial instruments (including notional principal contracts, futures, forwards, and options) that is registered as such under the laws of the United States or any state
 - G—A real estate investment trust
- H—A regulated investment company as defined in section 851 or an entity registered at all times during the tax year under the Investment Company Act of
- I-A common trust fund as defined in section 584(a)
- J-A bank as defined in section 581
- K-A broker
- L-A trust exempt from tax under section 664 or described in section 4947(a)(1)
- M-A tax exempt trust under a section 403(b) plan or section 457(g) plan

Note. You may wish to consult with the financial institution requesting this form to determine whether the FATCA code and/or exempt payee code should be completed.

Line 5

Enter your address (number, street, and apartment or suite number). This is where the requester of this Form W-9 will mail your information returns.

Line 6

Enter your city, state, and ZIP code.

Part I. Taxpayer Identification Number (TIN)

Enter your TIN in the appropriate box. If you are a resident alien and you do not have and are not eligible to get an SSN, your TIN is your IRS individual taxpayer identification number (ITIN). Enter it in the social security number box. If you do not have an ITIN, see *How to get a TIN* below.

If you are a sole proprietor and you have an EIN, you may enter either your SSN or EIN. However, the IRS prefers that you use your SSN.

If you are a single-member LLC that is disregarded as an entity separate from its owner (see *Limited Liability Company (LLC)* on this page), enter the owner's SSN (or EIN, if the owner has one). Do not enter the disregarded entity's EIN. If the LLC is classified as a corporation or partnership, enter the entity's EIN.

Note. See the chart on page 4 for further clarification of name and TIN combinations.

How to get a TIN. If you do not have a TIN, apply for one immediately. To apply for an SSN, get Form SS-5, Application for a Social Security Card, from your local SSA office or get this form online at www.ssa.gov. You may also get this form by calling 1-800-772-1213. Use Form W-7, Application for IRS Individual Taxpayer Identification Number, to apply for an ITIN, or Form SS-4, Application for Employer Identification Number, to apply for an EIN. You can apply for an EIN online by accessing the IRS website at www.irs.gov/businesses and clicking on Employer Identification Number (EIN) under Starting a Business. You can get Forms W-7 and SS-4 from the IRS by visiting IRS.gov or by calling 1-800-TAX-FORM (1-800-829-3676).

If you are asked to complete Form W-9 but do not have a TIN, apply for a TIN and write "Applied For" in the space for the TIN, sign and date the form, and give it to the requester. For interest and dividend payments, and certain payments made with respect to readily tradable instruments, generally you will have 60 days to get a TIN and give it to the requester before you are subject to backup withholding on payments. The 60-day rule does not apply to other types of payments. You will be subject to backup withholding on all such payments until you provide your TIN to the requester.

Note. Entering "Applied For" means that you have already applied for a TIN or that you intend to apply for one soon.

Caution: A disregarded U.S. entity that has a foreign owner must use the appropriate Form W-8.

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Part II. Certification

To establish to the withholding agent that you are a U.S. person, or resident alien, sign Form W-9. You may be requested to sign by the withholding agent even if items 1, 4, or 5 below indicate otherwise.

For a joint account, only the person whose TIN is shown in Part I should sign (when required). In the case of a disregarded entity, the person identified on line 1 must sign. Exempt payees, see Exempt payee code earlier.

Signature requirements. Complete the certification as indicated in items 1 through 5 below

- 1. Interest, dividend, and barter exchange accounts opened before 1984 and broker accounts considered active during 1983. You must give your correct TIN, but you do not have to sign the certification.
- 2. Interest, dividend, broker, and barter exchange accounts opened after 1983 and broker accounts considered inactive during 1983. You must sign the certification or backup withholding will apply. If you are subject to backup withholding and you are merely providing your correct TIN to the requester, you must cross out item 2 in the certification before signing the form.
- 3. Real estate transactions. You must sign the certification. You may cross out item 2 of the certification.
- 4. Other payments. You must give your correct TIN, but you do not have to sign the certification unless you have been notified that you have previously given an incorrect TIN. "Other payments" include payments made in the course of the requester's trade or business for rents, royalties, goods (other than bills for merchandise), medical and health care services (including payments to corporations), payments to a nonemployee for services, payments made in settlement of payment card and third party network transactions, payments to certain fishing boat crew members and fishermen, and gross proceeds paid to attorneys (including payments to corporations).
- 5. Mortgage interest paid by you, acquisition or abandonment of secured property, cancellation of debt, qualified tuition program payments (under section 529), IRA, Coverdell ESA, Archer MSA or HSA contributions or distributions, and pension distributions. You must give your correct TIN, but you do not have to sign the certification.

What Name and Number To Give the Requester

For this type of account:	Give name and SSN of:		
Individual Two or more individuals (joint account)	The individual The actual owner of the account or, if combined funds, the first individual on the account		
Custodian account of a minor (Uniform Gift to Minors Act)	The minor ²		
a. The usual revocable savings trust (grantor is also trustee) b. So-called trust account that is not a legal or valid trust under state law	The grantor-trustee' The actual owner'		
Sole proprietorship or disregarded entity owned by an individual	The owner ³		
6. Grantor trust filing under Optional Form 1099 Filing Method 1 (see Regulations section 1.671-4(b)(2)(i) (A))	The grantor*		
For this type of account:	Give name and EIN of:		
Disregarded entity not owned by an individual	The owner		
8. A valid trust, estate, or pension trust	Legal entity ⁴		
Corporation or LLC electing corporate status on Form 8832 or Form 2553	The corporation		
Association, club, religious, charitable, educational, or other tax- exempt organization	The organization		
11. Partnership or multi-member LLC	The partnership		
12. A broker or registered nominee	The broker or nominee		
13. Account with the Department of Agriculture in the name of a public entity (such as a state or local government, school district, or prison) that receives agricultural program payments	The public entity		
 Grantor trust filing under the Form 1041 Filing Method or the Optional Form 1099 Filing Method 2 (see Regulations section 1.671-4(b)(2)(i) (B)) 	The trust		

List first and circle the name of the person whose number you furnish. If only one person on a joint account has an SSN, that person's number must be furnished.

You must show your individual name and you may also enter your business or DBA name on the "Business name/disregarded entity" name line. You may use either your SSN or EIN (if you have one), but the IRS encourages you to use your SSN.

List first and circle the name of the trust, estate, or pension trust. (Do not furnish the TIN of the personal representative or trustee unless the legal entity itself is not designated in the account title.) Also see Special rules for partnerships on page 2. *Note. Grantor also must provide a Form W-9 to trustee of trust.

Note. If no name is circled when more than one name is listed, the number will be considered to be that of the first name listed.

Secure Your Tax Records from Identity Theft

Identity theft occurs when someone uses your personal information such as your name, SSN, or other identifying information, without your permission, to commit fraud or other crimes. An identity thief may use your SSN to get a job or may file a tax return using your SSN to receive a refund.

To reduce your risk:

- · Protect your SSN,
- Ensure your employer is protecting your SSN, and
- Be careful when choosing a tax preparer.

If your tax records are affected by identity theft and you receive a notice from the IRS, respond right away to the name and phone number printed on the IRS notice or letter.

If your tax records are not currently affected by identity theft but you think you are at risk due to a lost or stolen purse or wallet, questionable credit card activity or credit report, contact the IRS Identity Theft Hotline at 1-800-908-4490 or submit Form 14039

For more information, see Publication 4535, Identity Theft Prevention and Victim

Victims of identity theft who are experiencing economic harm or a system problem, or are seeking help in resolving tax problems that have not been resolved through normal channels, may be eligible for Taxpayer Advocate Service (TAS) assistance. You can reach TAS by calling the TAS toll-free case intake line at 1-877-777-4778 or TTY/TDD 1-800-829-4059.

Protect yourself from suspicious emails or phishing schemes. Phishing is the creation and use of email and websites designed to mimic legitimate business emails and websites. The most common act is sending an email to a user falsely claiming to be an established legitimate enterprise in an attempt to scam the user into surrendering private information that will be used for identity theft.

The IRS does not initiate contacts with taxpayers via emails. Also, the IRS does not request personal detailed information through email or ask taxpayers for the PIN numbers, passwords, or similar secret access information for their credit card, bank, or other financial accounts.

If you receive an unsolicited email claiming to be from the IRS, forward this message to phishing@irs.gov. You may also report misuse of the IRS name, logo, or other IRS property to the Treasury Inspector General for Tax Administration (TIGTA) at 1-800-366-4484. You can forward suspicious emails to the Federal Trade Commission at: spam@uce.gov or contact them at www.ftc.gov/idtheft or 1-877-IDTHEFT (1-877-438-4338).

Visit IRS.gov to learn more about identity theft and how to reduce your risk.

Privacy Act Notice

Section 6109 of the Internal Revenue Code requires you to provide your correct TIN to persons (including federal agencies) who are required to file information returns with the IRS to report interest, dividends, or certain other income paid to you; mortgage interest you paid; the acquisition or abandonment of secured property; the cancellation of debt; or contributions you made to an IRA, Archer MSA, or HSA. The person collecting this form uses the information on the form to file information returns with the IRS, reporting the above information. Routine uses of this information include giving it to the Department of Justice for civil and criminal litigation and to cities, states, the District of Columbia, and U.S. commonwealths and possessions for use in administering their laws. The information also may be disclosed to other countries under a treaty, to federal and state agencies to enforce civil and criminal laws, or to federal law enforcement and intelligence agencies to combat terrorism. You must provide your TIN whether or not you are required to file a tax return. Under section 3406, payers must generally withhold a percentage of taxable interest, dividend, and certain other payments to a payee who does not give a TIN to the payer. Certain penalties may also apply for providing false or fraudulent information.

Circle the minor's name and furnish the minor's SSN.

TAX FORM/DEBT/ RESIDENCE CERTIFICATION

(for Advertised Projects)

Taxpa	ayer Identification Numbe	(T.I.N.):
Comp	oany Name submitting Bio	Proposal:
- Maili	ng Address:	
		s in the State of Texas? Yes No
	are an individual, list the ned name(s) under which	names and addresses of any partnership of which you are a general partner or any ou operate your business
I.		ele property in Fort Bend County owned by you or above partnerships as well as any d/b/a personal property as well as mineral interest accounts. (Use a second sheet of paper if
Fort I	Bend County Tax Acct. No	* Property address or location**
** Fo	or real property, specify Idress where the property ay be stored at a warehou.	lentification number assigned by the Fort Bend County Appraisal District. the property address or legal description. For business personal property, specify the is located. For example, office equipment will normally be at your office, but inventory or other location. t - Do you owe any debts to Fort Bend County (taxes on properties listed in I above,
	tickets, fines, tolls, cou	
	☐ Yes☐ No	If yes, attach a separate page explaining the debt.
III.	requests Residence Cer	- Pursuant to Texas Government Code §2252.001 <i>et seq.</i> , as amended, Fort Bend County ification. §2252.001 <i>et seq.</i> of the Government Code provides some restrictions on the contracts; pertinent provisions of §2252.001 are stated below:
	(3) "Nonresident bide	er" refers to a person who is not a resident.
		refers to a person whose principal place of business is in this state, including a se ultimate parent company or majority owner has its principal place of business in
	I certify that §2252.001.	is a Resident Bidder of Texas as defined in Government Code [Company Name]
	I certify that	is a Nonresident Bidder as defined in Government Code [Company Name]
	82252.001 and ot	principal place of business is [City and State]



Contractor Acknowledgement of Storm Water Management Program

I hereby acknowledge that I am aware of the stormwater management program and standard operating procedures developed by Fort Bend County in compliance with the TPDES General Permit No. TXR040000. I agree to comply with all applicable best management practices and standard operating procedures while conducting my services for Fort Bend County. I agree to conduct all services in a manner that does not introduce illicit discharges of pollutants to streets, stormwater inlets, drainage ditches or any portion of the drainage system. The following materials and/or pollutant sources must not be discharged to the drainage system as a result of any services provided:

- 1. Grass clippings, leaves, mulch, rocks, sand, dirt or other waste materials resulting from landscaping activities, (except those materials resulting from ditch mowing or maintenance activities)
- 2. Herbicides, pesticides and/or fertilizers, (except those intended for aquatic use)
- 3. Detergents, fuels, solvents, oils and/or lubricants, other equipment and/or vehicle fluids,
- 4. Other hazardous materials including paints, thinners, chemicals or related waste materials,
- 5. Uncontrolled dewatering discharges, equipment and/or vehicle wash waters,
- 6. Sanitary waste, trash, debris, or other waste products
- 7. Wastewater from wet saw machinery,

Title

8. Other pollutants that degrade water quality or pose a threat to human health or the environment.

Furthermore, I agree to notify Fort Bend County immediately of any issue caused by or identified by:

(Company/Contractor)

that is believed to be an immediate threat to human health or the environment.

Contractor Signature

Date

Printed Name

SCOPE OF WORK

STELLA ROAD

The project scope includes demolishing the existing two-lane asphalt roadway and reconstructing it as a two 12-foot lane asphalt roadway with 6-foot shoulders on either side. The existing open ditch storm drainage is to be regraded and detention is to be provided to help mitigate impacts from the roadway improvements to the drainage outfall locations.

Work includes hot mix asphalt concrete surface course, hot mix asphalt concrete base course, lime stabilized subgrade, grading, driveways, culverts, seeding and sodding, placement of signs, pavement markings, etc.

Contractor is responsible for establishing and maintaining a traffic control plan in accordance to the latest version of Texas Manual on Uniform Traffic Control Devices (TMUTCD) and measures shown in the plans.

This description of the scope of work is general in nature and is intended as an overview of the project only. The complete detailed scope of work and bid items are contained in the construction drawings and specifications.

TECHNICAL SPECIFICATIONS

Technical Specifications are to the latest version of specifications from Harris County Engineering Department, Harris County Flood Control District, and City of Houston. These referenced specifications are incorporated herein as if they are copied verbatim including any supplementary specification, or amendments thereto and related specifications herein unless indicated otherwise in the drawings or specifications. Specifications can be found in the following links:

http://www.eng.hctx.net/Consultants/Standards-Specifications/Standard-Engineering-Design-Specifications

https://www.hcfcd.org/Resources/Technical-Manuals/Standard-Specifications-Related-Drawings?folderId=15985&view=gridview&pageSize=10

https://www.houstonpermittingcenter.org/office-city-engineer/design-and-construction-standards#agency-links-1476

Stella Road: From Cottonwood School Road to W Fairgrounds Road for Fort Bend County Bid 24-076

INDEX OF TECHNICAL SPECIFICATIONS

 $Reference\ Harris\ Count \underline{y}\ \underline{Standard\ Engineering\ Design\ Specifications\ (2023\ revision)\ where\ applicable.$

Harris County Specifications

Item No.	Specification Title		
100	Preparing Right-of-Way		
103	Existing Fence and Gates		
104	Removing Concrete		
105	Removing Base and Asphalt Pavament		
106	Salvage, Hauling, and Stockpiling Reclaimable Asphalt		
108	Pavament Removing Structures		
110	Excavation		
130	Borrow		
132	Embankment		
140	Eliminating Existing Pavment Markings and Markers		
160	Topsoil		
162 164	Sodding for Erosion Control Seeding for Erosion Control		
166	Fertilizer		
216	Subgrade		
247	Flexible Base		
260	Lime Treatment (Road-Mixed)		
265	Fly Ash or Lime-Fly Ash Treatment (Road-Mixed)		
275	Cement Treatment (Road-Mixed)		
276	Cement Treatment (Noad-Mixed) Cement Treatment (Plant-Mixed)		
292	Dense-Graded Hot-Mix Asphalt Base Course		
295	Full-Depth Reclamation Using Cement (Road-Mixed)		
296	Full-Depth Reclamation Using Asphalt Emulsion (Road-		
290	Mixed)		
312	Tack Coat		
314	Emulsified Asphalt (Prime Coat)		
340	Dense-Graded Hot-Mix Asphalt		
341	Dense-Graded Hot-Mix Asphalt Surface Course		
358	Hot In-Place Recycling of Asphalt Concrete Surfaces		
360 361	Concrete Pavement		
400	Repair of Concrete Pavement Excavation and Backfill for Strcutures		
401	Flowable Backfill		
402	Trench Excavation Protection		
404	Driving Piling		
407	Temporary Steel Sheet Piling		
409	Prestressed Concrete Piling		
416	Drilled Shaft Foundations		
420	Concrete Structures		
421	Hydraulic Cement Structures		
424	Prestressed Concrete Members (Fabrication)		
425	Precast Prestressed Concrete Structural Members		
426	Bridge Plaque		
432	RipRap		
434	Elastomeric Materials		
440	Joint Sealants Reinforcement for Concrete		
440	Steel Structures		
441	Metal for Structures		
446	Painting and Protective Coating		
447	Structural Bolting		
448	Structural Field Welding		
450	Railing		
451	Retrofit Railing		
462	Reinforced Concrete Box Culverts		
464	Reinforced Concrete Pipe		
465	Concrete Manholes and Junction Boxes		
466	Inlets		
467	Safety End Treatment		
476 479	Jacking, Boring, or Tunneling Pipe or Box Adjusting Monholes, Junction Boxes, and Inlets		
4/5	rujusting Monnoies, Juniction Duxes, and innets		

Item No.	Specification Title
481	Pipe for Drains
482	
501	Thermoplastic Pipe Culverts and Drains
502	Tree protection and Trimming
302	Traffic Signs, Roadside Signs, and Mailboxes
520	Weighing and Measurement Equipment
528	Colored Concrete for Median Noses
530	Concrete Curb, Concrete Curb and Gutter,
F26	Sidewalks and Driveways
536 540	Concrete Medians and Directional Islands Metal Beam Guard Fence
550	Chain Link Fencing
552	Wire Fence
554	Wood Fence
556	Construction Safety Fence
560	Project Site Cleanup and Maintenance
561	Project Site Documentation
580	Precast Concrete Wheel Stop
590	Notice of Intent
350	Temporary Erosion, Sedimentation, and
591	Enironmental Controls
610	Roadway Illumination Assemblies
618	Conduit
625	Zinc-Coated Steel Wire Strand
627	Treated Timber Poles
636	Signs
644	Roadside Sign Supports
658	Delineators and Object Markers
662	Work Zone Pavement Markings
666	Reflectorized Pavement Markings
668	Prefabricated Pavement Markers
672	Raised Pavement Markers
678	Pavement Surface Preparation for Markings
682	Traffic Signal Heads
683	Pedestrian Signal Head
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	Intelligent Transportation System (ITS) Controller
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698	Low Profile Concrete Barrier
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720	Repair Base Using Dense-Graded Hot-Mix Asphalt
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800	Hydraulic Cement
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802	Membrane Curing
820	Lime and Lime Slurry
821	Fly Ash for Soil Treatment
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861	Traffic Paint (Solvent Based)
862	Glass Reflective Spheres for Traffic Paint
863	Twelve Inch LED Traffic Signal Lamp Unit
864	Pedestrian LED Traffic Signal Lamp Unit
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866	LED Sign Lights for Traffic Signals

Stella Road: From Cottonwood School Road to W Fairgrounds Road for Fort Bend County

Bid 24-076

INDEX OF TECHNICAL SPECIFICATIONS (CONTINUED)

City of Houston Standard Specifications

For water line construction of this project, the contractor shall reference the City of Houston, Department of Public Works and Engineering, STANDARD CONSTRUCTION SPECIFICATIONS (2021 revision), when identified in the Bid Form specification reference as "COH"

Item No.	Specification Title	
02511	Water Lines	
02512	Water Tap and Service Line Installation	
02513	Wet Connections	
02514	Disinfection of Water Lines	
02515	Hydrostatic Testing of Pipelines	
02516	Cut, Plug, and Abandonment of Mains	
02517	Water Line in Tunnels	
02520	Fire Hydrant	
02521	Gate Valves	
02525	Tapping Sleeves and Valves	
02526	Water Meters	
02527	Polyurethane Coatings on Steel or Ductile Iron Pipe	
02528	Polyethylene Wrap	
02529	Tape Coatings on Steel Pipe	

Harris County Flood Control District Specifications

Reference 2020 Standard Construction Specifications and Details for Harris County Flood Control District where applicable when identified in the Bid Form specification reference as "HCFCD"

Item No.	Specification Title
02120	Material Disposal
02316	Structural Excavating and Backfilling
02321	Cement Stabilized Sand
02922	Sod
03310	Concrete

Other specifications to be listed as applicable.

Geotechnical Investigation (included herein)





GEOTECHNICAL INVESTIGATION STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD FORT BEND COUNTY, TEXAS

Reported to:
McDonough Engineering Corporation
Houston, Texas

by

Aviles Engineering Corporation 5790 Windfern Houston, Texas 77041 713-895-7645

REPORT NO. G126-21 (Revision 1)

August 2022



August 29, 2022

Austin McLean, P.E. Project Manager McDonough Engineering Corporation 5625 Schumacher Lane Houston, Texas 77057

Reference: Geotechnical Investigation

Stella Road Improvements

From Cottonwood School Road to Band Road Fort Bend County Mobility Bond Project #21060

Fort Bend County, Texas

AEC Report No. G126-21 (Revision 1)

Dear Mr. McLean,

Aviles Engineering Corporation (AEC) is pleased to present this report of the results of our geotechnical investigation for the above referenced project. Project terms and conditions were in accordance with the Professional Services Consultant Agreement between McDonough Engineering Corporation (MEC) and AEC, dated May 13, 2021. The project scope of services is in accordance with AEC Proposal No. G2021-03-02R2, dated March 31, 2021, and AEC Proposal G2021-03-02SR, dated January 14, 2022.

AEC appreciates the opportunity to be of service to you. Please call us if you have any questions or comments concerning this report or when we can be of further assistance.

Respectfully submitted,

Aviles Engineering Corporation

(TBPELS Firm Registration No. F-42)

Wilber L. Wang, P.E. Senior Engineer

08/29/2022

Reports Submitted:

McDonough Engineering Corporation (electronic)

1 File (electronic)

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June 28, 2022

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EXECUTIVE SUMMARY

The report submitted herein presents the results of Aviles Engineering Corporation's (AEC) geotechnical investigation for Fort Bend County's (FBC) proposed Stella Road Improvements from Cottonwood School Road to Band Road project in Precinct 1, Fort Bend County, Texas (Fort Bend County Key Map Nos.: 604X, and 644 B & C). A project vicinity map is presented on Plate A-1, in Appendix A. According to the information provided by McDonough Engineering Corporation (MEC), Stella Road along the project alignment is an existing 2 lane (one lane in each direction) asphalt roadway, which will be reconstructed to a new 2 lane (one lane in each direction) asphalt roadway with the main traffic lanes widened to 12 feet plus new 6 foot wide shoulders. The roadway will have roadside drainage swales. There will be a detention basin with a depth of 3.4 to 4.3 feet and slope inclination of H:V = 4:1 at the approximate mid-point of the alignment.

AEC notes that the project alignment has been updated after AEC completed its original borings (Borings B-1 through B-17). At its west limit, Stella Road will connect to Cottonwood School Road approximately 400 feet to the south from its current location. The east limit of the project has also changed from Band Road to W. Fairgrounds Road. The S-curve portion of the alignment from approximately Station 25+00 to 48+00 no longer follows the existing Stella Road alignment. After the alignment was updated, additional borings (Borings B-18 and B-19) were drilled along the new roadway alignment in March 2022, and detention basin borings (Borings B-20 through B-23) were drilled in May 2022.

- 1. Existing Pavement Conditions: In general, approximately 3.25 to 7 inches of asphalt surface with 5 to 15 inches of sand and gravel base was encountered along Stella Road. A summary of existing pavement sections encountered in AEC's borings is presented on Table 4 in Section 4.0 of this report.
- 2. <u>Subsurface Soil Conditions:</u> Details of the soils encountered during drilling are presented in the boring logs (see Plates A-3 through A-25, in Appendix A). Generalized soil profiles along the roadway alignment are presented on Plates B-1a through B-1c, in Appendix B. Based on Borings B-1 through B-19, the subsurface conditions along the project alignment generally consist of soft to hard lean/fat clay (CL/CH), including fill. Approximately 1 to 3 feet of clayey sand (SC) fill material was encountered at the ground surface in Borings B-2, B-5, B-10, B-13, and B-21. Granular soils such as loose to very dense silty sands (SM) and medium dense to very dense silts (ML) were encountered beginning from a depth of 14 to 16 feet down to the boring termination depths in Borings B-1, B-2, B-9, B-11 through B-13, B-15 through B-17, B-19, and B-22.
- 3. <u>Subsurface Soil Properties:</u> The subsurface clayey soils (including fill but excluding clayey sand) encountered in the borings have medium to very high plasticity (see "Degree of Plasticity of Cohesive Soils" on Plate A-27, in Appendix A), with liquid limits (LL) ranging from 27 to 98, and plasticity indices (PI) ranging from 11 to 69. The cohesive soils encountered are classified as "CL", "CL-ML", and "CH" type soils, and granular soils encountered are classified as "ML", "SC", "SC-SM", and "SM" type soils in accordance with ASTM D 2487.
- 4. <u>Groundwater Conditions:</u> Groundwater was not encountered in any of the boreholes during or upon completion of drilling.
- 5. <u>Hazardous Materials</u>: No signs of visual staining or odors were encountered during field drilling or during processing of the soil samples in the laboratory.



EXECUTIVE SUMMARY (Cont.)

- 6. Geologic Conditions: AEC performed a desk top fault study which included a review of public maps, available literature, and aerial photographs. AEC reviewed a University of Houston Master's Thesis entitled A Geophysical Study of Active Faulting in Fort Bend County, Texas concerning faulting in Fort Bend County, Texas (Schmidt, May 2013). According to the thesis, the closest fault to the project alignment is the northeast-southwest oriented Pleak Fault located approximately 3.9 miles south of the southeastern end of the project alignment. This fault is too distant to have an impact on the project alignment. Additional fault study is not recommended.
- 7. Roadway Reconstruction: Recommendations for reconstructing the existing asphalt pavement roadway are presented in Section 5.1 of this report. As directed, AEC first considered an asphalt pavement section that meets Fort Bend County's minimum pavement thickness requirements, which includes a 3 inch asphalt surface, 8 inch asphalt stabilized base, and 8 inch thick stabilized subgrade. Based on available traffic data, AEC estimates that FBC's minimum pavement section would provide a service life of approximately 8 years. AEC estimates an asphalt pavement section consisting of 3.5 inch asphalt surface, 10 inch asphalt-stabilized base, and 8 inch thick stabilized subgrade will be needed to provide a 20 year service life. Based on our borings, AEC estimates that most of the new roadway subgrade will be within highly expansive fat clay (CH) soil, which will require stabilization with a minimum of 7 percent lime. However, clayey sand (SC) fill material (likely placed when the original roadway was constructed) was encountered at the ground surface in Borings B-2 and B-5. Where sandy soils are exposed at the ground surface, the subgrade should be stabilized with a minimum of 3 percent lime and 7 percent fly ash.
- 8. <u>Detention Basin:</u> Recommendations for the detention basin is presented in Section 5.2 of this report. Based on Borings B-22 and B-23, the detention basin excavation will encounter very stiff to hard fat clay (CH). Groundwater was not encountered in the borings during drilling. The calculated minimum factor of safety (FS) for slope stability of the southeast bank of the detention basin ranges from 17.66 to 19.43 for short term condition, ranges from 1.85 to 2.22 for long term condition, and ranges from 1.31 to 1.52 for rapid drawdown condition. None of the soil excavated from the detention basin can be reused as select clay fill.

This Executive Summary is intended as a summary of the investigation and should not be used without the full text of this report.



GEOTECHNICAL INVESTIGATION

STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD FORT BEND COUNTY, TEXAS

1.0 INTRODUCTION

The contents of this revised report (G126-21 Revision 1, dated August 29. 2022) prepared by Aviles Engineering Corporation (AEC) supersede AEC's geotechnical investigation report (G126-21, dated May 9, 2022) previously issued for this project.

1.1 General

The report submitted herein presents the results of AEC's geotechnical investigation for Fort Bend County's (FBC) proposed Stella Road Improvements from Cottonwood School Road to Band Road project in Precinct 1, Fort Bend County, Texas (Fort Bend County Key Map Nos.: 604X, and 644 B & C). A project vicinity map is presented on Plate A-1, in Appendix A. According to the information provided by McDonough Engineering Corporation (MEC), Stella Road along the project alignment is an existing 2 lane (one lane in each direction) asphalt roadway, which will be reconstructed to a new 2 lane (one lane in each direction) asphalt roadway with the main traffic lanes widened to 12 feet plus new 6 foot wide shoulders. The roadway will have roadside drainage swales. There will be a detention basin with a depth of 3.4 to 4.3 feet and slope inclination of H:V = 4:1 at the approximate mid-point of the alignment.

AEC notes that the project alignment has been updated after AEC completed its original borings (Borings B-1 through B-17). At its west limit, Stella Road will connect to Cottonwood School Road approximately 400 feet to the south from its current location. The east limit of the project has also changed from Band Road to W. Fairgrounds Road. The S-curve portion of the alignment from approximately Station 25+00 to 48+00 no longer follows the existing Stella Road alignment. After the alignment was updated, additional borings (Borings B-18 and B-19) were drilled along the new roadway alignment in March 2022, and detention basin borings (Borings B-20 through B-23) were drilled in May 2022.



1.2 Purpose and Scope

The purpose of this geotechnical investigation is to evaluate the subsurface soil and groundwater conditions along the project alignment and develop geotechnical engineering recommendations for design and construction of asphalt pavement and the detention basin. The scope of this geotechnical investigation is summarized below:

- 1. Drilling and sampling twenty-three geotechnical borings ranging from 15 to 20 feet below existing grade.
- 2. Soil laboratory testing on selected soil samples.
- 3. Engineering analyses and recommendations for reconstruction of roadways with asphalt pavement, including pavement thickness design and subgrade preparation.
- 4. Engineering analyses and recommendations for the detention basin, including slope stability analysis, erosion protection requirements (if required), and evaluation of excavated soil for use as select fill.
- 5. Construction recommendations and groundwater control guidelines for the proposed roadway and detention basin.

2.0 <u>SUBSURFACE EXPLORATION</u>

2.1 Soil Borings

Boring spacing and depths were selected in general accordance with Chapter 8 of the August 2020 FBC Engineering Design Manual (Draft). AEC drilled a total of twenty-three soil borings (Borings B-1 through B-23) for the project improvements to depths ranging from 15 to 20 feet below existing grade. Boring locations were marked by AEC personnel in the field using a handheld GPS unit. The total drilling footage is 440 feet. After completion of drilling, the locations of Borings B-1 through B-19 were surveyed by Landtech, Inc. The locations of Borings B-20 through B-23 were not surveyed; AEC estimated the coordinates of the borings from the handheld GPS unit, and estimated the boring elevations from available topographic data in the drawings provided by MEC. The boring locations are shown on the Boring Location Plan on Plate A-2, in Appendix A. Boring survey data (in State Plane Grid Coordinates, Texas South Central Zone 4204, US Survey Feet) is summarized on Table 1 below and is also included on the representative boring logs.



Table 1. Summary of Boring Survey Data

Boring No.	Boring Depth (ft)	Northing (1) (Grid, ft)	Easting (1) (Grid, ft)	Boring Surface Elevation (ft)	Station (Stella Rd Baseline)	Offset (ft)
B-1	20	13,752,613.63	2,977,451.48	99.65	3+13.50	-29.76
B-2	20	13,752,122.69	2,977,409.05	97.70	7+91.95	-4.35
B-3	20	13,751,769.18	2,977,721.50	97.86	12+85.68	-5.67
B-4	20	13,751,439.93	2,978,079.39	97.58	17+72.04	-6.05
B-5	20	13,751,103.01	2,978,427.64	97.30	22+56.53	5.76
B-6	20	13,750,779.58	2,978,795.46	96.28	27+46.25	-5.63
B-7	20	13,750,466,84	2,979,177.94	96.38	32+44.94	-6.86
B-8	20	13,750,519.24	2,979,671.65	96.64	37+71.16	-6.59
B-9	20	13,750,858.76	2,980,018.92	96.11	42+58.29	7.52
B-10	20	13,751,072.08	2,980,400.76	96.26	47+14.00	6.22
B-11	20	13,750,890.73	2,980,828.65	96.32	52+00.53	7.47
B-12	20	13,750,586.11	2,981,181.36	96.07	56+66.37	-8.33
B-13	20	13,750,262.50	2,981,532.35	95.97	61+43.85	-9.00
B-14	20	13,749,931.30	2,981,888.21	95.67	66+30.04	-7.41
B-15	20	13,749,608.87	2,982,234.96	95.35	71+03.59	-6.08
B-16	20	13,749,294.44	2,982,576.06	95.16	75+67.57	-6.79
B-17	20	13,748,987.65	2,982,888.19	94.52	80+05.09	6.57
B-18	20	13,750,595.24	2,979,443.158	93.80	31+30.51	-6.82
B-19	20	13,750,882.72	2,980,368.432	94.19	41+31.40	73.75
B-20 ⁽²⁾	15	13,751,108.27	2,980,993.076	98	-	-
B-21 ⁽²⁾	15	13,750,814.12	2,981,100.064	97	-	-
B-22 ⁽²⁾	16	13,750,851.89	2,979,703.639	95	-	-
B-23 ⁽²⁾	15	13,750,724.23	2,979,319.16	95	-	-

Note: (1) Northing and easting coordinates referenced to Texas Coordinate System, South Central Zone 4204. Coordinates are provided in grid format.

2.2 Drilling and Sampling Methods

Prior to drilling, existing pavement at Borings B-1, B-4, B-8, B-11, and B-14 through B-17 were first cut with a core barrel. Borings were drilled using either a truck-mounted (Borings B-1 through B-17) or buggy-

⁽²⁾ Boring location not surveyed. Coordinates and elevations are estimated.



mounted (Borings B-18 through B-23) drilling rig and advanced using dry auger method alone. Undisturbed samples of cohesive soils were obtained from the borings by pushing 3-inch diameter thin-wall, seamless steel Shelby tube samplers in general accordance with ASTM D 1587. Granular soils were sampled with a 2-inch split-barrel sampler in accordance with ASTM D 1586. Standard Penetration Test resistance (N) values were recorded for the granular soils as "Blows per Foot" and are shown on the boring logs. Strength of the cohesive soils was estimated in the field using a hand penetrometer. The undisturbed samples of cohesive soils were extruded mechanically from the core barrels in the field and wrapped in aluminum foil; all samples were sealed in plastic bags to reduce moisture loss and disturbance. The samples were then placed in core boxes and transported to the AEC laboratory for testing and further study. Groundwater was not encountered in any of the borings during or after completion of drilling. After completion of drilling, boreholes located on existing pavement were grouted with cement-bentonite grout and existing pavement was patched with cold-placed asphalt patch. Boreholes located on grass were backfilled with bentonite chips.

3.0 <u>LABORATORY TESTING PROGRAM</u>

Soil laboratory testing was performed by AEC personnel. Samples from the borings were examined and classified in the laboratory by a technician under the supervision of a geotechnical engineer. Laboratory tests were performed on selected soil samples to evaluate the engineering properties of the foundation soils in accordance with applicable ASTM Standards. Atterberg limits, moisture contents, percent passing a No. 200 sieve, and dry unit weight tests were performed on selected samples to establish the index properties and confirm field classification of the subsurface soils. Strength properties of cohesive soils were determined by means of torvane (TV), unconfined compression (UC), unconsolidated-undrained (UU), and consolidated-undrained (CU) triaxial tests performed on relatively undisturbed samples. The laboratory test results are presented on the representative boring logs (see Plates A-3 through A-25, in Appendix A). A key to the boring logs, classification of soils for engineering purposes, terms used on boring logs, and reference ASTM Standards for laboratory testing are presented on Plates A-26 through A-29, in Appendix A.

<u>Crumb Dispersion Tests:</u> To evaluate the dispersive characteristics of clayey soils in the detention basin, crumb tests were performed on selected soil samples in accordance with ASTM D 6572, Method A. The results of the crumb tests are summarized on Table 2 and are presented on Plate A-30, in Appendix A.



Table 2. Summary of Crumb Test Results

Sample ID and Description	Dispersive Grade	Dispersive Classification
B-20, 0'-2', Fill: Fat Clay (CH)	1	Non-dispersive
B-20, 6'-8', Fat Clay (CH)	1	Non-dispersive
B-21, 0'-2', Fill: Fat Clay (CH)	1	Non-dispersive
B-21, 8'-10', Fat Clay (CH)	1	Non-dispersive
B-22, 2'-4', Fat Clay (CH)	1	Non-dispersive
B-22, 6'-8', Fat Clay (CH)	1	Non-dispersive
B-23, 0'-2', Fat Clay (CH)	1	Non-dispersive
B-23, 6'-8', Fat Clay (CH)	1	Non-dispersive

Consolidated-Undrained Triaxial Tests: CU triaxial tests were performed in accordance with ASTM D 4767 to determine shear strength parameters of the soils in the detention basin. Using the CU data, AEC plotted the stress paths and determined the k_f (critical state) line from the stress paths in accordance with the US Army Corps of Engineers Engineering Manual, Appendix D, Section D-4. Based on the k_f line, AEC determined the strength parameters (cohesion and friction angle) of the soil. Mohr's circles were developed based on the failure criteria (either maximum effective stress obliquity or maximum deviator stress) presented in ASTM D 4767. The Mohr-Coulomb diagrams (with Mohr's Circles at failure) generated from the CU triaxial tests are included on Plates A-31 and A-32, in Appendix A. The shear strength parameters obtained from the CU triaxial tests are summarized below in Table 3.

Table 3. Summary of Shear Strength Parameters from CU Triaxial Tests

Sample ID and Description	Effectiv	e Stress	Total Stress	
Sample 1D and Description	c' (psf)	φ' (deg)	ccu (psf)	φ _{cu} (deg)
B-21, 6'-8', Fat Clay (CH)	480	12.5	450	9.7
B-23, 4'-6', Fat Clay (CH)	250	17.7	230	13.4

Notes: (1) c' = effective cohesion, $\phi' =$ effective friction angle, obtained from CU tests with pore pressure measurements. (2) $c_{cu} =$ cohesion in total stress, $\phi_{cu} =$ friction angle in total stress, obtained from CU tests.

4.0 SITE CONDITIONS

The existing roadway along the project alignment is a two-lane (one lane in each direction) asphalt roadway (no shoulder) with roadside drainage swales. During our site visit, AEC observed that the existing roadway



between W. Fairground Road and Band Road appears to be in average to good condition, while the roadway between W. Fairground Road and Cottonwood School Road is generally in poor condition. For the segment between W. Fairground Road and Cottonwood School Road, AEC observed numerous roadway longitudinal cracks, occasional transverse cracks, alligator cracking, and rutting in wheel paths AEC also observed asphalt patches at several areas along the roadway. A summary of existing pavement sections encountered in AEC's borings is presented on Table 4 below.

Table 4. Summary of Existing Pavement Thickness

Boring No.	Street	Pavement Section
B-1	Stella Road	5.5" asphalt, 6" gravel base, and 7.75" stabilized sand subbase
B-2	Stella Road	3.5" asphalt, 8.5" stabilized sand and gravel base
B-3	Stella Road	4.5" asphalt, 14" stabilized sand and gravel base
B-4	Stella Road	3.5" asphalt, 6" sand and gravel base, 2.5" asphalt, and 7.5" sand and gravel subbase
B-5	Stella Road	4.5" asphalt, 15" stabilized sand and gravel base
B-6	Stella Road	7" asphalt, 5" stabilized sand and gravel base
B-7	Stella Road	5" asphalt, 12" stabilized sand and gravel base
B-8	Stella Road	4" asphalt, 14.5" stabilized sand base
B-9	Stella Road	4" asphalt, 10" stabilized sand and gravel base
B-10	Stella Road	4" asphalt, 8" stabilized sand and gravel base
B-11	Stella Road	3.5" asphalt, 5.5" stabilized sand and gravel base, and 10" sand and gravel subbase
B-12	Stella Road	4" asphalt, 8" stabilized sand and gravel base
B-13	Stella Road	4" asphalt, 8" stabilized sand and gravel base
B-14	Stella Road	3.5" asphalt, 13.5" sand and gravel base
B-15	Stella Road	4" asphalt, 14" sand and gravel base
B-16	Stella Road	3.5" asphalt, 12.5" sand and gravel base
B-17	Stella Road	3.25" asphalt, 14.25" sand and gravel base

4.1 Subsurface Conditions

Details of the soils encountered during drilling are presented in the boring logs on Plates A-3 through A-25, in Appendix A. Soil strata encountered in the borings are summarized below. Boring log profiles along the project alignment are presented on Plates B-1a through B-1c, in Appendix B.



Boring B-1	Depth (ft) 0 - 1.6 1.6 - 12 12 - 16 16 - 20	Description of Stratum Pavement and base: see Table 4 in Section 4.0 of this report. Soft to very stiff, Fat Clay (CH), with slickensides Soft to very stiff, Lean Clay (CL), with calcareous nodules Loose to medium dense, Silty Sand (SM), wet
B-2	0 - 1 1 - 2 2 - 8 8 - 10 10 - 12 12 - 14	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Stiff to very stiff, Fat Clay (CH), with ferrous nodules Very stiff, Lean Clay (CL), with fat clay and calcareous powder pockets, and calcareous and ferrous nodules Very stiff, Silty Clay (CL-ML), with lean clay pockets, calcareous nodules, and ferrous stains Very stiff, Sandy Lean Clay (CL), with sandy silt partings, silty clay pockets, calcareous nodules, and ferrous stains Sandy Silt (ML), with lean clay pockets, calcareous nodules, and ferrous
	16 - 18 18 - 20	stains Firm to very stiff, Lean Clay (CL), with silt and calcareous powder pockets, and calcareous nodules Stiff to very stiff, Fat Clay (CH), with slickensides, silt pockets and calcareous nodules
B-3	0 - 1.5 1.5 - 2 2 - 6 6 - 10 10 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Fill: stiff to very stiff, Fat Clay (CH), with gravel Very stiff, Fat Clay (CH), with slickensides Very stiff to hard, Lean Clay (CL), with calcareous nodules, pockets, and seams Stiff to hard, Fat Clay (CH), with slickensides
B-4	0 - 1.6 1.6 - 12 12 - 14 14 - 16 16 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Very stiff to hard, Fat Clay (CH) Very stiff, Silty Clay (CL-ML), with ferrous and calcareous nodules, and fat clay pockets Very stiff, Lean Clay (CL), with calcareous nodules and powder pockets Very stiff to hard, Fat Clay (CH), with slickensides
B-5	0 - 1.6 1.6 - 4 4 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Silty Clayey Sand (SC-SM), with gravel Stiff to hard, Fat Clay (CH) Hard, Lean Clay (CL), with calcareous nodules and powder pockets Very stiff to hard, Fat Clay (CH), with slickensides
B-6	0 - 1 1 - 4 4 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: stiff to very stiff, Lean Clay (CL), with gravel Stiff to hard, Fat Clay (CH), with slickensides.



Boring B-7	Depth (ft) 0 - 1.4 1.4 - 2 2 - 4 4 - 20	Description of Stratum Pavement and base: see Table 4 in Section 4.0 of this report. Fill: hard, Sandy Lean Clay (CL), with gravel Fill: very stiff, Fat Clay (CH), with gravel and lean clay seams Stiff to hard, Fat Clay (CH), with slickensides
B-8	0 - 1.5 1.5 - 2 2 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: stiff to very stiff, Fat Clay (CH), with gravel and sand pockets Stiff to hard, Fat Clay (CH), with slickensides
B-9	0 - 1.2 1.2 - 2 2 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: stiff, Fat Clay (CH), with gravel and lean clay pockets Stiff to very stiff, Fat Clay (CH), with slickensides Very stiff, Lean Clay (CL), with sandy silt partings Medium dense to dense, Sandy Silt (ML)
B-10	0 - 1 1 - 4 4 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Stiff to hard, Fat Clay (CH), with slickensides
B-11	0 - 1.6 1.6 - 2 2 - 18 18 - 20	Pavement: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Stiff to hard, Fat Clay (CH), with slickensides Dense, Sandy Silt (ML), with lean clay pockets and siltstone nodules
B-12	0 - 1 1 - 2 2 - 16 16 - 20	Pavement: see Table 4 in Section 4.0 of this report. Fill: stiff to very stiff, Fat Clay (CH), with sandy lean clay seams and gravel Stiff to hard, Fat Clay (CH), with slickensides Dense to very dense, Sandy Silt (ML)
B-13	0 - 1 1 - 4 4 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Firm to hard, Fat Clay (CH), with slickensides Very dense, Sandy Silt (ML), with siltstone nodules
B-14	0 - 1.4 1.4 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Stiff to hard, Fat Clay (CH), with slickensides
B-15	0 - 1.5 1.5 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Stiff to very stiff, Fat Clay (CH), with slickensides Stiff to hard, Lean Clay (CL), with sandy silt seams Dense to very dense, Silt with Sand (ML)
B-16	0 - 1.3 1.3 - 2 2 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: very stiff, Fat Clay (CH), with gravel, sandy lean clay seams, and asphalt pieces Stiff to very stiff, Fat Clay (CH), with slickensides Stiff to very stiff, Lean Clay (CL), with calcareous nodules and silt partings Medium dense, Silt with Sand (ML)



Boring	Depth (ft)	Description of Stratum
B-17	0 - 1.5	Pavement and base: see Table 4 in Section 4.0 of this report.
	1.5 - 2	Fill: very stiff, Sandy Fat Clay (CH), with gravel seams and shell fragments
	2 - 10	Stiff to very stiff, Fat Clay (CH), with slickensides
	10 - 12	Silt with Sand (ML)
	12 - 14	Stiff to very stiff, Lean Clay (CL), with silt seams
	14 - 20	Medium dense, Silt with Sand (ML)
	1. 20	Western delise, Site With Saila (MD)
B-18	0 - 20	Firm to hard, Fat Clay (CH), with slickensides
		··, - ···, (), ··
B-19	0 - 14	Stiff to hard, Fat Clay (CH), with slickensides
	14 - 16	Stiff, Lean Clay (CL), with silt partings and siltstone nodules
	16 - 20	Very dense, Silty Sand (SM), with cement sand nodules
	10 20	to the state of th
B-20	0 - 4	Fill: hard, Fat Clay (CH), with silty sand pockets and gravel
	4 - 14	Stiff to hard, Fat Clay (CH), with slickensides
	14 - 15	Hard, Lean Clay (CL), with slickensides and ferrous nodules
	1. 10	Traita, Boar Gray (GE), With brokensiass and forfous housies
B-21	0 - 2	Fill: hard, Fat Clay (CH), with calcareous nodules and roots
2 2.	2 - 4	Fill: Clayey Sand (SC), with gravel and asphalt pieces
	4 - 15	Stiff to hard, Fat Clay (CH), with slickensides and ferrous nodules
	1 13	Still to hard, I at City (CII), with shekensides and lerrous hodges
B-22	0 - 14	Very stiff to hard, Fat Clay (CH), with slickensides and ferrous nodules
2 	14 - 16	Very dense, Silt (ML), with sandy lean clay pockets
	1. 10	. The state of the
B-23	0 - 15	Stiff to hard, Fat Clay (CH), with slickensides and ferrous nodules
		z

Subsurface Soil Properties: The subsurface clayey soils (including fill but excluding clayey sand) encountered in the borings have medium to very high plasticity (see "Degree of Plasticity of Cohesive Soils" on Plate A-27, in Appendix A), with liquid limits (LL) ranging from 27 to 98, and plasticity indices (PI) ranging from 11 to 69. The cohesive soils encountered are classified as "CL", "CL-ML", and "CH" type soils, and granular soils encountered are classified as "ML", "SC", "SC-SM", and "SM" type soils in accordance with ASTM D 2487. "CH" soils undergo significant volume changes due to seasonal changes in soil moisture contents. "CL" type soils with lower LL (less than 40) and PI (less than 20) generally do not undergo significant volume changes with changes in moisture content. However, "CL" soils with LL approaching 50 and PI greater than 20 essentially behave as "CH" soils and could undergo significant volume changes. Slickensides were encountered in most fat clay (CH) and some lean clay (CL) soils.

<u>Groundwater Conditions:</u> Groundwater was not encountered in any of the boreholes during or upon completion of drilling.



The information in this report summarizes conditions found on the dates the borings were drilled. However, it should be noted that our groundwater observations are short-term; groundwater depths and subsurface soil moisture contents will vary with environmental variations such as frequency and magnitude of rainfall and the time of year when construction is in progress.

4.2 Hazardous Materials

No signs of visual staining or odors were encountered during field drilling or during processing of the soil samples in the laboratory. However, AEC notes that the presence of potential hazardous material at other locations along the project alignments cannot be discounted based upon the very small and limited number of samples taken.

4.3 Geologic Hazards

AEC performed a desk top fault study which included a review of public maps, available literature, and aerial photographs. The project alignment is not covered by the maps entitled "Principal Active Faults of the Houston Area (after O'Neill and Van Siclen, May 1984)", and "Principal Surface Faults in the Central Houston Metropolitan Area (after O'Neill, Van Siclen, with additions by C. Norman, May 13, 2004)". AEC reviewed a University of Houston Master's Thesis entitled A Geophysical Study of Active Faulting in Fort Bend County, Texas concerning faulting in Fort Bend County, Texas (Schmidt, May 2013). According to the thesis, the following fault systems are located in Fort Bend County: the Addicks, the Longpoint, the Needville, the Pleak, the Thompsons, and the Arcola, along with some other short unnamed faults in the eastern portion of the county. The closest fault to the project alignment is the northeast-southwest oriented Pleak Fault located approximately 3.9 miles south of the southeastern end of the project alignment. This fault is too distant to have an impact on the project alignment.

Twenty-three Google Earth aerial photographs from 1985 to 2021 were reviewed. No evidence of faulting was observed from the review of the aerial photographs in or near the project alignment.

AEC does not recommend any further fault studies.



<u>Limitations</u>: The preliminary fault study investigation provided in this report is limited to a review of available literature, aerial photographs and maps, and limited field observations. Distances are scaled from maps. Faults may exist in, cross, or adjoin the project area which were not identified in this report due to the following reasons: limitations of the scope of work and cost; lack of documentation in the literature; the scale of the maps available; lack of visible displacement in the field; and not observed during the reconnaissance due to the presence of obscuring vegetation, man-made structures and environmental features, and modification of the land surface by human activities. Faults may also be present below ground but do not currently have surface expressions. Identification of these faults is beyond the scope of work for this study.

4.4 Subsurface Variations

It should be emphasized that: (i) at any given time, groundwater depths can vary from location to location, and (ii) at any given location, groundwater depths can change with time. Groundwater depths will vary with seasonal rainfall and other climatic/environmental events. Subsurface conditions may vary away from and between the boring locations.

Clay soils in the Greater Houston area typically have secondary features such as slickensides, calcareous and ferrous nodules, and contain sand/silt seams/lenses/layers/pockets/partings. It should be noted that the information in the boring logs is based on 3-inch diameter soil samples which were obtained continuously at intervals of 2 feet from the ground surface to the boring termination depths of 15 to 20 feet below grade. A detailed description of the soil secondary features may not have been obtained due to the small sample size and sampling interval between the samples. Therefore, while a boring log shows some soil secondary features, it should not be assumed that the features are absent where not indicated on the boring logs.

5.0 GEOTECHNICAL ENGINEERING RECOMMENDATIONS

According to the information provided by MEC, Stella Road along the project alignment is an existing 2 lane (one lane in each direction) asphalt roadway, which will be reconstructed to a new 2 lane (one lane in each direction) asphalt roadway with the main traffic lanes widened to 12 feet plus new 6 foot wide shoulders. The roadway will have roadside drainage swales. There will be a detention basin with a depth of 3.4 to 4.3 feet and slope inclination of H:V = 4:1 at the approximate mid-point of the alignment.



<u>Design and Construction Standards:</u> AEC performed the roadway design for Stella Road based on the May 2021 (Draft) FBC Engineering Design Manual (EDM). AEC has also referenced applicable Fort Bend County Construction Details (FBCCD) (dated March 1, 2022) wherever applicable for this project. For technical specifications, the FBCCD makes referrals to Harris County Standard Engineering Design Specifications (HCSEDS). AEC should be notified if different construction specifications should be used, so that our recommendations can be updated if necessary.

5.1 Roadway Reconstruction

The existing roadway along the project alignment is a two-lane (one lane in each direction) asphalt roadway (no shoulder) with roadside drainage swales. Based on drawings (dated April 21, 2022) prepared by MEC, the existing roadway will be reconstructed with two lanes (one lane in each direction), with the main lanes widened to 12 feet, plus 6 foot wide shoulders on each side of the roadway. The reconstructed roadway will have roadside drainage swales on both sides. Based on preliminary plan and profile drawings, AEC portions of the centerline of the new pavement will be placed at or near existing grade, although additional fill will need to be placed in some areas, such as within existing roadside swales to accommodate the roadway widening.

Fort Bend County's Engineering Design Manual (Draft) Requirements: Section 3.12 of the May 2021 FBC EDM requires that the minimum pavement structure for permanent asphalt roadways shall consist of a 3 inch asphalt surface, 8 inch asphalt stabilized base, and 8 inch thick stabilized subgrade.

Traffic Volume: AEC checked the FBC Engineering GIS, Texas A&M Transportation Institute (TTI) "Houston Regional Traffic Count Map", and the Texas Department of Transportation (TxDOT) "District Traffic Web Map" websites to determine the classification of the existing roadways within the project area and to check if any traffic count data was available. The FBC GIS website does not list Stella Road as either a 'collector' or 'major thoroughfare' roadway and traffic data for the Stella Road was not available at the time this report was prepared. The TTI and TxDOT websites also do not have traffic data for Stella Road, however it has 24 hour traffic volume counts for Cottonwood School Road with which Stella Road intersects. Traffic data for Cottonwood School Road are shown on Table 5 below.



Table 5. Summary of Available Traffic Data

Traffic Count Location	Year	24 Hour Traffic Volume (vpd)	Source of Traffic Data
1100 Cottonwood School Road	2006 2012	2,940 2,250	TTI
	2021	1,177	TxDOT

Form the traffic volume count from the TTI and TxDOT websites, it appears that the traffic volume on Cottonwood School Drive decreased from 2006 to 2021. However, traffic volume changes hour to hour, from weekdays to weekend, and season to season. AEC cannot validate the traffic volume decrease based on 24 hour volume counts from 2006, 2012, and 2021 unless further data becomes available. In the absence of other data, AEC conservatively used the 2006 traffic count data obtained from the TTI website for analyses. An annual traffic growth rate of 3 percent was assumed. A design life of 20 years was considered, which is typical for asphalt pavement roadways in the area. Based on the 2006 TTI traffic data and the assumed traffic volume growth rate, AEC estimated a 2022 ADT of 4,611 vpd. AEC should be notified if different traffic loads, design parameters, or pavement design life are required for pavement design at the site so that our recommendations can be updated accordingly.

5.1.1 Roadway Design Traffic Load

A traffic analysis was not available at the time this report was prepared and AEC understands that FBC is not planning to perform one for the Stella Road reconstruction project. As noted in Section 5.1 of this report, AEC has estimated roadway design traffic loading based on traffic count data from Cottonwood School Road in 2006, 2012, and 2021. AEC notes that traffic data that is from an adjoining roadway should be considered *unreliable* for estimating the traffic loading on Stella Road. AEC recommends that a current traffic count be performed along the Stella Road alignment, if possible, so that the roadway design traffic load estimate can be updated.

Estimated Traffic Loads: Pavement design is based on the anticipated design number of 18-kip Equivalent Single Axle Loads (ESAL) the pavement is subjected to during its design life. The equation to calculate the number of 18-kip ESAL repetitions to use for pavement design is presented in Equation (1). Assumptions made by AEC to estimate 18-kip ESAL repetitions are presented on Table 6.

18-kip ESAL = $(ADT)(T)(T_f)(D)(L)(G)(Y)(365)$ Equation (1)



where: ESAL = 18-kip Equivalent Single-Axle Load repetitions.

ADT = Average Daily Traffic, vehicles per day.

T = Percent of heavy trucks.

T_f = Truck factor. D = Directional factor.

L = Lane factor. G = Growth factor.

Y = Design life, in years.

Table 6. Traffic Volume Parameters for Asphalt Pavement Design

Traffic Parameter	Stella Road from Band Road to Cottonwood School Road	
Average Daily Traffic (ADT) projected for 2022	4,611 vpd (conservatively estimated from 2006 data)	
ADT projected for 2042	8,401 vpd	
ADT Percent Heavy Duty Truck (T), FHWA Class 5 and higher	5% (assumed)	
Truck Factor (T _f)	0.60 (assumed)	
Directional Factor (D)	0.5 (two way road)	
Lane Factor (L)	1.0 (one lane in each direction)	
Total Growth Rate Factor (G)	1.34 (3% annual growth rate from 2022 to 2042, assumed)	
Design Life (Y)	20 years (assumed)	
Estimated 18-kip ESAL Loading over Design Life	678,526	

AEC notes that the calculated number of 18-kip ESAL repetitions is highly sensitive to parameters such as percent heavy trucks, truck factor, and traffic volume growth rate in pavement design. Differences between assumed and actual traffic parameters can have significant effects on overall pavement thickness design and ultimate pavement performance. AEC should be notified if different traffic loads or design parameters are required for pavement design at the site so that our analysis can be updated accordingly.

5.1.2 Asphalt Pavement Design

Flexible pavement design procedure includes determination of the structural number (SN) for the proposed pavement, as well as the thickness of individual components of the surface course, base course, and subgrade. The basic equation developed by the AASHTO Road Test is:



$$SN = a_1(D_1) + a_2(D_2) + a_3(D_3)$$
Equation (2)

where: SN = Structural Number for the total flexible pavement structure.

 a_1, a_2, a_3 = layer coefficients for surface, base, and subgrade course, respectively. D_1, D_2, D_3 = thickness of surface, base, and subgrade course, respectively, in inches.

Layer coefficients used for design are presented on Table 7.

Table 7. Layer Coefficients for Asphalt Pavements

Pavement Layer	Layer Coefficient
Hot Mix Asphaltic Concrete (HMAC)	a1 = 0.44
Black Base	a2 = 0.34
Stabilized Subgrade*	a3 = 0.11

Note: (*) Subgrade stabilization recommendations are presented in Section 5.1.4 of this report.

The parameters that were used in computing the flexible pavement for the Stella Road are as follows:

Roadbed Soil Resilient Modulus (M _R)	1,500 psi
Drainage Coefficient (C _d)	1.0
Overall Standard Deviation (S ₀)	0.45
Reliability Level (R)	90%
Initial Serviceability (P ₀)	4.2
Terminal Serviceability (P _t)	2.5

AEC should be notified if different parameters are required for asphalt pavement design. As directed by MEC, AEC performed designs for two asphalt pavement sections. AEC first considered a pavement section that meets the FBC EDM minimum requirements and then estimated a service life based on the design roadway traffic load presented on Table 6 in Section 5.1.1 of this report. AEC then considered a pavement section that will provide a load capacity that meets the estimated 20 year design life roadway traffic load that was presented in Table 6. The pavement section that meets FBC EDM minimum thickness requirements is presented on Table 8.

Table 8. Asphalt Pavement Section based on FBC Minimum Thickness Requirement

Pavement Layer	Stella Road from Band Road to Cottonwood School Road	
Hot Mix Asphaltic Concrete	3"	



Pavement Layer	Stella Road from Band Road to Cottonwood School Road
Black Base (BB)	8"
Stabilized Subgrade*	8"
Structural Number (SN)	4.92
Pavement 18-kip ESAL Load Capacity	226,828
Estimated 18-kip ESAL Loading over Design Life (See Table 6)	678,526
Estimated Service Life	8 years

Note: (*) Subgrade stabilization recommendations are presented in Section 5.1.4 of this report.

Based on the DARWin v3.0 computer program, the pavement section presented in Table 8 should sustain 226,828 repetitions of 18-kip ESALs. As presented on Table 6 in Section 5.1.1 of this report, AEC estimated a traffic loading of 678,526 18-kip ESALs over a 20 year design life. Based on AEC's current assumptions, AEC estimates that the service life of the FBC EDM minimum pavement section would be reached in about 8 years. As noted in Section 5.1.1 of this report, AEC notes that our estimate of service life is based on traffic data from an adjacent roadway, as well as assumptions made without supporting data being available. The service life presented by AEC should therefore be considered a very rough estimate.

As noted above, AEC performed a second pavement design which provides a load capacity that meets the estimated traffic loading of 678,526 18-kip ESALs over a 20 year design life. The pavement section that meets AEC's estimated design loading is provided on Table 9 below.

Table 9. Recommended Asphalt Pavement Section based on 20 year Design Life

Pavement Layer	Stella Road from Band Road to Cottonwood School Road
Hot Mix Asphaltic Concrete	3.5"
Black Base (BB)	10"
Stabilized Subgrade*	8"
Structural Number (SN)	5.82



Pavement Layer	Stella Road from Band Road to Cottonwood School Road
Pavement 18-kip ESAL Load Capacity	760,267
Estimated 18-kip ESAL Loading over Design Life (See Table 6)	678,526

Note: (*) Subgrade stabilization recommendations are presented in Section 5.1.4 of this report.

Based on the DARWin v3.0 computer program, the pavement section presented on Table 9, should provide an estimated load capacity of 760,267 18-kip ESALs, which exceeds the estimated 20 year design traffic load. AEC notes that MEC's drawings (dated April 21, 2022) show a proposed pavement section that matches the section presented in Table 9 above.

The DARWin outputs for asphalt pavement are presented on Plate C-1 through C-3, in Appendix C, for reference. The design engineer should verify whether the proposed pavement section will provide enough ESALs for the anticipated amount of site traffic. AEC should be notified if different standards or constants are required for pavement design at the site, so that our recommendations can be updated accordingly.

Asphalt Pavement: HMAC pavement should be constructed in general accordance with Item 340 of the 2017 HCSEDS. The HMAC shall be placed and compacted in accordance with 'Compaction Criteria' requirements of Section 340.14 of HCSEDS Item 340, which is 3 to 8 percent air voids as determined by TxDOT test method Tex-207-F, Part III.

<u>Black Base:</u> Asphalt-stabilized (black) base shall be in accordance with Item 250 of the 2017 HCSEDS. The black base shall be placed and compacted in accordance with 'Compaction Criteria' requirements of Section 250.12 of HCSEDS Item 250, which is 3 to 8 percent air voids as determined by TxDOT test method Tex-207-F, Part III.

<u>Prime Coat:</u> The surface of the completed subgrade and base course should be primed in accordance with Item 310 of the 2017 HCSEDS.

5.1.3 Roadway Fill

For roadway areas that require fill to achieve final grade, existing pavement and base (if any), vegetation, trees, roots, organic soils, and other deleterious materials should first be removed and wasted. The exposed



soils should then be proof-rolled in accordance with Item 216 of the 2014 TxDOT Standard Specifications for Construction and Maintenance of Highways, Streets, and Bridges to identify and remove any weak, compressible, or other unsuitable materials; such materials should be replaced with compacted clay fill. After proof rolling, compacted general clay fill should be used to raise existing grade to final grade.

For areas where new pavement will be placed, the top 8 inches of the exposed subgrade (after general clay fill has been placed) should then be stabilized with a minimum of 7 percent hydrated lime (by dry soil weight). Recommendations for lime subgrade stabilization and general clay fill are included in Sections 5.1.4 and 5.3.3 of this report, respectively.

5.1.4 Pavement Subgrade Preparation

As noted in Section 5.1 of this report, based on preliminary plan and profile drawings, AEC portions of the centerline of the new pavement will be placed at or near existing grade, although additional fill will need to be placed in some areas, such as within existing roadside swales to accommodate the roadway widening.

Based on Borings, B-2 through B-6, B-9, B-11, B-12, B-18, and B-19, the surficial soils along the existing Stella Road alignment primarily consist of lean/fat clay (CL/CH). However, more than 6 inches of clayey sand (SC) fill material was encountered at the ground surface in Borings B-2 and B-5, which AEC assumes is fill material placed for the original roadway construction, and AEC also assumes that this material will be left in place. In general, AEC anticipates that most of the pavement subgrade will require stabilization with hydrated lime. However, lime and fly ash stabilization will be required whenever clayey sand (SC) fill material is exposed along the roadway alignment.

Subgrade Preparation: Subgrade preparation should extend a minimum of 2 feet beyond the paved area perimeters. Existing pavement and base should be removed in accordance with Item 540 of the 2017 HCSEDS. Reclaimed asphalt pavement (RAP) can be reused for HMAC and/or asphalt-stabilized (black) base mixes in accordance with Items 340 and 250 of the 2017 HCSEDS, respectively. For new widening areas to be paved (that did not previously have pavement and base on top), a minimum of 3 inches of surface soils, existing vegetation, trees, roots, and other deleterious materials should be removed and wasted. The excavation depth should be increased when inspection indicates the presence of organics and deleterious materials to greater depths. The exposed soils should then be proof-rolled in accordance with Item 216 of the 2014 TxDOT Standard Specifications for Construction and Maintenance of Highways,



Streets, and Bridges to identify and remove any weak, compressible, or other unsuitable materials; such materials should be replaced with compacted clay fill. General clay fill recommendations are presented in Section 5.3.3 of this report.

After proof rolling, scarify the exposed subgrade to a depth of 8 inches and stabilize with either hydrated lime or lime fly ash, depending on type of exposed subgrade soil. Exposed clay should be stabilized with a minimum of 7 percent lime (by dry soil weight). Exposed sand or silt should be stabilized with a minimum of 3 percent hydrated lime and 7 percent fly ash (by dry soil weight). Lime and lime fly ash stabilization shall be performed in accordance with Items 220 and 223 of the 2017 HCSEDS, respectively. The percentage of lime and lime fly ash required for stabilization is a preliminary estimate for planning purposes only; laboratory testing (such as optimum lime content versus pH, in accordance with ASTM D6276) should be performed to determine optimum contents for stabilization prior to construction. The stabilized soils should be compacted to 95 percent of their ASTM D698 (Standard Proctor) dry density at a moisture content ranging from optimum to 3 percent above optimum.

5.1.5 Roadside Drainage Swales

Based on the drawings prepared by MEC, roadside drainage swales will be added on both sides of the new roadway. According to FBC EDM, a maximum slope inclination of H:V = 4:1 should be considered for the slope adjacent to the roadway, and a slope inclination of H:V = 3:1 for the slope on the opposite side can be considered. In general, AEC recommends that the flattest side slopes that are possible along the project alignments be used for swale design. Roadside drainage swale construction shall be performed in accordance with applicable 2018 FBCCD Construction Details.

5.2 Detention Basin

AEC notes that in April 2022, there were two detention basins shown on MEC's drawings that were available at the time Borings B-20 through B-23 were drilled. However, based on updated drawings provided by MEC in July 2022, the eastern detention basin (near the intersection of Stella Road and W. Fairground Road) was deleted from the project area. As a result, only Borings B-22 and B-23 are in the vicinity of the remaining west detention basin (near the S-curve at the mid-point of the project alignment).



Detention basin plan and cross section drawings (prepared by r.g. Miller Engineers) are presented on Plates D-1 and D-2, in Appendix D, for reference. The basin detention storage volume is 9.6 acre-feet (considering 1 foot of freeboard); the 100 year water surface elevation (WSE) is +94.20 feet Mean Sea Level (MSL). Top of bank around the basin has an elevation of +94.85 feet MSL. The toe of slope around the basin has an elevation range of +91.44 to +91.45 feet MSL. The bottom of basin flowline (i.e. pilot channels) is at an elevation of approximately +89.7 feet MSL. The resulting basin depths range from 3.4 to 4.3 feet, and the basin slopes will have an inclination of H:V = 4:1.

<u>Soil and Groundwater Conditions in Basin Area:</u> Based on Borings B-22 and B-23, the soil conditions in the detention basin area generally consist of stiff to hard fat clay (CH), considering a basin depth of 3.4 to 4.3 feet. Groundwater was not encountered in the borings during drilling.

5.2.1 Slope Stability Analysis

Based on AEC's borings and the basin cross section drawings (see Plate D-2, in Appendix D), AEC selected the southeast bank (closest to the Stella Road S-curve) of the detention basin (using Cross Section A-A) as the 'most critical' section to perform basin slope stability analysis on. AEC performed the slope stability analyses based on three different conditions: the short-term condition, long-term condition, and rapid drawdown condition. Based on AEC's previous discussions with the Fort Bend County Drainage District (FBCDD), there are no minimum factor of safety (FS) requirements for slope stability analyses for detention basins in Fort Bend County. AEC should be notified if FBCDD has different requirements so that our analyses can be revised if necessary.

Clay Desiccation Zone: AEC notes that the clay soils present in the top 10 feet of Borings B-22 and B-23 predominantly consist of fat clay (CH). These clays have plasticity indices that range from 41 to 57, indicating that the clay soils have high to very high expansive potential (see "Degree of Plasticity of Cohesive Soils" on Plate A-27, in Appendix A). Exposing these clays to the atmosphere and cycles of wetting-drying from seasonal moisture changes will result in desiccation, cracking, and a reduction in their shear strengths, which in turn will result in progressive slope movement and eventual slope failure. For fat clay soils, we considered a desiccation zone of approximately 8 feet below the ground/slope surface. For fat clay within the desiccation zone, we estimated effective stress residual shear strengths (c'_r and ϕ '_r) to evaluate slope stability for both the long-term condition and rapid drawdown condition based on A. Saleh and S. Wright (1977). We also reduced the c' and c_{cu} of lean clay soils (with a PI greater than 20) within the



non-desiccated (i.e. weathered) zone based on a combination of methods by G. Mesri (1999) and S. Wright (2005).

<u>Soil Parameters:</u> Soil parameters used in the analyses include moist unit weights, unconsolidated-undrained (UU) shear strengths, effective stress shear strength (developed using total stress parameters and pore water pressure measurements), and total stress shear strength obtained from consolidated-undrained (CU) triaxial tests. Design soil parameters used for AEC's slope stability analyses on the southeast bank of the detention basin are presented on Plate D-3, in Appendix D.

We used the Simplified Bishop Method of Slices option in the 2021 GeoStudio (SLOPE/W) computer program to analyze slope stability for 2-dimensional limiting equilibrium. The program has the capability to compute pore water pressures based on a defined piezometric surface.

<u>Groundwater Level:</u> For the analyses, we considered different groundwater conditions for short-term, long-term, and rapid drawdown conditions. Since groundwater was not encountered in Borings B-22 or B-23 during drilling, a groundwater table was not considered for either the short term or long term condition. For rapid drawdown condition, AEC considered the groundwater level to be at the ground surface, from top of the slope to the proposed basin bottom; this models a post-flood condition, where the basin fills with stormwater and then drains quickly, before pore-water pressures in the slope are allowed to dissipate.

<u>Required Safety Factor:</u> Stability analyses for the basin slopes were conducted for the short-term (end-of-construction), long-term, and rapid drawdown conditions. A brief description of these conditions is presented below:

- 1. <u>Short Term/End-of-Construction Condition</u> This condition models rapid construction loading taking place, so that there is no time for the induced excess pore water pressure to dissipate or for consolidation to occur during the loading period. UU shear strength parameters were used for this analysis.
- 2. <u>Long-Term Condition</u> This condition models long-term steady seepage through embankments and the long-term stability of slopes in stiff clays. Effective stress shear strength parameters (obtained from CU triaxial tests with pore water pressure measurements) were used for this analysis.
- 3. <u>Rapid Drawdown Condition</u> Most slope failures in the Gulf Coast area occur under rapid drawdown conditions. This condition models when the slope becomes fully saturated and



consolidated and is at equilibrium with the existing stress system, then encounters rapid drawdown and simultaneously allows no drainage to occur. Total stress shear strength parameters (obtained from CU triaxial tests) were used for this analysis.

<u>Basin Slope Stability:</u> Using the soil information encountered in Boring B-23, AEC performed slope stability analyses on the southeast bank of the basin based on the "A-A" cross-section presented on r.g. Miller Engineers drawings (see Plate D-2, in Appendix D). Design soil parameters used for the slope stability analyses are presented on Plate D-3, in Appendix D. A 300 psf construction surcharge was added to the top of bank for the short-term condition while a 250 psf surcharge was added to the top of the bank for the long-term and rapid drawdown conditions.

The results of the basin slope stability analyses under short-term, long-term, and rapid drawdown conditions are presented on Plates D-4 through D-9, in Appendix D. A summary of the FS for the proposed basin slopes under short-term, long-term, and rapid drawdown conditions is presented on Table 10.

Table 10. Detention Basin Slope Stability Analysis Results (Based on Boring B-23)

Section Analyzed	Estimated Minimum Factor of Safety (FS)		
Section Analyzed	Short-Term	Long-Term	Rapid Drawdown
Southeast Bank of Basin, H:V = 4:1, Global Slide	17.66 (Plate D-4)	2.22 (Plate D-6)	1.52 (Plate D-8)
Southeast Bank of Basin, H:V = 4:1, Local Slide	19.43 (Plate D-5)	1.85 (Plate D-7)	1.31 (Plate D-9)

5.2.2 Protection of Basin Slopes and Bottom

<u>Erosion Protection:</u> Based on AEC's borings, most of the soils that will be encountered in the basin slopes and bottom areas will consist of stiff to hard fat clay (CH) soils with low erosion potential. AEC does not anticipate that erosion protection (such as riprap, liners, or articulating blocks) will be required for the basin slopes or bottom.

<u>Dispersive Soils:</u> Based on the results of the crumb dispersion tests (see Table 2 in Section 3.0 of this report), the soils within the proposed basin consist of non-dispersive clays. Additional remediation of the basin slopes or bottoms for potentially dispersive soils is not required.



5.2.3 <u>Basin Excavation</u>

Basin Soil and Groundwater Conditions: Considering a basin depth ranging from 3.4 to 4.3 feet below grade, AEC anticipates that the basin excavation will generally encounter stiff to hard fat clay (CH) soils. Since groundwater was not encountered in Borings B-22 and B-23 (or any of AEC's borings), AEC does not anticipate that groundwater will be encountered during basin excavation. However, there could still be some groundwater seepage into the excavation; open drainage method (such as sump and pump) will likely be sufficient for groundwater control, if necessary. However, if the seepage rate into the excavation is insufficient for open drainage method to control, then pre-drainage method (such as ejector/eductor type systems) may be necessary. Generalized groundwater control recommendations are presented in Section 6.2 of this report.

<u>Excavations</u>: The contractor should be responsible for designing, constructing, and maintaining safe excavations and protecting existing structures in the vicinity of the proposed detention basin. Excavations should be in accordance with OSHA, Safety and Health Regulations, 29 CFR, Part 1926, Subpart P (Excavation and Trenches).

We recommend that the surcharge on the basin banks be limited to 300 psf or less during construction. AEC recommends that general clay fill be used for basin berm construction. If fill will be placed on slopes steeper than H:V=4:1, then the slopes should be cut back into benches to provide a good construction joint between the existing soil and new fill. Where possible, each bench should be a minimum of 8 feet wide and a maximum of 3 feet high. Voids in the excavated slopes (if any) should be backfilled with general clay fill in accordance with Section 5.3.3 of this report.

Re-use of Basin Borrow Soil as Select Clay Fill: Based on Borings B-22 and B-23, none of the clay soils that will be excavated in the basin area (considering a basin depth of 3.4 to 4.3 feet) meet the requirements of select clay fill, as presented in Section 5.3.2 of this report. AEC recommends that the excavated basin soils be limited to use as general clay fill soil only, as presented in Section 5.3.3 of this report. Alternatively, the excavated soil can be treated with lime and then used as lime-stabilized clay, in accordance with Section 5.3.1 of this report.



5.3 Fill Requirements

5.3.1 Lime Stabilized Clay

Soils Stabilized with Hydrated Lime: AEC prefers that lime-stabilized clay be used as structural fill. Either: (i) imported lime-stabilized clay soils (stabilized offsite before delivery to the project site); or (ii) clay soils excavated onsite and treated with hydrated lime can be used. Clay soils excavated onsite should first be stabilized with a minimum of 7 percent hydrated lime (by dry soil weight). The amount of hydrated lime provided in this report is for estimation purposes only. The actual amount of lime required for stabilization should be determined by lime-series curve or pH method in a laboratory prior to construction. Lime stabilization should be done in general accordance with Item 220 of the 2017 HCSEDS. AEC prefers using stabilized soil as structural fill since compacted stabilized soil generally has high strength, low compressibility, and relatively low permeability.

<u>Lifts and Compaction:</u> Lime-stabilized clay fill should be placed in loose lifts not exceeding 8 inches in thickness. Backfill within 3 feet of walls or columns should be placed in loose lifts no more than 4-inches thick and compacted using hand tampers, or small self-propelled compactors.

Lime-stabilized clay should be compacted to a minimum of 95 percent of the ASTM D 698 (Standard Proctor) maximum dry unit weight at a moisture content ranging between optimum and 3 percent above optimum.

5.3.2 Select Clay Fill

<u>'Select' Clay Fill:</u> It is AEC's experience that 'select' clay fill material imported from sand and clay pits in the Greater Houston area is generally non-homogenous (i.e., composed of a mixture of sands, silts, and clays, instead of a homogenous sandy clay material) and of poor quality, and either contains too much sand or has large clay clods with high expansive potential. Use of this non-homogenous soil can result in poor long term performance of structures and pavements placed on top of the fill.

<u>Precautions:</u> Prior to construction, the Contractor should determine if they can obtain qualified select clay fill meeting the below select fill criteria. The closest sand and clay pit to the project site may not be able to deliver fill material that meets the requirements below. The Contractor should also be aware



that testing of select clay fill (see below) typically takes a minimum of 1.5 days to complete and they should accommodate testing in their fill placement in their project schedule. In addition, imported fill that is delivered to the project site may vary from day to day; material delivered to the site may pass one day but fail the next.

AEC notes that although lime-stabilized clay (see Section 5.3.1 of this report) may have a higher material and/or labor cost compared to select clay fill; the delays associated with locating, testing, and approving qualified select clay fill may exceed the costs of using lime stabilized clay instead. Potential risks associated with importing poor quality fill material to the site include: (i) accepting delivery of fill material that does not meet specifications, which could end up as wasted material if there is no use for it in other applications; (ii) removal of already placed lifts of compacted soil prior to laboratory testing results becoming available, resulting in schedule delays; and (iii) bringing additional equipment onsite to further manipulate the fill, such as a pulvimixer.

<u>Requirements:</u> Select clay fill (whether imported from offsite or if it is already onsite) should consist of <u>uniform</u>, non-active inorganic lean clays with a PI between 10 and 20 percent, and more than 50 percent passing a No. 200 sieve. Material intended for use as select fill shall not have clay clods with PI greater than 20, clay clods greater than 2 inches in diameter, or contain sands/silts with PI less than 10. Sand and clay mixtures/blends are unacceptable for use as select fill. Sand/silt with clay clods is unacceptable for use as select fill. Mixing sand into clay or mixing clay into sand/silt is also unacceptable for use as select fill. The testing lab shall <u>reject</u> any material intended for use as select fill that does not meet the PI, sieve, and clay clod requirements above, without exceptions.

<u>Lifts and Compaction:</u> All material intended for use as select fill should be tested prior to use to confirm that it meets select fill criteria. The fill should be placed in loose lifts not exceeding 8 inches in thickness. Backfill within 3 feet of walls or columns should be placed in loose lifts no more than 4-inches thick and compacted using hand tampers, or small self-propelled compactors.

Select fill should be compacted to a minimum of 95 percent of the ASTM D 698 (Standard Proctor) maximum dry unit weight at a moisture content ranging between optimum and 3 percent above optimum.



5.3.3 General Clay Fill

General Clay Fill: AEC recommends that general clay fill consist of a clean, cohesive soil (USCS Classification "CL" or "CH"). Granular soils (i.e., sands, silts, and gravel; not more than 50 percent retained on No. 200 sieve) should not be used as general clay fill.

General clay fill should be placed in loose lifts not exceeding 8 inches in thickness. General clay fill should be compacted to 95 percent of its ASTM D 698 (Standard Proctor) maximum dry unit weight at a moisture content ranging between optimum and 3 percent above optimum.

6.0 <u>CONSTRUCTION CONSIDERATIONS</u>

6.1 Site Preparation

To mitigate site problems that may develop following prolonged periods of rainfall, it is essential to have adequate drainage to maintain a relatively dry and firm surface prior to starting any work at the site. Adequate drainage should be maintained throughout the construction period. Methods for controlling surface runoff and basining include proper site grading, berm construction around exposed areas, and installation of sump pits with pumps.

6.2 Groundwater Control

The need for groundwater control will depend on the depth of excavation relative to the groundwater depth at the time of construction. If there is heavy rain prior to or during construction, the groundwater table may be higher than indicated in this report; higher seepage is also likely and may require a more extensive groundwater control program. In addition, groundwater may be pressurized in certain areas of the alignment, requiring further evaluation and consideration of the excess hydrostatic pressures. Groundwater control should be in general accordance with Item 436 of the 2017 HCSEDS.

The Contractor should be responsible for selecting, designing, constructing, maintaining, and monitoring a groundwater control system and adapt his operations to ensure the stability of the excavations. Groundwater information presented in Section 4.1 of this report and elsewhere, along with consideration for potential environmental and site variation between the time of our field exploration and construction, should



be incorporated in evaluating groundwater depths. The following recommendations are intended to guide the Contractor during design and construction of the dewatering system.

Groundwater control methods typically can be classified into three categories: (i) open pumping, where water is allowed to flow into an excavation and is collected in ditches or sumps and pumped away; (ii) predrainage, where the water table is lowered before excavation using wellpoints, ejector/eductor systems, deep wells, etc.; and (iii) cut off or exclusion, where the groundwater is prevented from entering the excavation by an impermeable barrier, such as by sheet piling, grouting, deep soil mixing, ground freezing, slurry shields, etc.

<u>Cohesive Soils:</u> Groundwater control in cohesive soils can typically be performed using open pumping methods. Seepage rates are lower than in granular soils and groundwater is usually collected in sumps and/or channeled by gravity flow to storm sewers. If cohesive soils contain significant secondary features, seepage rates will be higher. This may require larger sumps and drainage channels, or if significant granular layers are interbedded within the cohesive soils, methods used for granular soils may be required. Where it is present, pressurized groundwater will also yield higher seepage rates.

Granular Soils: Groundwater control in granular soils will typically require predrainage methods or cutoff/exclusion methods. For excavations that are less than 15 feet deep that will occur within saturated sands, a predrainage method such as wellpoints can be considered. For excavations that are greater than 15 feet deep, other predrainage methods that can be considered include multiple staged wellpoints, ejectors/eductors (primarily for use when silty soils are present), or deep wells with submersible pumps. Generally, with predrainage methods, the groundwater depth should be lowered at least 3 feet below the excavation bottom to be able to work on a firm surface when water-bearing granular soils are encountered.

If predrainage methods cannot be used, then a cutoff/exclusion method such as interlocking water-tight sheet piles, drilled shaft/secant pile wall (with grout between the shafts/piles), or jet grouting of the granular strata may be necessary.

Extended Dewatering: Extended and/or excessive dewatering can result in settlement of existing structures in the vicinity of the dewatering operations; the Contractor should take the necessary precautions to minimize the effect on existing structures in the vicinity of the dewatering operation. We recommend that the Contractor verify the groundwater depths and seepage rates prior to and during construction and retain



the services of a dewatering expert (if necessary) to assist them in identifying, implementing, and monitoring the most suitable and cost-effective method of controlling groundwater.

Bottom Heave or Boiling: For excavation in cohesive soils, the possibility of bottom heave must be considered due to the removal of the weight of excavated soil. In lean and fat clays, heave normally does not occur unless the ratio of Critical Height to Depth of Cut approaches one. In silty clays, heave does not typically occur unless an artificially large head of water is created using impervious sheeting in bracing the cut. If the excavation extends below groundwater and the soils at or near the bottom of the excavation are mainly sands or silts, the bottom can fail by blow-out (boiling) when a sufficient hydraulic head exists. The potential for boiling or in-flow of granular soils increases where the groundwater is pressurized. To reduce the potential for boiling of excavations terminating in granular soils below pressurized groundwater, the groundwater table should be lowered at least 3 feet below the excavation.

<u>Perched Water (If Present):</u> Although it may be present at a shallower depth than the normal groundwater level, perched water should still be considered a form of groundwater. If perched water is encountered during the construction phase, the groundwater control methods mentioned above would still be the same. Depending on the size of the perched reservoir and recharge rates, the contractor should not assume that perched water can be completely dewatered during a normal construction period.

6.3 Construction Monitoring

Pavement construction and subgrade preparation, as well as excavation of detention basin should be monitored by qualified geotechnical professionals to check for compliance with project documents and changed conditions, if encountered. AEC should be allowed to review the design and construction plans and specifications prior to release to check that the geotechnical recommendations and design criteria presented herein are properly interpreted.

7.0 <u>LIMITATIONS</u>

The information contained in this report summarizes conditions found on the dates the borings were drilled. The attached boring logs are true representations of the soils encountered at the specific boring locations on the dates of drilling. Reasonable variations from the subsurface information presented in this report should be anticipated. If conditions encountered during construction are significantly different from those



presented in this report; AEC should be notified immediately.

This investigation was performed using the standard level of care and diligence normally practiced by recognized geotechnical engineering firms in this area, presently performing similar services under similar circumstances. This report is intended to be used in its entirety. The report has been prepared exclusively for the project and location described in this report. If pertinent project details change or otherwise differ from those described herein, AEC should be notified immediately and retained to evaluate the effect of the changes on the recommendations presented in this report and revise the recommendations if necessary. The recommendations presented in this report should not be used for other structures located along the alignments or similar structures located elsewhere, without additional evaluation and/or investigation.



APPENDIX A

Plate A-1 Vicinity Map

Plate A-2 Boring Location Plan

Plates A-3 to A-25 Boring Logs
Plate A-26 Key to Symbols

Plate A-27 Classification of Soils for Engineering Purposes

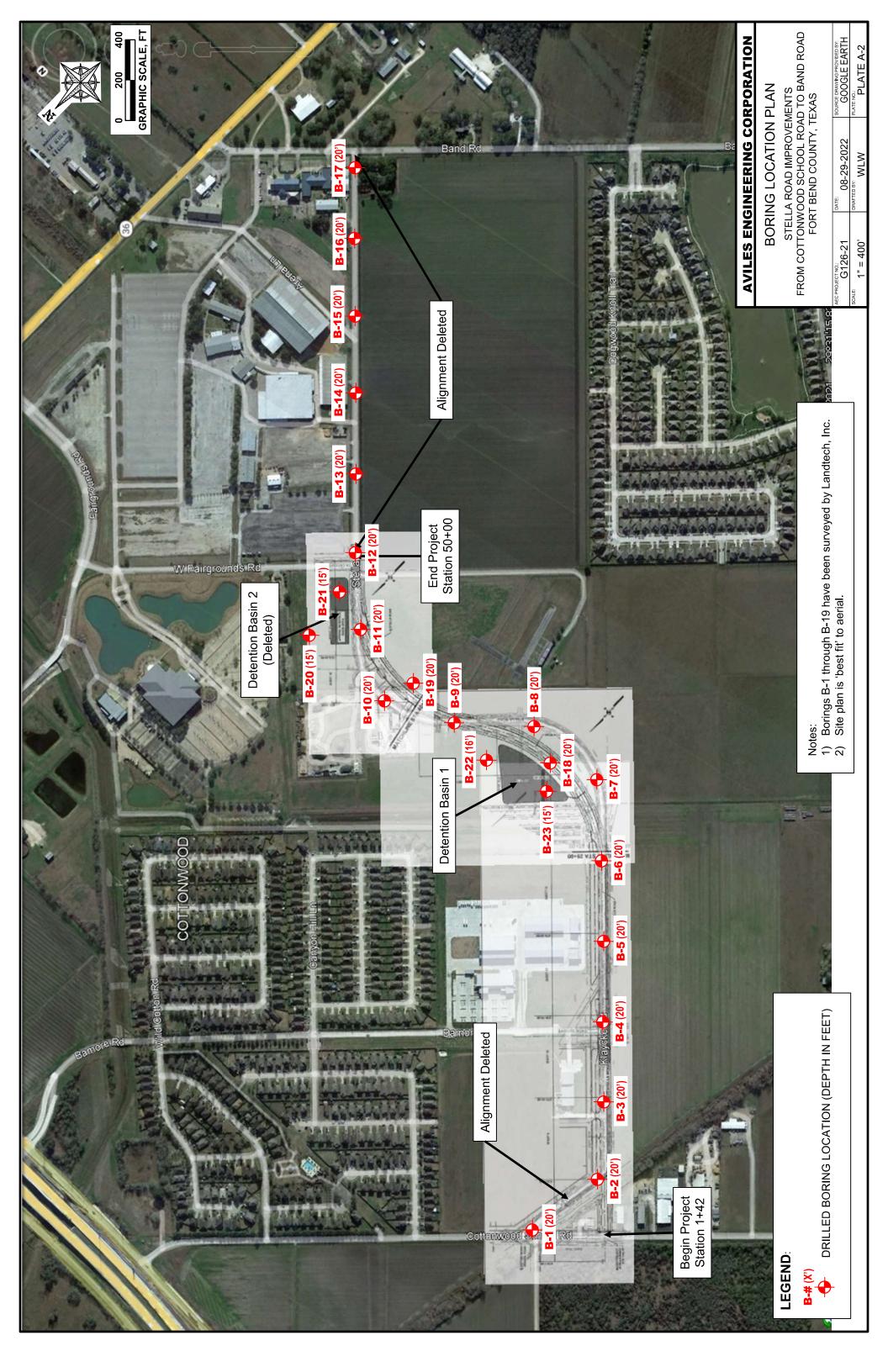
Plate A-28 Terms Used on Boring Logs

Plate A-29 ASTM & TXDOT Designation for Soil Laboratory Tests

Plate A-30 Crumb Test Results

Plates A-31 and A-32 Mohr-Coulomb Diagrams (from CU Triaxial Tests)







ENGINEERING CORP. BORING

B-1

DATE 6/10/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** PLASTICITY Eastina: 2977451.48 Pocket Penetrometer SYMBOL РΤ Northing: 13752613.63 Torvane Elevation: 99.65 Pavement: 5.5" asphalt Base: 6" gravel Subbase: 7.75" stabilized sand 96 67 25 42 35 Soft to very stiff, dark gray Fat Clay (CH), with slickensides 41 79.6 -with ferrous nodules 2'-12' -tannish gray 4'-6', with calcareous nodules 4'-32 -dark brown and gray 6'-8' 96 60 19 | 41 25 8 -tannish gray 8'-10' 30 94.1 -tan, with calcareous powder pockets 10'-12' 24 12 Soft to very stiff, tan and gray Lean Clay (CL), 40 15 | 25 with calcareous nodules 17 -with calcareous powder pockets 12'-14' -with silty clay pockets and and silt seams 14'-19 16 Loose to medium dense, tan Silty Sand (SM), wet 13 22 -tan and light gray, with fat clay seams 18'- 20' 45 10 28 20 Termination Depth = 20 feet 24 28 BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons YY YY/DN



PROJECT: Stella Road Improvements ENGINEERING CORP. **BORING B-2** DATE 6/10/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF DESCRIPTION MOISTURE CONTENT, DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** PLASTICITY 200 MESH Eastina: 2977409.05 \bigcirc Pocket Penetrometer SYMBOL РΤ 13752122.69 Northing: ЖY Torvane Elevation: 97.70 Pavement: 3.5" asphalt 8 Base: 8.5" stabilized sand and gravel 26 36 22 14 13 Fill: brown Clayey Sand (SC), with gravel -with fat clay seams 1.5'-2' Stiff to very stiff, gray Fat Clay (CH), with 26 98.6 ferrous nodules 4 25 -tan and gray, with calcareous nodules 6'-8' 63 19 | 44 21 8 Very stiff, tan and gray Lean Clay (CL), with fat clay and calcareous powder pockets, and 112.9 calcareous and ferrous nodules Very stiff, tan Silty Clay (CL-ML), with lean clay pockets, calcareous nodules, and ferrous 17 stains 12 Very stiff, tan Sandy Lean Clay (CL), with 70 27 16 11 sandy silt partings, silty clay pockets, 16 calcareous nodules, and ferrous stains Tan Sandy Silt (ML), with lean clay pockets, calcareous nodules, and ferrous stains 21 Firm to very stiff, tan Lean Clay (CL), with silt 97 and calcareous powder pockets, and 7 24 calcareous nodules Stiff to very stiff, reddish tan Fat Clay (CH), with slickensides, silt pockets, and calcareous 26 97.3 nodules 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20

WATER ENCOUNTERED AT N/A FEET WHILE DRILLING ₩

WATER LEVEL AT N/A FEET AFTER COMPLETE

DRILLED BY Van and Sons DRAFTED BY YY LOGGED BY YY/DN



PROJECT: Stella Road Improvements ENGINEERING CORP. **BORING B-3**

DATE 6/10/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** PLASTICITY 200 MESH Eastina: 2977721.50 Pocket Penetrometer SYMBOL ЬΤ Northing: 13751769.18 **JRY** Torvane Elevation: 97.86 Pavement: 4.5" asphalt 10 Base: 14" stabilized sand with gravel 10 36 63 20 43 Fill: dark gray Clayey Sand (SC), with gravel Fill: stiff to very stiff, dark gray Fat Clay (CH), 25 97.1 with gravel 4 -with sand seams 4'-6' 24 Very stiff, gray Fat Clay (CH), with slickensides 57 16 | 41 -with ferrous nodules 6'-10' 22 8 -grayish tan, with calcareous nodules 8'-10' 25 99.3 Very stiff to hard, reddish brown Lean Clay (CL), with calcareous nodules, pockets, and 17 12 -with silt pockets and ferrous stains 12'-14' 39 16 23 18 Stiff to hard, reddish tan Fat Clay (CH), with 96 slickensides 114.9 16 -with silt seams 14'-16' 16 -with calcareous nodules 16'-20' 25 61 22 | 39 25 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY **Van and Sons** YY/DN

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PROJECT NO. G126-21

DRILLED BY



ENGINEERING CORP. **BORING B-4**

PROJECT: Stella Road Improvements DATE 6/10/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** 200 MESH 2978079.39 Eastina: Pocket Penetrometer SYMBOL РТ 13751439.93 Northing:)RY Torvane Elevation: 97.58 Pavement: 3.5" asphalt Base: 6" sand and gravel Pavement: 2.5" asphalt 26 Subbase: 7.5" sand and gravel 96 67 17 | 50 24 Very stiff to hard, dark gray Fat Clay (CH) -with sandy lean clay partings 1.6'-2' and 4 ferrous nodules 1.6'-10' -gray, with calcareous nodules 2'-4' 23 101.6 -tan and grayish tan 6'-8', with calcareous nodules 6'-12' 24 8 -brown and gray 8'-10', with calcareous powder 89 56 17 | 39 pockets 8'-12' 20 -tan 10'-12' 16 113.7 12 Very stiff, reddish tan Silty Clay (CL-ML), with ferrous and calcareous nodules, and fat clay 19 pockets Very stiff, tan and light gray Lean Clay (CL), 47 17 30 with calcareous nodules and powder pockets 16 16 Very stiff to hard, tan and light gray Fat Clay (CH), with slickensides 23 102.9 -with calcareous nodules 16'-18' -with calcareous powder pockets 18'-20' 24 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺

DRAFTED BY LOGGED BY DRILLED BY **Van and Sons** YY YY/DN PROJECT NO. G126-21 PLATE A-6



ENGINEERING CORP.
GEOTECHNICAL ENGINEERS
BORING

B-5

D,	ATE 6/10/2021 TYPE 4" Dry Auger				GEOTECHNICAL ENGINEERS CATION See Boring Locat	ion P	lan	
	DESCRIPTION				SHEAR STRENGTH, TSF			T
DEPTH IN FEET	GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2978427.64 Northing: 13751103.01 Elevation: 97.30	S.P.T. BLOWS/FT.	MOISTURE CONTENT,	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 	-200 MESH	LIQUID LIMIT PLASTIC LIMIT	PLASTICITY INDEX
0	Pavement: 4.5" asphalt Base: 15" stabilized sand with gravel	1	12					
	Fill: gray Silty Clayey Sand (SC-SM), with	-	7			15	24 20	4
	gravel -with fat clay seams and lime pockets 2'-4'		8			-		
4 -	Stiff to hard, gray and dark gray Fat Clay (CH) -with ferrous nodules 4'-6'		25	96.0	•			
	-gray, with calcareous nodules 6'-8'		24			95	62 18	3 44
- 8 -	-light grayish tan 8'-10'		27			-		
	-tan and grayish tan, with calcareous and ferrous nodules, and lean clay pockets 10'-12'		19	109.0				
- 12 -	Hard, tan Lean Clay (CL), with calcareous nodules and powder pockets	-	15				39 16	3 23
	Very stiff to hard, tan Fat Clay (CH), with	_						
- 16 -	slickensides -with calcareous nodules and ferrous stains 14'-18'		26					
	-tan and gray 16'-20'		21	108.4		-		
			25				65 25	40
- 20 -	Termination Depth = 20 feet	_						
- 24 -								
- 28 -								
	BORING DRILLED TO <u>20</u> FEET WITHOUT DF WATER ENCOUNTERED AT N/A FEET WHILI							
١	WATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPL</u>		<u> </u>			(V/IDA)		
	DRILLED BY <u>Van and Sons</u> DRAFTED BY _		Y	Y	LOGGED BY	Y/DN		



PROJECT: Stella Road Improvements ENGINEERING CORP. GEOTECHNICAL ENGINEERS BORING B-6

DATE 6/9/2021 TYPE 4" Dry Augus LOCATION See Boring Location Plan

D/	ATE <u>6/9/2021</u> TYPE <u>4" Dry Auger</u>			LOC	CATION See Boring Location Plan
	DECORPORTION		%		SHEAR STRENGTH, TSF
DEPTH IN FEET	DESCRIPTION Representation DESCRIPTION DES	S.P.T. BLOWS / FT.	MOISTURE CONTENT,	DRY DENSITY, PCF	△ Confined Compression
0	Pavement: 7" asphalt Base: 5" stabilized sand and gravel		8		93 46 15 31
- 4 -	Fill: stiff to very stiff, dark brown Lean Clay (CL), with gravel -with ferrous nodules 1'-2' -with fat clay seams 2'- 4'		22 24		33 40 13 31
	Stiff to hard, gray and dark gray Fat Clay (CH), with slickensides -with ferrous nodules 4'-14' -tannish gray 6'-8', with calcareous nodules 6'- 16'		28	93.5	68 19 49
- 8 -	-tan 8'-14'		26		
- 12 -	-with calcareous powder pockets 10'-12'			111.5	92 55 17 38
	-reddish tan 14'-18', with silt partings 14'-16'		19		
- 16 -	-with ferrous stains 16'-18'		27	100.4	
	-tan and light gray, with silt partings 18'-20'		22		56 19 37
- 20 -	Termination Depth = 20 feet				
- 24 -					
- 28 -	_				
	BORING DRILLED TO <u>20</u> FEET WITHOUT DF WATER ENCOUNTERED AT N/A FEET WHIL!				
\	WATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPL</u>		<u> </u>		
	DRILLED BY <u>Van and Sons</u> DRAFTED BY _		Y	Υ	LOGGED BY YY/DN

PROJECT NO. G126-21



PROJECT: Stella Road Improvements ENGINEERING CORP.
GEOTECHNICAL ENGINEERS
BORING
B-7

DAT	E <u>6/9/2021</u> TYPE <u>4" Dry Auger</u>			LOC	CATION See Boring Location Plan
DEPTH IN FEET	DESCRIPTION RANGE GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2979177.94 Northing: 13750466.84 Elevation: 96.38	S.P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF A Confined Compression Unconfined Compression Pocket Penetrometer Torvane 0.5 1 1.5 2
4	Pavement: 5" asphalt Base: 12" stabilized sand and gravel Fill: hard, brown and dark gray Sandy Lean Clay (CL), with gravel Fill: very stiff, dark brown Fat Clay (CH), with gravel and lean clay seams Stiff to hard, dark gray Fat Clay (CH), with slickensides -with ferrous nodules 4'-6'	-	6 14 23 30	91.9	52 61 20 41
8	-gray 8'-10', with ferrous nodules 8'-12' -tan 10'-12', with calcareous nodules 10'-14'		30	99.7	96 71 21 50
12	-reddish tan 12'-16'		20	00.7	62 23 39
16	-tan and gray 16'-20', with calcareous nodules 16'-18' -with silt partings 18'-20'		25 22	102.8	
20	Termination Depth = 20 feet				
- 24 -					
W.A W.A	ORING DRILLED TO <u>20</u> FEET WITHOUT DE ATER ENCOUNTERED AT <u>N/A</u> FEET WHIL ATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPL</u> BILLED BY <u>Van and Sons</u> DRAFTED BY	E DR	ILLI	NG ÷	



PROJECT: Stella Road Improvements ENGINEERING CORP. BORING B-8

DATE 6/9/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** Eastina: 2979671.65 Pocket Penetrometer SYMBOL ЬΤ Northing: 13750519.24 Torvane Elevation: 96.64 Pavement: 4" asphalt Base: 14.5" stabilized sand 97 76 52 24 l 28 Fill: stiff to very stiff, dark gray Fat Clay (CH), with gravel and sand pockets 36 86.2 Stiff to hard, dark gray Fat Clay (CH), with slickensides -with ferrous nodules 2'-4' 31 97 85 24 61 32 8 -gray 8'-10', with calcareous nodules 8'-12' 90.7 32 -reddish tan and light brown 10'-12' 30 12 -tan 12'-14' 61 20 | 41 24 -tan and light gray, with calcareous powder pockets 14'-16' 103.6 22 16 -reddish tan and gray, with silt seams 16'-18' 24 -tan and gray, with sandy silt seams 18'-20' 97 26 20 20 Termination Depth = 20 feet 24 28 BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY **Van and Sons** YY YY/DN

PROJECT NO. G126-21



PROJECT: Stella Road Improvements ENGINEERING CORP. **BORING B-9**

DATE 6/8/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression 200 MESH** Eastina: 2980018.92 Pocket Penetrometer SYMBOL ЬΤ Northing: 13750858.76 Torvane Elevation: 96.11 Pavement: 4" asphalt 6 Base: 10" stabilized sand and gravel 96 63

DEPTH IN FEET PLASTICITY 26 37 32 Fill: stiff, dark brown Fat Clay (CH), with gravel and lean clay pockets Stiff to very stiff, dark gray Fat Clay (CH), with 34 85.9 slickensides 4 33 -with ferrous nodules 6'-12' 90 25 | 65 32 8 -tan and gray, with calcareous nodules 8'-10' 31 91.1 -tan 10'-12' 103.5 22 12 Very stiff, tan Lean Clay (CL), with sandy silt 89 31 14 17 15 partings Medium dense to dense, tan Sandy Silt (ML) 35 7 16 -with cemented sand nodules 16'-18' 31 9 -with lean clay seams 18'-20' 60 28 10 20 Termination Depth = 20 feet 24 28

BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺

DRAFTED BY LOGGED BY DRILLED BY Van and Sons YY



PROJECT: Stella Road Improvements ENGINEERING CORP. **BORING** B-10

DATE 6/8/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DRY DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** PLASTICITY **200 MESH** Eastina: 2980400.76 Pocket Penetrometer SYMBOL ЬΤ Northing: 13751072.08 Torvane Elevation: 96.26 Pavement: 4" asphalt 5 Base: 8" stabilized sand and gravel 21 17 l 7 Fill: dark brown Clayey Sand (SC), with gravel Stiff to hard, dark gray Fat Clay (CH), with 39 84.6 slickensides 4 -with ferrous nodules 4'-12' 39 96 83 26 | 57 36 8 -tan and gray, with calcareous nodules 8'-10' 85.9 36 -tan 10'-14' 28 12 65 27 38 26 -tan and light gray 14'-20' 98.4 29 16 28 26 34 60 24 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₩ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons

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PROJECT NO. G126-21



PROJECT: Stella Road Improvements ENGINEERING CORP. BORING B-11

DATE 6/8/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** 200 MESH Eastina: 2980828.65 Pocket Penetrometer SYMBOL РΤ Northing: 13750890.73 Torvane Elevation: 96.32 Pavement: 3.5" asphalt Base: 5.5" stabilized sand and gravel 4 Subbase: 10" sand and gravel 46 61 23 38 26 Fill: dark brown Clayey Sand (SC), with gravel 35 85.4 Stiff to hard, dark brownish gray Fat Clay (CH), with slickensides 4 -with ferrous nodules 2'-6' 24 57 81 -dark gray 4'-8' 34 35 8 -dark gray and tannish gray 8'-10', with ferrous stains 8'-12' 90.9 34 -reddish tan 10'-14', with calcareous nodules 24 | 51 10'-12' 30 12 26 -tan and reddish brown, with ferrous stains and silt partings 14'-16' 105.3 24 16 -tan and light gray 16'-18' 100 | 66 45 21 30 Dense, tan and light gray Sandy Silt (ML), with lean clay pockets and siltstone nodules 43 15 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons YY

PROJECT NO. G126-21



PROJECT: Stella Road Improvements ENGINEERING CORP. GEOTECHNICAL ENGINEERS BORING B-12

DATE 6/8/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan

DA1	ΓΕ <u>6</u>	/8/2021 TYPE 4" Dry Auger			LOC	CATION See Boring Location Plan
DEPTH IN FEET	SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2981181.36 Northing: 13750586.11	P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF A Confined Compression Unconfined Compression Pocket Penetrometer Torvane 0.5 1.5 2.0 SHEAR STRENGTH, TSF WASHICITY INDEX DEPTITE TO
0	S S	Pavement: 4" asphalt	S)	≥ 10		0.5 1 1.5 2
4		Base: 8" stabilized sand and gravel Fill: stiff to very stiff, dark gray Fat Clay (CH), with sandy lean clay seams and gravel Stiff to hard, dark gray Fat Clay (CH), with slickensides		33	83.7	96 67 23 44
				37		
		-dark gray and browinsh gray 6'-8', with ferrous nodules 6'-16'		33		81 24 57
8		-reddish tan 8'-10'		31	91.7	
		-tan and light gray 10'-12'		27		
12		-reddish tan and light gray 12'-14'		26		68 22 46
		-tan and light gray, with silt seams 14'-16'		24	102.2	
16		Dense to very dense, tan and light gray Sandy Silt (ML) -with calcareous nodules 16'-18' -tan, with silty clay pockets and siltstone	62	11		52
20	ЩХ	nodules 18'-20'	37	9		
- 24 -		Termination Depth = 20 feet				
		G DRILLED TO <u>20</u> FEET WITHOUT DR R ENCOUNTERED AT N/A FEET WHILE				

WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \Rightarrow

WATER LEVEL AT N/A FEET AFTER COMPLETE

DRILLED BY <u>Van and Sons</u> DRAFTED BY <u>YY</u> LOGGED BY <u>YY</u>



PROJECT: Stella Road Improvements ENGINEERING CORP. GEOTECHNICAL ENGINEERS BORING B-13

DATE 6/8/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan

DAT	ΓΕ <u>6</u>	/8/2021 TYPE 4" Dry Auger			LOC	CATION See Boring Location Plan
	SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2981532.35 Northing: 13750262.50 Elevation: 95.97	S.P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF Confined Compression Unconfined Compression Pocket Penetrometer Torvane 0.5 1 1.5 2
0		Pavement: 4" asphalt Base: 8" stabilized sand and gravel Fill: dark brown Clayey Sand (SC), with gravel -with fat clay pockets 1'-2' -brown, with fat clay seams 2'-4'		11 10 16	104.4	17 31 13 18
4		Firm to hard, dark gray Fat Clay (CH), with slickensides		33	104.4	78 27 51
8 -		-with calcareous nodules 6'-10' -tan and gray 8'-10', with ferrous stains 8'- 12'		32	90.4	
12		-tan 10'-12'		28		98 69 24 45
		-tan and light gray, with calcareous nodules, and lean clay and calcareous powder pockets 12'-14' Very dense, light tan Sandy Silt (ML), with	-	18	108.5	
- 16 -		siltstone nodules	77	5		51
		-with sandy lean clay pockets 18'-20'	61 50/5"	7 8		
20		Termination Depth = 20 feet				
- 24 -						
- 28 -						
W	ATEF	G DRILLED TO <u>20</u> FEET WITHOUT DF R ENCOUNTERED AT <u>N/A</u> FEET WHILE DIEVEL AT N/A FEET AFTER COMPLE				

WATER LEVEL AT N/A FEET AFTER COMPLETE

DRILLED BY Van and Sons DRAFTED BY YY LOGGED BY YY



ENGINEERING CORP. **BORING**

B-14 DATE 6/7/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH, TSF MOISTURE CONTENT, % **DESCRIPTION** DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression 200 MESH** Eastina: 2981888.21 Pocket Penetrometer SYMBOL ЬΤ Northing: 13749931.30 Torvane Elevation: 95.67 Pavement: 3.5" asphalt Base: 13.5" sand and gravel 96 78 28 50 33 Stiff to hard, dark gray Fat Clay (CH), with slickensides 33 86.6 4 34 -tannish gray and dark gray 6'-8', with 87 24 63 calcareous nodules 6'-10' 32 8 -reddish tan 8'-14' 30 91.7 27 12 69 24 45 27 -tan and light gray 14'-16' 101.6 25 16 -reddish tan and light gray, with silt seams 16'-18' 20 -tan and light gray, with silt pockets 18'-20' 100 | 56 21 35 25 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons YY



PROJECT: Stella Road Improvements ENGINEERING CORP. BORING B-15
GEOTECHNICAL ENGINEERS

DA	TE <u>6/</u>	7/2021 TYPE <u>4" Dry Auger</u>			LOC	CATION See Boring Location Plan	
O DEPTH IN FEET	SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2982234.96 Northing: 13749608.87 Elevation: 95.35	S.P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2	PLASTICITY INDEX
- 4		Pavement: 4" asphalt Base: 14" sand and gravel Stiff to very stiff, dark gray Fat Clay (CH), with slickensides -with ferrous nodules 2'-6'		32 36	83.3	96 80 27	53
		-brown and gray 6'-8'		36 32		86 23	63
8		-tan and gray 8'-10', with calcareous and ferrous nodules 8'-12' -reddish tan 10'-12'		30 25	91.9		
- 12		Stiff to hard, light reddish tan and gray Lean Clay (CL), with sandy silt seams Dense to very dense, tan Silt with Sand (ML)	-	28	90.9	91 31 15	16
- 16 -		-with fat clay seams and silty clay pockeys 16'-	37 70	15 19			
- 20 -		-light tan 18'-20' Termination Depth = 20 feet	50/5"	8		77	
		·					
- 24 -							
- 28 - B	BORING	G DRILLED TO 20 FEET WITHOUT DR	 RILLII	VG F	 -LUIC		
\ \	VATER VATER	R ENCOUNTERED AT <u>N/A</u> FEET WHILE R LEVEL AT <u>N/A</u> FEET AFTER <u>COMPL</u> D BY <u>Van and Sons</u> DRAFTED BY	E DR	ILLI	NG -		



ENGINEERING CORP. B

BORING **B-16**

DATE 6/7/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** 200 MESH Eastina: 2982576.06 \bigcirc Pocket Penetrometer SYMBOL РΤ Northing: 13749294.44)RY Torvane Elevation: 95.16 Pavement: 3.5" asphalt Base: 12.5" sand and gravel 72 25 47 95 Fill: very stiff, dark gray Fat Clay (CH), with 27 gravel, sandy lean clay seams, and asphalt pieces 30 90.8 Stiff to very stiff, dark gray Fat Clay (CH), with 4 slickensides -with ferrous nodules 2'-6' 29 -dark brownish gray 4'-6' -tan and gray 6'-8' 73 23 | 50 94.7 29 8 -tan and dark gray, with ferrous stains 8'-10', and calcareous nodules 8'-12' 27 -red and tan, with silt pockets and seams 10'-12' 22 12 Stiff to very stiff, tan and gray Lean Clay (CL), 98 29 18 11 with calcareous nodules and silt partings 21 Medium dense, tan Silt with Sand (ML) -with siltstone seams 14'-16' 20 20 16 79 20 18 -with silty clay pockets 18'-20' 21 15 20 Termnation Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons YY



PROJECT: Stella Road Improvements ENGINEERING CORP. **BORING** B-17

DATE 6/7/2021 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** 200 MESH Eastina: 2982888.19 Pocket Penetrometer SYMBOL ЬΤ Northing: 13748987.65 Torvane Elevation: 94.52 Pavement: 3.25" asphalt Base: 14.25" sand and gravel 22 63 50 17 33 Fill: very stiff, dark gray Sandy Fat Clay (CH), with gravel seams and shell fragments 23 Stiff to very stiff, gray and dark gray Fat Clay (CH), with slickensides 4 -with ferrous nodules 2'-6' 100.9 -tan and gray, with calcareous nodules 4'-6' 25 -reddish tan, with lean clay pockets 6'-8' and 20 | 47 67 calcareous powder pockets 6'-10' 21 8 -reddish brown, with silt pockets 8'-10' 109.6 19 Reddish tan Silt with Sand (ML) 16 12 Stiff to very stiff, tan and light gray Lean Clay 93 32 15 17 (CL), with silt seams 109.0 18 Medium dense, tan Silt with Sand (ML) -with silty clay pockets 14'-16' 16 21 16 -boring cave-in at 16' after completion of drilling 81 19 24 17 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT **N/A** FEET WHILE DRILLING \\\\ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY Van and Sons

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PROJECT NO. G126-21

DRILLED BY



PROJECT: Stella Road Improvements **ENGINEERING CORP.** BORING B-18 DATE **3/25/2022** TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** PLASTICITY Easting: 2979443.158 Pocket Penetrometer SYMBOL ЬΤ Northing: 13750595.24 Torvane Elevation: 93.80 Firm to hard, gray Fat Clay (CH), with 67 20 47 96 slickensides 29 -with roots 0'-2' -dark gray 2'-6', with ferrous nodules 2'-18' 36 85.5 4 32 -tan 6'-12' 20 | 48 97 68 22 8 -with calcareous nodules 8'-10' 106.3 21 12 -tan and gray 12'-18' 23 103.0 24 16 -with silty sand seams and partings 16'-20' 18 33 99 51 18 -tan 18'-20' 20 20 Termination Depth = 20 feet 24 28

BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₩ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺

DRAFTED BY LOGGED BY DRILLED BY Van and Son SA ΑZ



ENGINEERING CORP. BORING

B-19

DATE 3/25/2022 TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF DESCRIPTION MOISTURE CONTENT, DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT PLASTICITY **Unconfined Compression** 200 MESH Eastina: 2980368.432 Pocket Penetrometer SYMBOL ЬΤ Northing: 13750882.72 **JRY** Torvane Elevation: 94.19 Stiff to hard, dark gray Fat Clay (CH), with 79 25 54 96 slickensides 42 -with roots 0'-4' 37 4 -gray and tan 4'-6', with ferrous nodules 4'-8' 35 87.9 -reddish tan 6'-8', with calcareous nodules 6'-23 8 -tan and tannish gray 8'-10' 98 68 22 | 46 28 96.4 -gray and reddish tan 10'-12' 29 12 -tan and light gray, with calcareous nodules 12'-14' 105.7 22 Stiff, tan and light gray Lean Clay (CL), with silt 32 15 | 17 partings and siltstone nodules 12 16 Very dense, tan Silty Sand (SM), with 37 cemented sand nodules 54 5 -with clayey sand pockets 18'-20' 7 61 20 Termination Depth = 20 feet 24 28 BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons SA AZ



PROJECT: Stella Road Improvements ENGINEERING CORP. **BORING B-20**

DATE **5/12/2022** TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % **DESCRIPTION** DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** 200 MESH 2980993.076 Eastina: \bigcirc Pocket Penetrometer SYMBOL ЬΤ Northing: 13751108.265 **JRY** Torvane Elevation: 98* Fill: hard, black and gray Fat Clay (CH), with silty sand pockets and gravel 23 -with roots and calcareous nodules 0'-2' -with ferrous nodules 2'-4' 87 77 24 | 53 32 4 Stiff to hard, dark gray Fat Clay (CH), with slickensides 33 87.8 -with ferrous nodules 4'-6' 28 8 -tan and tannish gray, with calcareous nodules 79 25 54 8'-10', and ferrous nodules 8'-14' 23 -tan 10'-12' 104.5 23 12 -tan and gray, with calcareous nodules 12'-14' 24 49 18 31 Hard, tan and gray Lean Clay (CL), with 17 slickensides and ferrous nodules Termination Depth = 15 feet 16 *: boring not surveyed; coordinates and elevation are estimated. 20 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 15 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY **Van and Sons** LW

PROJECT NO. G126-21 PLATE A-22

BA



PROJECT: Stella Road Improvements **BORING** ENGINEERING CORP. B-21

DATE **5/12/2022** TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % **DESCRIPTION** DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** PLASTICITY Eastina: 2981100.064 \bigcirc Pocket Penetrometer SYMBOL ЬΤ Northing: 13750814.117 Torvane Elevation: 97* Fill: hard, black and reddish tan Fat Clay (CH), with calcareous nodules and roots 22 Fill: dark brown Clayey Sand (SC), with gravel 16 31 17 47 and asphalt pieces 9 4 Stiff to hard, gray Fat Clay (CH), with slickensides and ferrous nodules 31 90.9 -olive gray and brown, with calcareous nodules 95 98 29 | 69 29 95.5 -tan 8'-10' 26 -tan and gray 10'-12' 102 24 12 -tan 12'-14' 66 24 42 24 -tan and gray 14'-15' 24 Termination Depth = 15 feet *: boring not surveyed; coordinates and 16 elevation are estimated. 20 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 15 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons LW BA



ENGINEERING CORP. BORING

B-22

DATE **5/12/2022** TYPE 4" Dry Auger **LOCATION See Boring Location Plan** SHEAR STRENGTH, TSF MOISTURE CONTENT, % **DESCRIPTION** DRY DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** 200 MESH Eastina: 2979703.639 \bigcirc Pocket Penetrometer SYMBOL ЬΤ Northing: 13750851.892 Torvane Elevation: 95* Very stiff to hard, dark gray Fat Clay (CH), with slickensides and ferrous nodules 27 -with roots 0'-4' 26 96.4 4 -tan and gray 4'-6' 96 82 25 57 32 -tan 6'-8' 22 8 -reddish tan and gray 8'-10', with calcareous nodules and pockets 8'-14' 102.7 24 -tan and gray 10'-14', with silt partings 10'-12' 100 | 67 24 | 43 22 12 18 Very dense, tan Silt (ML), with sandy lean clay 93 pockets 59 6 16 Termination Depth = 16 feet *: boring not surveyed; coordinates and elevation are estimated. 20 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 15 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₩ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons LW BA



ENGINEERING CORP. BORING

BORING B-23

DATE **5/12/2022** TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH, TSF MOISTURE CONTENT, % **DESCRIPTION** DENSITY, PCF BLOWS / FT. PLASTICITY INDEX GRID Coordinates (US Survey ft): **Confined Compression** DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT **Unconfined Compression** Eastina: 2979319.157 \bigcirc Pocket Penetrometer SYMBOL ЬΤ Northing: 13750724.229 Torvane 95* Elevation: Stiff to hard, dark gray Fat Clay (CH), with slickensides and ferrous nodules 25 -with roots 2'-4' 25 99.7 4 -gray and reddish brown 4'-6' 76 23 | 53 97 29 92.1 -tan 6'-8' 24 8 -tan and gray 8'-12', with calcareous nodules 8'-10' 102.3 25 100 | 63 22 | 41 24 12 -reddish tan 12'-14' 24 -tan and gray 14'-15' 25 99.0 Termination Depth = 15 feet *: boring not surveyed; coordinates and 16 elevation are estimated. 20 24 28 BORING DRILLED TO 15 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING \₹ WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 **DRAFTED BY** LOGGED BY DRILLED BY Van and Sons LW BA

KEY TO SYMBOLS

Symbol Description Symbol Description Strata symbols Auger Paving Fill High plasticity clay Low plasticity clay Silty sand Silty low plasticity clay Silt Misc. Symbols Torvane Pocket Penetrometer \bigcirc Unconfined Compression Confined Compression Δ Soil Samplers Rock core Undisturbed thin wall Shelby tube Standard penetration test

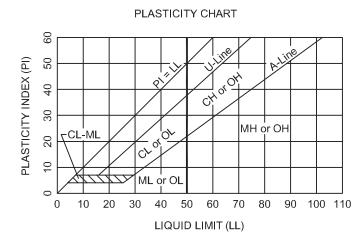


CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES

ASTM Designation D-2487

	MAJOR DIVISIONS			GROUP SYMBOL	TYPICAL NAMES
	oarse sieve)	CLEAN GRAVELS			Well-graded gravel, well-graded gravel with sand
eve)	GRAVELS (Less than 50% of coarse fraction passes No. 4 sieve)		an 5% passes 200 sieve)	GP	Poorly-graded gravel, poorly-graded gravel with sand
COARSE-GRAINED SOILS (Less than 50% passes No. 200 sieve)	GRAVELS than 50% of n passes No.	GRAVELS WITH FINES (More than 12% passes	Limits plot below "A" line & hatched zone on plasticity chart	GM	Silty gravel, silty gravel with sand
COARSE-GRAINED SOILS than 50% passes No. 200 s	(Less fraction	No. 200 sieve)	Limits plot above "A" line & hatched zone on plasticity chart	GC	Clayey gravel, clayey gravel with sand
SE-GR	arse sieve)		AN SANDS	sw	Well-graded sand, well-graded sand with gravel
COAR s than {	CCEAN SANDS (Less than 5% passes No. 200 sieve) SANDS WITH FINES Limits plot below "A" line & hatched zone on plasticity chart		oasses No. 200 sieve)	SP	Poorly-graded sand, poorly-graded sand with gravel
(Les	SANDS (50% or more of coarse fraction passes No. 4 sieve)	SANDS WITH FINES (More than 12% passes	Limits plot below "A" line & hatched zone on plasticity chart	SM	Silty sand, silty sand with gravel
	(50% fraction	No. 200 sieve)	Limits plot above "A" line & hatched zone on plasticity chart	sc	Clayey sand, clayey sand with gravel
	/e)				Silt, silt with sand, silt with gravel, sandy silt, gravelly silt
SIICS	200 siev		AND CLAYS t Less Than 50%)	CL	Lean clay, lean clay with sand, lean clay with gravel, sandy lean clay, gravelly lean clay
NED SC	ses No.			OL	Organic clay, organic clay with sand, sandy organic clay, organic silt, sandy organic silt
FINE-GRAINED SOILS	ore pas:			МН	Elastic silt, elastic silt with sand, sandy elastic silt, gravelly elastic silt
	(50% or more passes No. 200 sieve)		AND CLAYS nit 50% or More)	СН	Fat clay, fat clay with sand, fat clay with gravel, sandy fat clay, gravelly fat clay
	(20			ОН	Organic clay, organic clay with sand, sandy organic clay, organic silt, sandy organic silt

NOTE: Coarse soils between 5% and 12% passing the No. 200 sieve and fine-grained soils with limits plotting in the hatched zone of the plasticity chart are to have dual symbols.

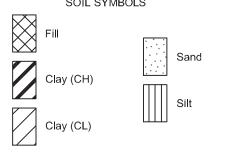


Equation of A-Line: Horizontal at PI=4 to LL=25.5, then PI=0.73(LL-20) Equation of U-Line: Vertical at LL=16 to PI=7, then PI=0.9(LL-8)

DEGREE OF PLASTICITY OF COHESIVE SOILS

Degree of Plasticity	Plasticity Index
None Slight Medium	5 - 10
High Very High	

SOIL SYMBOLS

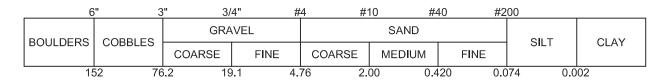




TERMS USED ON BORING LOGS

SOIL GRAIN SIZE

U.S. STANDARD SIEVE



SOIL GRAIN SIZE IN MILLIMETERS

STRENGTH OF COHESIVE SOILS

	Undrained		SOILS FROM STANDARD F	PENETRATION TEST
<u>Consistency</u>	Shear Strength,	SPT Blowcount	Vomiliana	<1 hnf
	<u>Kips per Sq. ft.</u>		Very Loose	
			Loose	5-10 bpf
Very Soft	less than 0.25	< 2 bpf	Medium Dense	11-30 bpf
Soft	0.25 to 0.50	2-4 bpf	Dense	31-50 bpf
Firm	0.50 to 1.00	4-8 bpf	Very Dense	>50 bpf
Stiff	1.00 to 2.00	8-16 bpf		
Very Stiff	2.00 to 4.00	16-32 bpf		
Hard	greater than 4.00	>32 bpf		

SPLIT-BARREL SAMPLER DRIVING RECORD

Description

25	25 blows	driving sam	pler 12	2 inches,	after i	initial	6 inche	s of s	eating.	
E 0 /7#	E0 11							_		

NOTE: To avoid change to sampling tools, driving is limited to 50 blows during or after seating interval.

DRY STRENGTH ASTM D2488

Calcareous

Blows per Foot

MOISTURE CONDITION ASTM D2488

RELATIVE DENSITY OF COHESIONLESS

None	Dry specimen crumbles into powder with mere pressure of handling	Dry	Absence of moisture, dusty, dry to the touch
Low	Dry specimen crumbles into powder with some finger pressure	Moist	Damp but no visible water

Medium Dry specimen breaks into pieces or crumbles with considerable pressure Wet Visible free water

High Dry specimen cannot be broken with finger pressure, it can be

Having appreciable quantities of calcium material.

broken between thumb and hard surface

Very High Dry specimen cannot be broken between thumb and hard surface

SOIL STRUCTURE

Slickensided	Having planes of weakness that appear slick and glossy. The degree of slickensidedness depends upon the spacing of slickensides and the easiness of breaking along these planes.
Fissured	Containing shrinkage or relief cracks, often filled with fine sand or silt; usually more or less vertical.
Friable	Crumbly, can be easily crushed with light pressure.
Blocky	Clays that have a block-like or polyhedral structure.
Pocket	Inclusion of material of different texture that is smaller than the diameter of the sample.
Parting	Inclusion less than 1/8 inch thick extending through the sample.
Seam	Inclusion 1/8 inch to 3 inches thick extending through the sample.
Layer	Inclusion greater than 3 inches thick extending through the sample.
Laminated	Soil sample composed of alternating partings or seams of different soil types.
Interlayered	Soil sample composed of alternating layers of different soil types.
Intermixed	Soil sample composed of pockets of different soil types and layered or laminated structure is not evident.



ASTM & TXDOT DESIGNATION FOR SOIL LABORATORY TESTS

SOIL TEST	ASTM TEST DESIGNATION	TXDOT TEST DESIGNATION	
Unified Soil Classification System	D 2487	Tex-142-E	
Moisture Content	D 2216	Tex-103-E	
Specific Gravity	D 854	Tex-108-E	
Sieve Analysis	D 6913	Tex-110-E (Part 1)	
Hydrometer Analysis	D 7928	Tex-110-E (Part 2)	
Minus No. 200 Sieve	D 1140	Tex-111-E	
Liquid Limit	D 4318	Tex-104-E	
Plastic Limit	D 4318	Tex-105-E	
Standard Proctor Compaction	D 698	Tex-114-E	
Modified Proctor Compaction	D 1557	Tex-113-E	
California Bearing Ratio	D 1883	-	
Swell	D 4546	-	
Consolidation	D 2435	-	
Unconfined Compression	D 2166	-	
Unconsolidated-Undrained Triaxial	D 2850	Tex-118-E	
Consolidated-Undrained Triaxial	D 4767	Tex-131-E	
Permeability (constant head)	D 5084	-	
Pinhole	D 4647	-	
Crumb	D 6572	-	
Double Hydrometer	D 4221	-	
pH of Soil	D 4972	Tex-128-E	
Soil Suction	D 5298	_	
Soil Sulfate	C 1580	Tex-145-E	
Organics	D 2974 Tex-148-E		

AVILES ENGINEERING CORPORATION

Consulting Engineers - Geotechnical, Construction Materials Testing, Environmental

RESULTS OF CRUMB TESTS (ASTM D 6572)

Project Name: Stella Road Reconstruction from Cottonwood School to Band Road, Fort Bend County, Texas

Project No.: <u>G126-21</u> Test Date: <u>5/26/22</u>

Boring Number	Depth, feet	2 Minutes		1 Hour		6 Hours	
Number	ieet	Grade	C (deg)	Grade	C (deg)	Grade	C (deg)
B-20	0-2	1	22.2	1	22.2	1	22.3
B-20	6-8	1	22.2	1	22.2	1	22.3
B-21	0-2	1	22.2	1	22.2	1	22.3
B-21	8-10	1	22.2	1	22.2	1	22.3
B-22	2-4	1	22.2	1	22.2	1	22.3
B-22	6-8	1	22.2	1	22.2	1	22.3
B-23	0-2	1	22.2	1	22.2	1	22.3
B-23	6-8	1	22.2	1	22.2	1	22.3

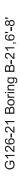
Grade Classification:

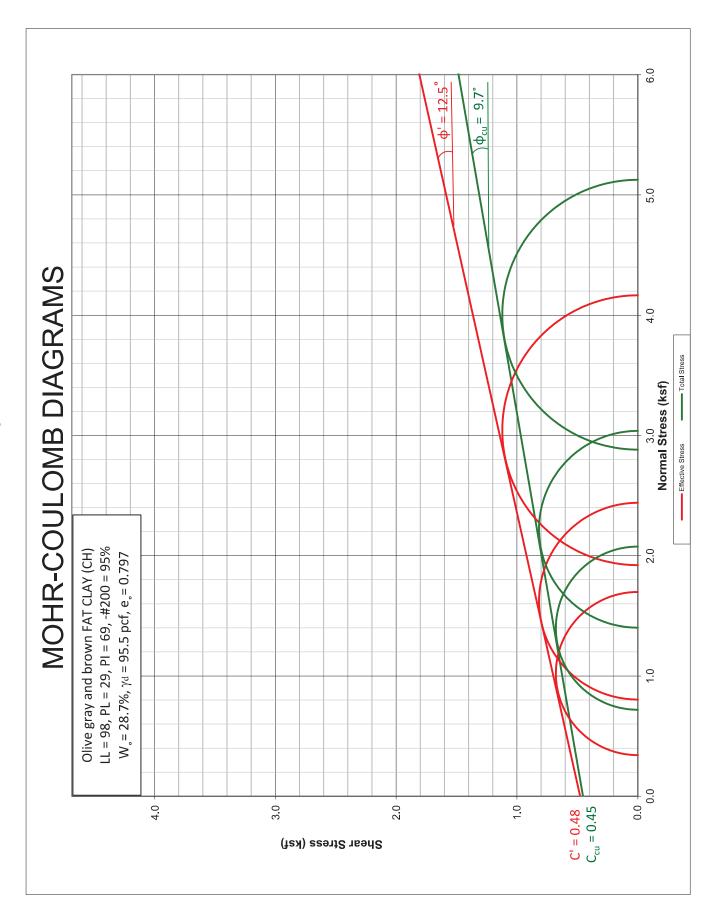
Grade 1 Non-dispersive; No reaction
 Grade 2 Intermediate; Slight reaction
 Grade 3 Dispersive; Moderate reaction
 Grade 4 Highly Dispersive; Strong reaction

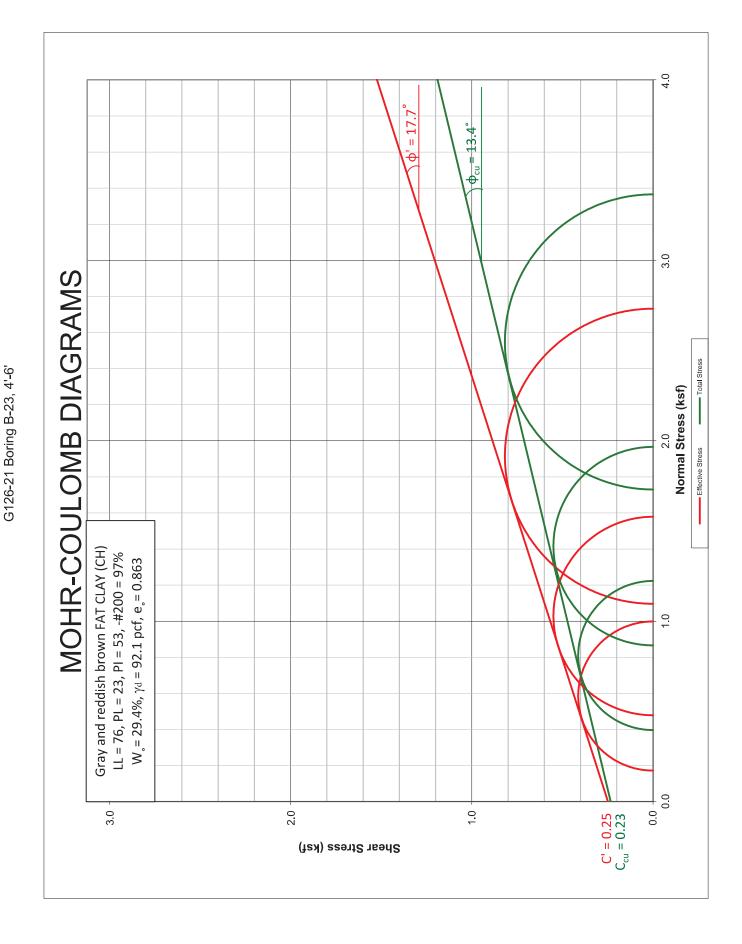
Interpretation:

Under normal conditions, use the 1 hour reading to determine dispersive grade.

However, if the dispersive grade changes from 2 to 3 or from 3 to 4 between the 1 and 6 hour readings, use the 6 hour reading instead.



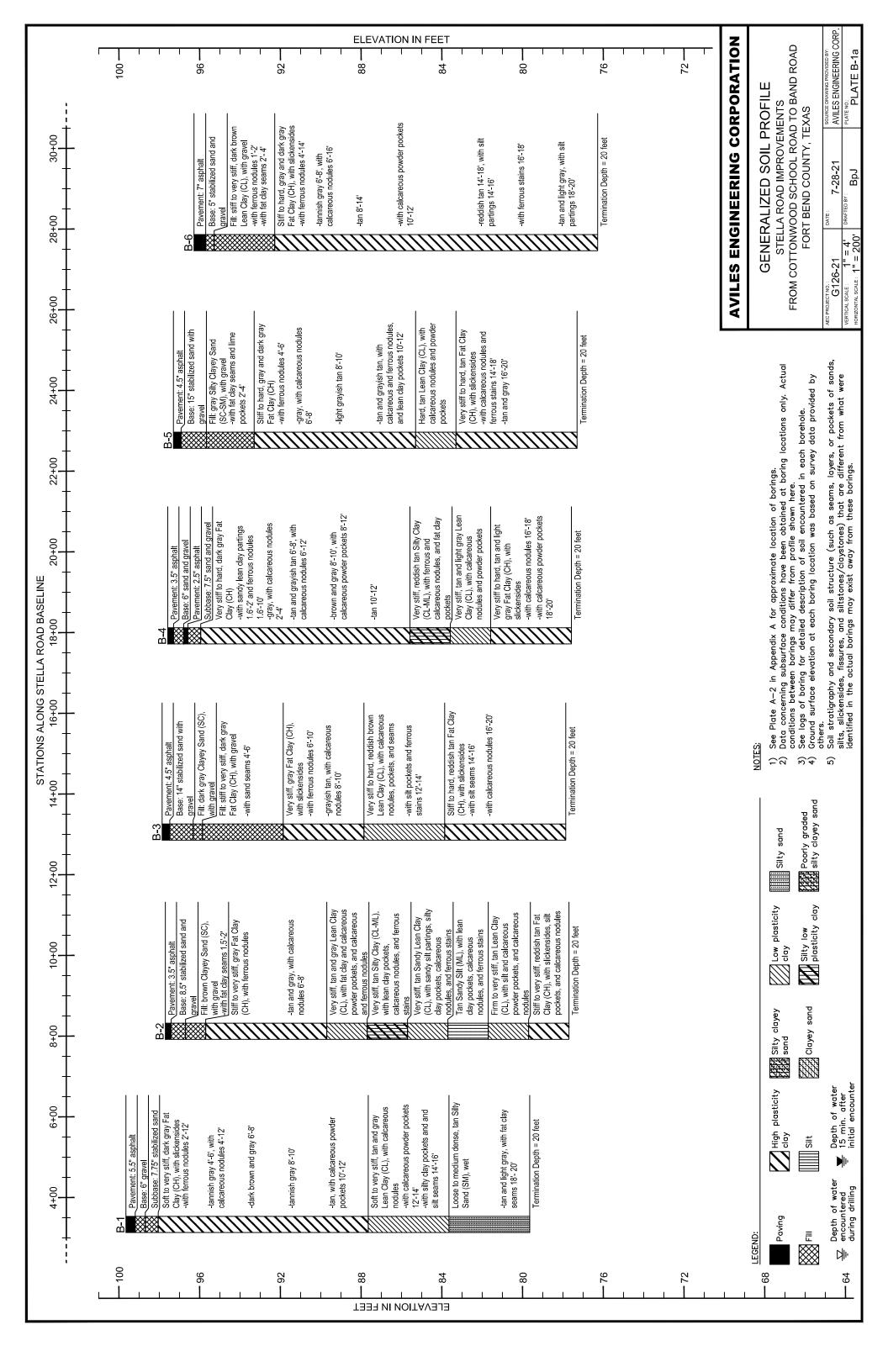


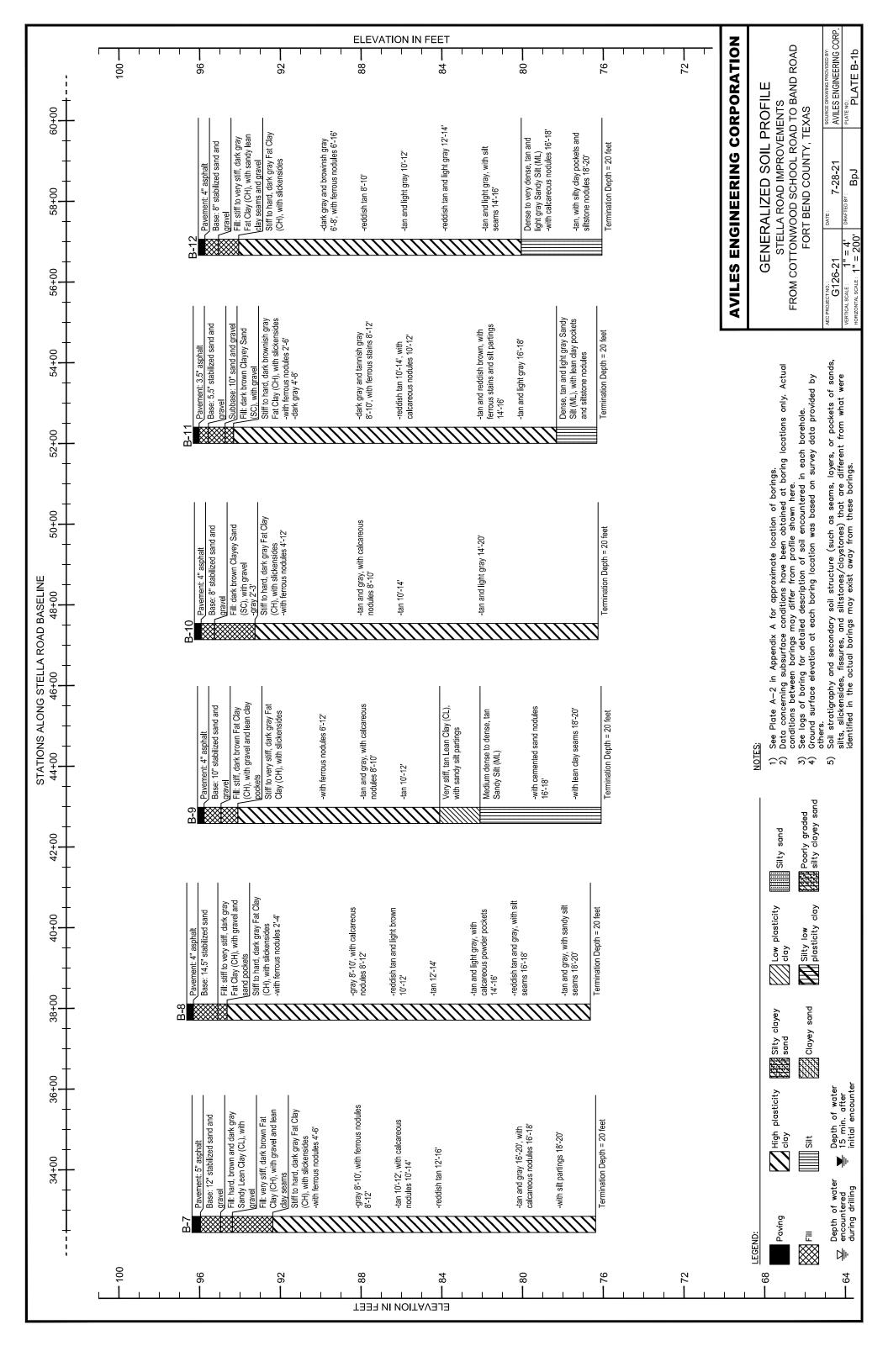


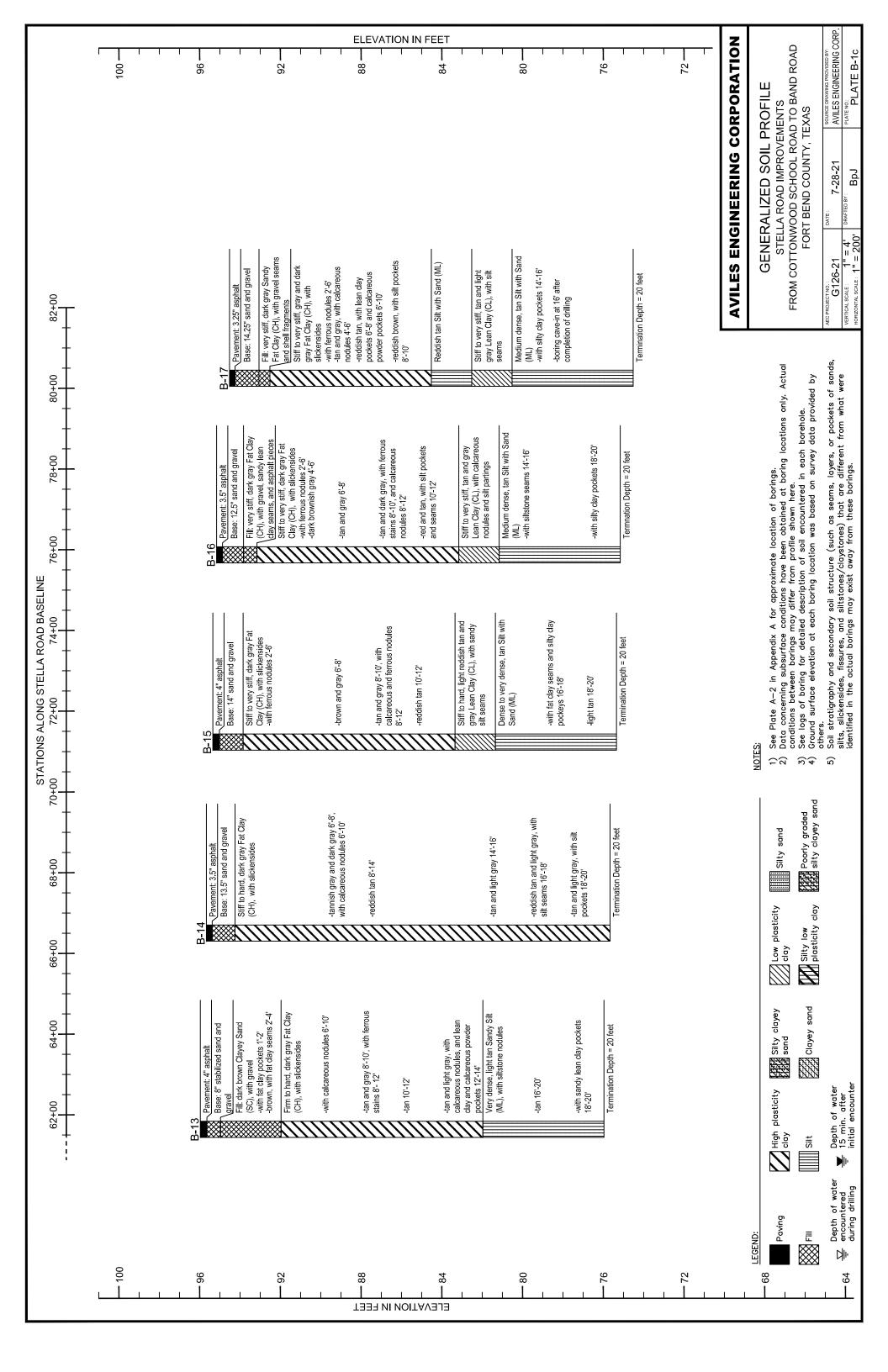


APPENDIX B

Plates B-1a to B-1c Generalized Soil Profiles









APPENDIX C

Plates C-1 to C-3 DARWin v3.0 Outputs for Asphalt Pavement Design

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

Stella Road Reconstruction ESALs Calculation with FBC's Minumun Thickness Requirements

Flexible Structural Design

Structural Number	4.92 in
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	90 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	1,500 psi
Stage Construction	1

18-kip ESALs Over Initial Performance Period 226,282

Effective Roadbed Soil Resilient Modulus

		Roadbed
		Resilient
<u>Period</u>	<u>Description</u>	Modulus (psi)
1	1	1.500

Calculated Effective Modulus 1,500 psi

Specified Layer Design

		Struct Coef.	Drain Coef.	Thickness	Width	Calculated
<u>Layer</u>	Material Description	<u>(Ai)</u>	<u>(Mi)</u>	<u>(Di)(in)</u>	<u>(ft)</u>	<u>SN (in)</u>
1	Asphalt	0.44	1	3	-	1.32
2	Black Base	0.34	1	8	-	2.72
3	Stabilized Subgrade	0.11	1	8	-	0.88
Total	-	-	-	19.00	-	4.92

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

Stella Road Reconstruction Structural Number Calculation to Meet Estimated Design Life ESALs

Flexible Structural Design

18-kip ESALs Over Initial Performance Period	678,526
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	90 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	1,500 psi
Stage Construction	1
Calculated Design Structural Number	5.73 in

Effective Roadbed Soil Resilient Modulus

		Roadbed
		Resilient
<u>Period</u>	<u>Description</u>	<u>Modulus (psi)</u>
1	1	1,500

Calculated Effective Modulus 1,500 psi

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

Stella Road Reconstruction ESALs Calculation with Recommended Pavement Design

Flexible Structural Design

Structural Number	5.82 in
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	90 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	1,500 psi
Stage Construction	1

18-kip ESALs Over Initial Performance Period 760,267

Effective Roadbed Soil Resilient Modulus

		Roadbed
		Resilient
<u>Period</u>	<u>Description</u>	Modulus (psi)
1	1	1 500

Calculated Effective Modulus 1,500 psi

Specified Layer Design

		Struct	Drain			
		Coef.	Coef.	Thickness	Width	Calculated
<u>Layer</u>	Material Description	<u>(Ai)</u>	<u>(Mi)</u>	(Di)(in)	<u>(ft)</u>	<u>SN (in)</u>
1	Asphalt	0.44	1	3.5	=	1.54
2	Black Base	0.34	1	10	=	3.40
3	Stabilized Subgrade	0.11	1	8	-	0.88
Total	-	_	_	21.50	_	5.82



APPENDIX D

Plates D-1 and D-2 r.g. Miller Engineers drawings, Detention Basin Layout and Cross Sections, dated

June 28, 2022

Plate D-3 Design Soil Parameters for Slope Stability Analyses

Plates D-4 to D-9 Detention Basin Slope Stability Analyses

DATE: 6/28/22 SHEET NO: 52 /100 CIVIL STANDARD PLATE D-2 FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD SHEET DESCRIPTION:
SHEET DESCRIPTION:
PROPOSED DETENTION BASIN SECTION STELLA ROAD 1" = 40PROJECT TITLE: DRAWN BY: NS CK'D BY: MJ INTERIM REVIEW ONLY
DOCUMENT INCOMPLETE: Not Intended
for permit, or construction.
Figureer: Wengowng Jung
P.E. License No. 758195
DATE: June, 2022 ENGINEERING DEPARTMENT FORT BEND COUNTY

PROP. CONCRETE PILOT CHANNEL STA. 5+35— EL. 93.56 STA. 5+30— EL. 94.25 30' MAINT. ACCESS -STA. 4+86 EL. 91.45 2+00 PROP. TOB— STA. 5+00 EL. 94.85 4+00 100-YR WSE:94.20

S-YR WSE:93.40 € PROP. BASIN SECTION A-A PROPOSED CONC. PILOT CHANNEL -STA. 2+37-EL. 89.71 STA. 2+32-EL. 89.70 2+28-90.69 STA. EL. 9 % - ST - F. ST - F. ST 30' MAINT. ACCESS 0+44 93.70 STA. EL.

98

96

94

92

90

88 0+0

88 5+50

90

94

STA. 5+40— EL. 94.27

LEGEND

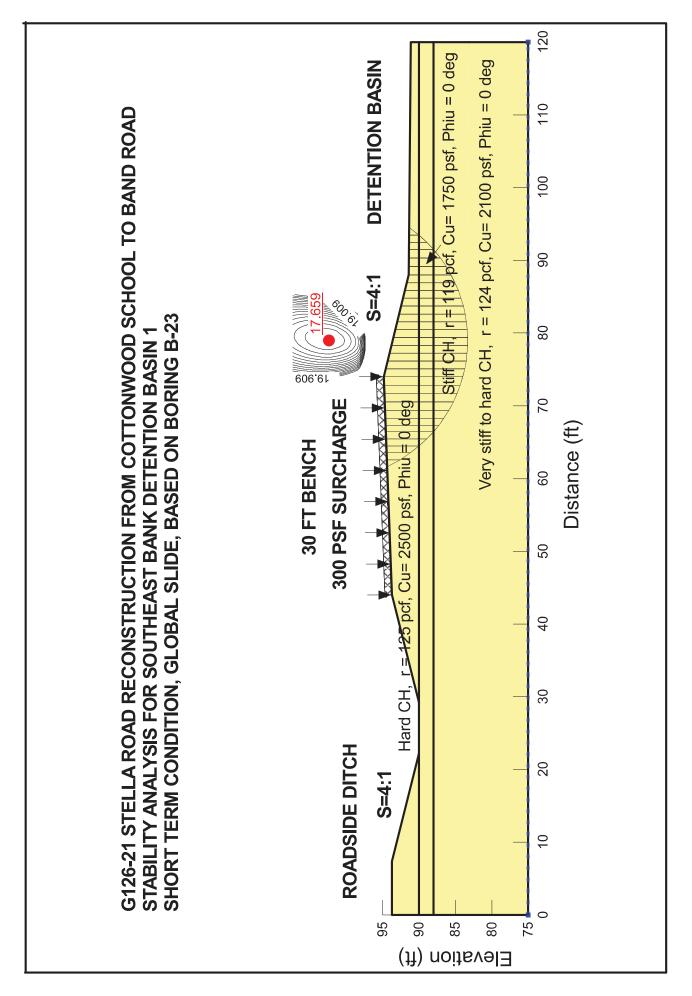


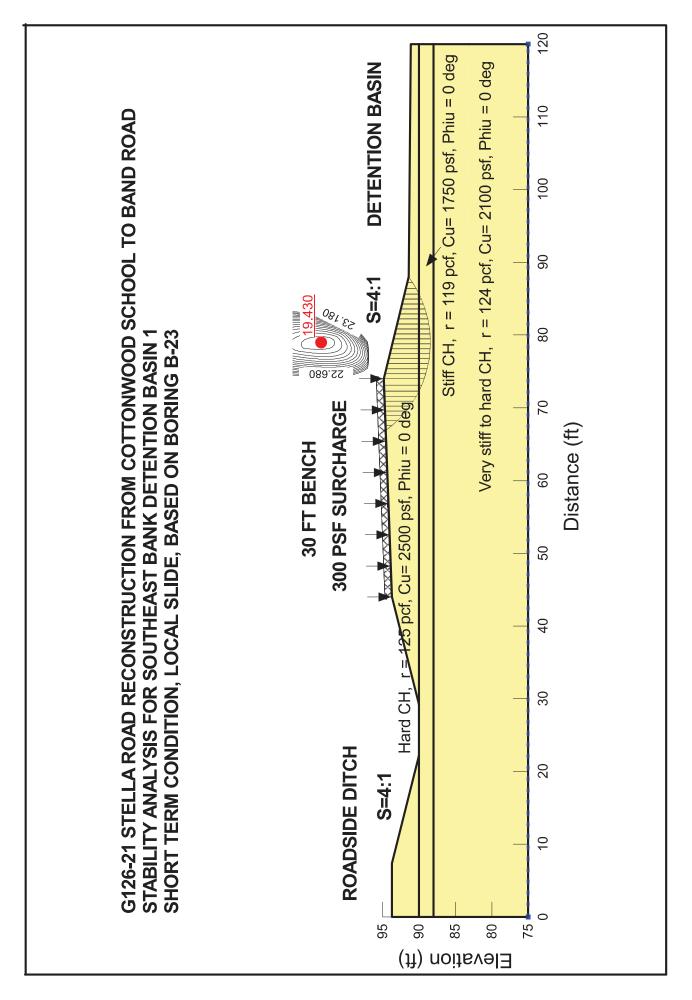
Design Soil Parameters for Slope Stability Analyses Detention Basin (Based on Boring B-23)

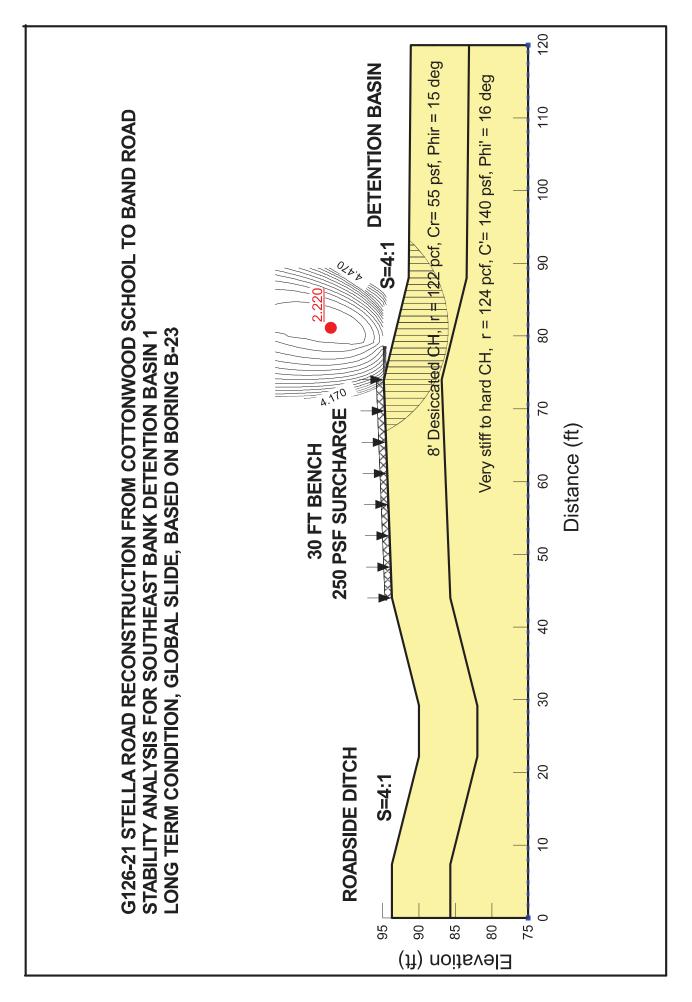
Elevation (ft)	Soil Type	γm	Sho Term/Un Param	drained	Effective Param	e Stress	Total Paran	
		(pcf)	C _u (psf)	φ _u (deg)	C' (psf)	φ' (deg)	C _{cu} (psf)	фси (deg)
94 to 90	Hard CH	125	2500	0	160 (C _r = 55)	$ \begin{array}{c} 16 \\ (\phi_{\rm r} = 15) \end{array} $	150 (C _r = 55)	$15 \\ (\phi_r = 15)$
90 to 88	Stiff CH	119	1750	0	120 (C _r = 55)	$ \begin{array}{c} 16 \\ (\phi_{\rm r} = 15) \end{array} $	110 (C _r = 55)	$15 \\ (\phi_r = 15)$
88 to 79	Very stiff to hard CH	124	2,100	0	140 (C _r = 55)	$16 $ $(\phi_r = 15)$	120 (C _r = 55)	$15 \\ (\phi_r = 15)$

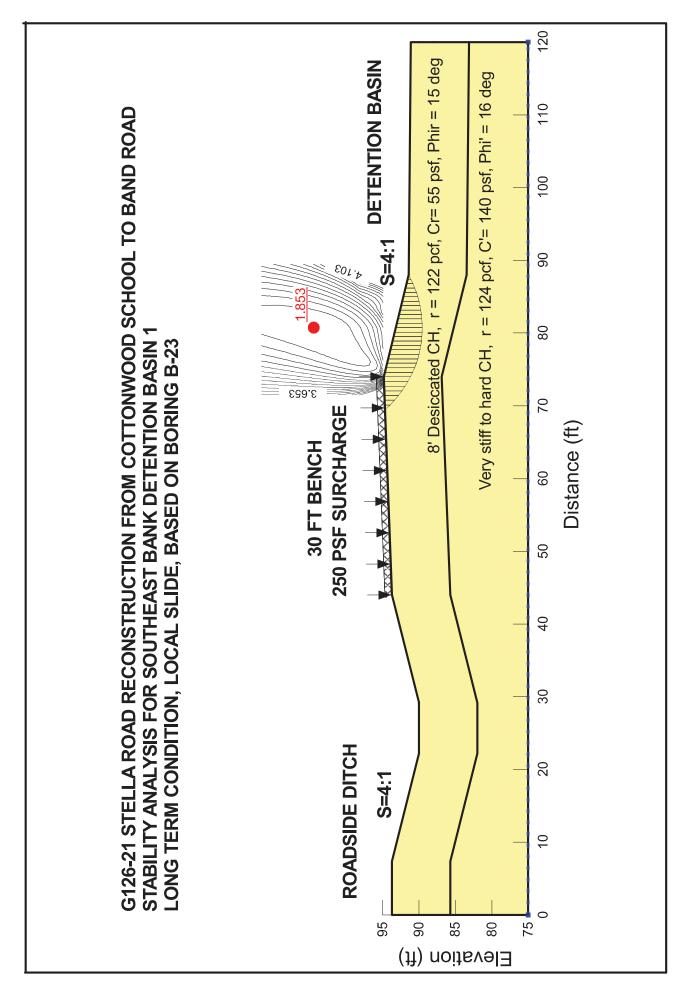
Notes:

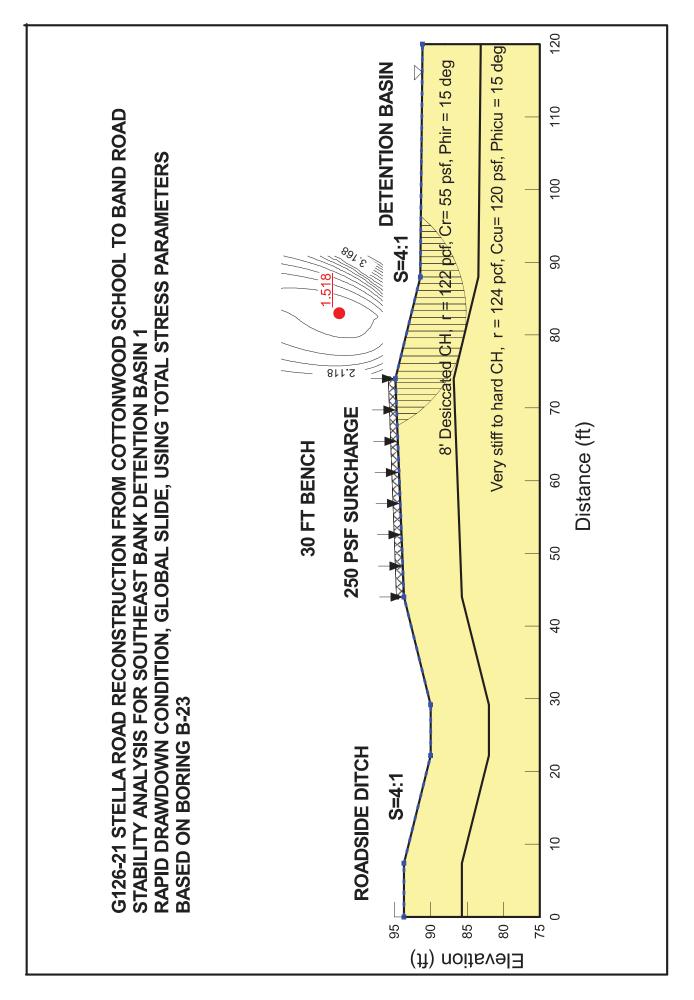
- (1) γ_m = moist unit weight of soil.
- (2) C_u =undrained cohesion, ϕ_u = angle of internal friction, under short term conditions. UU = strength parameters that were determined from Unconsolidated-Undrained (UU)triaxial tests.
- (3) C' =effective cohesion, ϕ ' =effective friction angle, effective stress parameters that were determined from Consolidated-Undrained (CU) triaxial tests with pore water pressure measurements.
- (4) C_{cu} = cohesion, ϕ_{cu} = friction angle, total stress parameters that were developed from CU triaxial tests.
- (5) C_r = cohesion for desiccated fat clay, ϕ_r = friction angle for desiccated fat clay.
- (6) CH = Fat Clay

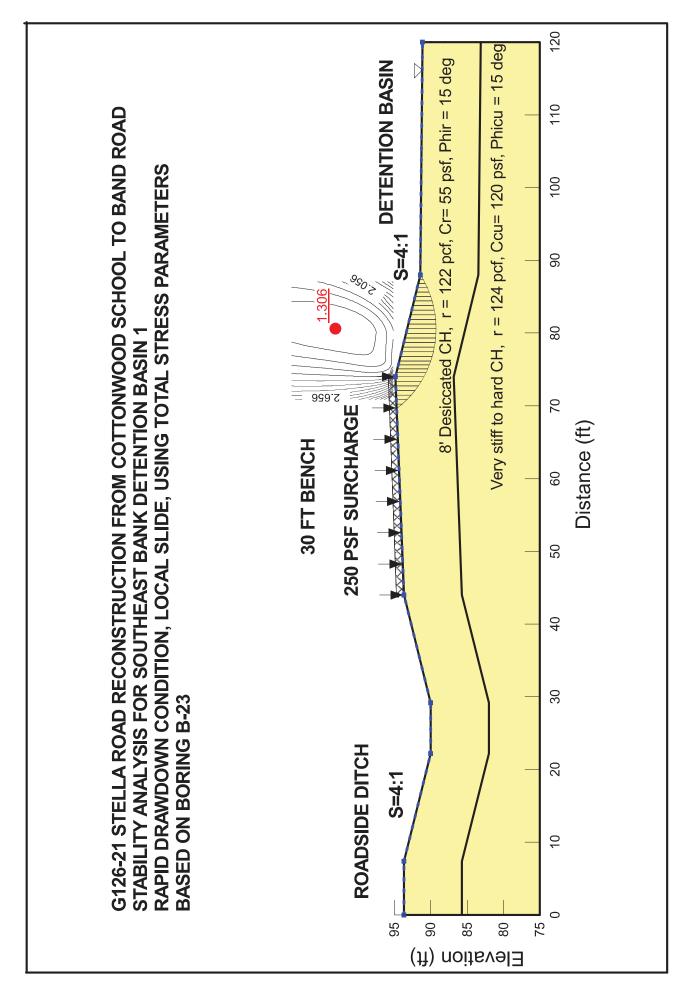












FORT BEND COUNTY ENGINEERING DEPARTMENT

STELLA ROAD FROM COTTONWOOD SCHOOL RD TO W FAIRGROUNDS RD

FORT BEND COUNTY MOBILITY BOND PROJECT #20116 PRECINCT 1

VINCENT MORALES, JR.

COMMISSIONER

PRECINCT 1

GEORGE COUNTY JUDGE

ANDY MEYERS

COMMISSIONER

PRECINCT 3

GRADY PRESTAGE

COMMISSIONER

PROJECT LOCATION

PRECINCT 2

DEXTER McCOY

COMMISSIONER

PRECINCT 4 Digitally signed by Rigoberto Rigoberto Calzoncin Reason: I am approving this Date: 2024.07.12 08:02:36-05'00'

RIGO CALZONCIN, EXECUTIVE DIRECTOR OF PUBLIC SERVICES

DATE

Digitally signed by Charles A. Kalkomey
DN: C=US, E=ckalkomey@rosenbergtx.gov
CN=Charles A. Kalkomey
Reason: I am approving this document
Date: 2024.07.11 12:14:18-05'00'

CHARLES A. KALKOMEY, P.E., CITY ENGINEER

DATE

THESE SIGNATURES ARE VOID IF CONSTRUCTION HAS NOT COMMENCED IN ONE (1) YEAR FROM DATE OF APPROVALS.



July 2024 PRECINCT 1 Fort Bend County, Texas





x:\engineering\2021\21060 - stella road\1

PROJECT

LOCATION

PROJECT LOCATION KEY MAPS # 606T, 606X

GENERAL

COVER SHEET INDEX SHEET LEGEND 4 GENERAL NOTES

5 CONSTRUCTION NOTES 6-7 TYPICAL PAVEMENT SECTIONS

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REMOVAL PLAN

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SIGNING AND PAVEMENT MARKING

SIGNING AND PAVEMENT MARKING PLAN

SWPPP

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105 FBCED STORM SEWER CONSTRUCTION DETAILS

106 FBCED PRECAST CONCRETE STORM SEWER MANHOLE DETAILS

107 FBCED JUNCTION BOX AND MANHOLE DETAILS 108 TXDOT PRECAST S.E.T.-TYII-PARALLEL DRAINAGE (PSET-SP)

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121 FBCED STREET SIGN NAME DETAILS

122 FBCED SWPPP DETAILS

FBCED PROJECT SIGN DETAILS

CROSS SECTIONS

125-133 CROSS SECTIONS

REVISIONS DATE NAME

FORT BEND COUNTY







PROJECT TITL	E: STELLA ROAD	
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY:	SHEET DESCRIPTION: INDEX SHEET	
SCALE:		SHEET NO
DATE: 1/16/2023	APPROVED BY:	2 / 13

LEGEND

SURVEY

- GUY ANCHOR

- POWER POLE - TELE. PEDESTAL

- WATER SPIGOT Ø - LIGHT POLE - MAILBOX

- STREET SIGN - FLUSH VALVE

- WATER VALVE - BLOW OFF VALVE - WATER METER

- OVERHEAD UTILITY LINE - BARBWIRE FENCE

-//- - WOOD FENCE "S" - SET 5/8" IR W/CAP

P.R.F.B.C.T. - PLAT RECORDS, FORT BEND CO.

D.R.F.B.C.T. - DEED RECORDS, FORT BEND CO.

O.R.F.B.C.T. - OFFICIAL RECORDS, FORT BEND CO.

O.P.R.F.B.C.T. - OFFICIAL PUBLIC RECORDS, FORT BEND CO.

PROPOSED

SOUTH/WEST ROW NORTH/EAST ROW SWALE STORM SEWER SANITARY SEWER WATERLINE (WL) CURB PHONE DUCTS GAS LINE OVERHEAD POWER

SAWCUT TRAFFIC SIGN BARRICADE

> F.H. W/VALVE GATE VALVE

GUY WIRE PIPELINE MARKER TYPE "BB" INLET

TYPE "C" INLET

TYPE "A" INLET STORM MANHOLE

SANITARY MANHOLE

0

TOP OF CURB TC FLOW LINE FL CENTER LINE

CL PAVEMENT PVMT. STREET LIGHT

PROFILE GRADE LINE

PGL
HYDRAULIC GRADE LINE
HGL
VERTICAL POINT OF INTERSECTION

POINT OF INTERSECTION
VPI
POINT OF TANGENCY
PT
POINT OF COMMENCEMENT
PC
LINEAR FEET

LF WATER SURFACE ELEVATION WSEL INVERT

INV DIRECTION OF FLOW DRAINAGE BOUNDARY

CUMULATIVE ACREAGE DRAINAGE AREA ACREAGE SLOPE

EXISTING SOUTH/WEST ROW

NORTH/EAST ROW SWALE STORM SEWER SANITARY SEWER WATERLINE (WL) CURB PHONE DUCTS GAS LINE OVERHEAD POWER ESMT. LINE TRAFFIC SIGN BARRICADE F.H. W/ VALVE GATE VALVE GUY WIRE PIPELINE MARKER TYPE "BB" INLET TYPE "H-2" INLET TYPE "A" INLET STORM MANHOLE SANITARY MANHOLE TOP OF CURB FLOW LINE FL CENTER LINE CL PAVEMENT PVMT. STREET LIGHT BUSINESS SINGN POWER POLE PHONE MH

GRAVEL

EBOX WATER SURFACE ELEVATION
WSEL

DIRECTION OF FLOW

REMOVE CONCRETE

SLOPE

NO.	REVISIONS	DATE	NAME
\Box			

FORT BEND COUNTY TEXAS







	PROJECT TITLE: STELLA ROAD					
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.				
	CK'D BY:	SHEET DESCRIPTION: LEGEND				
į	SCALE:		SHEET NO:			
	DATE: 1/16/2023	APPROVED BY:	3 / 133			

- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING SECURITY TO PROTECT THE PROJECT SITE, CONTRACTOR PROPERTY, EQUIPMENT, AND WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR CLEANING STREETS OF CONSTRUCTION DIRT AND DEBRIS AT CLOSE OF EACH WORK DAY.
- 4. THE CONDITION OF THE ROAD AND/OR RIGHT-OF-WAY, UPON COMPLETION OF THE JOB SHALL BE AS GOOD AS OR BETTER THAN PRIOR TO STARTING WORK.
- 5. PRIOR TO CONSTRUCTION, THE CONTRACTOR, ALONG WITH CONCURRENCE FROM THE FIELD ENGINEER, SHALL DETERMINE HIS/HER LAY-DOWN AND/OR STAGING AREA LOCATIONS.
- 6. THE CONTRACTOR SHALL NOTIFY ALL PROPERTY OWNERS A MINIMUM OF 24 HOURS PRIOR TO BLOCKING DRIVEWAYS OR ENTERING UTILITY EASEMENTS.
- 7. TRAFFIC INGRESS AND EGRESS FOR DRIVEWAYS AND PEDESTRIAN ACCESS FACILITIES SHALL BE
- 8. THE CONTRACTOR SHALL REMOVE ANY FENCES, POSTS, MAILBOXES, PLANTERS, PERMANENT TRASH CONTAINERS, CULVERTS, ETC. OR SECTIONS THEREOF, THAT ENCROACH WITHIN THE COUNTY'S RIGHT-OF-WAY. NOTE: PRIOR TO CONSTRUCTION, THE PROPERTY OWNER WAS PAID TO RELOCATE OR REPLACE THESE ITEMS OUTSIDE OF THE COUNTY'S RIGHT-OF-WAY.

 IF THE OWNER HAS FAILED TO DO SO, THE CONTRACTOR WILL REPLACE THEM WITH THE
 MINIMUM LEVEL OF QUALITY NEEDED TO SECURE THE PROPERTY AND/OR MAINTAIN MAIL
 DELIVERY, IN THAT CASE, PAYMENT FOR THESE INSTALLATIONS WILL BE INCLUDED AS EXTRA
 WORK ITEMS OR AS OVERRUNS TO EXISTING PAY ITEMS.

ANY DAMAGE CAUSED BY THE CONTRACTOR TO SUCH ITEMS LOCATED OUTSIDE OF THE COUNTY'S RIGHT-OF-WAY, SHALL BE REPLACED WITH LIKE-KIND OR BETTER AT THE CONTRACTOR'S EXPENSE.

ALSO, IF THESE ITEMS ARE LOCATED WITHIN THE PROJECT RIGHT-OF-WAY AND ARE DESIGNATED TO REMAIN, ANY DAMAGE CAUSED BY THE CONTRACTOR TO SUCH ITEMS. SHALL BE REPLACED WITH LIKE-KIND OR BETTER AT THE CONTRACTOR'S EXPENSE.

TREES, BUSHES, SHRUBBERY AND OTHER DAMAGED PLANTINGS DESIGNATED TO REMAIN SHALL BE REPLACED WITHIN 72 HOURS OF REMOVAL AND ARE TO BE THOROUGHLY WATERED-IN. NO SEPARATE PAY.

- 9. PAVED SURFACES, PAVEMENT MARKERS AND MARKINGS SHALL BE PROTECTED FROM DAMAGE
- 10. IRON RODS DISTURBED DURING CONSTRUCTION ARE TO BE REPLACED BY A REGISTERED PROFESSIONAL LAND SURVEYOR FOR THE ORIGINAL PROPERTY OWNER AT NO SEPARATE PAY.
- 11. CONSTRUCTION STAKING WILL BE PROVIDED BY THE CONTRACTOR. TWO COPIES OF STAKING NOTES TO BE PROVIDED TO THE ENGINEER PRIOR TO CONSTRUCTION.
- 12. THE COUNTY OR THE COUNTY'S SURVEYOR SHALL PROVIDE A BENCHMARK OR TEMPORARY BENCHMARK AND SURVEY CONTROLS.
- 13. THE CONTRACTOR SHALL MAINTAIN UPDATED RED-LINED RECORD DRAWINGS ON SITE FOR INSPECTION BY THE ENGINEER.
- 14. MOWING, MAINTENANCE, AND CLEAN-UP OF THE PROJECT SHALL MEET THE REQUIREMENT OF SPECIFICATION ITEM 560 (NO SEPARATE PAY). MOWING, MAINTENANCE, AND CLEAN-UP IS REQUIRED FOR THE PROJECT LIMITS AND DURATION, REGARDLESS OF THE CONTRACTOR'S SCOPE OF ACTIVITIES WITHIN THE PROJECT LIMITS.
- 15. THE REMOVAL OF ANY ABANDONED UTILITIES REQUIRED TO COMPLETE THE WORK SHALL BE INCIDENTAL AND NO SEPARATE PAYMENT SHALL BE MADE.
- 16. IT IS THE CONTRACTOR'S RESPONSIBILITY TO STOCKPILE NECESSARY MATERIAL ON—SITE OR AT A SECURED OFF—SITE LOCATION AT NO ADDITIONAL EXPENSE TO FORT BEND COUNTY. ANY SUITABLE EXCAVATED MATERIAL ON THE PROJECT WHICH IS AVAILABLE AT THE TIME OF NEED; WHETHER FROM STORM SEWER, ROADWAY, AND/OR CHANNEL EXCAVATION, SHALL BE USED BEFORE BORROW IS BROUGHT ON-SITE.
- 17. MANHOLES, JUNCTION BOXES, INLETS, AND RISERS ARE TO BE PRE-CAST OR CAST IN PLACE.
- 18. THE FOLLOWING DETAILS ARE MINIMUM REQUIREMENTS AND MAY BE SUPERSEDED BY GEOTECHNICAL ENGINEER RECOMMENDATIONS OR MORE STRINGENT REQUIREMENTS FROM THE CITY'S ETJ PROJECT IS
- 19. POP UP DRAINS ARE NOT ALLOWED IN FORT BEND COUNTY RIGHT OF WAY.
- 20. CONTRACTOR IS RESPONSIBLE FOR HIS OWN HORIZONTAL AND VERTICAL CONTROL. REFERENCE POINTS AND CONSTRUCTION STAKING AS INCIDENTAL TO THE PROJECT.
- 21. CONTRACTOR TO FIELD VERIFY ALL BOUNDARY AND TOPOGRAPHIC INFORMATION PRIOR TO BEGINNING
- 22. EXISTING UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY BASED ON THE BEST AVAILABLE INFORMATION. CONTRACTOR TO FIELD VERIFY LOCATION OF ANY EXISTING UTILITIES AND OTHER FACILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO LOCATE AND PRESERVE ANY
- 23 THE LENGTH OF PROPOSED LINDERGROUND LITHLITY LINES SHOWN ARE APPROXIMATE ONLY LENGTHS OF LINES MAY VARY DUE TO FIELD CONDITIONS ENCOUNTERED AT THE TIME OF CONSTRUCTION.

 CONTRACTOR SHALL PROVIDE ALL LABOR AND MATERIALS NECESSARY FOR UTILITY LINES TO SERVE
 THEIR INTENDED PURPOSE AND SHALL BE RESPONSIBLE FOR THE REROUTING OF LINES OCCASIONED BY CONFLICTS WITH OTHER UTILITIES AND SITE FEATURES.
- 24. WATER METERS, UTILITY LINES AND APPURTENANCES, DRIVEWAYS, AND ALL OTHER ITEMS TO BE LOCATED WITHIN THE STREET RIGHT-OF-WAY OR A PUBLIC EASEMENT, ARE TO BE CONSTRUCTED IN STRICT ACCORDANCE WITH CURRENT GOVERNING CITY, COUNTY AND STATE STANDARDS
- 25. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY BUILDING PERMITS AND FOR NOTIFICATION OF ALL AUTHORIZED INSPECTORS, SUPERINTENDENTS OR PERSONS IN CHARGE OF PRIVATE OR PUBLIC UTILITIES AFFECTED BY HIS OPERATIONS PRIOR TO COMMENCEMENT OF WORK.
- 26. CONTRACTOR SHALL NOTIFY UTILITY COORDINATING COMMITTEE BY TELEPHONE AT LEAST TWO FULL WORKING DAYS BEFORE STARTING WORK IN ANY STREET RIGHT-OF-WAY OR PUBLIC EASEMENT.

GENERAL NOTES CONT.

- 27. CONTRACTOR TO BE RESPONSIBLE FOR PROVIDING AND MAINTAINING ALL BARRICADES, WARNING SIGNS, FLASHING LIGHTS AND TRAFFIC CONTROL DEVICES IN CONFORMANCE WITH PART VI OF THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", (TEXAS M.U.T.C.D. MOST RECENT EDITION AS REVISED) DURING CONSTRUCTION.
- 28. IF CONTRACTOR OPTS TO USE OPEN CUT METHOD OF CONSTRUCTION, TRENCH BEDDING AND BACKFILL SHALL MEET CITY OF STAFFORD REQUIREMENTS AND ALL OPEN EXCAVATIONS IN VEHICULAR TRAFFIC AREAS SHALL BE COVERED WITH ANCHORED STEEL PLATES CAPABLE OF SUPPORTING HS 20 LOADING AT END OF EACH DAYS WORK OR

TRAFFIC CONTROL

- THE CONTRACTOR SHALL PROVIDE AND INSTALL TRAFFIC CONTROL DEVICES IN CONFORMANCE WITH PART VI OF THE MOST RECENT EDITION OF THE MANUAL UNIFORM TRAFFIC CONTROL DEVICES AND THE APPROVED TRAFFIC CONTROL F
- 2. THE CONTRACTOR SHALL MAINTAIN AT LEAST ONE LANE OF TRAFFIC IN EACH DIRECTION DURING WORKING HOURS EXCEPT DURING FLAGGING OPERATION OR PROVIDE DETOURS AROUND THE CONSTRUCTION SITE AND PROVIDE PUBLIC NOTIFICATION.
- 3. LANE CLOSURES SHALL BE DURING OFF-PEAK HOURS ONLY (MONDAY THROUGH FRIDAY 9 A.M. TO 4 P.M.) UNIFORMED PEACE OFFICERS OR FLAGGERS IN RADIO CONTACT ARE REQUIRED TO DIRÉCT TRAFFIC DURING LANE CLOSURES.
- 4. DETOURS REQUIRE PRIOR APPROVAL OF THE FIELD ENGINEER AND PRECINCT. DETOUR PLANS, IF ALLOWED, MUST INCLUDE APPROPRIATE DETOUR SIGNAGE, PUBLIC NOTICE VIA SIGNAGE TWO WEEKS IN ADVANCE STATING THE DATES OF THE AGREED UPON DATE OF CLOSURE AND DATE THE ROAD WILL RE-OPEN TO TRAFFIC. CONTRACTOR TO USE (WITH PRIOR APPROVAL OF THE FIELD ENGINEER) HIGH EARLY STRENGTH CONCRETE AND OTHER RELATED CONSTRUCTION METHODS TO MINIMIZE THE DURATION OF THE DETOUR AND TO ENSURE THAT THE ROADWAY IS OPEN ON, OR PRIOR TO, THE AGREED UPON DATE,
- 5. ONE DAY PRIOR TO THE IMPLEMENTATION OF A TRAFFIC CONTROL PLAN PHASE OR STEP, OR THE IMPLEMENTATION OF AN ADDITIONAL, REVISED, OR NEW TRAFFIC CONTROL ELEMENT, THE CONTRACTOR SHALL MEET WITH THE ENGINEER TO GIVE A DETAILED DESCRIPTION OF THE CONTRACTOR'S PLAN AND PREPARATIONS. THE CONTRACTOR SHALL OBTAIN WRITTEN CONCURRENCE FROM THE ENGINEER THAT ADEQUATE PROJECT SHALL OBTAIN WRITTEN CONCORRENCE FROM THE ENGINEER THAT ADEQUATE PROJECT PROGRESS HAS BEEN ACHIEVED AND THAT ADEQUATE PREPARATIONS ARE IN PLACE PRIOR TO SWITCHING TRAFFIC. IF, IN THE OPINION OF THE ENGINEER, REQUIRED PROGRESS AND ADEQUATE PREPARATIONS ARE NOT COMPLETE, THE CONTRACTOR SHALL NOT IMPLEMENT THE NEXT PHASE, STEP, OR ELEMENT OF TRAFFIC CONTROL UNTIL INCOMPLETE CONSTRUCTION ITEMS OR PREPARATIONS ARE COMPLETED. TIME EXTENSIONS WILL NOT BE GRANTED FOR DELAYS CAUSED BY THE INCOMPLETE CONSTRUCTION ITEMS OR INADEQUATE CONTRACTOR PREPARATIONS REQUIRED TO IMPLEMENT TRAFFIC CONTROL. TIME EXTENSIONS
- 6. TRAFFIC CONTROL PER THE CONTRACT IS REQUIRED FOR THE ENTIRE DURATION OF THE PROJECT, INCLUDING THE PUNCHLIST PERIOD. PAYMENT FOR TRAFFIC CONTROL THAT IS PROPERLY INSTALLED FOR LESS THAN A FULL MONTH SHALL BE BASED ON A PERCENTAGE BASIS OF THE TIME INSTALLED. TRAFFIC CONTROL PAYMENTS TO THE CONTRACTOR SHALL END 10 DAYS AFTER SUBSTANTIAL COMPLETION, ALTHOUGH PROPER TRAFFIC CONTROL MUST BE MAINTAINED UNTIL PUNCHLIST COMPLETION.
- 7. THE PURPOSE OF THE CONSTRUCTION SEQUENCE AND TRAFFIC HANDLING OUTLINED HEREIN IS TO DOCUMENT A VIABLE TOP THAT CAN BE UTILIZED TO CONSTRUCT THE PROJECT. IT IS THE BASIS OF ESTIMATION FOR THE TRAFFIC CONTROL BID ITEMS, AND IS TO BE UTILIZED AND IMPLEMENTED, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

IF THE CONTRACTOR CHOOSES TO USE A DIFFERENT TCP, HE/SHE SHALL PREPARE AND SUBMIT THE ALTERNATIVE TCP TO THE COUNTY FOR APPROVAL NO LESS THAN 10 WORKING DAY'S PRIOR TO THE PROPOSED IMPLEMENTATION DATE. THE TCP SHALL BE DRAWN TO SCALE AND SIGNED & SEALED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF TEXAS. UPON APPROVAL BY FORT BEND COUNTY, THE ALTERNATIVE PLAN SHALL BECOME THE BASIS FOR A "CHANGE IN CONTRACT" TO REVISE THE TRAFFIC CONTROL BID ITEMS ACCORDINGLY AND BECOME PART OF THE CONTRACT DOCUMENTS.

- 8. ALL TEMPORARY PAVEMENT MARKINGS ON PERMANENT PAVEMENT SHOULD BE RPMS OR TABS.
- 9. TRAFFIC PATTERN CHANGES REQUIRE CHANGEABLE MESSAGE BOARDS PLACED AT LEAST 2 WEEKS IN ADVANCE OF PROPOSED CHANGE. QUANTITY, PLACEMENT AND WORDING TBD BY FBC.

STORM SEWER CONSTRUCTION NOTES

- STORM SEWER PIPE TO BE POLYVINYL CHLORIDE PIPE (PVC) CONFORMING TO ASTM D3034, SDR 35 (PIPE SIZE 12"), AND REINFORCED CONCRETE PIPE (RCP) CONFORMING TO ASTM C-76, CLASS III, EXCEPT AS OTHERWISE NOTED ON THE PLANS.
- ALL STORM SEWERS TO RECEIVE BEDDING AND BACKFILL IN ACCORDANCE WITH THE DETAILS CONTAINED IN THE PLANS. STORM SEWERS WITHIN PUBLIC STREET RIGHTS—OF—WAY OR EASEMENTS TO RECEIVE BEDDING AND BACKFILL IN ACCORDANCE WITH FORT BEND COUNTY W.C.I.D. NO.2 SPECIFICATIONS FOR SEWER CONSTRUCTION, LATEST PRINTING AND AMENDMENTS
- 3. WHERE MANHOLES, GRATE INLETS, OR JUNCTION BOXES ARE LOCATED WITHIN PAVED AREAS, CONTRACTOR SHALL SET RIM ELEVATIONS TO MATCH TOP OF PAVEMENT ELEVATIONS.
- 4. CONTRACTOR TO ALLOW A MINIMUM OF 6-INCH VERTICAL CLEARANCE BETWEEN STORM SEWER

AT&T TEXAS/SWBT FACILITIES

- 1. THE LOCATIONS OF AT&T TEXAS/SWBT FACILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THIS FAILURE TO EXACTLY LOCATE AND PRESERVE THESE UNDERGROUND UTILITIES
- 2. THE CONTRACTOR SHALL CALL 1-800-344-8377 (TEXAS 811) A MINIMUM OF 48 HOURS PRIOR O CONSTRUCTION TO HAVE UNDERGROUND LINES FIELD LOCATED.
- 3. WHEN EXCAVATING WITHIN EIGHTEEN INCHES (18") OF THE INDICATED LOCATION OF AT&T TEXAS/SWBT FACILITIES, ALL EXCAVATIONS MUST BE ACCOMPLISHED USING NON-MECHANIZED EXCAVATION PROCEDURES. WHEN BORING, THE CONTRACTOR SHALL EXPOSE THE AT&T TEXAS CHAPT FOR CHAP
- 4. WHEN AT&T TEXAS/SWBT FACILITIES ARE EXPOSED, THE CONTRACTOR WILL PROVIDE SUPPORT TO PREVENT DAMAGE TO THE CONDUIT DUCTS OR CABLES. WHEN EXCAVATING NEAR TELEPHONE POLES THE CONTRACTOR SHALL BRACE THE POLE FOR SUPPORT.
- 5. THE PRESENCE OR ABSENCE OF AT&T TEXAS/SWBT UNDERGROUND CONDUIT FACILITIES OR BURIED CABLE FACILITIES SHOWN ON THESE PLANS DOES NOT MEAN THAT THERE ARE NO DIRECT BURIED CABLES OR OTHER CABLES IN CONDUIT IN THE AREA.
- 6. PLEASE CONTACT THE AT&T TEXAS DAMAGE PREVENTION MANAGER MR. ROOSEVELT LEE JR. AT (713) 567-4552 OR EMAIL HIM AT RL7259@ATT.COM, IF THERE ARE QUESTIONS ABOUT BORING OR EXCAVATING NEAR OUR AT&T TEXAS/SWBT FACILITIES.

CAUTION: UNDERGROUND GAS FACILITIES

THE CONTRACTOR SHALL CONTACT THE UTILITY COORDINATING COMMITEE AT 1-800-545-6005 OR 811 A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION TO HAVE MAIN AND SERVICE LINES FIELD

*WHEN CENTERPOINT ENERGY PIPE LINE MARKINGS ARE NOT VISIBLE, CALL (713) 945-8036 OR (713) 945-8037 (7:00 A.M. TO 4:30P.M.) FOR STATUS OF LINE LOCATION REQUEST BEFORE EXCAVATION REGINS.

*WHEN EXCAVATING WITHIN EIGHTEEN INCHES (18") OF THE INDICATED LOCATION OF CENTERPOINT ENERGY FACILITIES, ALL EXCAVATION MUST BE ACCOMPLISHED USING NON-MECHANIZED EXCAVATION

*WHEN CENTERPOINT ENERGY GAS FACILITIES ARE EXPOSED, SUFFICIENT SUPPORT MUST BE PROVIDED TO THE FACILITIES TO PREVENT EXCESSIVE STRESS ON THE PIPING.

* FOR EMERGENCIES REGARDING GAS LINES CALL (713) 659-3552 OR (713) 207-4200

THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY DAMAGES CAUSED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE THESE UNDERGROUND FACILITIES.

WARNING: OVERHEAD ELECTRICAL LINES

OVERHEAD LINES MAY EXIST ON THE PROPERTY. THE LOCATION OF OVERHEAD LINES HAS NOT BEEN SHOWN ON THESE DRAWINGS AS THE LINES ARE CLEARLY VISIBLE, BUT YOU SHOULD LOCATE THEM PRIOR TO BEGINNING ANY CONSTRUCTION. TEXAS LAW, SECTION 752, HEALTH & SAFETY CODE FORBIDS ACTIVITIES THAT OCCUR IN CLOSE PROXIMITY TO HIGH VOLTAGE LINES, SPECIFICALLY:

- · ANY ACTIVITY WHERE PERSON OR THINGS MAY COME WITHIN SIX(6) FEET OF LIVE OVERHEAD
- HIGH VOLTAGE LINES; AND

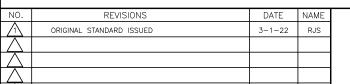
 OPERATING A CRANE, DERRICK, POWER SHOVEL, DRILLING RIG, PILE DRIVER, HOISTING EQUIPMENT, OR SIMILAR APPARATUS WITHIN 10 FEET OF LIVE OVERHEAD HIGH VOLTAGE LINES.

PARTIES RESPONSIBLE FOR THE WORK. INCLUDING CONTRACTORS ARE LEGALLY RESPONSIBLE FOR THE SAFETY OF CONSTRUCTION WORKERS UNDER THIS LAW. THIS LAW CARRIES BOTH CRIMINAL AND CIVIL LIABILITY. TO ARRANGE FOR LINES TO BE TURNED OFF OR REMOVED CALL CENTERPOINT ENERGY

ACTIVITIES ON OR ACROSS CENTERPOINT ENERGY FEE OR EASEMENT PROPERTY
NO APPROVAL TO USE, CROSS OR OCCUPY CENTERPOINT FEE OR EASEMENT PROPERTY IS GIVEN. IF
YOU NEED TO USE CENTERPOINT PROPERTY, PLEASE CONTACT OUR SURVEYING & RIGHT OF WAY

STORM WATER QUALITY

1. SWQMP: THIS PROJECT DOES NOT REQUIRE A STORM WATER QUALITY MANAGEMENT PLAN (SWOMP) BECAUSE IT DOES NOT MEET THE DEFINITION OF NEW DEVELOPMENT OR SIGNIFICANT REDEVELOPMENT UNDER THE FORT BEND COUNTY REGULATIONS.



FORT BEND COUNTY







	PROJECT TITLE: STELLA ROAD		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY:	SHEET DESCRIPTION: GENERAL NOTES	
10	SCALE:		SHEET NO:
fr.	DATE: 1/16/2023	APPROVED BY:	4 / 133

CONSTRUCTION

- 1. FORT BEND COUNTY MUST BE INVITED TO THE PRE-CONSTRUCTION MEETING.
- 2. CONTRACTOR SHALL NOTIFY FORT BEND COUNTY ENGINEERING DEPARTMENT 48 HOURS PRIOR TO COMMENCING CONSTRUCTION AND 48 HOUR NOTICE TO ANY CONSTRUCTION ACTIVITY WITHIN THE LIMITS OF THE PAVING AT CONSTRUCTION@FBCTX.GOV.
- 3. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED FROM FORT BEND COUNTY PRIOR TO COMMENCING CONSTRUCTION OF ANY IMPROVEMENTS WITHIN COUNTY ROAD RIGHT OF WAYS.
- 4. ALL PAVING IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH FORT BEND COUNTY "RULES, REGULATIONS AND REQUIREMENTS" RELATING TO THE APPROVAL AND ACCEPTANCE OF IMPROVEMENTS IN SUBDIVISIONS AS CURRENTLY AMENDED.
- ALL ROAD WIDTHS, CURB RADII AND CURB ALIGNMENT SHOWN INDICATES BACK OF CURB.
- 6. A CONTINUOUS LONGITUDINAL REINFORCING BAR SHALL BE USED IN THE CURBS.
- 7. ALL CONCRETE PAVEMENT SHALL BE 5½ SACK CEMENT WITH A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS. TRANSVERSE EXPANSION JOINTS SHALL BE INSTALLED AT EACH CURB RETURN AND AT A MAXIMUM SPACING OF 60 FFFT
- 8. ALL WEATHER ACCESS TO ALL EXISTING STREETS AND DRIVEWAYS SHALL BE MAINTAINED AT ALL TIMES.
- 9. 4" X 12" REINFORCED CONCRETE CURB SHALL BE PLACED IN FRONT OF SINGLE FAMILY LOTS ONLY. ALL OTHER AREAS SHALL BE 6" REINFORCED CONCRETE CURB.
- CURB HEADERS ARE REQUIRED AT CURB CONNECTIONS TO HANDICAP RAMPS, WITH NO CONSTRUCTION JOINT WITHIN 5' OF RAMPS.
- 11. GUIDELINES ARE SET FORTH IN THE TEXAS "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", AS CURRENTLY AMENDED, SHALL BE OBSERVED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE FLAGMEN, SIGNING, STRIPING AND WARNING DEVICES, ETC., DURING CONSTRUCTION BOTH DAY AND NIGHT.
- 12. ALL R1-1 STOP SIGNS SHALL BE A MINIMUM OF 36"X36" WITH DIAMOND GRADE SHEETING PER TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES.
- 13. STREET NAME SIGNAGE SHALL BE ON A 9" HIGH SIGN FLAT BLADE W/REFLECTIVE GREEN BACKGROUND. STREET NAMES SHALL BE UPPER AND LOWERCASE LETTERING WITH UPPERCASE LETTERS OF 6" MINIMUM AND LOWERCASE LETTERS OF 4.5" MINIMUM. THE LETTERS SHALL BE REFLECTIVE WHITE. STREET NAME SIGNS SHALL BE MOUNTED ON STOP SIGN POST.
- 14. A BLUE DOUBLE REFLECTORIZED BUTTON SHALL BE PLACED AT ALL FIRE HYDRANT LOCATIONS. THE BUTTON SHALL BE PLACED 12 INCHES OFF OF THE CENTERLINE OF THE STREET ON THE SAME SIDE AS THE HYDRANT.
- 15. THE PROJECT AND ALL PARTS THEREOF SHALL BE SUBJECT TO INSPECTION FROM TIME TO TIME BY INSPECTORS DESIGNATED BY FORT BEND COUNTY. NO SUCH INSPECTIONS SHALL RELIEVE THE CONTRACTOR OF ANY OF ITS OBLIGATIONS HEREUNDER. NEITHER FAILURE TO INSPECT NOR FAILURE TO DISCOVER OR REJECT ANY OF THE WORK AS NOT IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS, REQUIREMENTS AND SPECIFICATIONS OF FORT BEND COUNTY OR ANY PROVISION OF THIS PROJECT SHALL BE CONSTRUED TO IMPLY AN ACCEPTANCE OF SUCH WORK OR TO RELIEVE THE CONTRACTOR OF ANY OF ITS OBLIGATIONS HEREUNDER.
- 16. STABILIZED SUBGRADE: DETERMINE THE THICKNESS OF THE STABILIZED SUBGRADE AFTER CURING AND COMPACTION. IF THE SUBGRADE DEPTH IS GREATER THAN THE PROPOSED THICKNESS BY 20% OR MORE, THE CMT LAB MUST PROVIDE VERIFICATION THE PERCENTAGE OF MATERIAL BEING USED TO STABILIZE THE SUBGRADE MEETS OR EXCEEDS PROJECT REQUIREMENTS. TEST RESULTS REQUIRED.
- 17. CONTRACTOR TO PROVIDE MONTHLY SCHEDULE UPDATES AND WEEKLY LOOK AHEAD

NOTE: FORT BEND COUNTY NOTES SUPERSEDE ANY CONFLICTING NOTES.

NO.	REVISIONS	DATE	NAME
\triangle	ORIGINAL STANDARD ISSUED	3-1-22	RJS
2	ADDED NOTE 17	3-1-23	RJS
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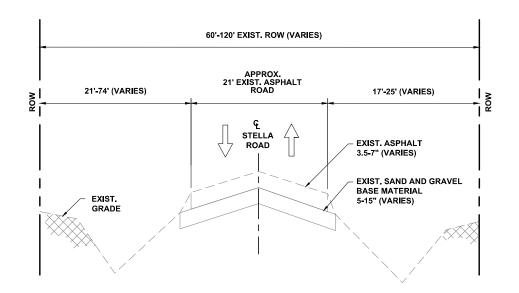
FORT BEND COUNTY
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PROJECT TITL	E: STELLA ROAD	
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY: AM	SHEET DESCRIPTION: CONSTRUCTION NOTES	
SCALE:		SHEET NO:
DATE: 1/16/2023	APPROVED BY:	5 / 133

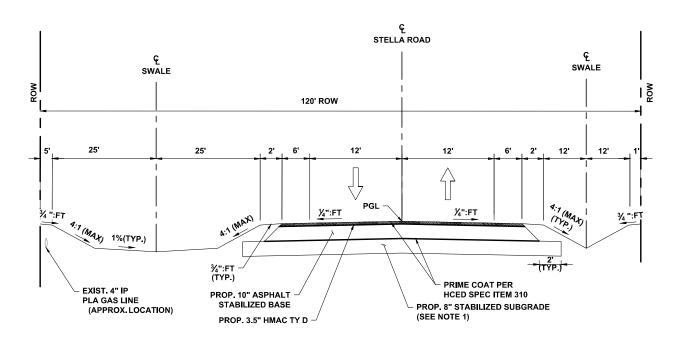


€ STELLA ROAD င့ SWALE ર્ SWALE 60' ROW 1' - 46' 4.5' - 8' 4.5' - 8' 2' (VARIES) (VARIES) 12' 12' 6' 2' 12' 12' TYP AX ½":FT PRIME COAT PER PROP. 10" ASPHALT HCED SPEC ITEM 310 PROP. 8" STABILIZED SUBGRADE (SEE NOTE 1) STABILIZED BASE PROP. 3.5" HMAC TY D

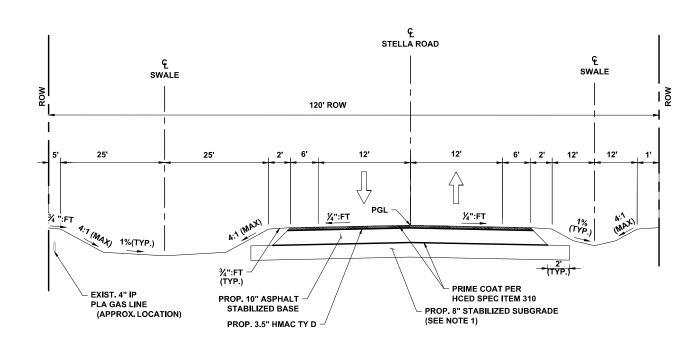
NOTES:

1. PER GEOTECHNICAL INVESTIGATION FOR STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD REPORT NO. G126-21 DATED JULY 2021, SUBGRADE WITHIN HIGHLY EXPANSIVE FAT CLAY (CH) SOIL SHALL BE STABILIZED WITH A MIN. OF 7% LIME. WHERE SANDY SOILS ARE EXPOSED AT GROUND SURFACE, SUBGRADE SHOULD BE STABILIZED WITH A MIN 3% LIME AND 7% FLY ASH.

EXISTING TYPICAL SECTION STELLA ROAD



PROPOSED TYPICAL SECTION STELLA ROAD N.T.S. STA 1+61.85 - 5+20.00



PROPOSED TYPICAL SECTION STELLA ROAD

N.T.S. STA 5+20.00 - 12+74.00 STA 19+21.07 - 25+50.00

PROPOSED TYPICAL SECTION **STELLA ROAD**

N.T.S. STA 12+74.00 - 19+21.07

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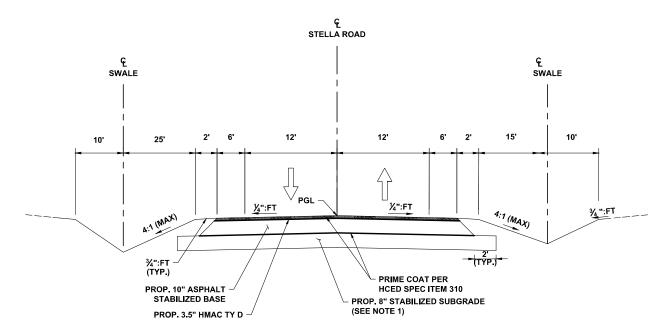
FORT BEND COUNTY TEXAS



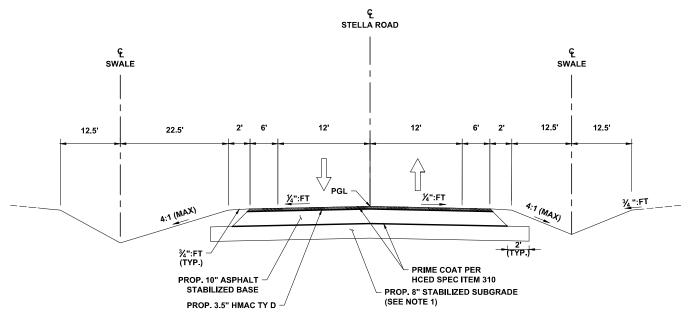




	PROJECT TITLE: STELLA ROAD		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY:	SHEET DESCRIPTION: TYPICAL PAVEMENT SECTIONS	
110	SCALE:		SHEET NO:
ch_	DATE: 1/16/2023	APPROVED BY:	6 / 133

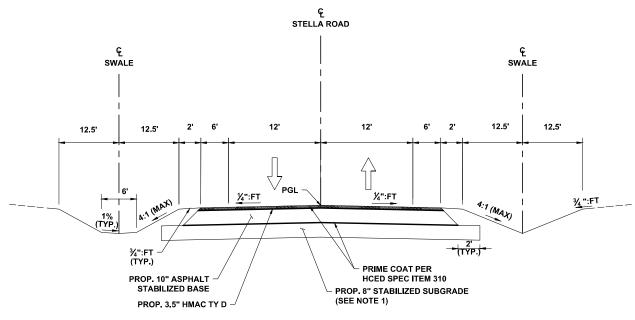


PROPOSED TYPICAL SECTION STELLA ROAD N.T.S. STA. 25+50.00 - 37+66.69



PROPOSED TYPICAL SECTION STELLA ROAD

<u>N.T.S.</u> STA. 37+66.69 - 47+00.00



PROPOSED TYPICAL SECTION STELLA ROAD N.T.S. STA. 47+00.00 - END

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FORT BEND COUNTY TEXAS







PROJECT TITE			
DRAWN BY: GB	COTTONINOOD SCHOOL BD TO W EXIDEDOLINDS DD 1		
CK'D BY:	SHEET DESCRIPTION: TYPICAL PAVEMENT SECTIONS		
SCALE:		SHEET NO:	
DATE: 1/16/2023	APPROVED BY:	7 / 133	

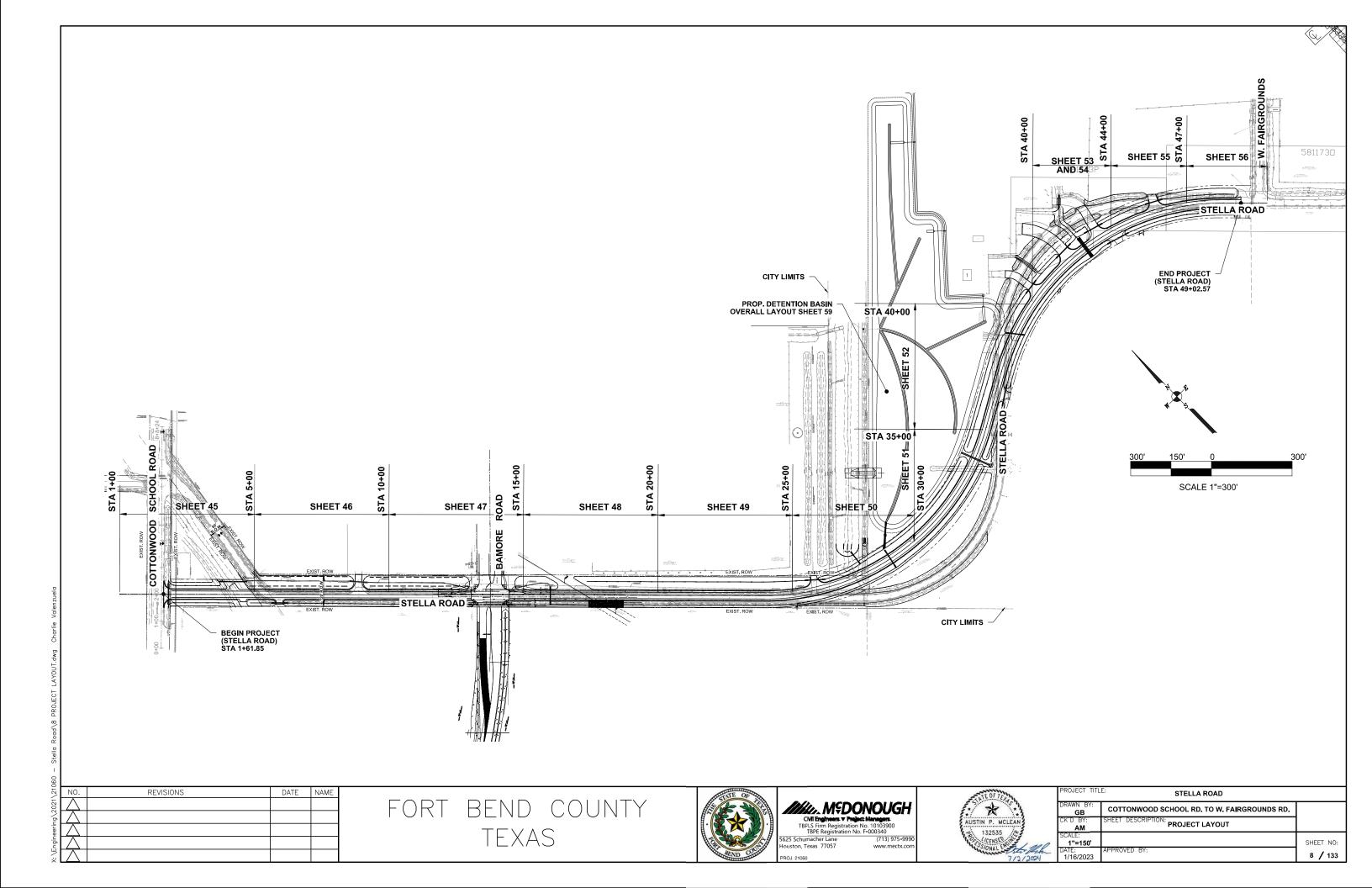
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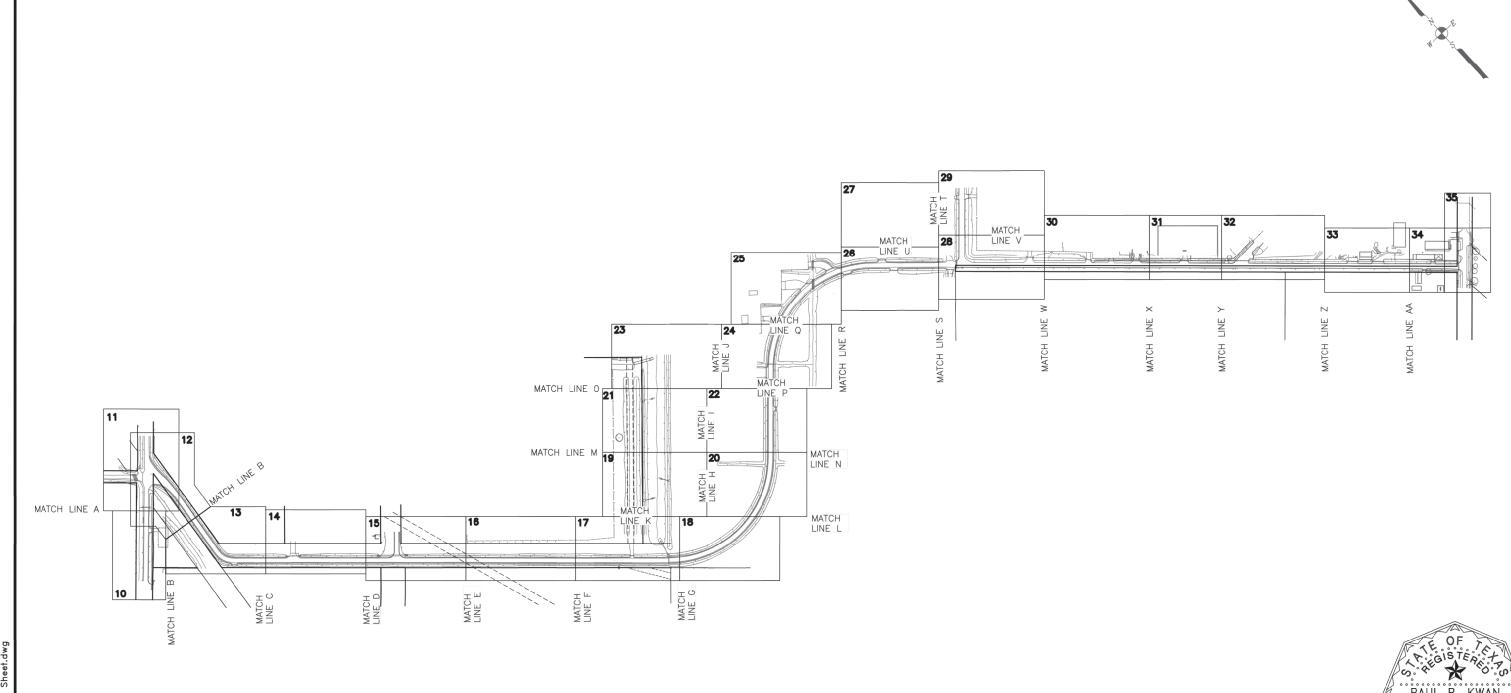
1. PER GEOTECHNICAL INVESTIGATION FOR

AND 7% FLY ASH.

STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD REPORT NO. G126-21 DATED JULY 2021,

SUBGRADE WITHIN HIGHLY EXPANSIVE FAT CLAY (CH) SOIL SHALL BE STABILIZED WITH A MIN. OF 7% LIME. WHERE SANDY SOILS ARE EXPOSED AT GROUND SURFACE, SUBGRADE SHOULD BE STABILIZED WITH A MIN 3% LIME







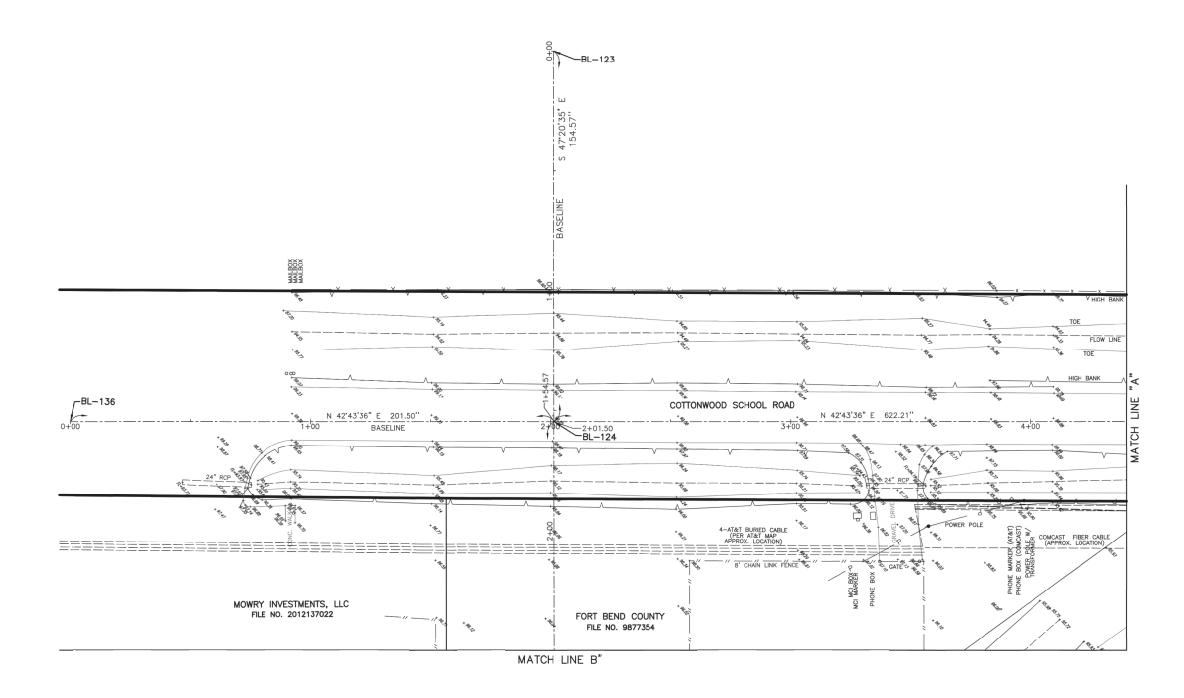
07/26/2022

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL

FORT BEND COUNTY
ENGINEERING DEPARTMENT



NDTECH	PROJECT TITI	LE: STELLA ROAD		
NDTECH	FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIV	
rth Loop West, Suite 300, buston, Texas 77008	SHEET DESCRI	RIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY		
51-7068 F: 713-861-4131	DRAWN BY:		DATE:	
S Registration No. 10019100	CK'D BY:	SCALE: AS SHOWN	SHEET N	NO: 133



BASELINE DATA

POINT	STATION	NORTHING	EASTING	DESCRIPTION
BL-123	0+00.00	13,754,244.13	2,977,397.57	STELLA 0+00
BL-124	1+54.57	13,754,139.39	2,977,511.24	STELLA ROAD INTERSECTION
BL-136	0+00.00	13,753,991.37	2,977,374.52	COTTONWOOD SCHOOL 0+00

NO. DATE NAME REVISIONS 08/06/21 ADDED BH-12&14, REVISED & SET BASELINE ADDED SHEET DESCRIPTION TITLE 09/20/21 05/06/22 YL UPDATED TOPOGRAPHIC & BASELINE 07/26/22 YL UPDATED SHEET NUMBERS

FORT BEND COUNTY ENGINEERING DEPARTMENT



PROJ	IECT TITL	:	STELLA ROAD		
	FROM	COTTONW	OOD SCHOOL ROAD TO BAN	ND ROAD	CIVIL STANDARD
SHEE	T DESCR		RAPHIC AND RIGHT OF WAY	' SURVEY	
DRAV	VN BY:				DATE:
CK'D	BY:	SCALE:	AS SHOWN		SHEET NO: 10 / 133

SCALE: 1" = 40'

1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), ANDBS (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.

ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS—GEOID 03).

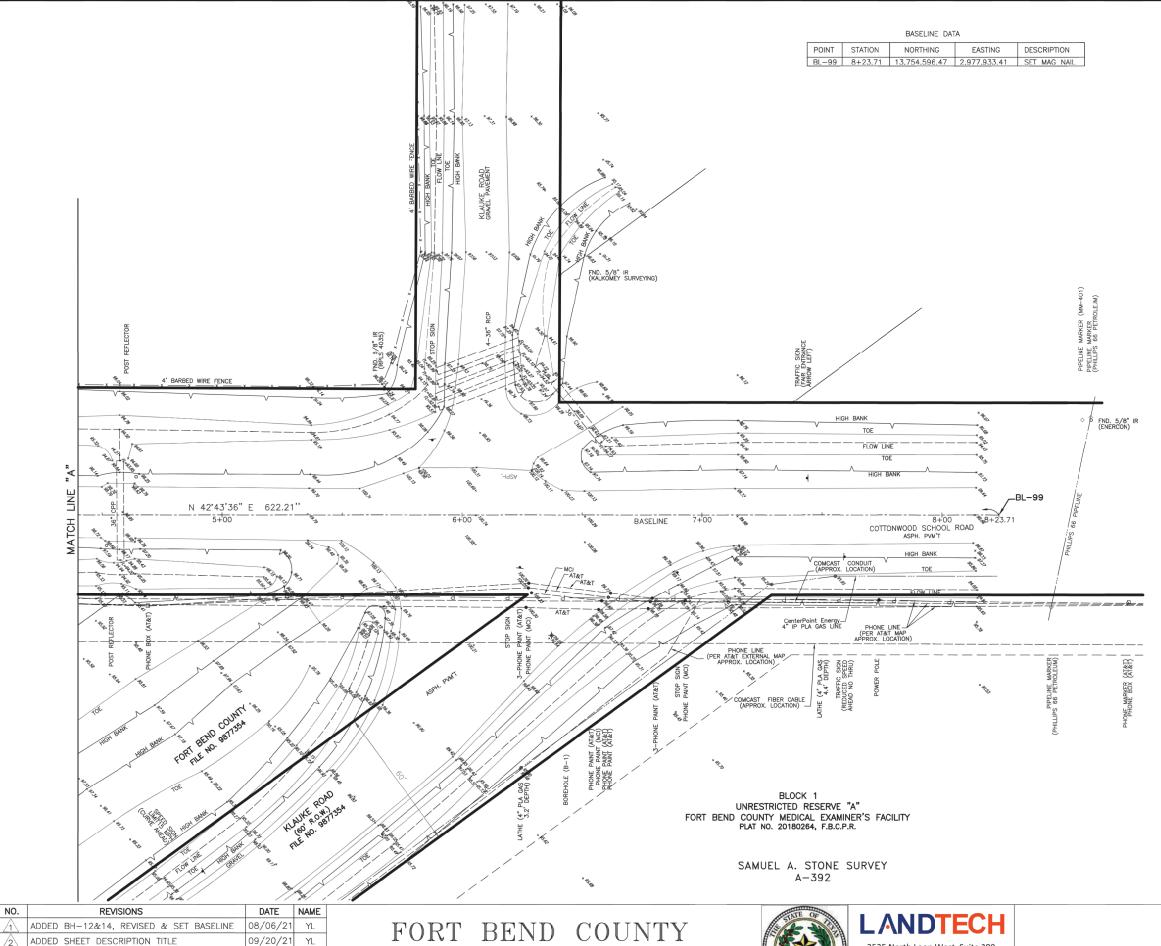
BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 90.26 FEET

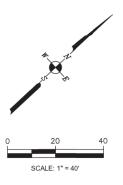
LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
VOL. = VOLUME
P.G. = PAGE
ESMT. = EASEMENT
SANLSWR.ESMT. = SANLTARY SEWER EASEMENT
SANLSWR.ESMT. = SANLTARY SEWER EASEMENT

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Pour Kurn 07/26/2022





- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
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BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEET

LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
A.E. = AERILA EASEMENT
VOL. = VOLUME
P.G. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANTARY SEWER EASEMENT
SAN.SWR.ESMT. = SANTARY SEWER EASEMENT



07/26/2022

09/20/21 UPDATED TOPOGRAPHIC & BASELINE 05/06/22 07/26/22 YL

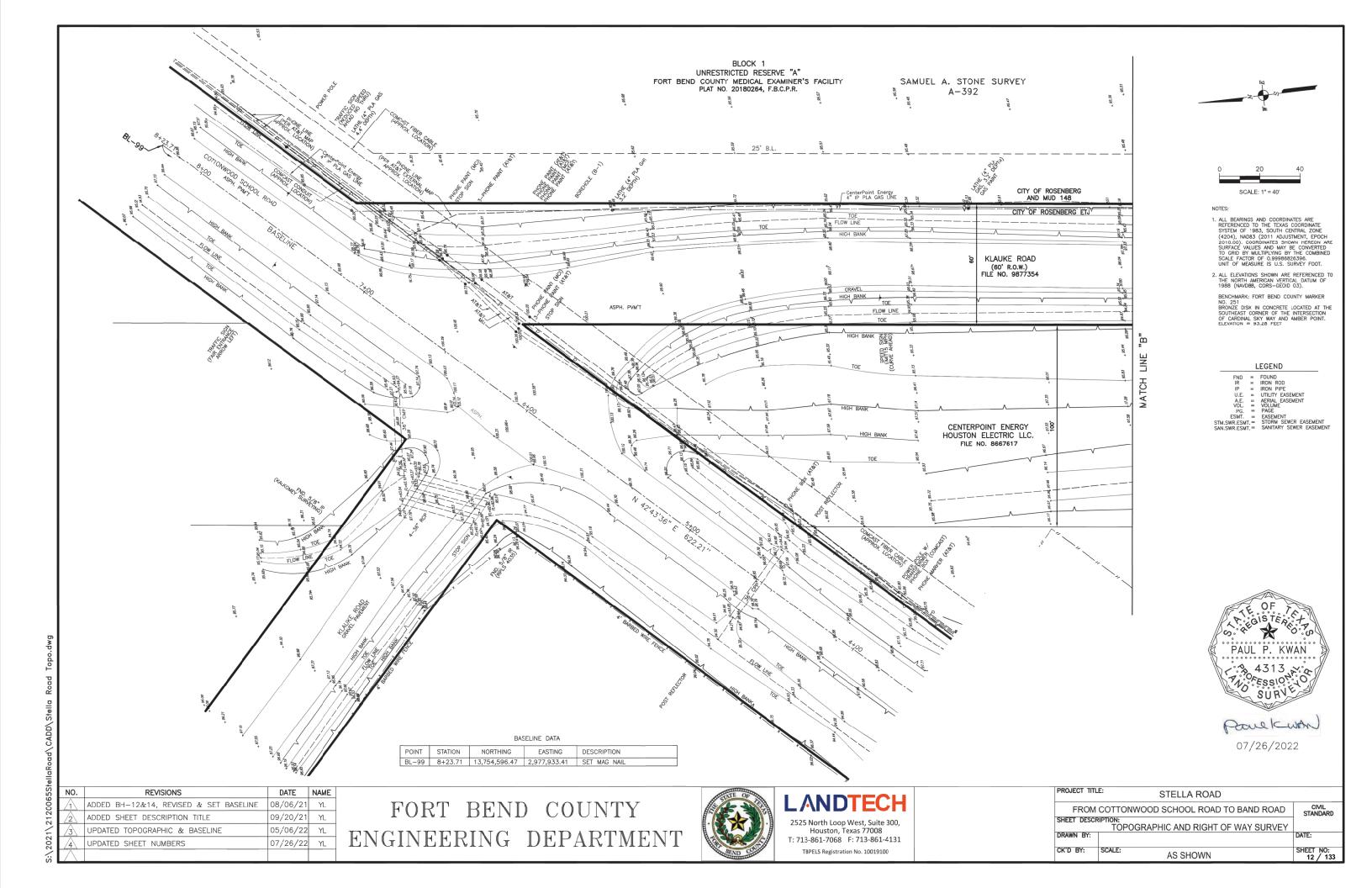
UPDATED SHEET NUMBERS

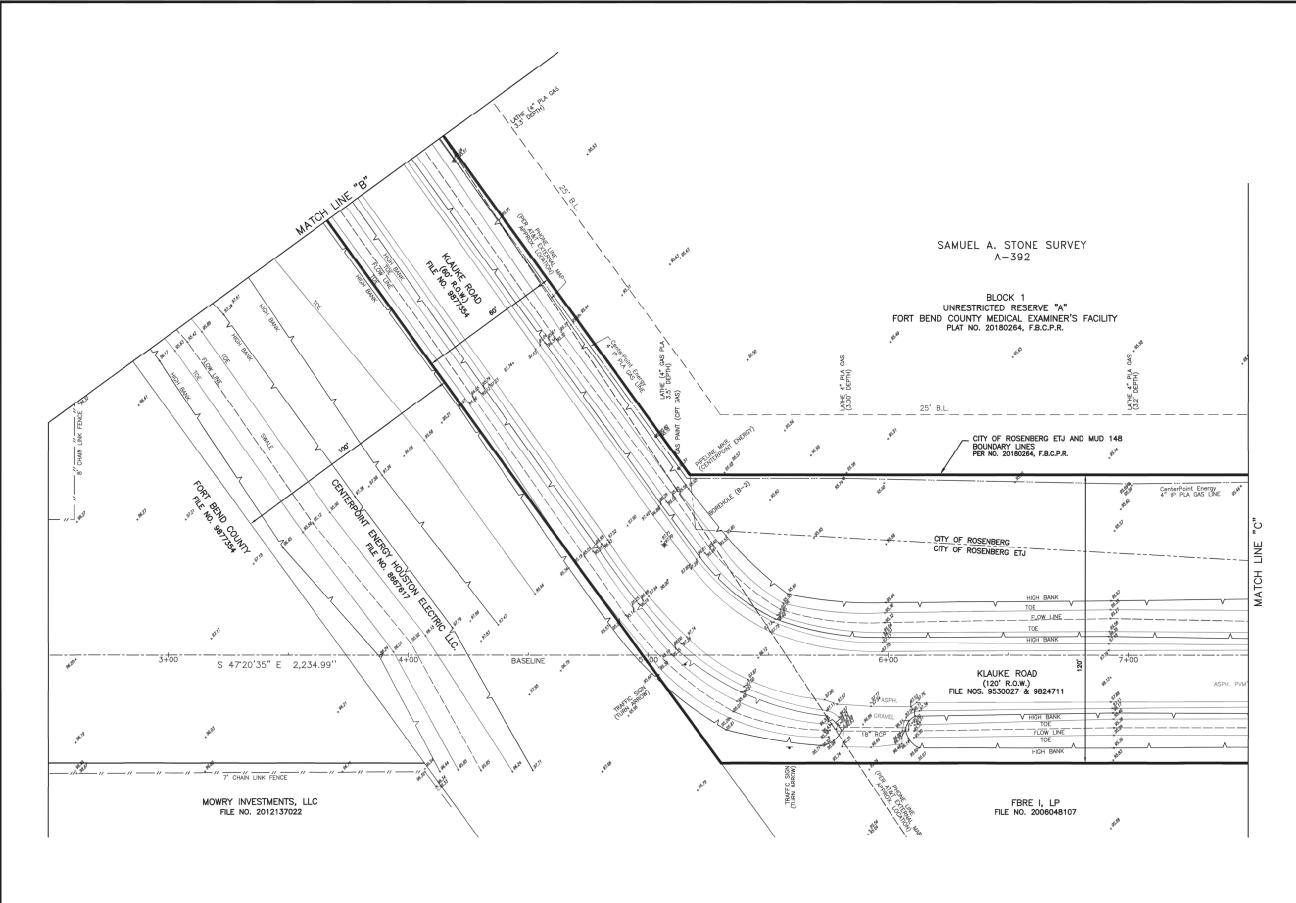
ENGINEERING DEPARTMENT



2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TITL		
FROM	CIVIL STANDARD	
SHEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 11 / 133









BENCHMARK: FORT BEND COUNTY MARKER NO. 251
BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 90.26 FEET

LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
VOL. = OLUME
PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT



07/26/2022

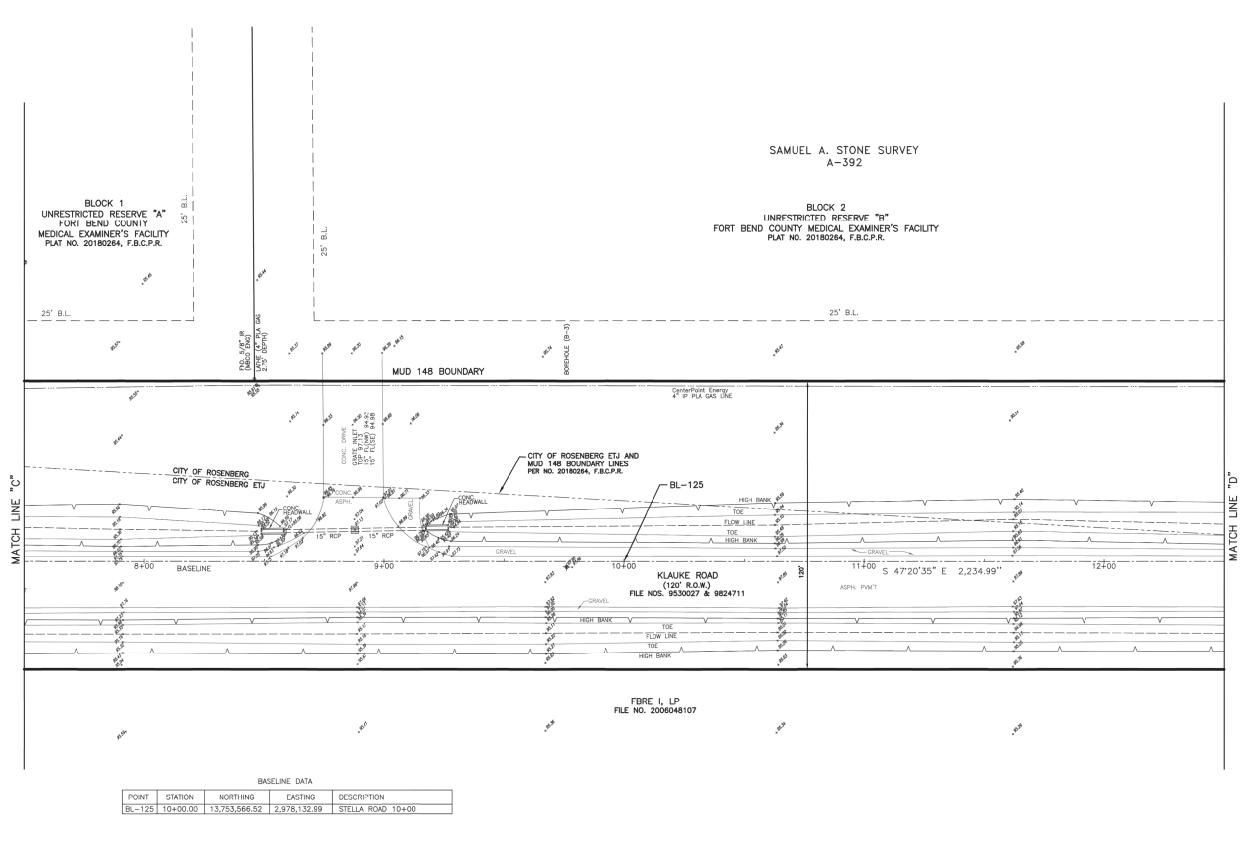
ı	NO.	REVISIONS	DATE	NAME
ı	1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
ı	2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
ı	3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
ı	4	UPDATED SHEET NUMBERS	07/26/22	YL
ı				

FORT BEND COUNTY ENGINEERING DEPARTMENT



ANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TITL	OJECT TITLE: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 13 / 133





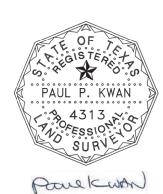


- I. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968026396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- . ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEE.

LEGEND

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IP = IRON PIPE
U.E. = UTILITY EASEMENT
VOL. = OLUME
PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT



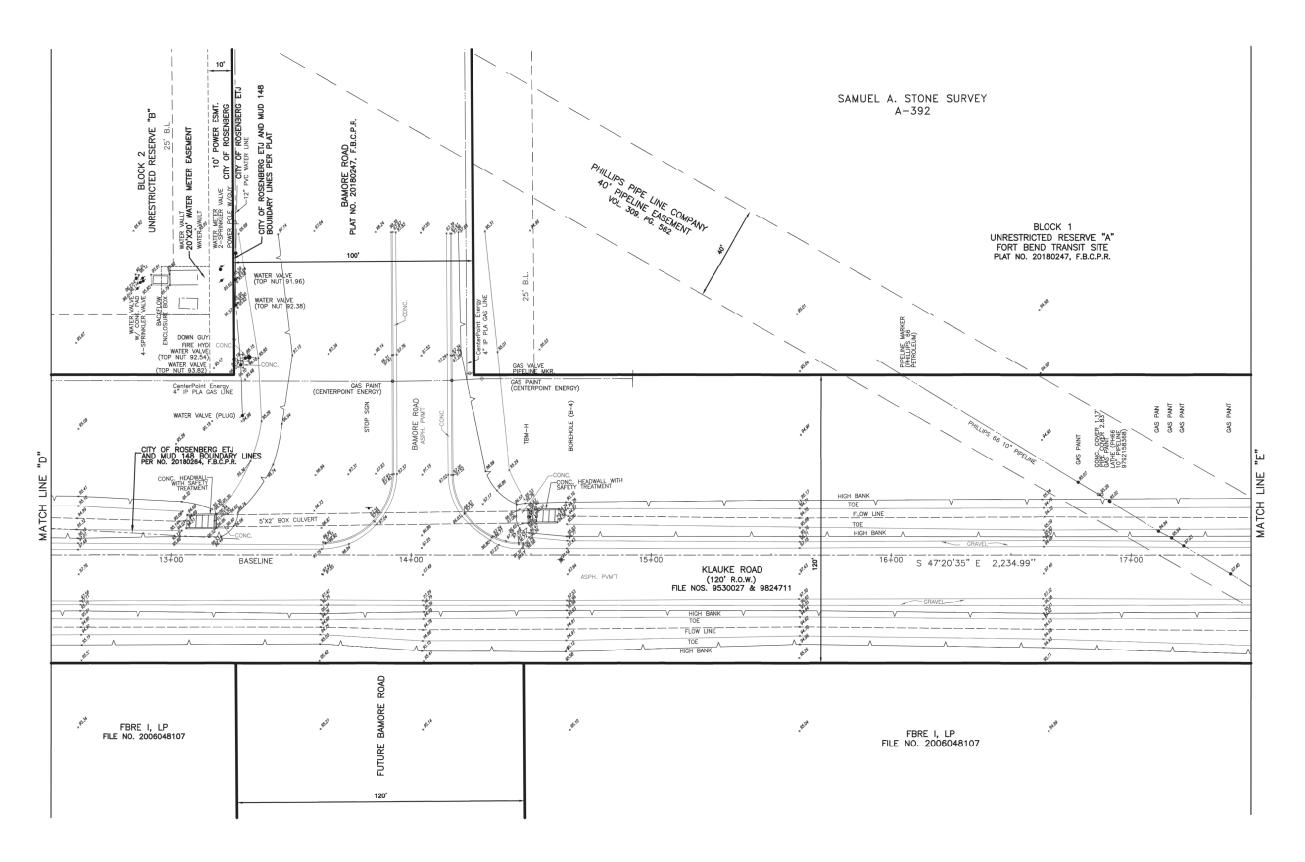
07/26/2022

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL
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FORT BEND COUNTY ENGINEERING DEPARTMENT



PROJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESCR	IPTION:	
DRAWN BY:	DATE:	
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 14 / 133







1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO ORID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396. UNIT OF MEASURE IS U.S. SURVEY FOOT.

BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEET

LEGEND

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PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT



07/26/2022

ı	NO.	REVISIONS	DATE	NAME
ı	1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
ı	2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
ı	3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
ı	4	UPDATED SHEET NUMBERS	07/26/22	YL
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FORT BEND COUNTY ENGINEERING DEPARTMENT



LANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	AS SHOWN	SHEET NO: 15 / 133

BASELINE DATA

POINT	STATION	NORTHING	EASTING	DESCRIPTION
BL-126	20+00.00	13.752.888.92	2.978.868.41	STELLA ROAD 20+00

POST

SAMUEL A. STONE SURVEY A-392





S 47*20'35" E 2,234.99"

HIGH BANK

GATE GATE GATE O 2" OAK ○ 2" OAK ⊙ 2" OAK O 2" OAK O 2" OAK ○ 2" OAK

_BL-126 FLOW LINE

BASELINE 19+00 KLAUKE ROAD (120' R.O.W.) FILE NOS. 9530027 & 9824711

FBRE I, LP FILE NO. 2006048107



- I. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NABAS (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
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BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 90.26 FEET

LEGEND



Pour Kurn 07/26/2022

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
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3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL

⊙ 2" OAK

GAS PAINT

⊙ 2" OAK

FORT BEND COUNTY ENGINEERING DEPARTMENT



LANDTECH
2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131
TBPELS Registration No. 10019100

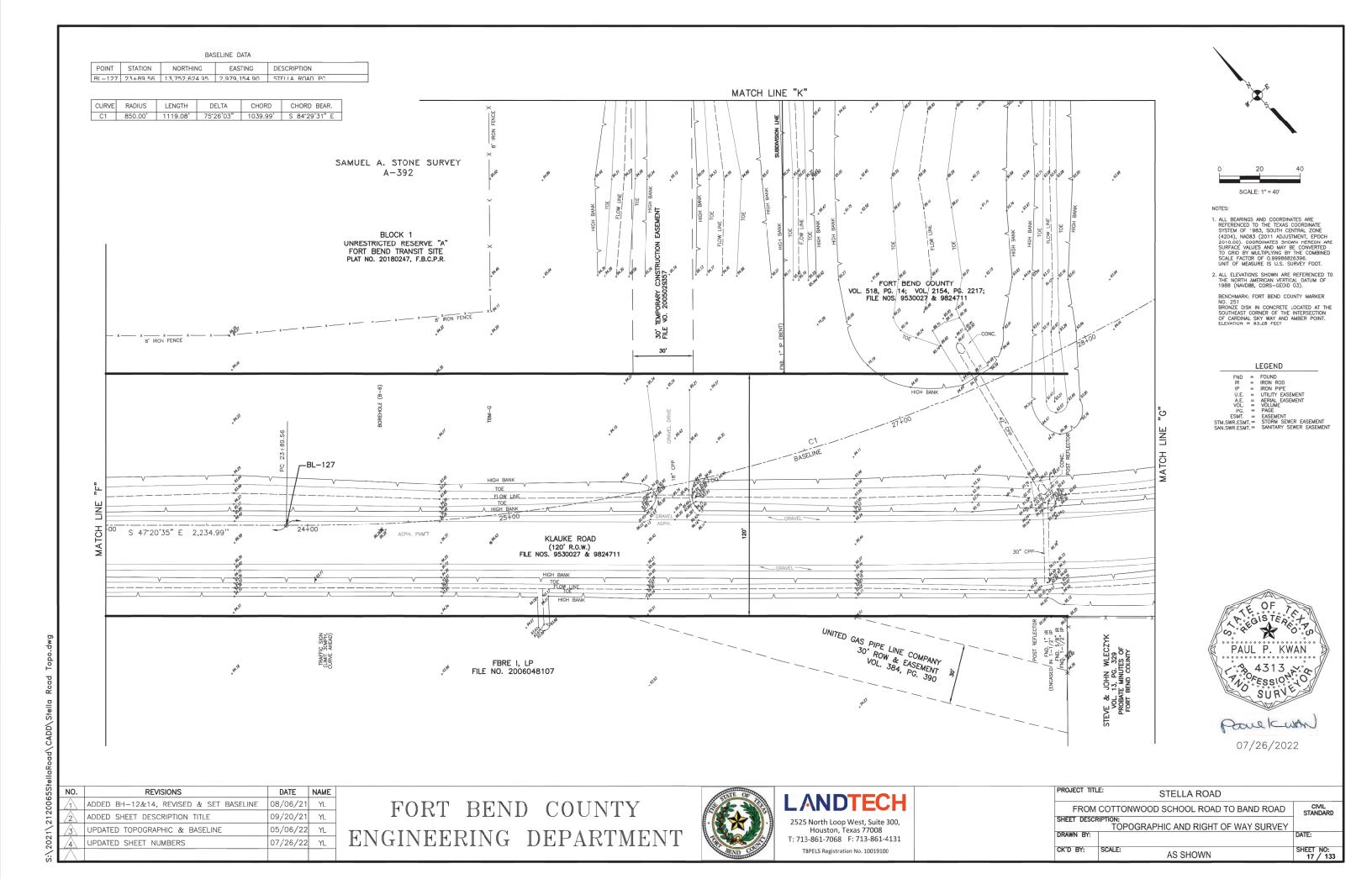
GATE

O 2" OAK

22+00

O 2" OAK

PROJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESCR	IPTION:	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 16 / 133



MATCH LINE "K&L" SAMUEL A. STONE SURVEY A-392 FORT BEND COUNTY VOL. 518, PG. 14; VOL. 2154, PG. 2217; FILE NOS. 9530027, 9824711 & 9877354 TRAFFIC LIMIT 30 FORT BEND COUNTY VOL. 518, PG. 14; VOL. 2154, PG. 2217; FILE NOS. 9530027, 9824711 & 9877354 STEVE & JOHN WLECZYK VOL. 13, PG. 329 PROBATE MINUTES OF FORT BEND COUNTY
 CURVE
 RADIUS
 LENGTH
 DELTA
 CHORD
 CHORD BEAR.

 C1
 850.00'
 1119.08'
 75'26'03"
 1039.99'
 S 84'29'31"
 E



- ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
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BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONGRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 93.26 FEET

LEGEND

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IP = IRON PIPE
U.E. = UTILITY EASEMENT
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VOL. = VOLUME
P.G. = PAGE
SEMT. = EASEMENT
STANSWR.ESMT. = SANITARY SEWER EASEMENT
SAN.SWR.ESMT. = SANITARY SEWER EASEMENT

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07/26/2022

DATE NAME 08/06/21 09/20/21 05/06/22 YL 07/26/22 YL

NO.

REVISIONS

ADDED SHEET DESCRIPTION TITLE

UPDATED SHEET NUMBERS

UPDATED TOPOGRAPHIC & BASELINE

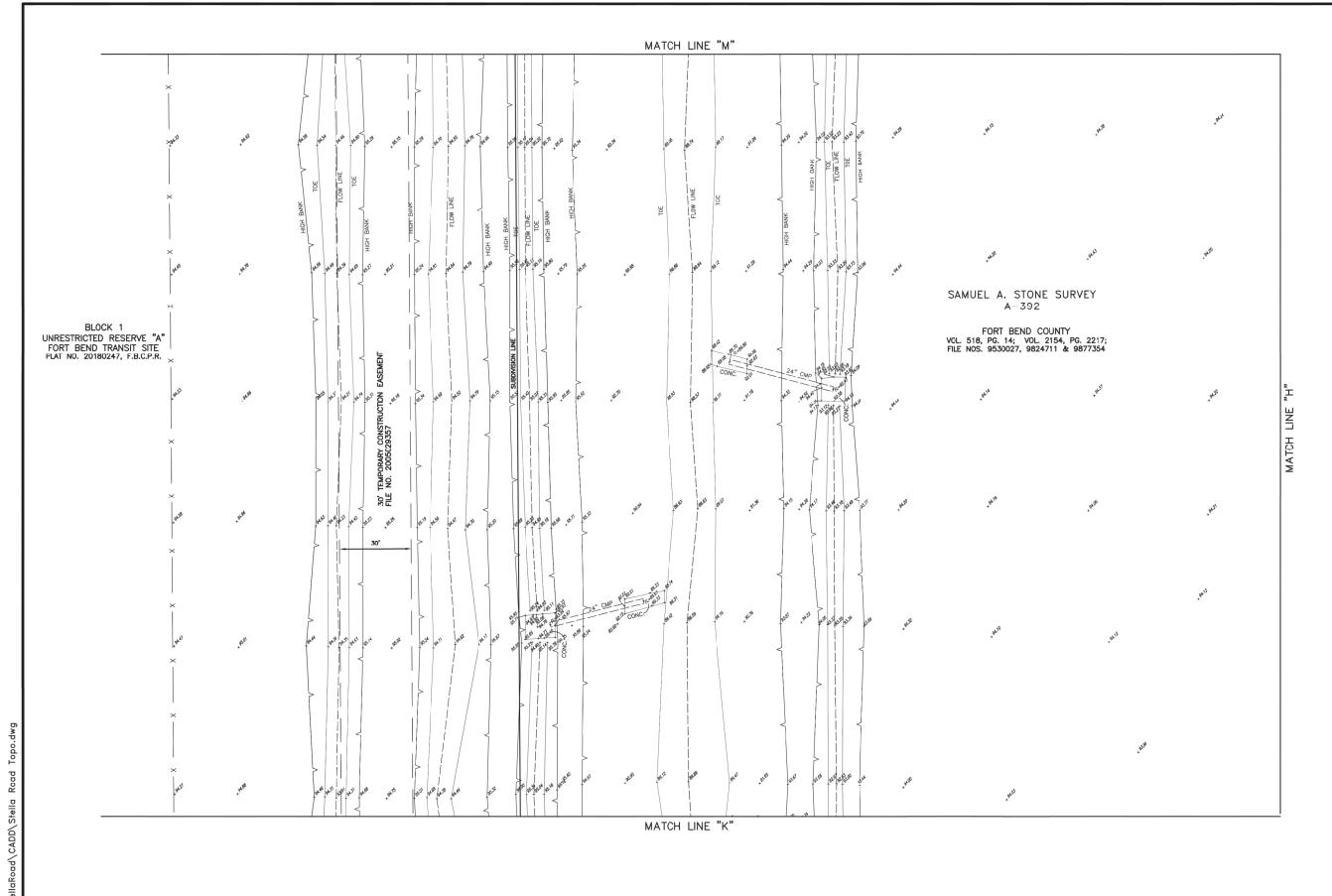
ADDED BH-12&14, REVISED & SET BASELINE

FORT BEND COUNTY ENGINEERING DEPARTMENT



LANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 18 / 133







- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1933, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00), COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO ORID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396, UNIT OF MEASURE IS U.S. SURVEY FOOT.
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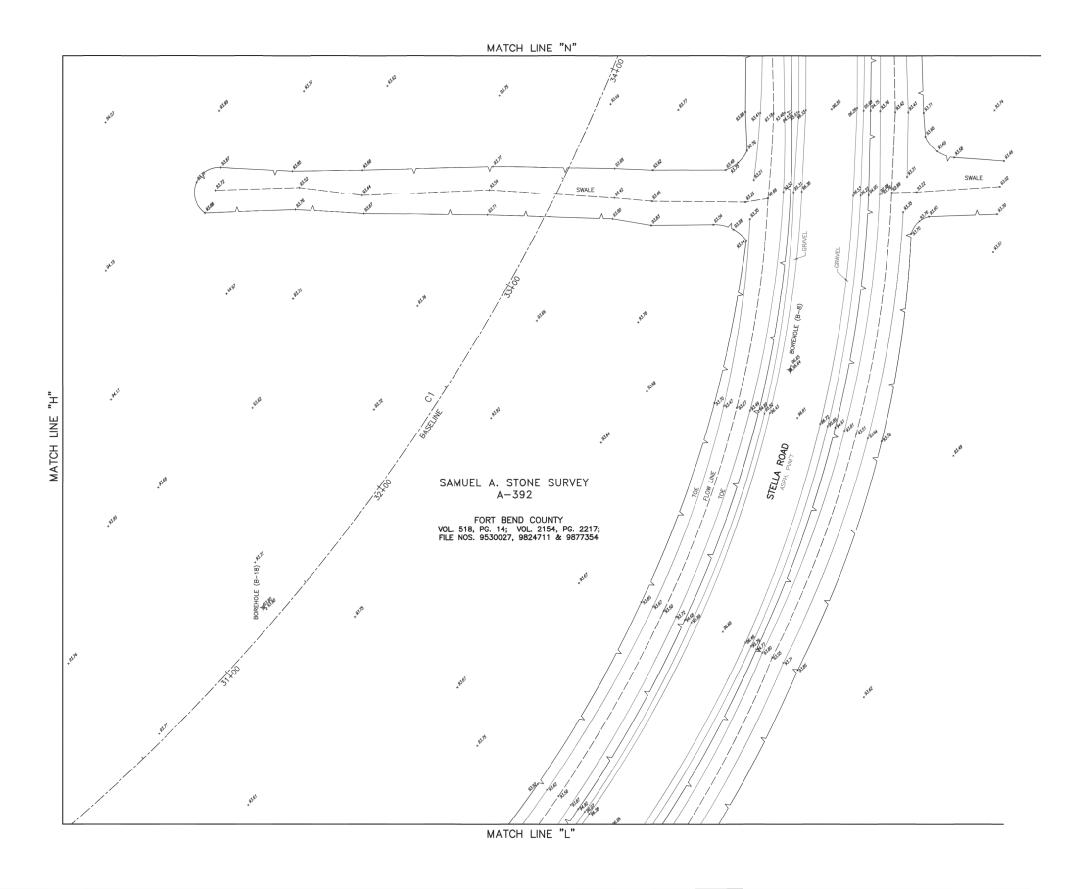


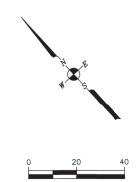
07/26/2022

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL



PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 19 / 133





LEGEND

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PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT

CURVE	RADIUS	LENGTH	DELTA	CHORD	CHORD BEAR.
C1	850.00'	1119.08'	75*26'03"	1039.99'	S 84°29'31" E

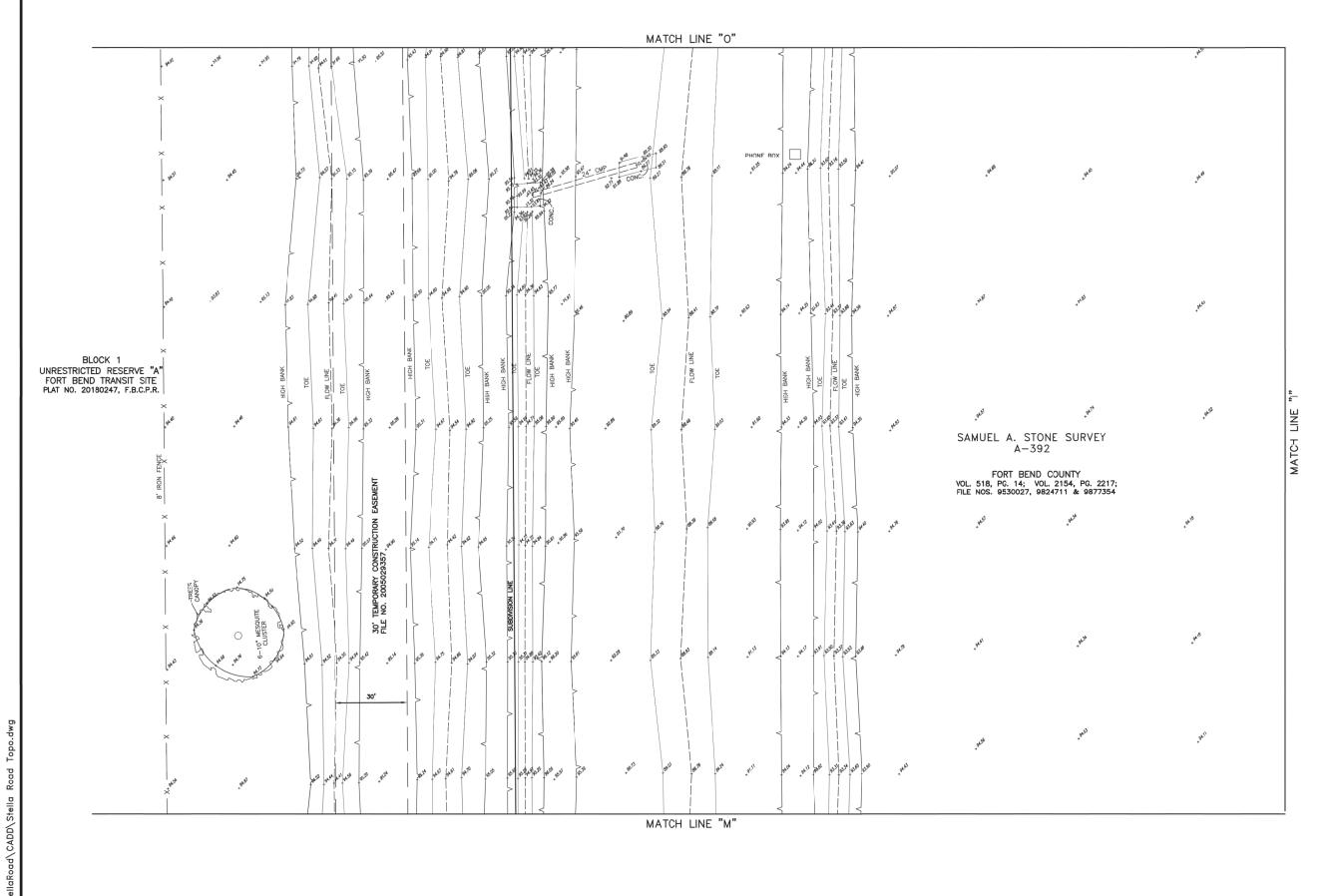


NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL
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A	LANDTECH
	2525 North Loop West, Suite 300, Houston, Texas 77008
7	T: 713-861-7068 F: 713-861-4131
	TBPELS Registration No. 10019100

PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 20 / 133







- ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
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- BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONGRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 93.26 FEET

LEGEND

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SANISWR.ESMT. = SANITARY SEWER EASEMENT



07/26/2022

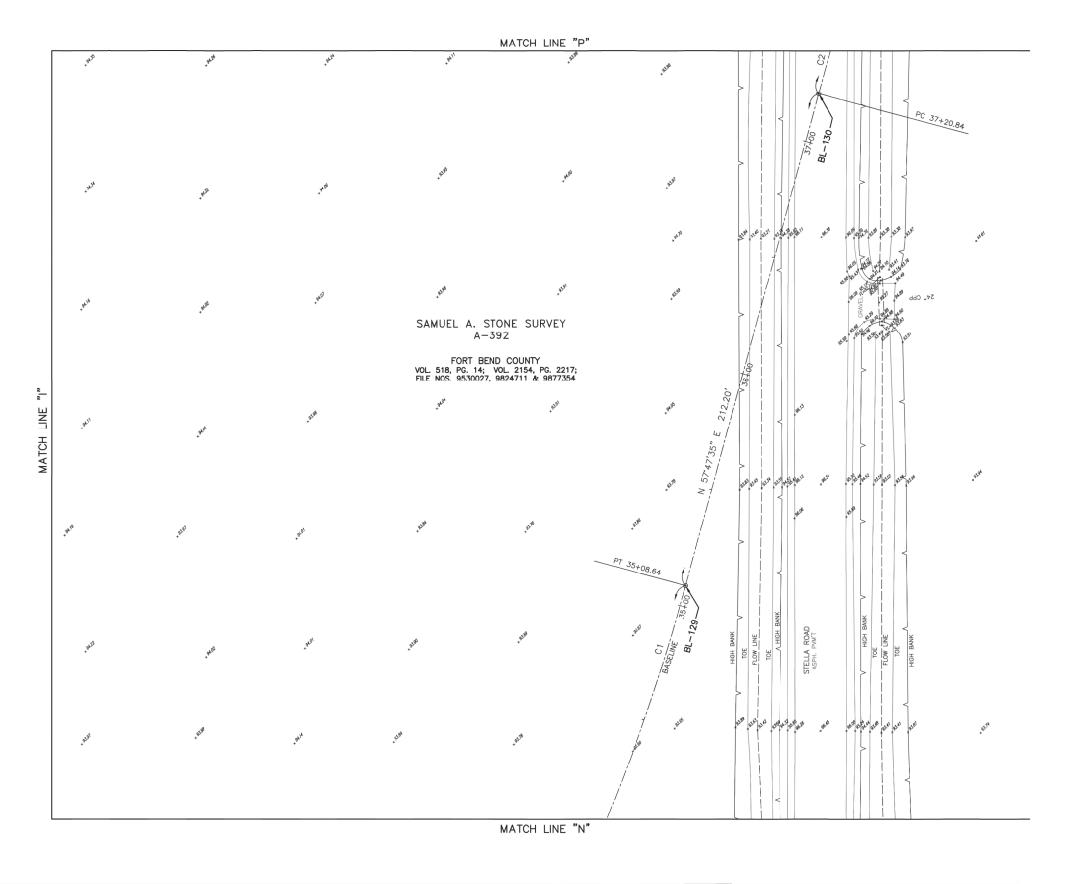
ı	NO.	REVISIONS	DATE	NAME
ı	1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
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ı	4	UPDATED SHEET NUMBERS	07/26/22	YL
ı				

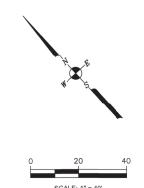
FORT BEND COUNTY ENGINEERING DEPARTMENT



LANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TITL	E: STELLA ROAD				
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD			
SHEET DESCR	SHEET DESCRIPTION:				
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY				
DRAWN BY:		DATE:			
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 21 / 133			





- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1933, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00), COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO ORID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396, UNIT OF MEASURE IS U.S. SURVEY FOOT.
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ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT

CURVE	RADIUS	LENGTH	DELTA	CHORD	CHORD BEAR.
C1	850.00'	1119.08'	75*26'03"	1039.99'	S 84°29'31" E
C2	850.00'	1115.61	75°12'00"	1037.25	S 84'50'34" E

BASELINE DATA

POINT	STATION	NORTHING	EASTING	DESCRIPTION
BL-129	35+08.64	13,752,525.13	2,980,190.09	STELLA ROAD PT
BL-130	37+20.84	13,752,638.23	2,980,369.64	STELLA ROAD PC

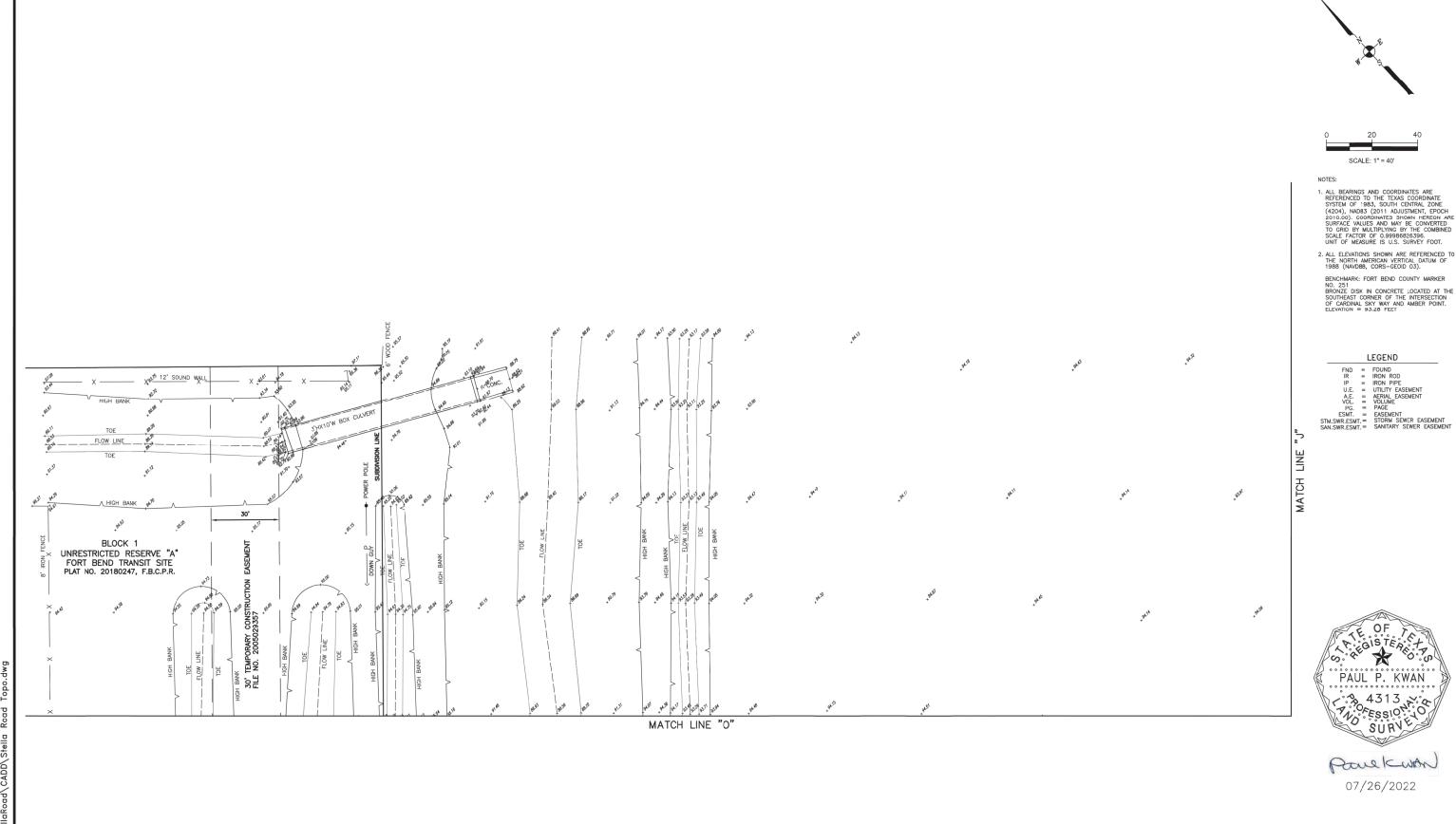


07/26/2022

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ı	\wedge			



PROJECT TITL	E: STELLA ROAD	
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION:	
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 22 / 133



FORT BEND COUNTY
ENGINEERING DEPARTMENT

NO.

REVISIONS

ADDED SHEET DESCRIPTION TITLE

UPDATED SHEET NUMBERS

UPDATED TOPOGRAPHIC & BASELINE

ADDED BH-12&14, REVISED & SET BASELINE

DATE NAME

08/06/21

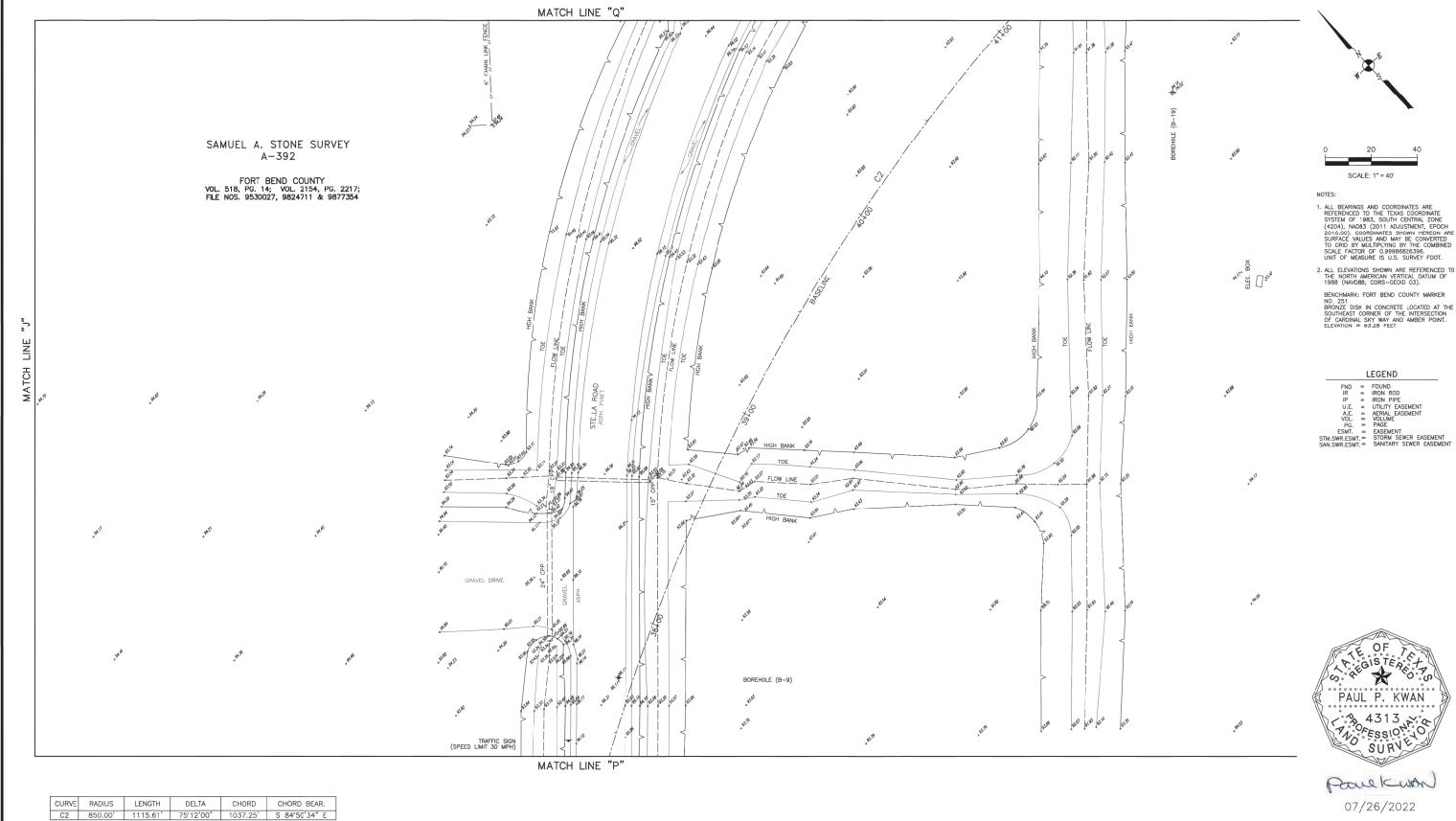
09/20/21

05/06/22

07/26/22 YL



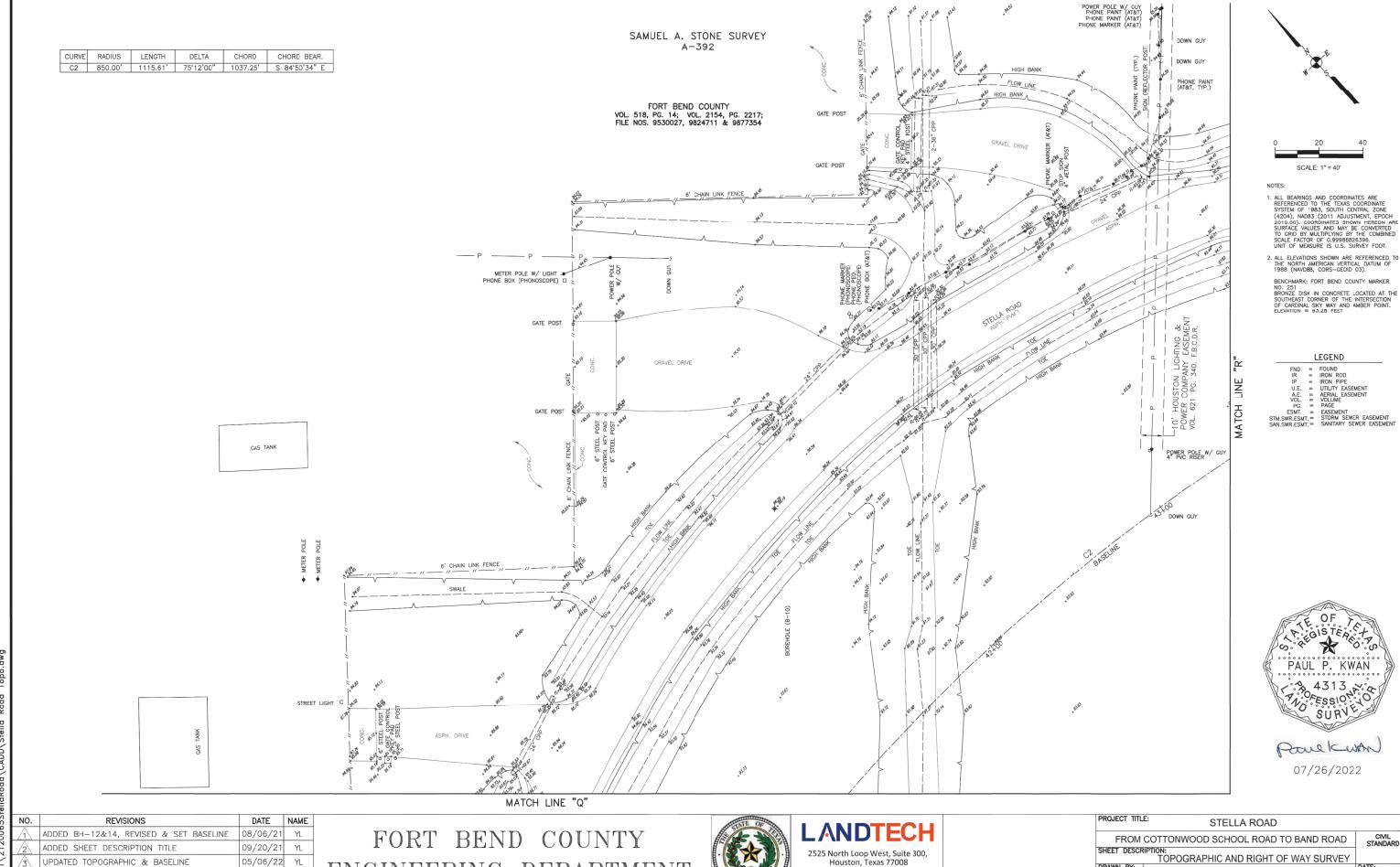
OJECT TITLE: STELLA ROAD	
	CIVIL ANDARD
IEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
PAWN BY: DATE:	
(TD BY: SCALE: AS SHOWN SHEE 2:	T NO: 3 / 133



NO.	REVISIONS	DATE	NAME	
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL	
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL	
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL	
4	UPDATED SHEET NUMBERS	07/26/22	YL	



OJECT TITLE: STELLA ROAD		
FROM	CIVIL STANDARD	
IEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
RAWN BY:		DATE:
(D BY:	SCALE: AS SHOWN	SHEET NO: 24 / 133



UPDATED SHEET NUMBERS

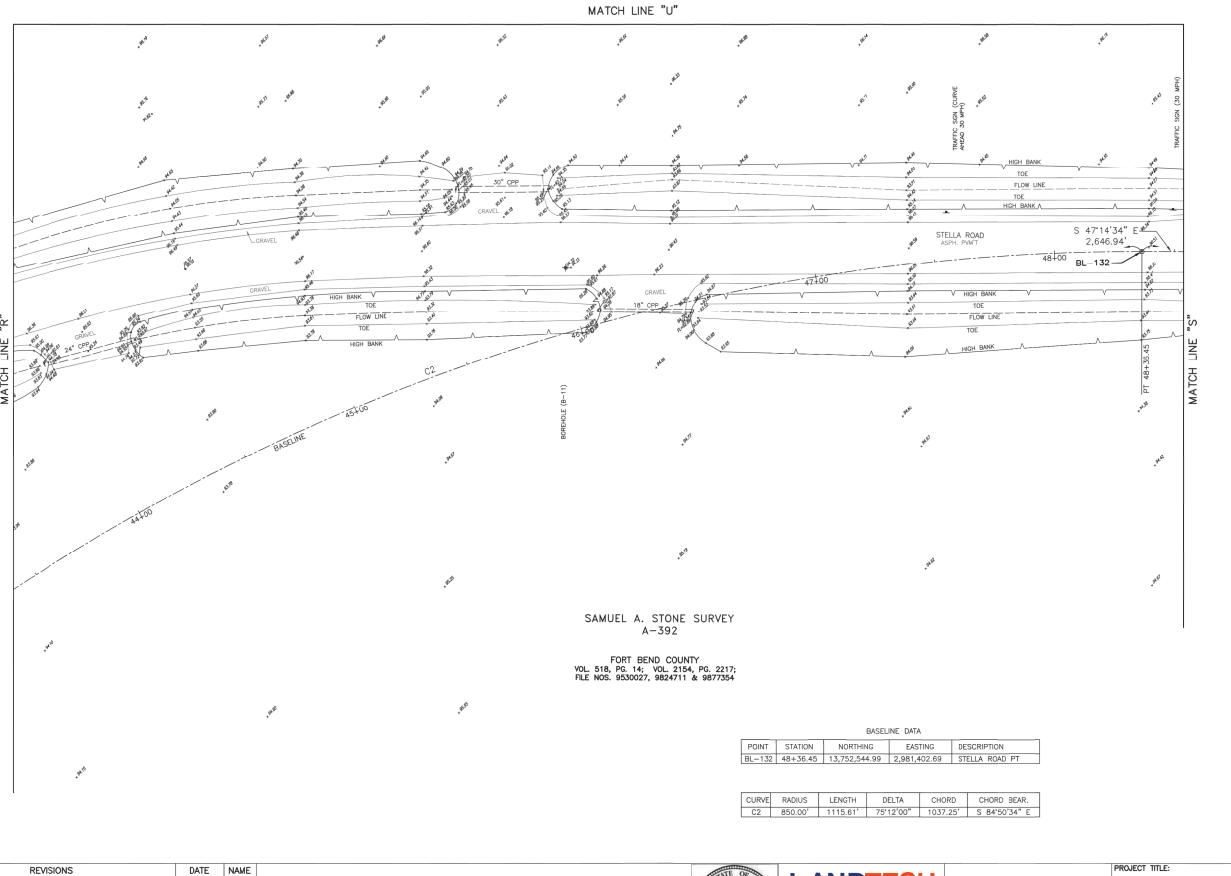
ENGINEERING DEPARTMENT

07/26/22 YL



LANDTECH
2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131
TBPELS Registration No. 10019100

PROJECT TITLE: STELLA ROAD		
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
HEET DESCR	IPTION:	
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
RAWN BY:		DATE:
K'D BY:	SCALE: AS SHOWN	SHEET NO: 25 / 133



I. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968026396. UNIT OF MEASURE IS U.S. SURVEY FOOT. ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03). BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONGRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 93.26 FEET LEGEND FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
VOL. = OLUME
P.G. = PAGE
ESMT. = SEMENT
SANLSWR.ESMT. = SANLSWR.ESWE EASEMENT
SANLSWR.ESMT. = SANLTARY SEWER EASEMENT

Pour Kush

07/26/2022

NO. 08/06/21 YL ADDED BH-12&14, REVISED & SET BASELINE ADDED SHEET DESCRIPTION TITLE 09/20/21 05/06/22 YL UPDATED TOPOGRAPHIC & BASELINE 07/26/22 YL UPDATED SHEET NUMBERS

FORT BEND COUNTY ENGINEERING DEPARTMENT



ANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

ROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
HEET DESCR	IPTION:	
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
RAWN BY:		DATE:
K'D BY:	SCALE: AS SHOWN	SHEET NO: 26 / 133

FORT BEND COUNTY VOL. 518, PG. 14; VOL. 2154, PG. 2217; FILE NOS. 9530027, 9824711 & 9877354

POWER POLE W/ LIGHT . \$559

POWER POLE W/ LIGHT

Pour Kurn

1. ALL BEARINGS AND COORDINATES ARE
REFERENCED TO THE TEXAS COORDINATE
SYSTEM OF 1933, SOUTH CENTRAL ZONE
(4204), NAD83 (2011 ADUSTMENT, EPOCH
2010.00). COORDINATES SHOWN HEREON ARE
SURFACE VALUES AND MAY BE CONVERTED
TO GRID BY MULTIPLYING BY THE COMBINED
SCALE FACTOR OF 0.99986826396.
UNIT OF MEASURE IS U.S. SURVEY FOOT.

ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS—GEOID 03).

BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 90.26 FEET

LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
VOL. = OLUME
PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT

07/26/2022

MATCH LINE "U"

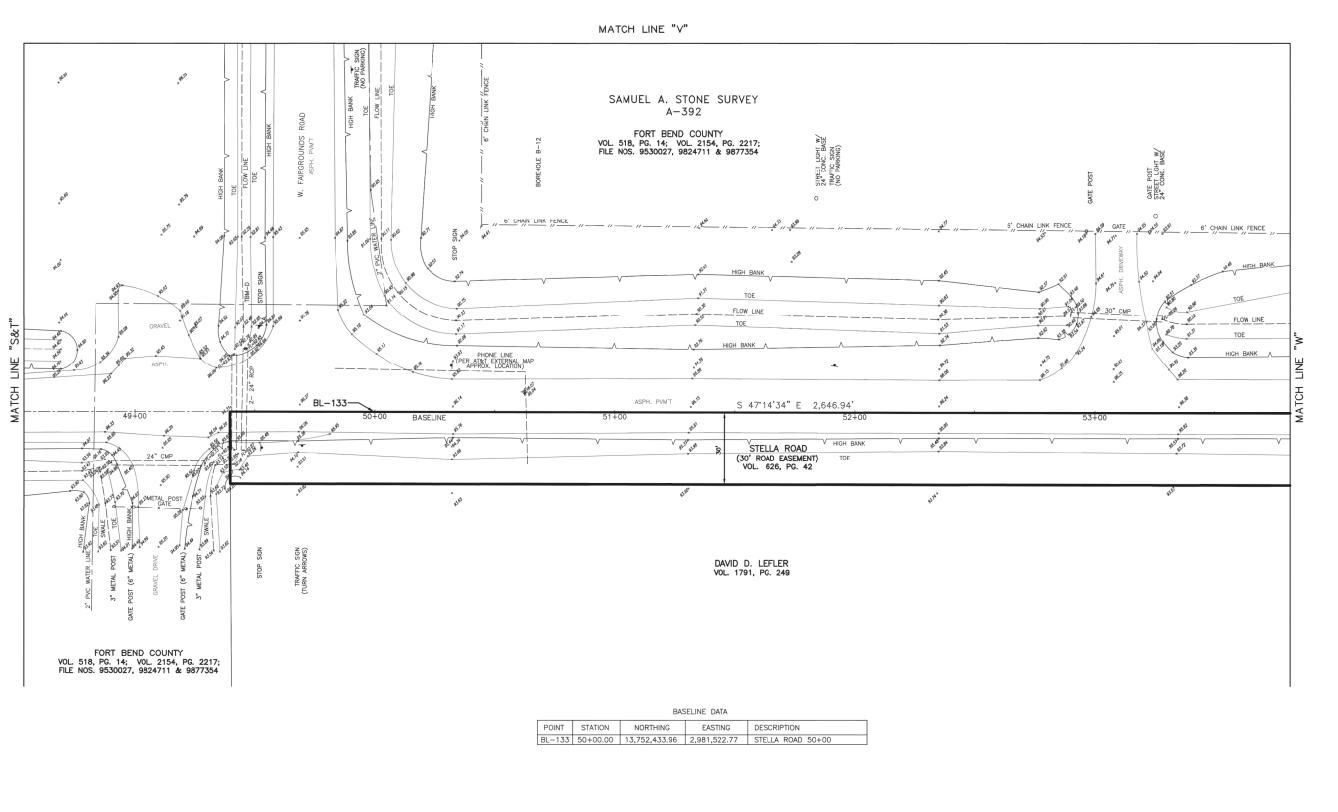
NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL
\wedge			

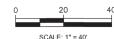
FORT BEND COUNTY ENGINEERING DEPARTMENT



2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

ROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
HEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
RAWN BY:		DATE:
K'D BY:	SCALE: AS SHOWN	SHEET NO: 27 / 133





- I. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HERCON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968026396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS—GEOID 03).

LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
A.E. = AERIAL EASEMENT
VOL. = VOLUME
P.C. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT



07/26/2022

FORT BEND COUNTY ENGINEERING DEPARTMENT

NO.

REVISIONS

ADDED SHEET DESCRIPTION TITLE

UPDATED SHEET NUMBERS

UPDATED TOPOGRAPHIC & BASELINE

ADDED BH-12&14, REVISED & SET BASELINE

DATE NAME

08/06/21

09/20/21

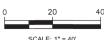
05/06/22 YL

07/26/22 YL



PROJECT TITL	E: STELLA ROAD	
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 28 / 133





- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
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OL. = VOLUME
PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT

SAMUEL A. STONE SURVEY A-392

FORT BEND COUNTY VOL. 518, PG. 14; VOL. 2154, PG. 2217; FILE NOS. 9530027, 9824711 & 9877354



07/26/2022

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL
\wedge			

FORT BEND COUNTY ENGINEERING DEPARTMENT

MATCH LINE "V"

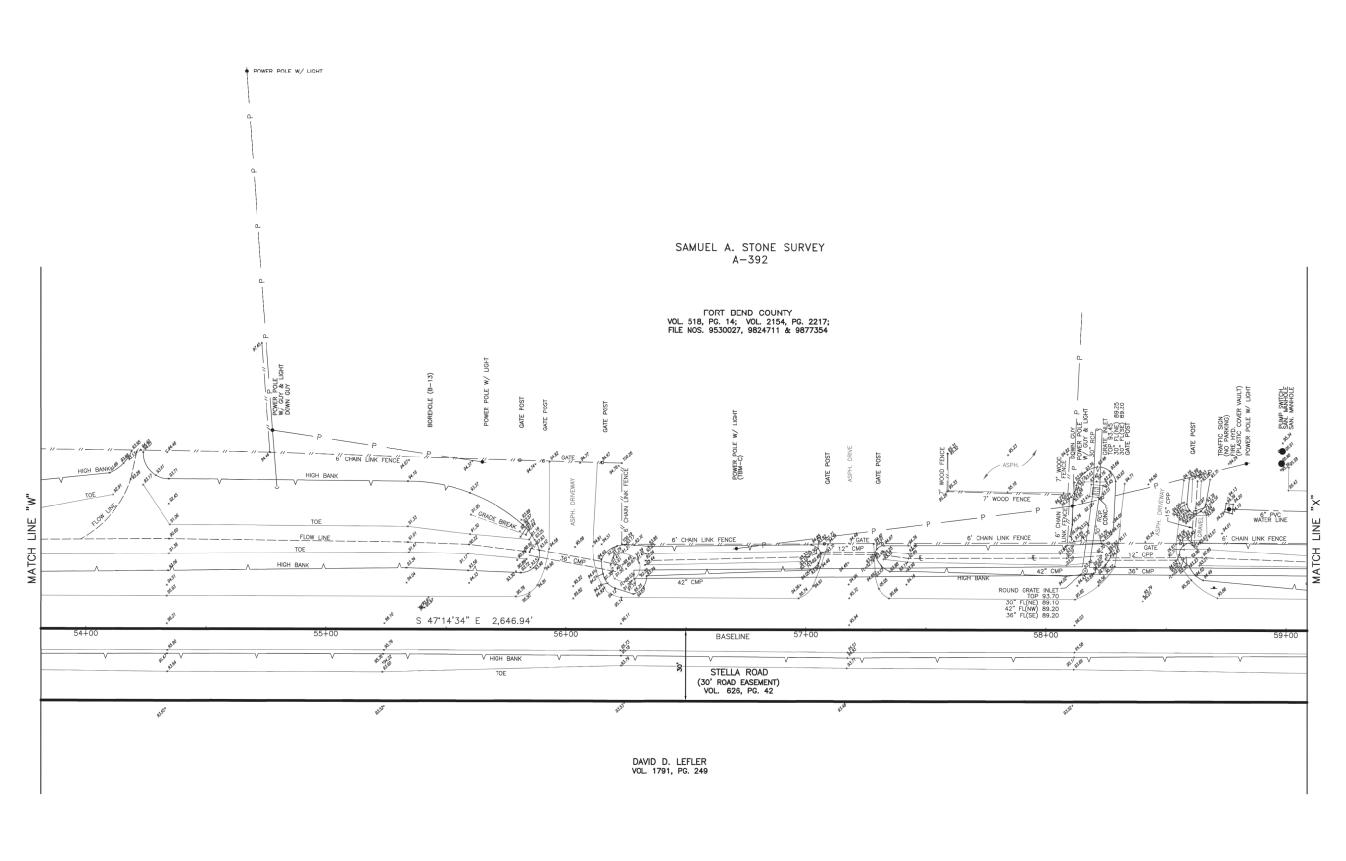
+ TRAFFIC SIGN (STOP SIGN AHEAD)

(NO PARKING ROAD AHEAD) NS.

SIGN MPH)



PROJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESCR		
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 29 / 133







LEGEND

FND = FOUND
IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
VOL. = OLUME
PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT

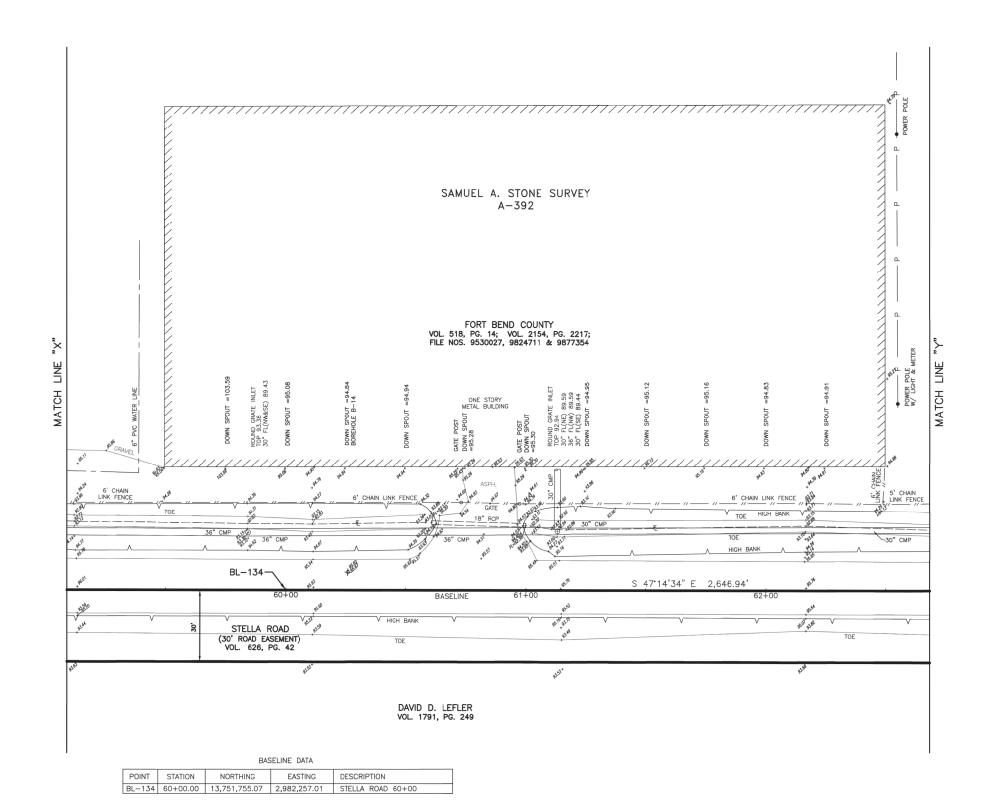
Pour Kurn

07/26/2022

1	NO.	REVISIONS	DATE	NAME
	1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
	2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
1	3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
	4	UPDATED SHEET NUMBERS	07/26/22	YL
1	\wedge			



ROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
HEET DESCR	IPTION:	
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
RAWN BY:		DATE:
K'D BY:	SCALE: AS SHOWN	SHEET NO: 30 / 133







- ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), ANDB3 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
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LEGEND

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P.G. = PAGE
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STM.SWR.ESMT. = SANTARY SEWER EASEMENT
SAN.SWR.ESMT. = SANTARY SEWER EASEMENT

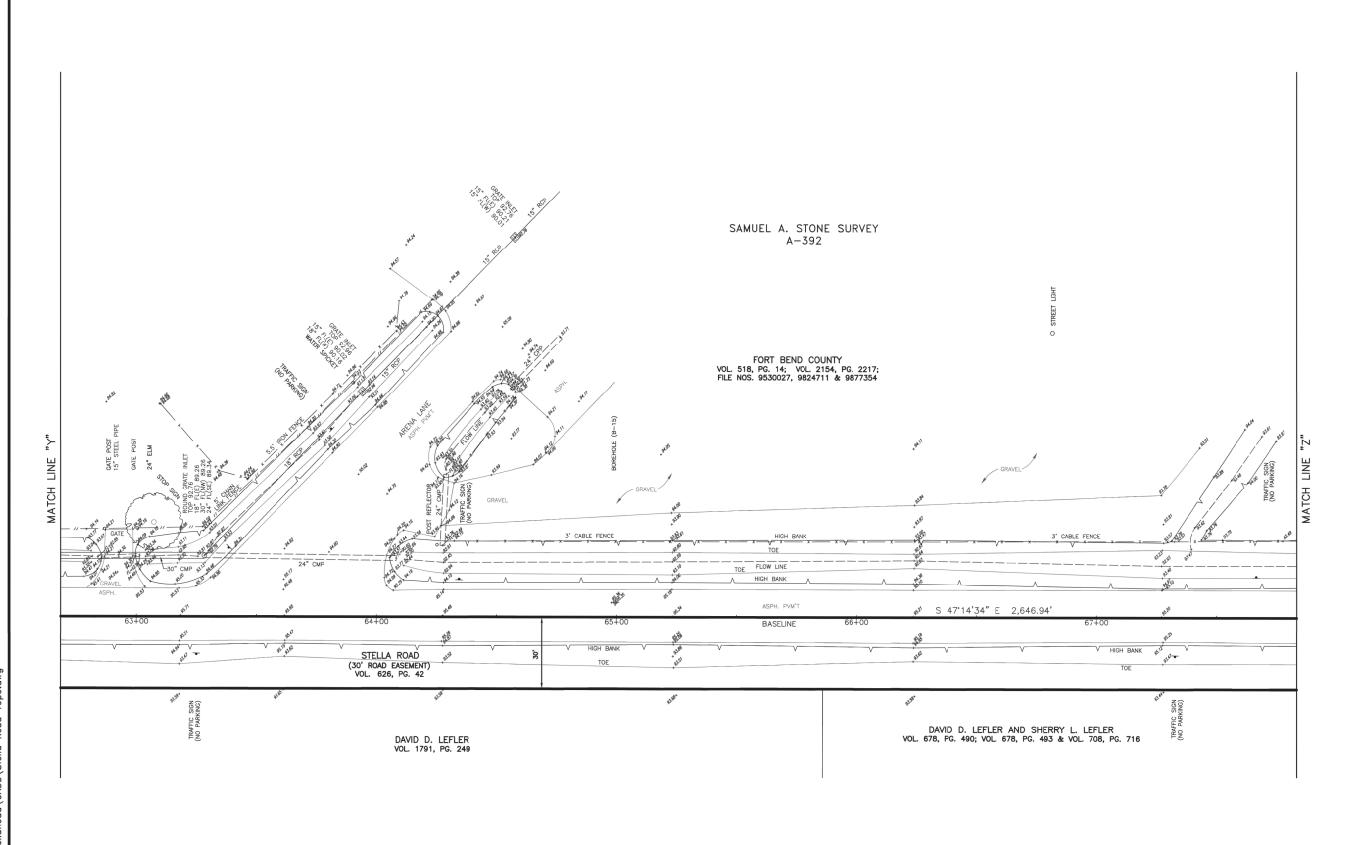
4313 X 0 710 Ession O SURNE Pour Kurn

07/26/2022

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL
\wedge			



ROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
HEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
RAWN BY:		DATE:
C'D BY:	AS SHOWN	SHEET NO: 31 / 133







- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1933, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADJUSTMENT, EPOCH 2010.00), COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO ORID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396, UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS—GEOID 03).

LEGEND



07/26/2022

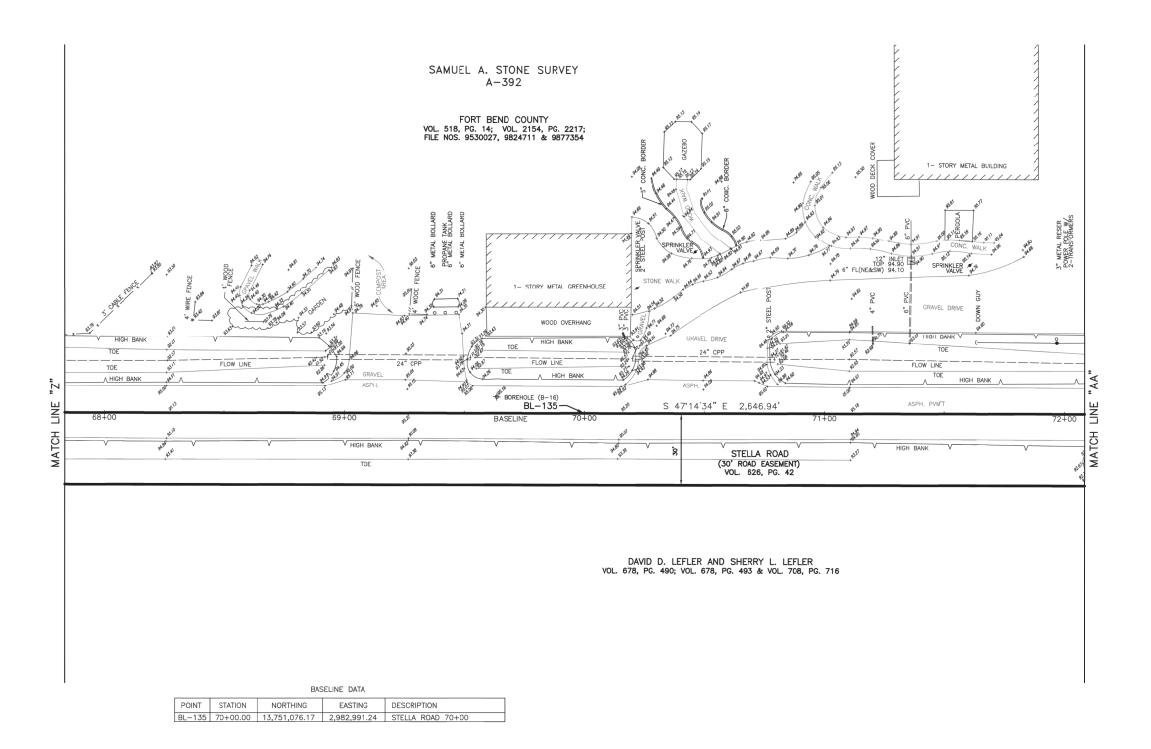
ı	NO.	REVISIONS		NAME
ı	1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
	2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
ı	3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
ı	4	UPDATED SHEET NUMBERS	07/26/22	YL
1	$\overline{}$			

FORT BEND COUNTY ENGINEERING DEPARTMENT



LANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TITL	E: STELLA ROAD	
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 32 / 133







- I. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NABAS (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626396. UNIT OF MEASURE IS U.S. SURVEY FOOT.

LEGEND

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IR = IRON ROD
IP = IRON PIPE
U.E. = UTILITY EASEMENT
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OL. = VOLUME
PG. = PAGE
ESMT. = EASEMENT
STM.SWR.ESMT. = SANITARY SEWER EASEMENT
SANISWR.ESMT. = SANITARY SEWER EASEMENT

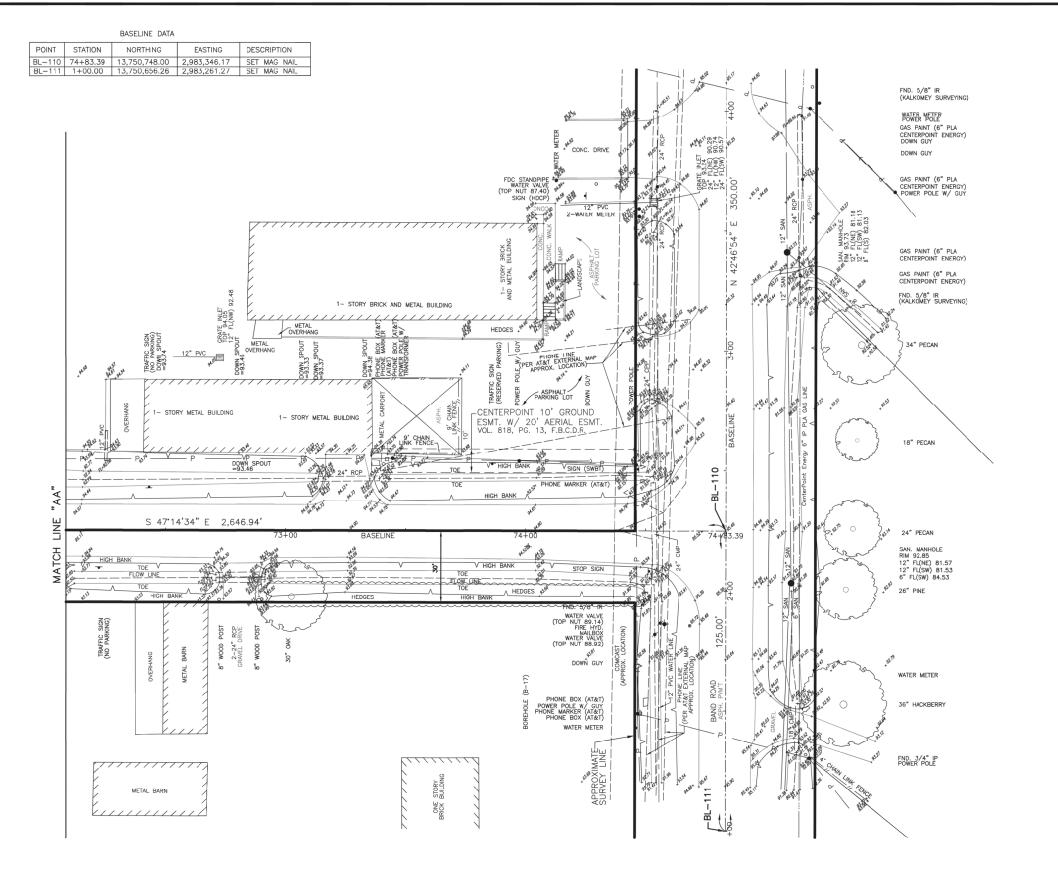
Paul Kwan)

07/26/2022

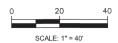
NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL



DJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
ET DESCR		
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
WN BY:		DATE:
D BY:	SCALE: AS SHOWN	SHEET NO: 33 / 133







1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 ADJUSTMENT, EPOCH 2010.00), COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396.
UNIT OF MESURE IS U.S. SURVEY FOOT.

BENCHMARK: FORT BEND COUNTY MARKER NO. 251
BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 93.20 FEET

LEGEND

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U.E. = UTILITY EASEMENT
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VOL. = VOLUME
P.G. = PAGE
ESMT. = EASEMENT
STIM.SWR.ESMT. = SANTARY SEWER EASEMENT
SAN.SWR.ESMT. = SANTARY SEWER EASEMENT

4313 X 6 Pour Kurn

07/26/2022

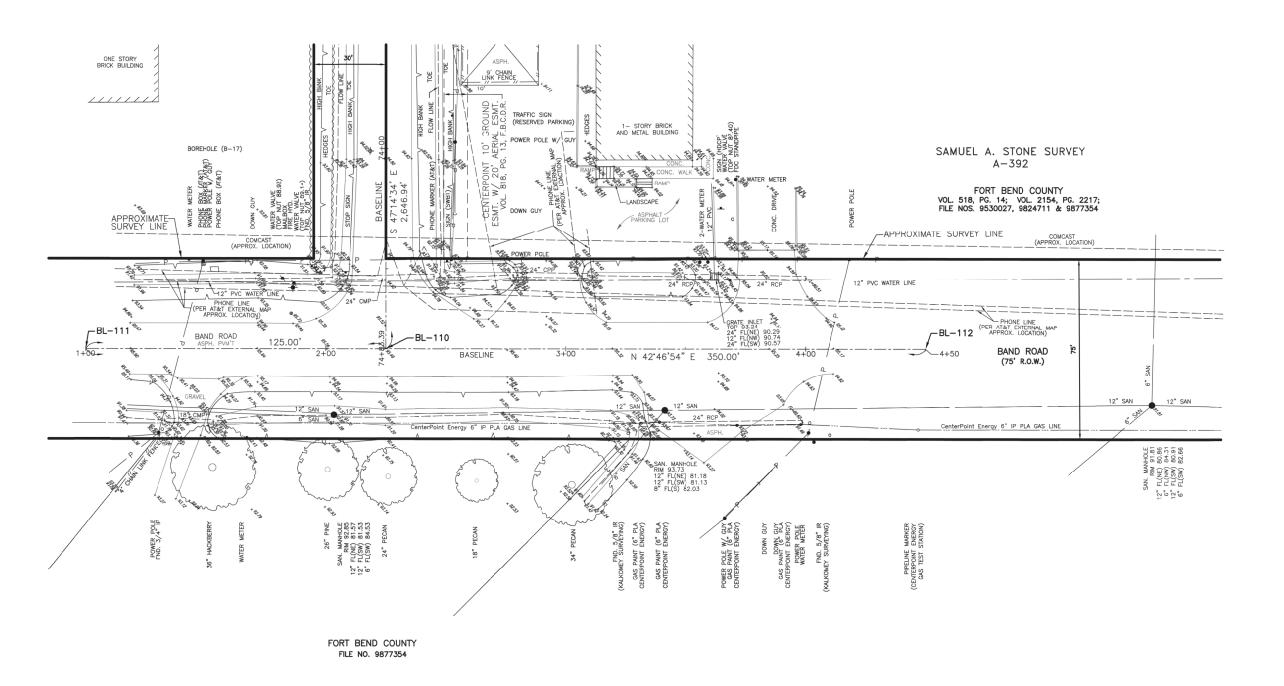
NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL

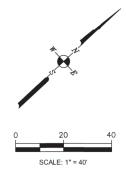
FORT BEND COUNTY ENGINEERING DEPARTMENT



ANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100

PROJECT TIT	E: STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESC		
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 34 / 133





ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 ADJUSTMENT, EPOCH 2010.00), COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396.
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BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 93.20 FEET

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VOL. = VOLUME
P.G. = PAGE
ESMT. = EASEMENT
STIM.SWR.ESMT. = SANTARY SEWER EASEMENT
SAN.SWR.ESMT. = SANTARY SEWER EASEMENT



07/26/2022

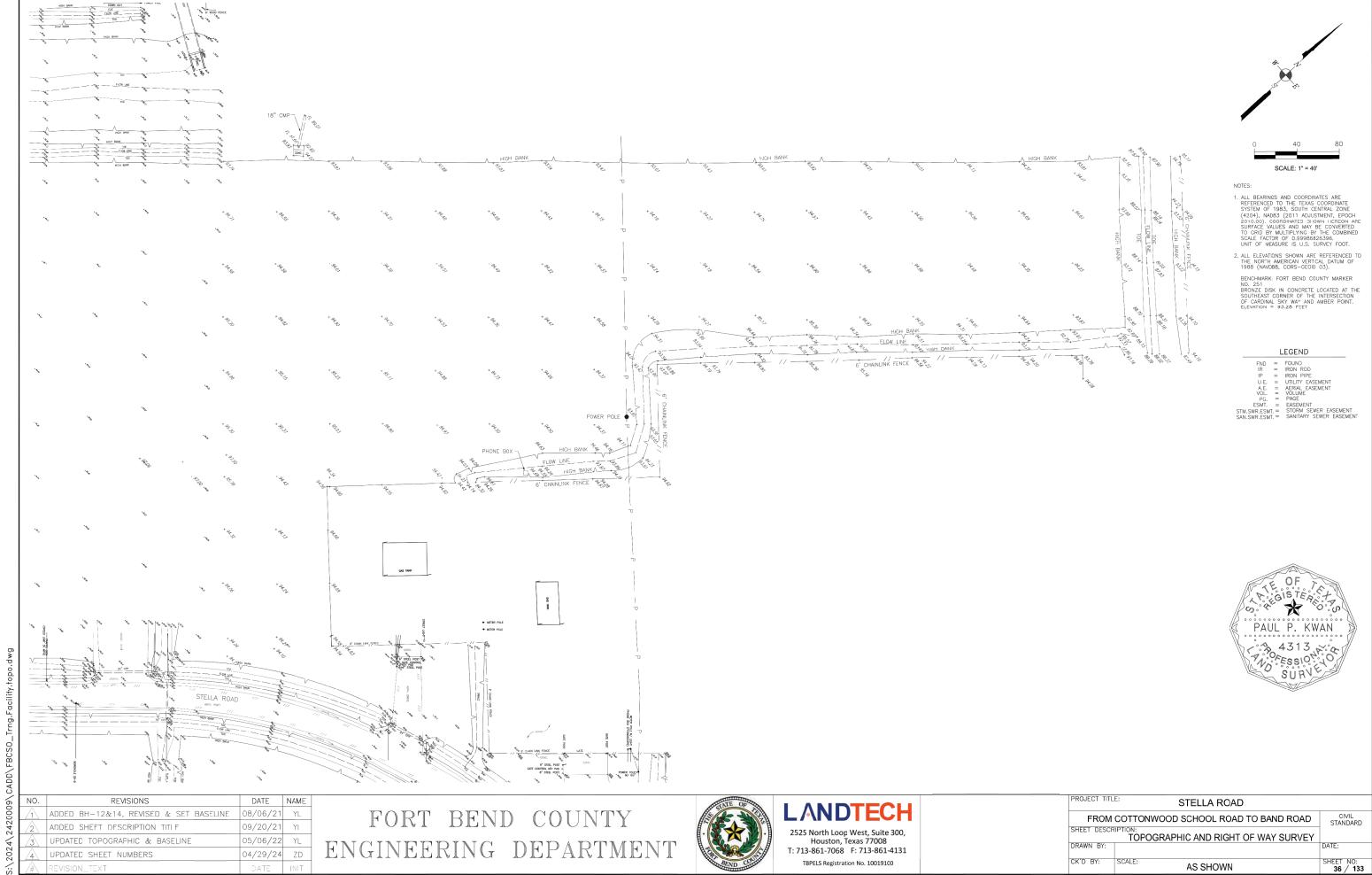
BASELINE DATA

POINT STAT	ION NORTHING	EASTING	DESCRIPTION
BL-110 74+8	3.39 13,750,748.0	00 2,983,346.17	7 SET MAG NAL
BL-111 1+00	0.00 13,750,656.2	26 2,983,261.27	7 SET MAG NAL
BL-112 4+50	0.00 13,750,913.1	14 2,983,498.99	SET MAG NAL

NO.	REVISIONS	DATE	NAME
1	ADDED BH-12&14, REVISED & SET BASELINE	08/06/21	YL
2	ADDED SHEET DESCRIPTION TITLE	09/20/21	YL
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22	YL
4	UPDATED SHEET NUMBERS	07/26/22	YL



PROJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 35 / 133



TBPELS Registration No. 10019100

SHEET NO: 36 / 133

AS SHOWN

Horizontal Alignment Report

Alignment Name: Stella Rd Station Range: Start: 0+00.00, End: 49+99.95 Description:

Begin Stella Rd N 13,754,244.1303 E 2,977,397.5658

0+00.00

Line (1) S47° 20' 34.99"E 2,397.992' N 13,752,619.2334 E 2,979,161.1068 23+97.99 Line (1)

Curve (2)
BC N 13,752,619.2334 E 2,979,161.1068
CTR N 13,753,244.3437 E 2,979,737.0730
PI N 13,752,178.3639 E 2,979,639.5934

Direction Back S47° 20' 34.99"E Radius 850.000' Delta 74°51'50"(LT) Length 1,110.630' Tangent 650.627'
Chord Direction S84° 46' 30.22"E Distance 1,033.293' Direction Ahead N57° 47' 34.54"E

EC N 13,752,525.1355 E 2,980,190.1066 35+08.62

Curve (2)

Line (3) N57° 47' 34.54"E 215.685' N 13,752,640.0913 E 2,980,372.6034 37+24.31

Curve (4) BC N 13,752,640.0913 E 2,980,372.6034 CTR N 13,751,920.8830 E 2,980,825.6370 PI N 13,752,987.4921 E 2,980,924.1155

Direction Back N57° 47' 34.54"E Radius 850,000' Delta 74°57'52"(RT) Length 1,112.118'

Tangent 651.808'
Chord Direction S84° 43' 29.69"E Distance 1,034.474'
Direction Ahead S47° 14' 33.92"E

EC N 13,752,544.9842 E 2,981,402.6965 48+36.43 Curve (4)

Line (5) S47° 14' 33.92"E 163.527'

N 13,752,433.9667 E 2,981,522.7641

49+99.95 Line (5)

N 13,752,433.9667 E 2,981,522.7641 End Stella Rd

49+99.95

Alignment Length: 4,999.952'

REVISIONS DATE NAME

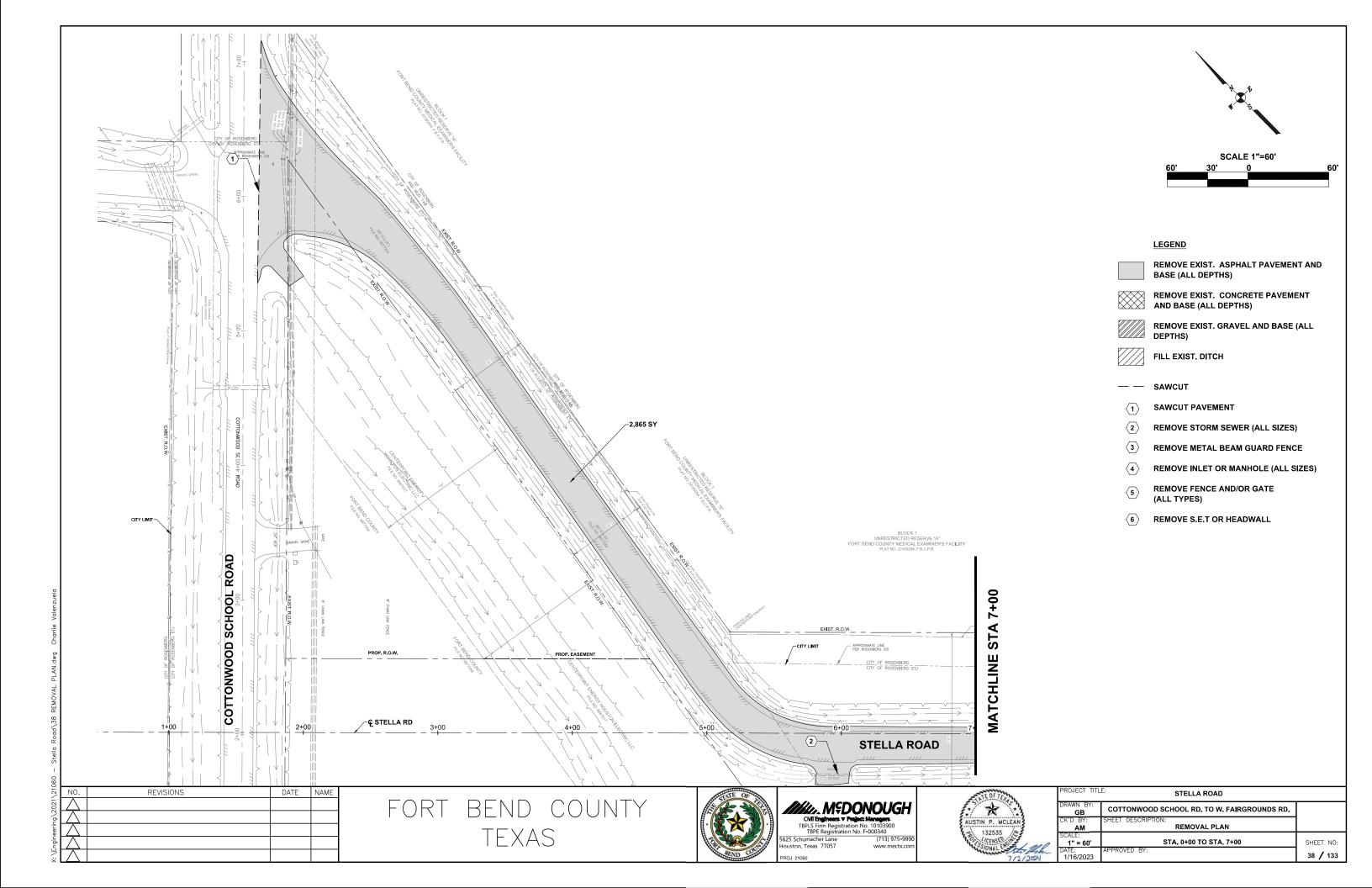
FORT BEND COUNTY TEXAS

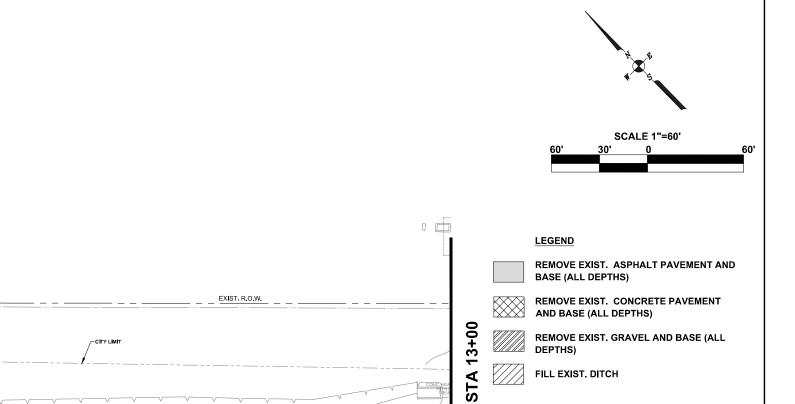






	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY:	SHEET DESCRIPTION: HORIZONTAL ALIGNMENT	
1	SCALE:	DATA SHEET	SHEET NO:
_	DATE: 1/16/2023	APPROVED BY:	37 / 133





REMOVE EXIST. GRAVEL AND BASE (ALL DEPTHS)

FILL EXIST. DITCH

 $-- \quad {\rm sawcut}$

MATCHLINE

SAWCUT PAVEMENT

2 REMOVE STORM SEWER (ALL SIZES)

REMOVE METAL BEAM GUARD FENCE

REMOVE INLET OR MANHOLE (ALL SIZES)

REMOVE FENCE AND/OR GATE (ALL TYPES)

REMOVE S.E.T OR HEADWALL

NO.	REVISIONS	DATE	NAME	
\triangle				
\triangle				
\triangle				
\triangle			·	
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EXIST. R.O.W.

STA 7+00

MATCHLINE

FORT BEND COUNTY TEXAS

~133.39 SY

STELLA ROAD

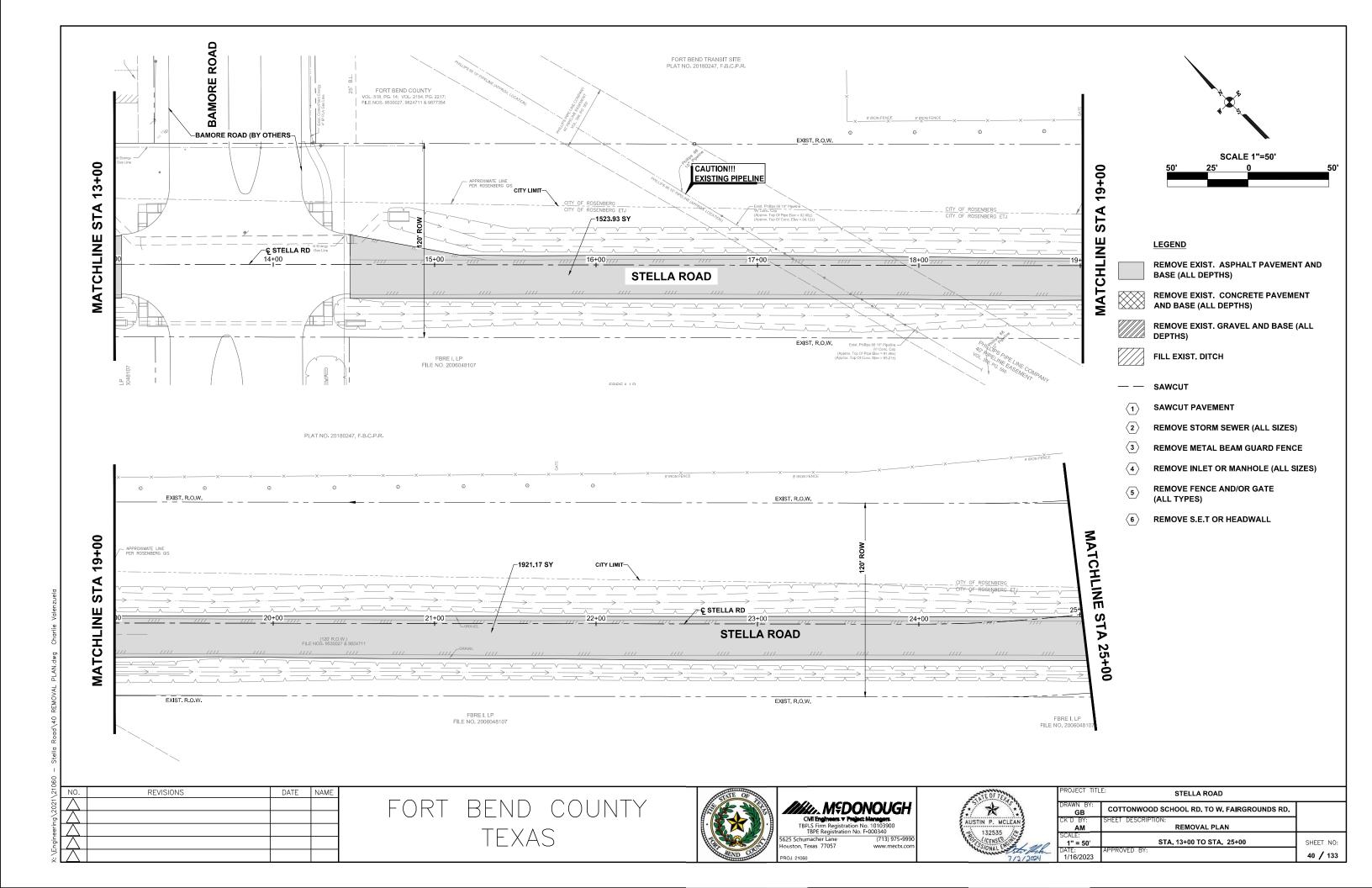


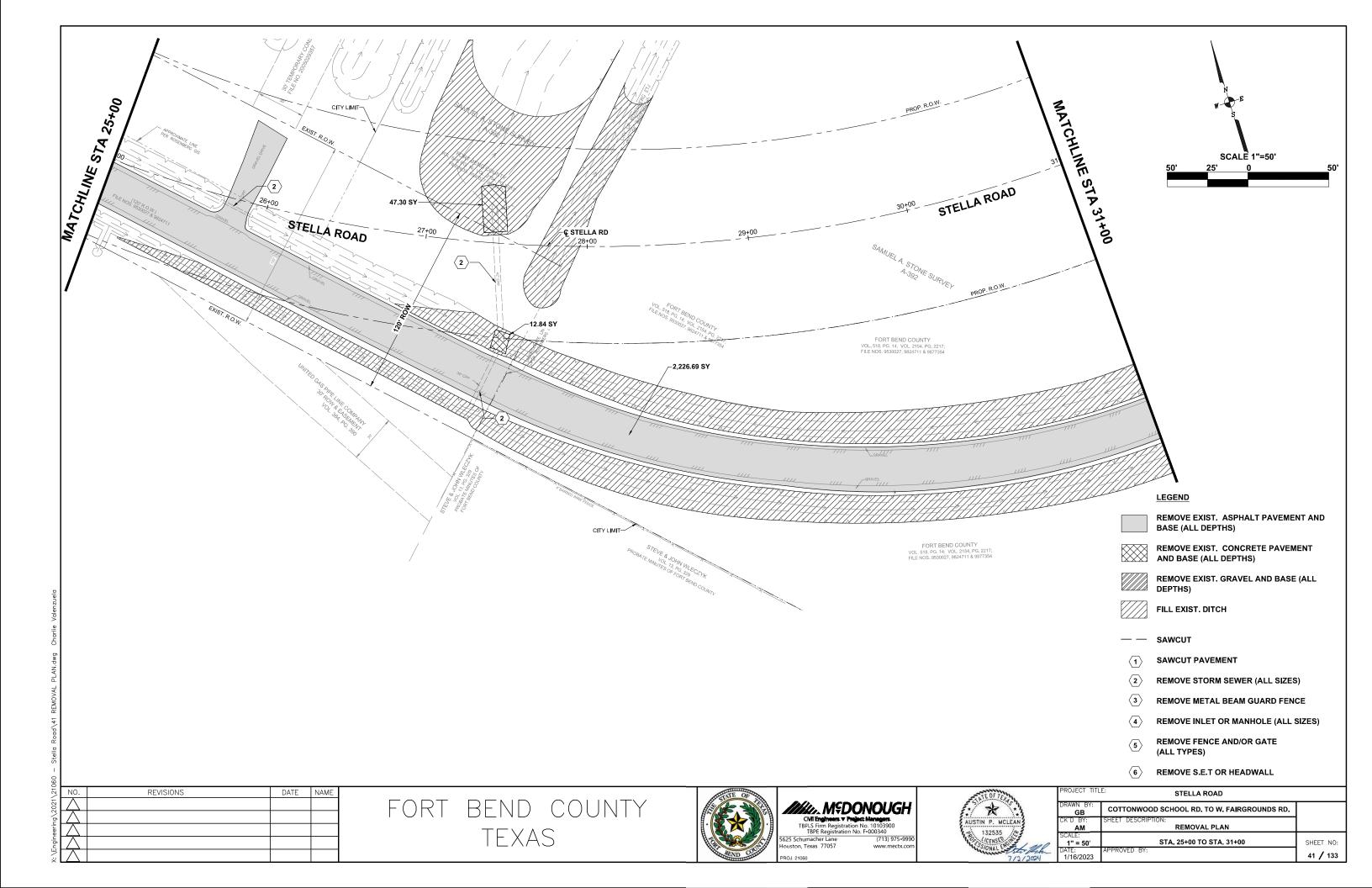
[∟]1,760.46 SY

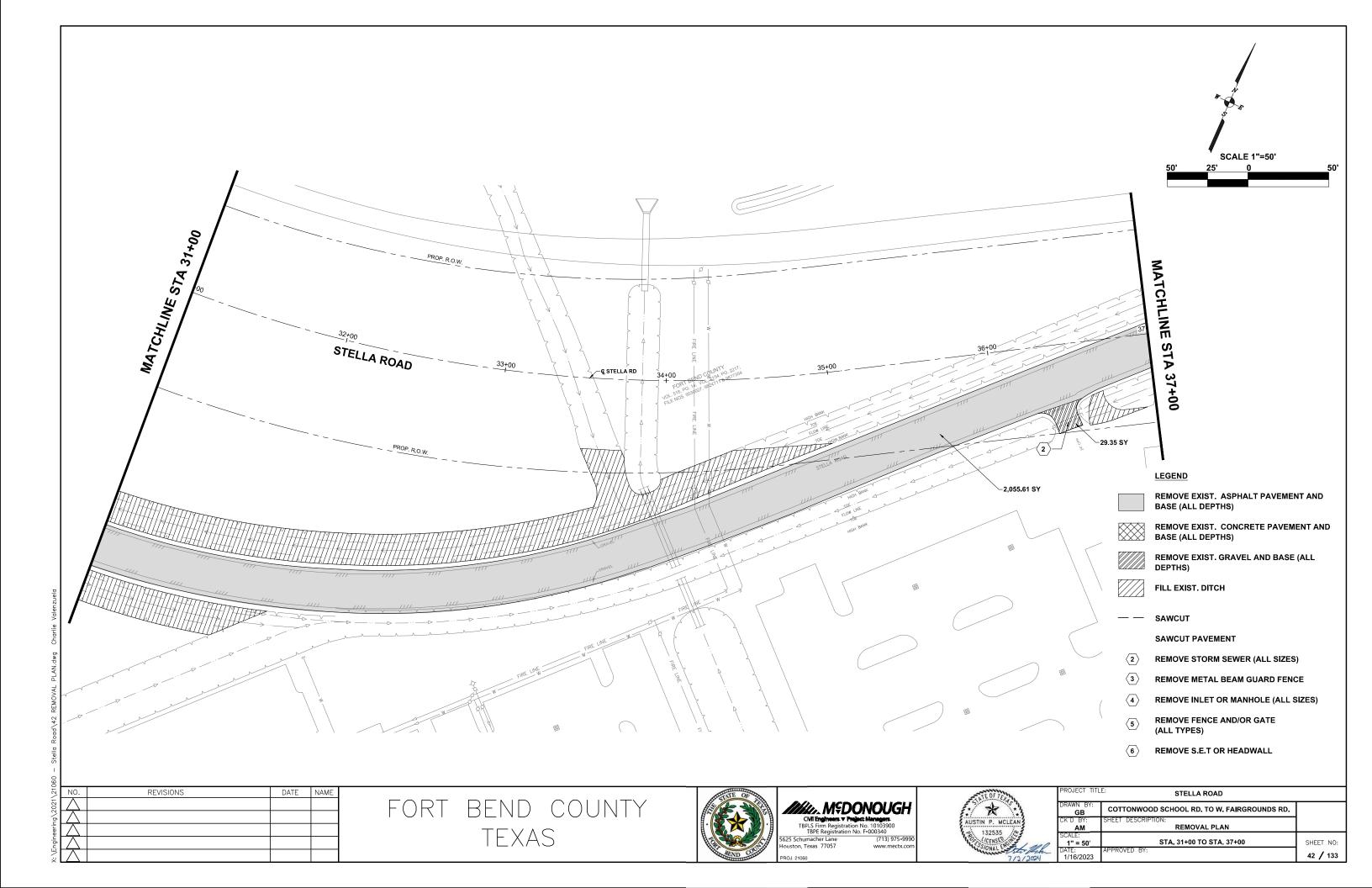


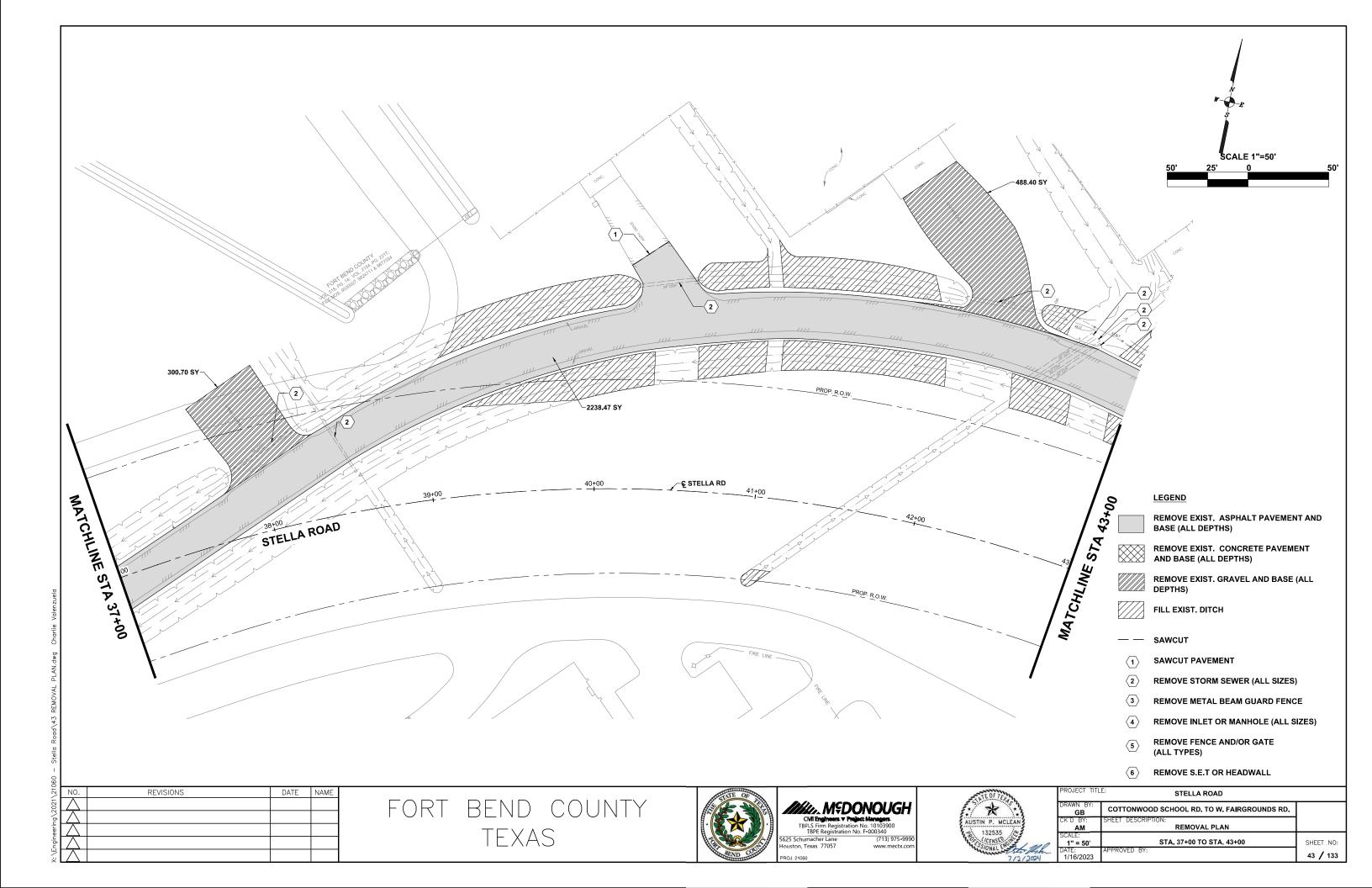


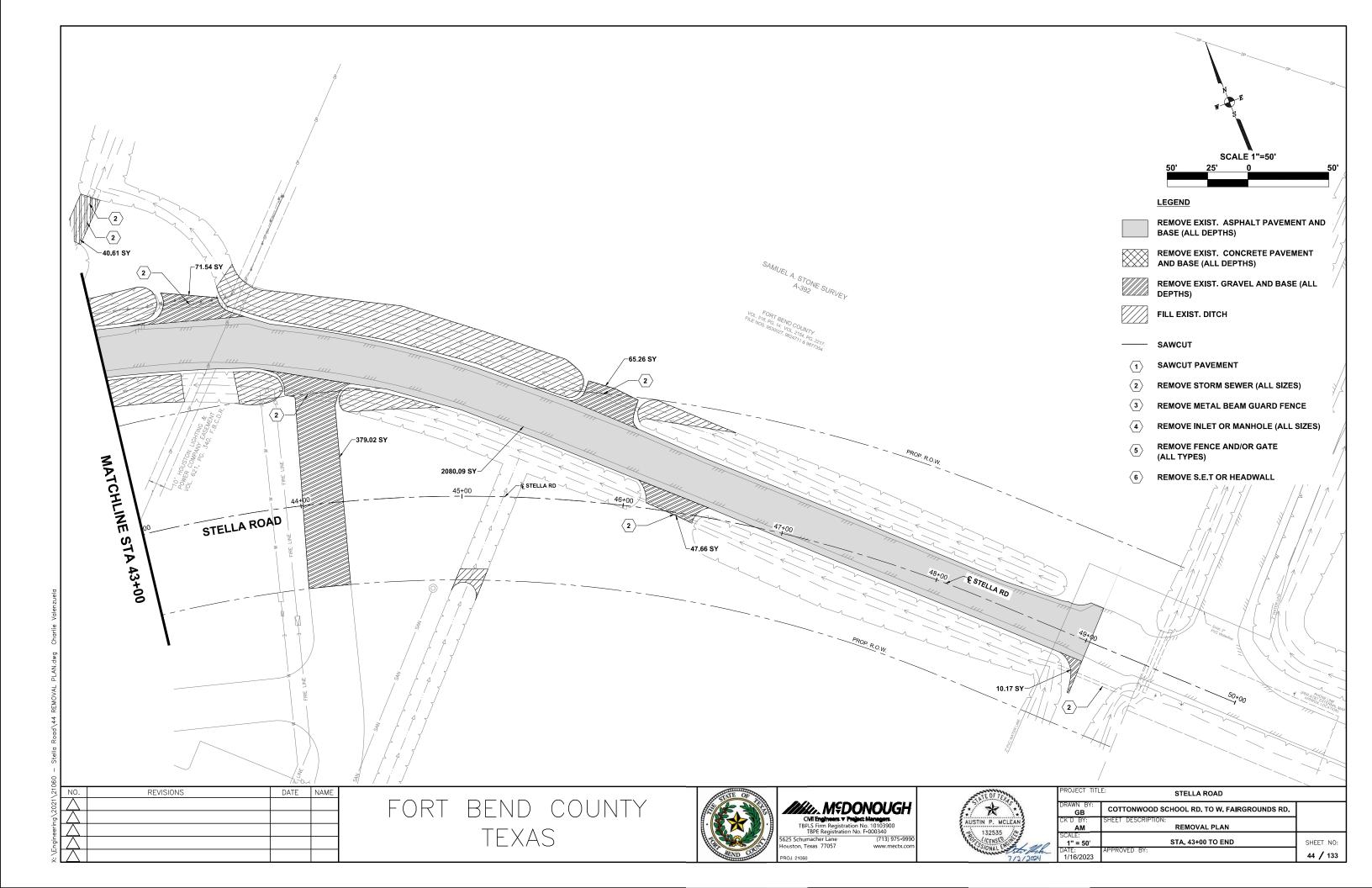
NO:
133

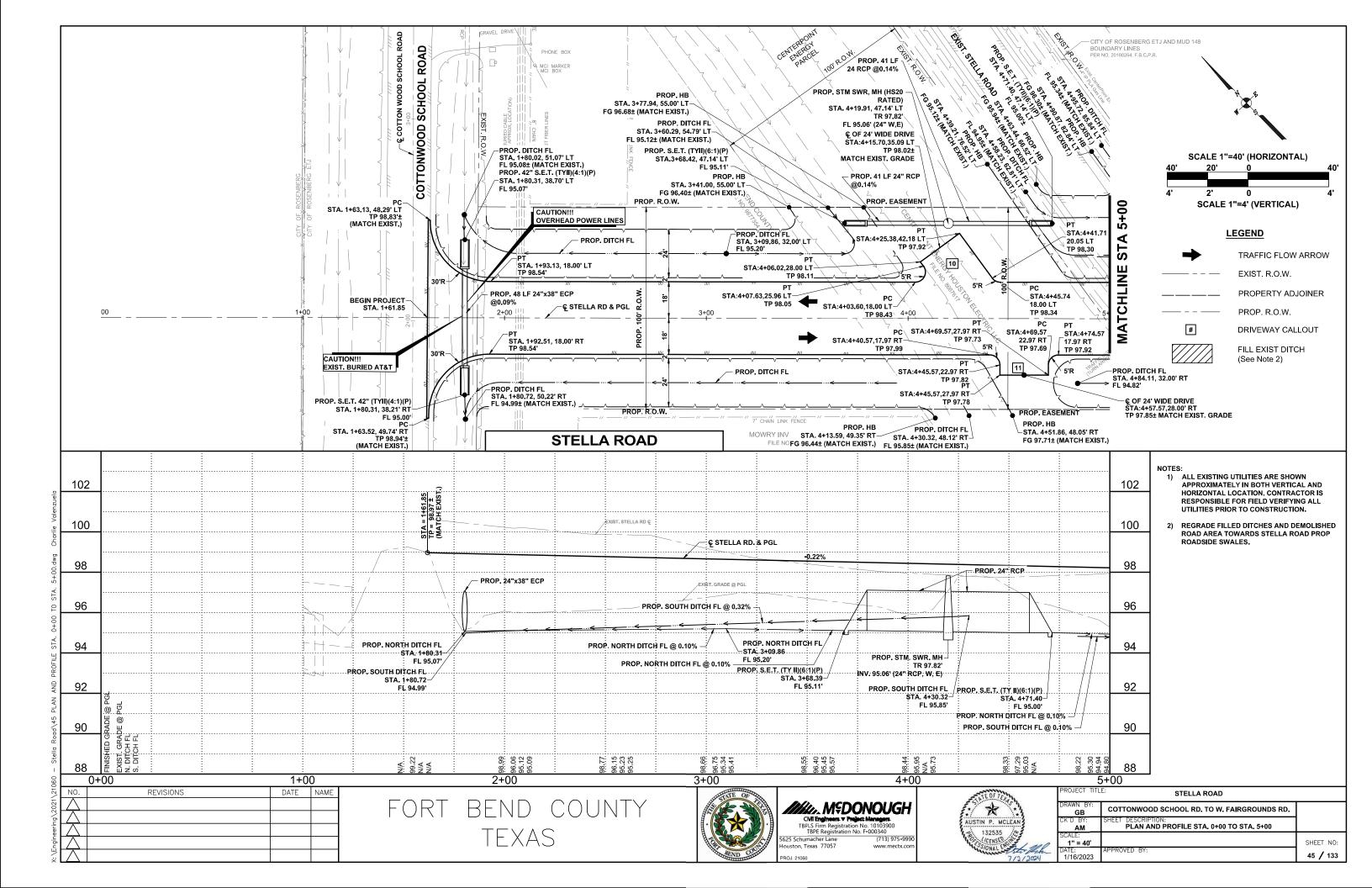


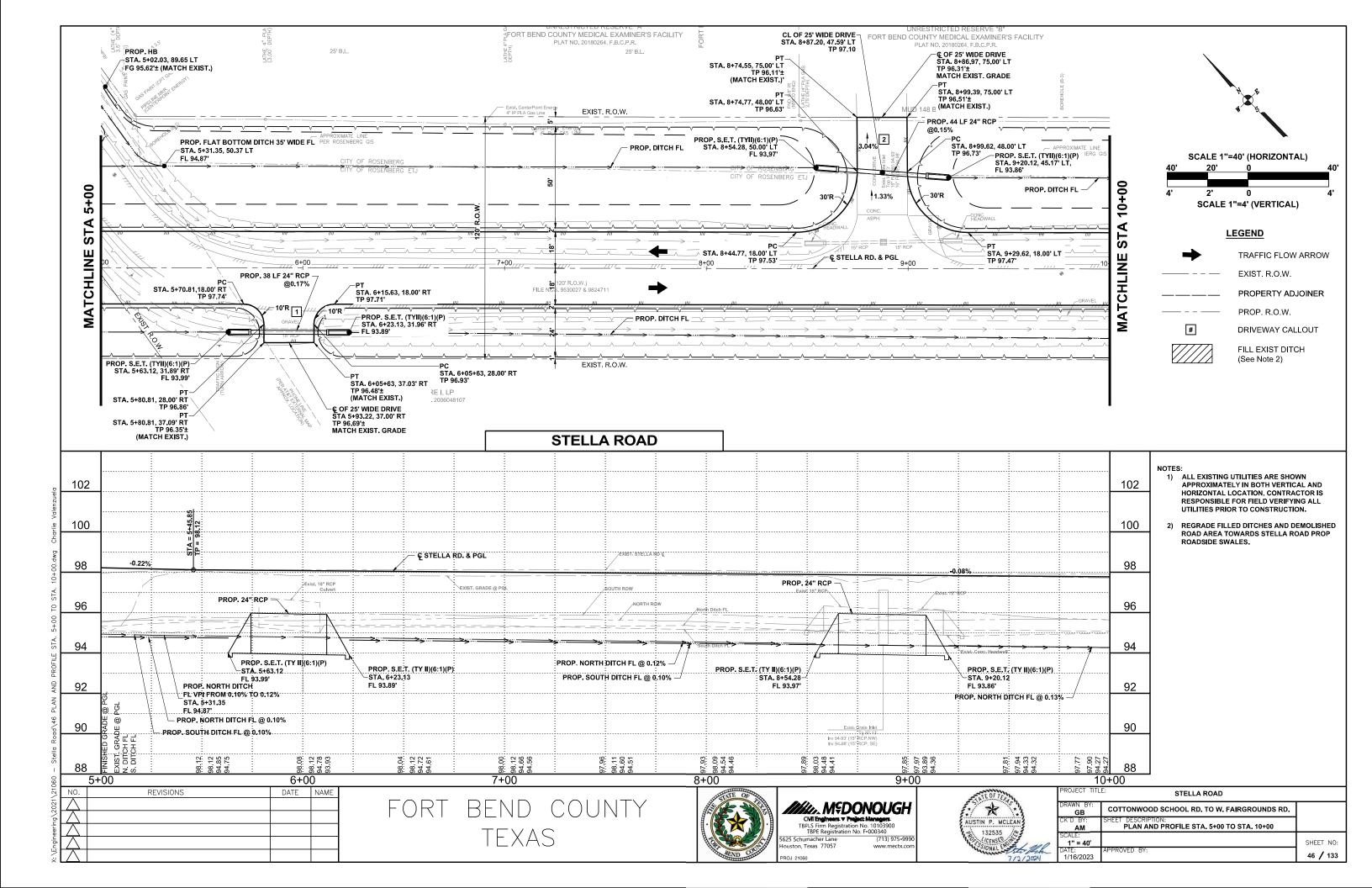


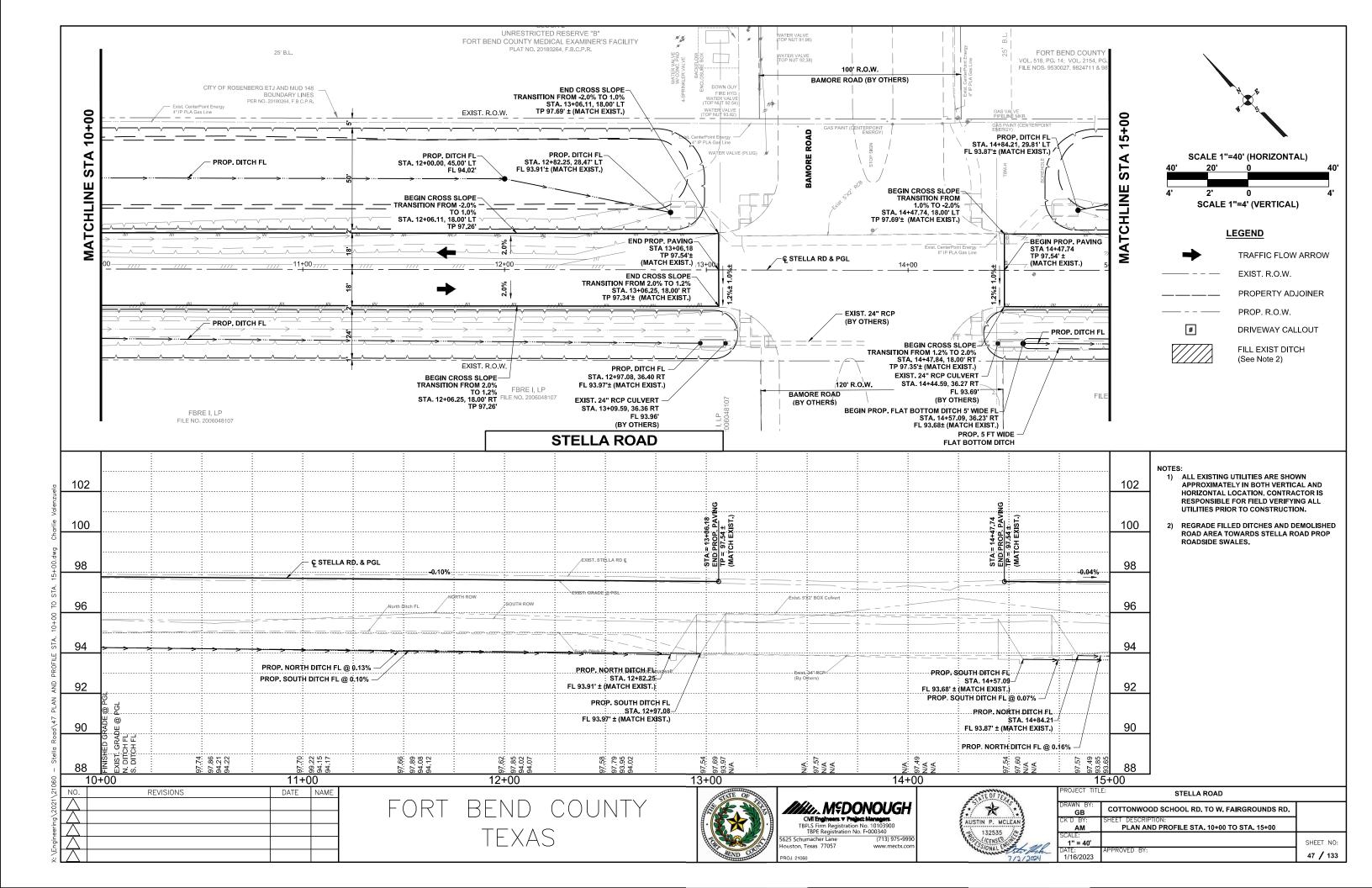


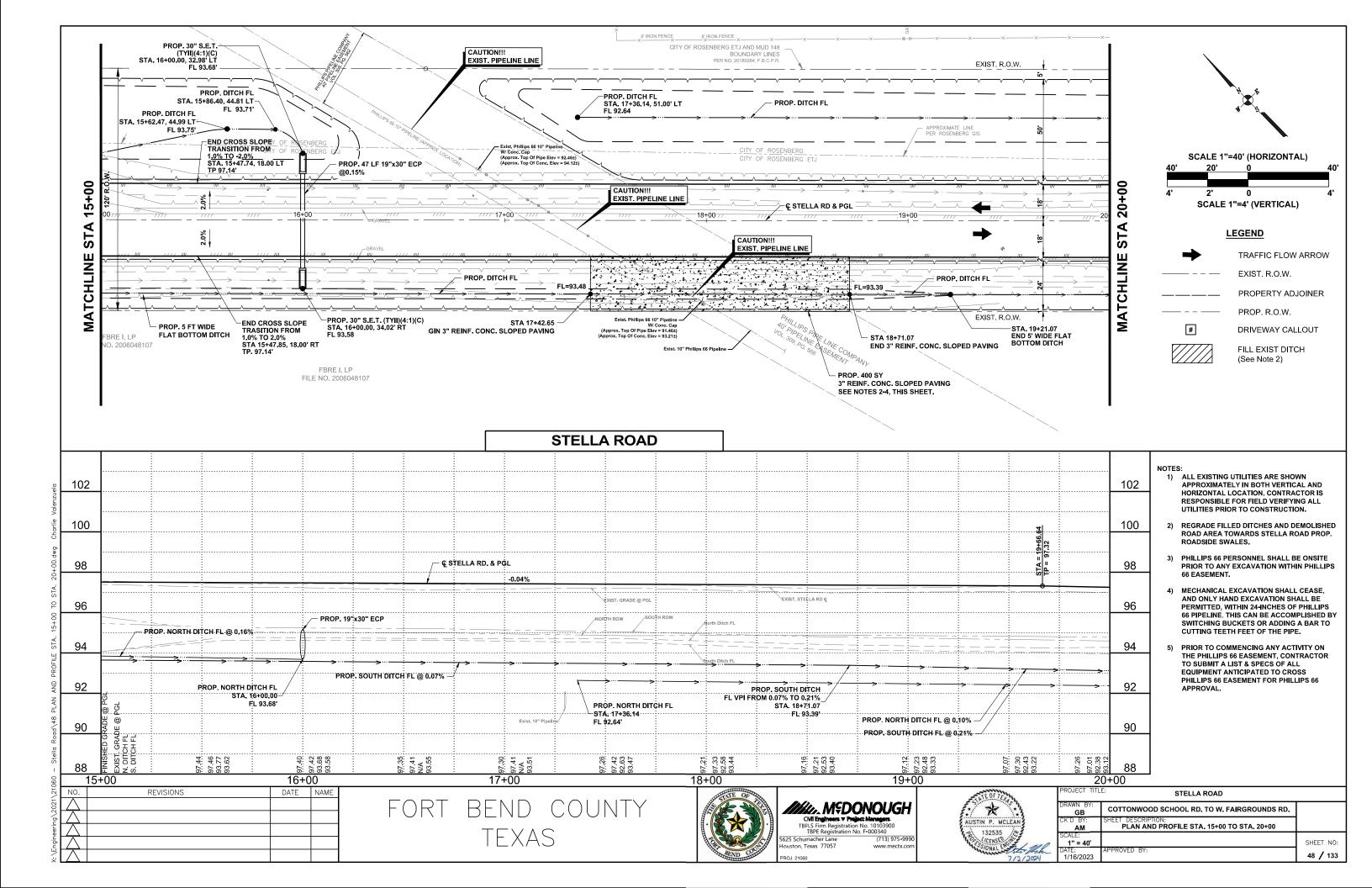


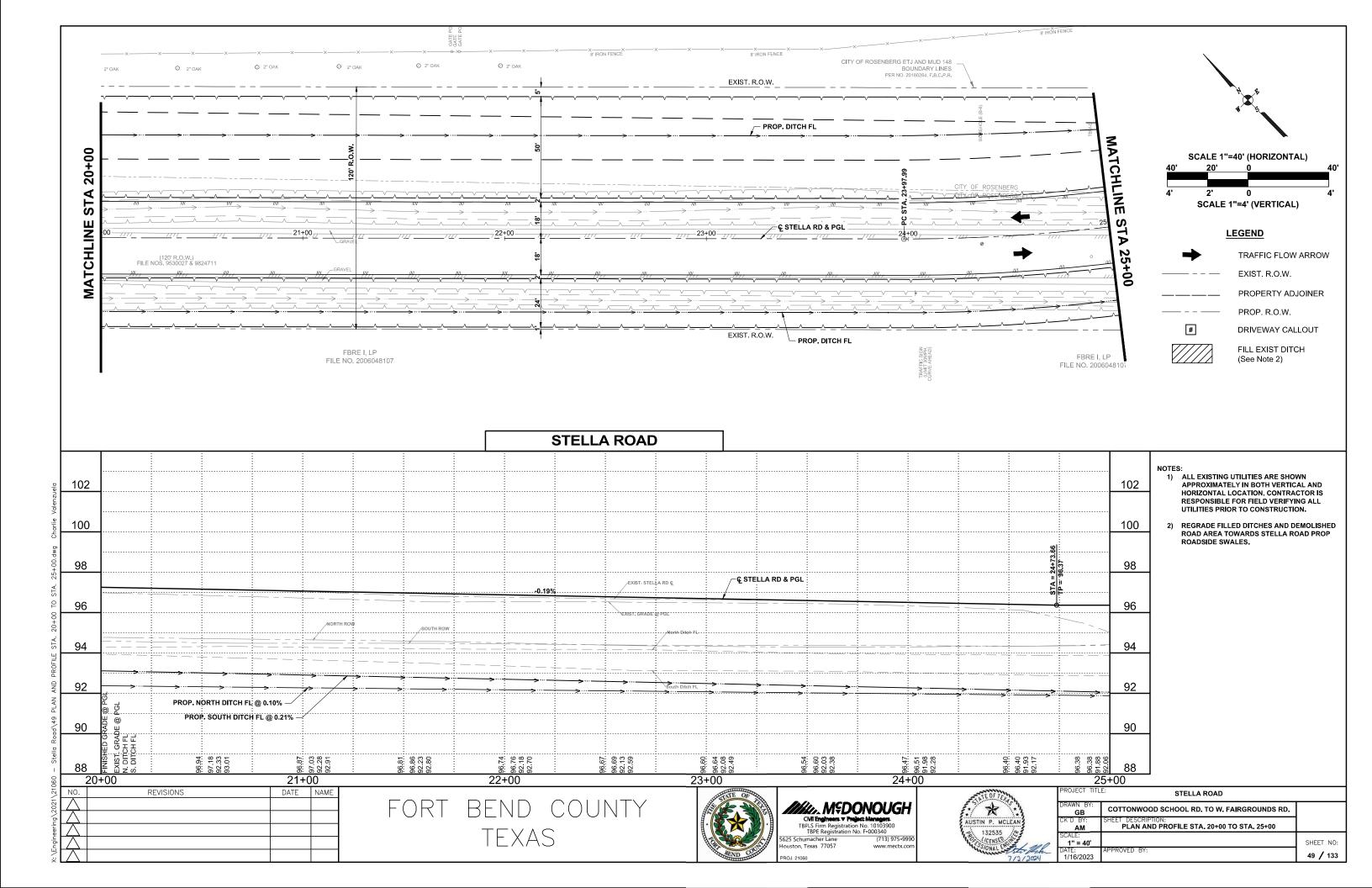


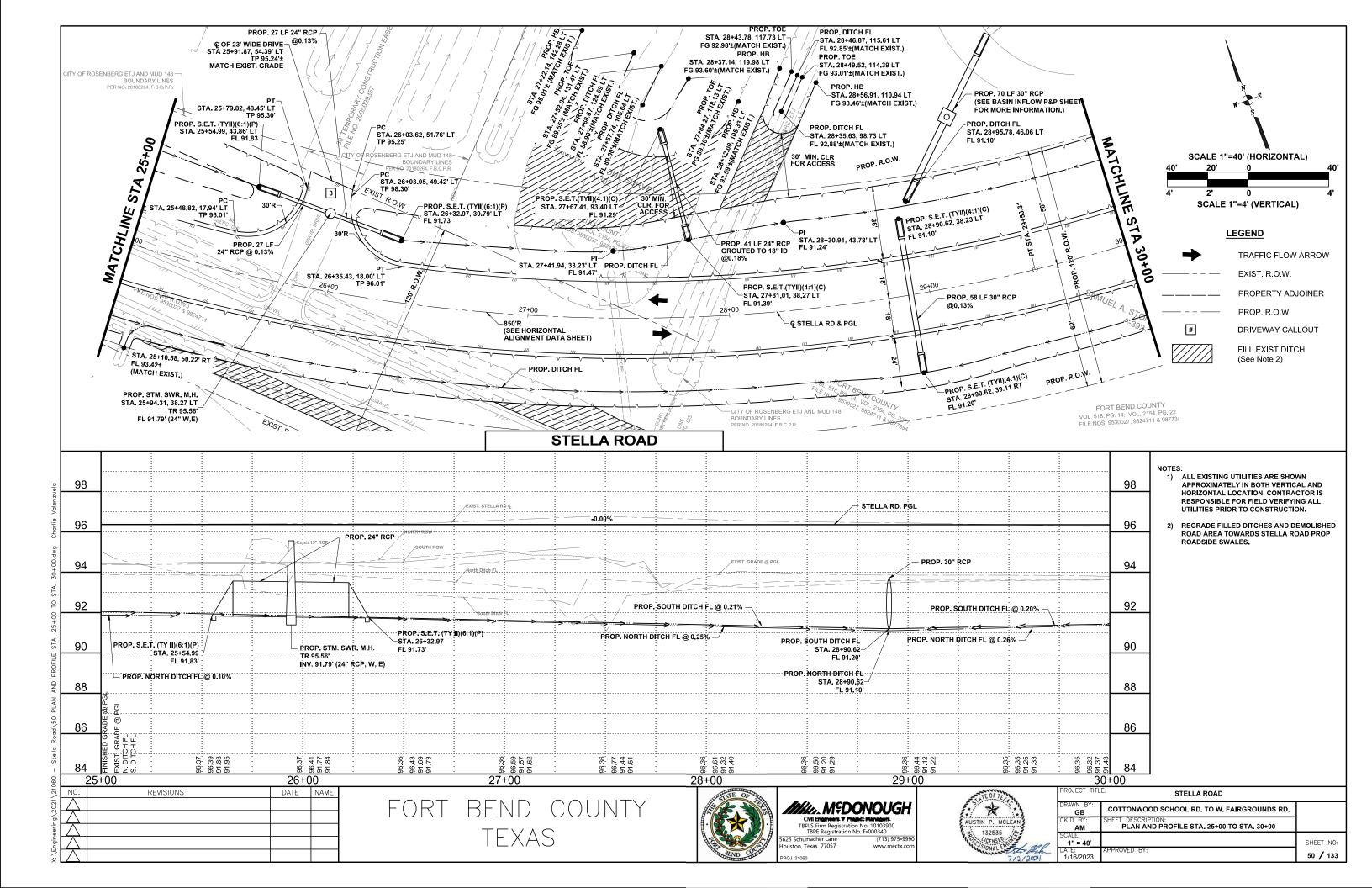


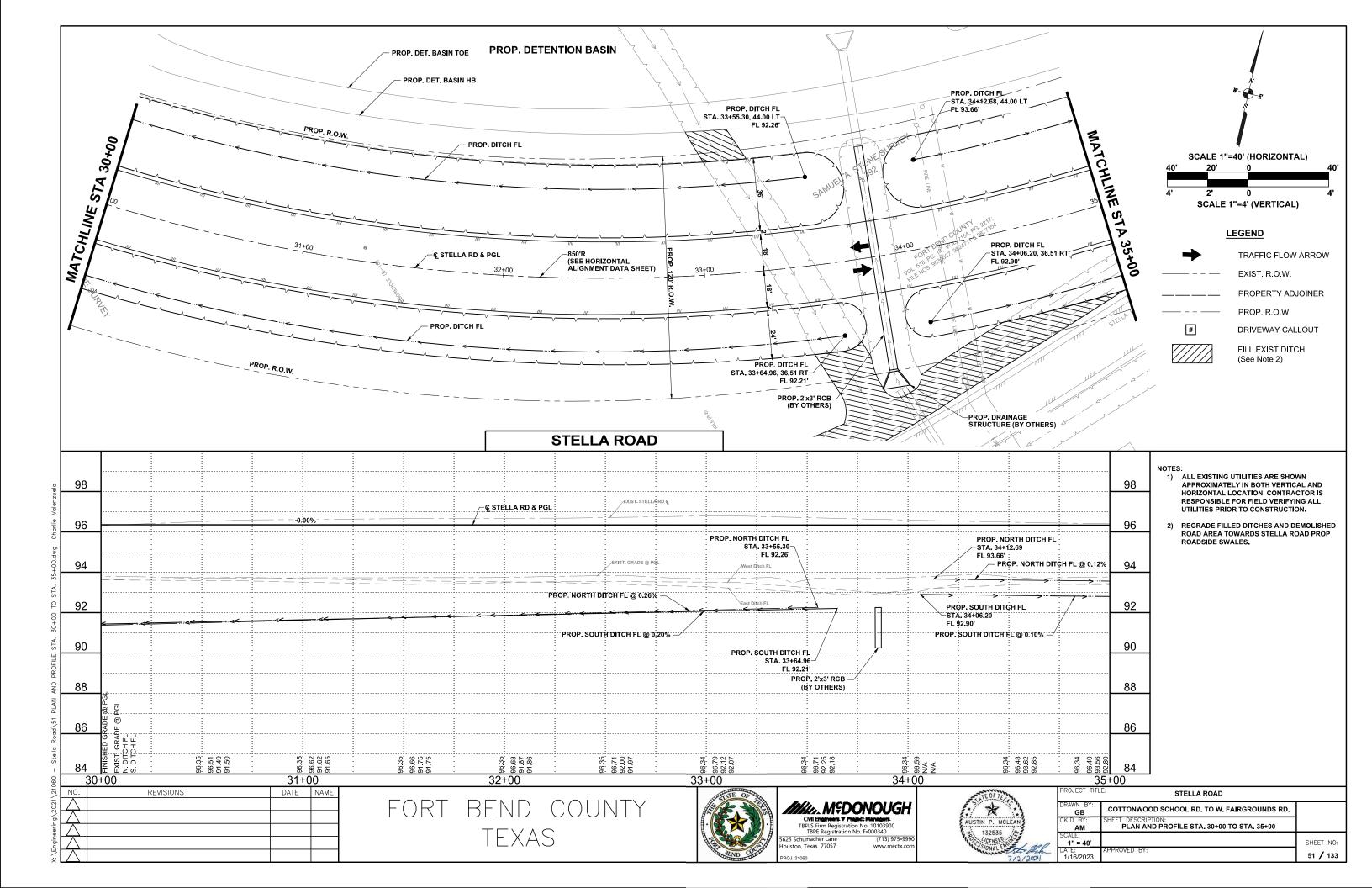


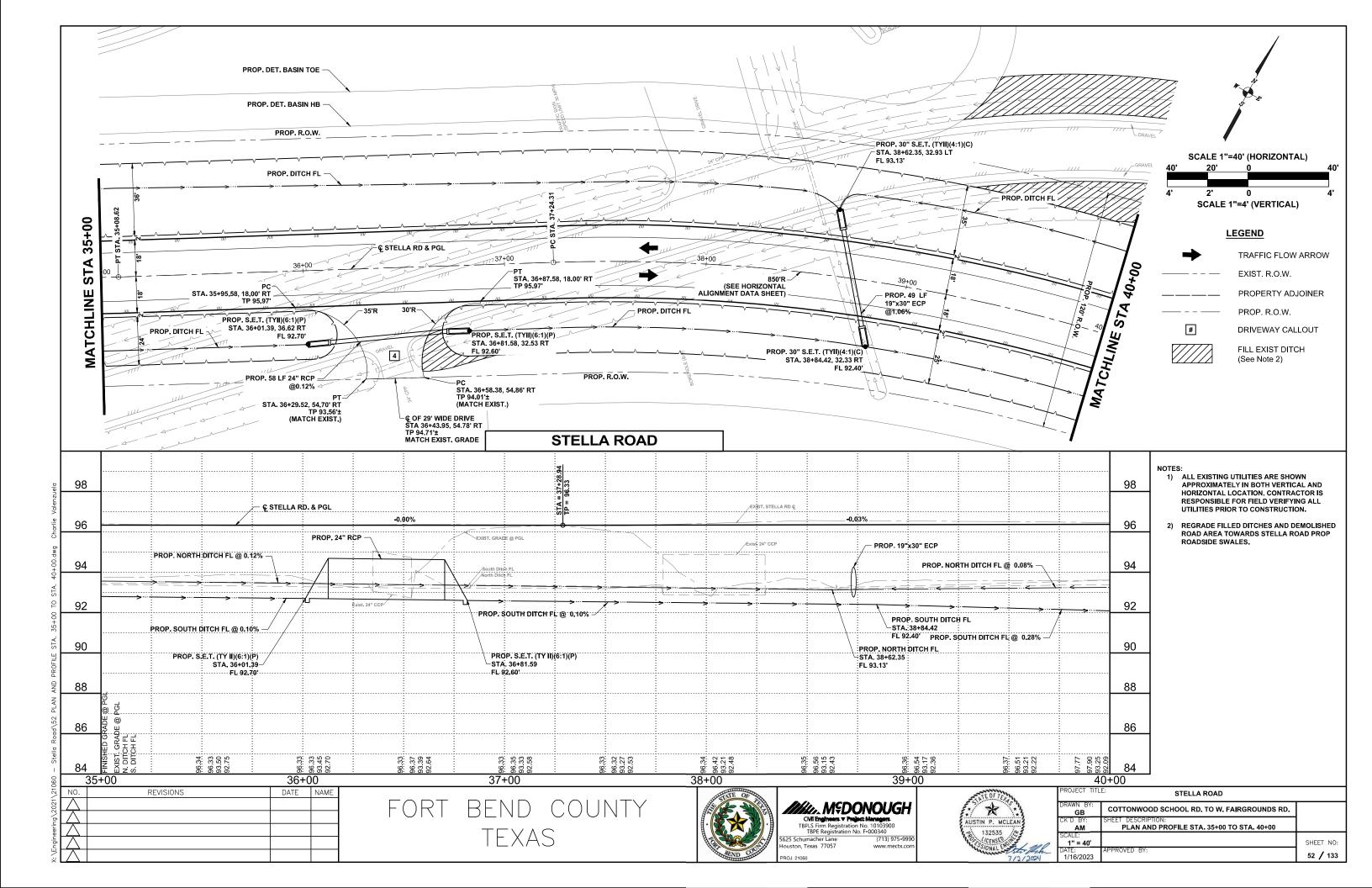


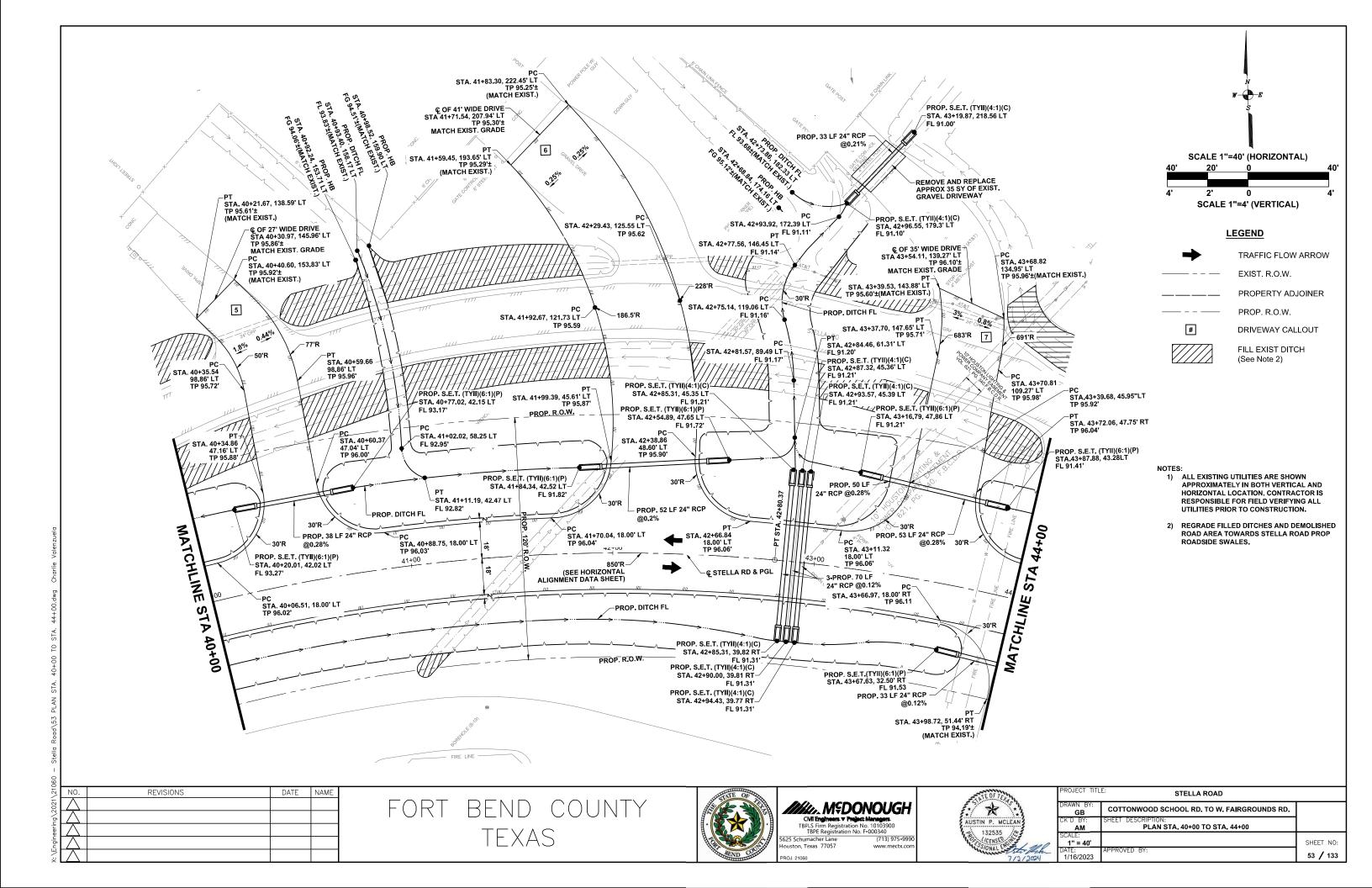


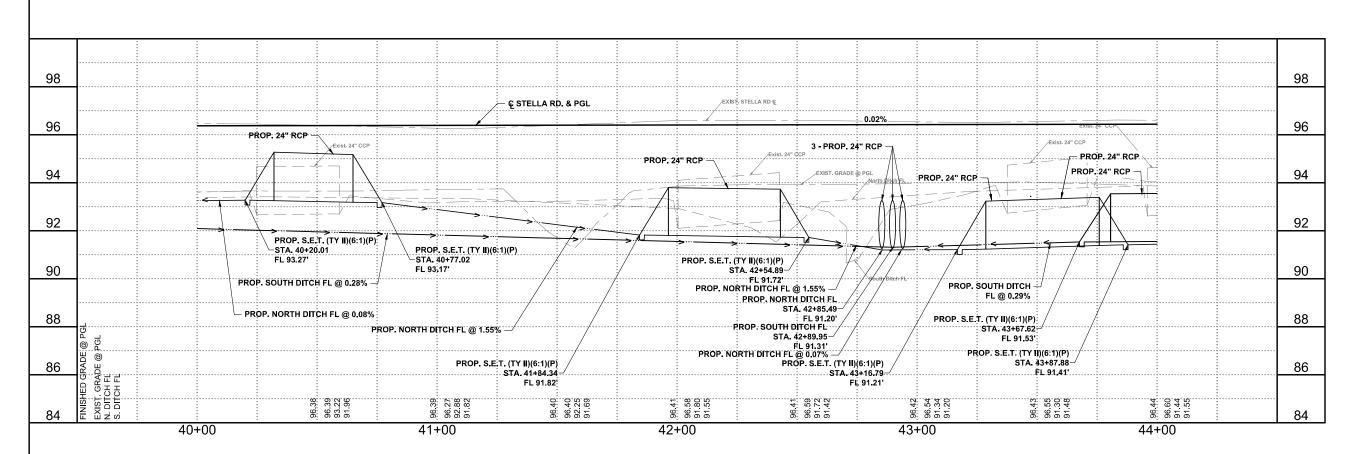






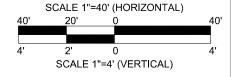






NOTES:

- 1) ALL EXISTING UTILITIES ARE SHOWN APPROXIMATELY IN BOTH VERTICAL AND HORIZONTAL LOCATION. CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL UTILITIES PRIOR TO CONSTRUCTION.
- 2) REGRADE FILLED DITCHES AND DEMOLISHED ROAD AREA TOWARDS STELLA ROAD PROP ROADSIDE SWALES.



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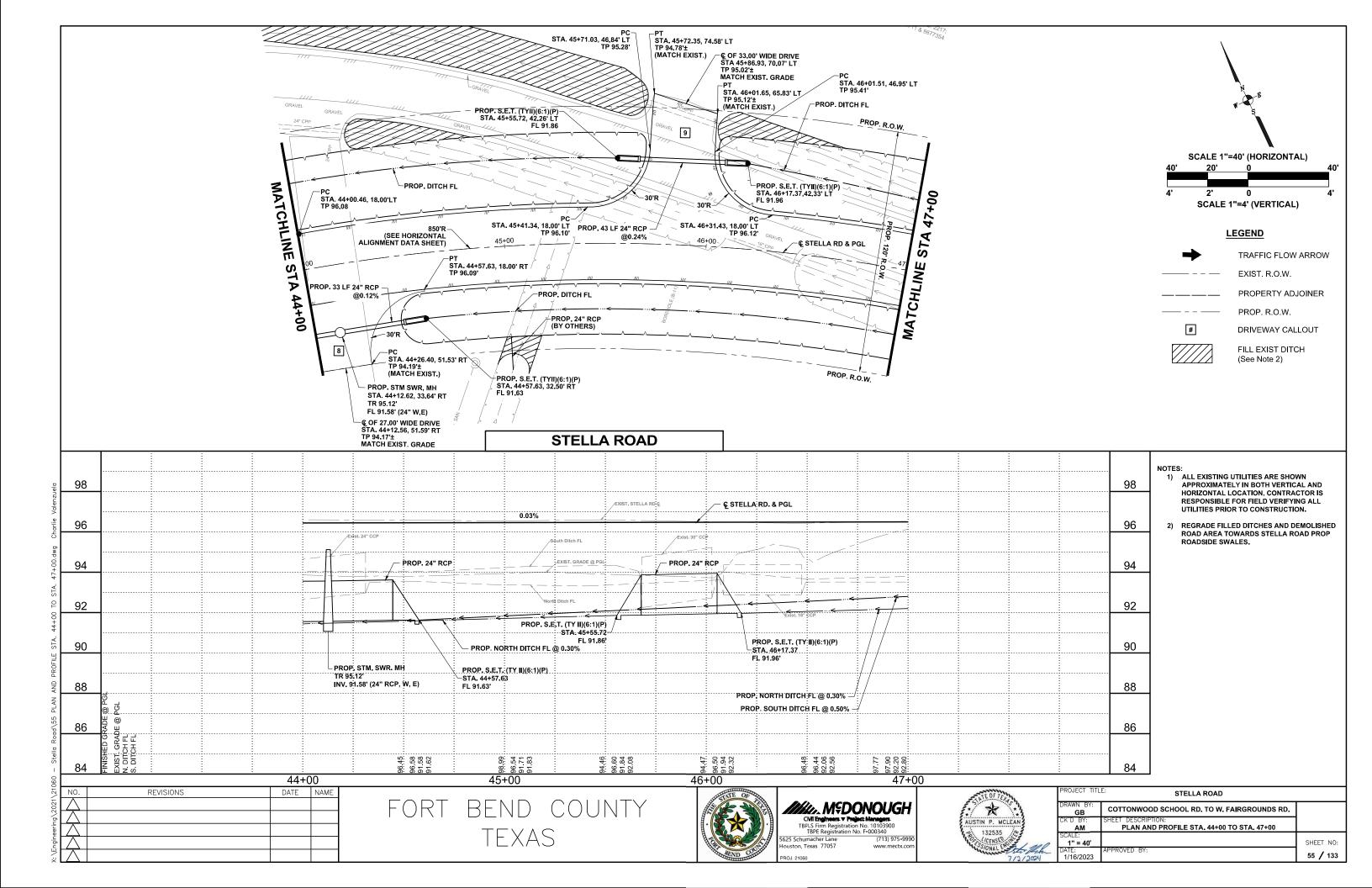
FORT BEND COUNTY TEXAS

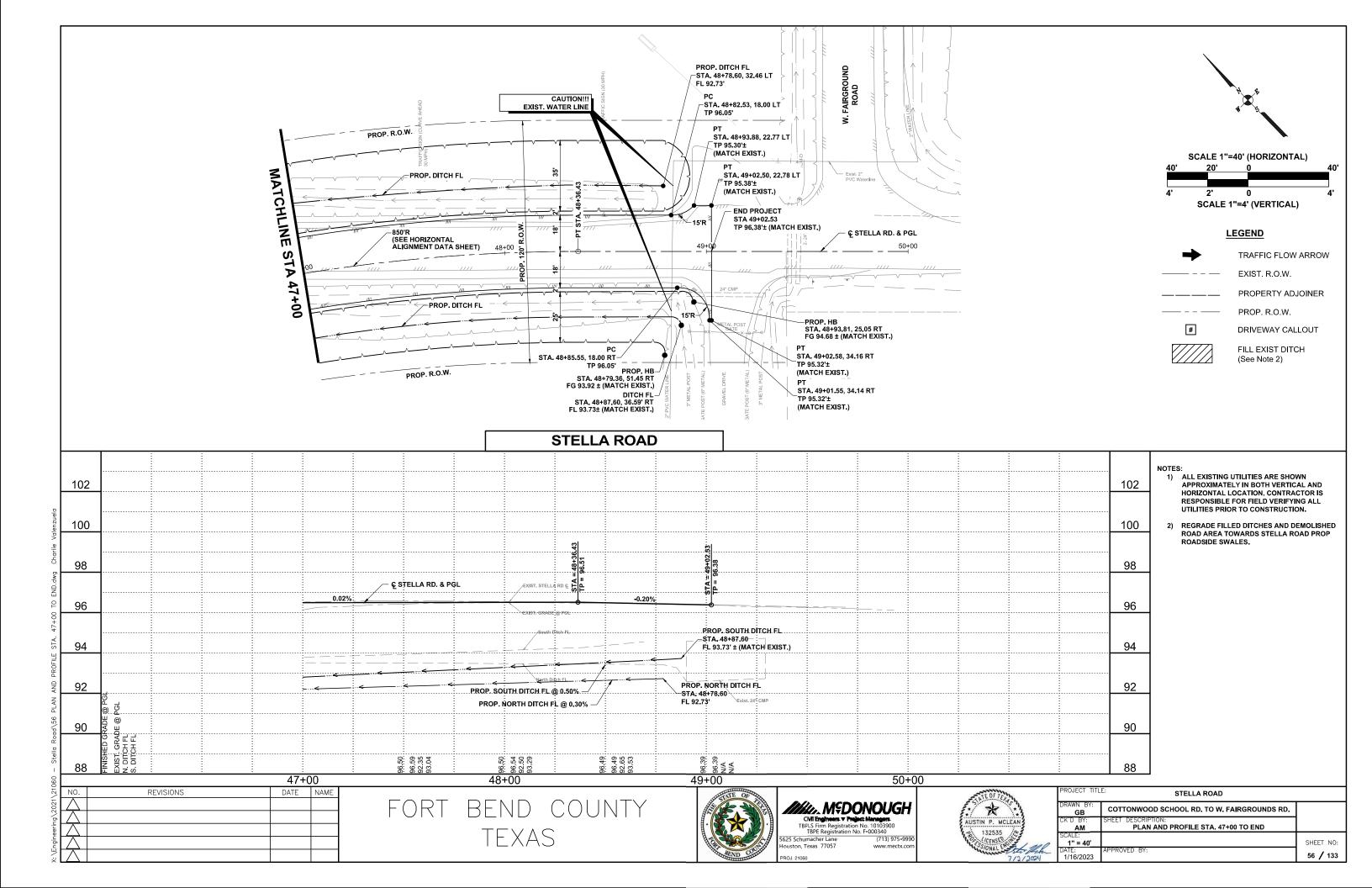


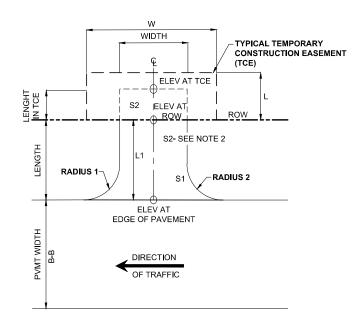




PROJECT TITL		
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY:	SHEET DESCRIPTION: PROFILE STA. 40+00 TO STA. 44+00	
SCALE: 1" = 40'		SHEET NO:
DATE: 12/9/2022 3	APPROVED BY:	54 / 133







NOTES

CONTRACTOR TO FIELD VERIFY EXIST. PAVING
 ELEVATION AND NOTIFY ENGINEER IF IT DOES NOT
 MATCH ELEV. SHOWN IN PLANS.

2. GRADE S2 ONLY NEEDED FOR DRIVEWAY NO. 2.

 $\frac{\text{DRIVEWAY DETAIL}}{\text{\tiny NTS}}$

PROP. PROP. PROP. EXIST. DRIVEWAY SHEET WIDTH LENGTH LENGTH IN RADIUS 1 RADIUS 2 EL. AT PAVING EL. LENGTH GRADE TCE GRADE (FT) NO. **CL STATION** (FT) TCE (FT) (FT) (FT) EOP @ TIE IN L1 (FT) S1(%) (LxW) S2(%) 1 45 STA 5+93.22, 37.00' RT 25 19 N/A 10 10 97.73 96.69 +/-19 5.47 N/A 2 45 30 57 STA 8+86.97, 75.00' LT 25 57 N/A **30** 97.50 96.31 +/-1.35 N/A 2.88 3 49 STA 25+92.16,54.37' LT 23 36.4 N/A 30 30 96.01 95.24 +/-36.4 2.12 N/A 4 51 STA 36+43.95, 54.78' RT 29 36.8 N/A 35 30 95.97 94.71 +/-36.8 3.42 N/A 5 52 27 STA 40+30.97, 145.96' LT 132.8 N/A 30 30 96.03 95.86 +/-132.8 N/A 0.13 6 52 STA 41+71.54, 207.94' LT 41 204.1 N/A 30 96.05 95.30 +/-204.1 N/A 30 0.37 7 52 STA 43+54.11, 139.27' LT 35 121.3 N/A 30 30 96.07 96.10 +/-121.1 -0.02 N/A 8 STA 44+12.56, 51.59' RT 27 54 33.6 N/A 30 30 96.10 94.17 +/-33.6 5.74 N/A STA 45+86.93, 70.07' RT 52.1 9 54 33 52.1 N/A 30 30 96.11 95.02 +/-2.09 N/A 10 44 STA 4+15.70, 35.09' LT 24 21.2 N/A 5 5 98.38 98.02 +/-21.2 1.70 N/A 11 STA 4+57.57, 28.00' RT 24 10 N/A 5 5 97.96 97.85 +/-10 1.10 N/A

NO.	REVISIONS	DATE	NAME	
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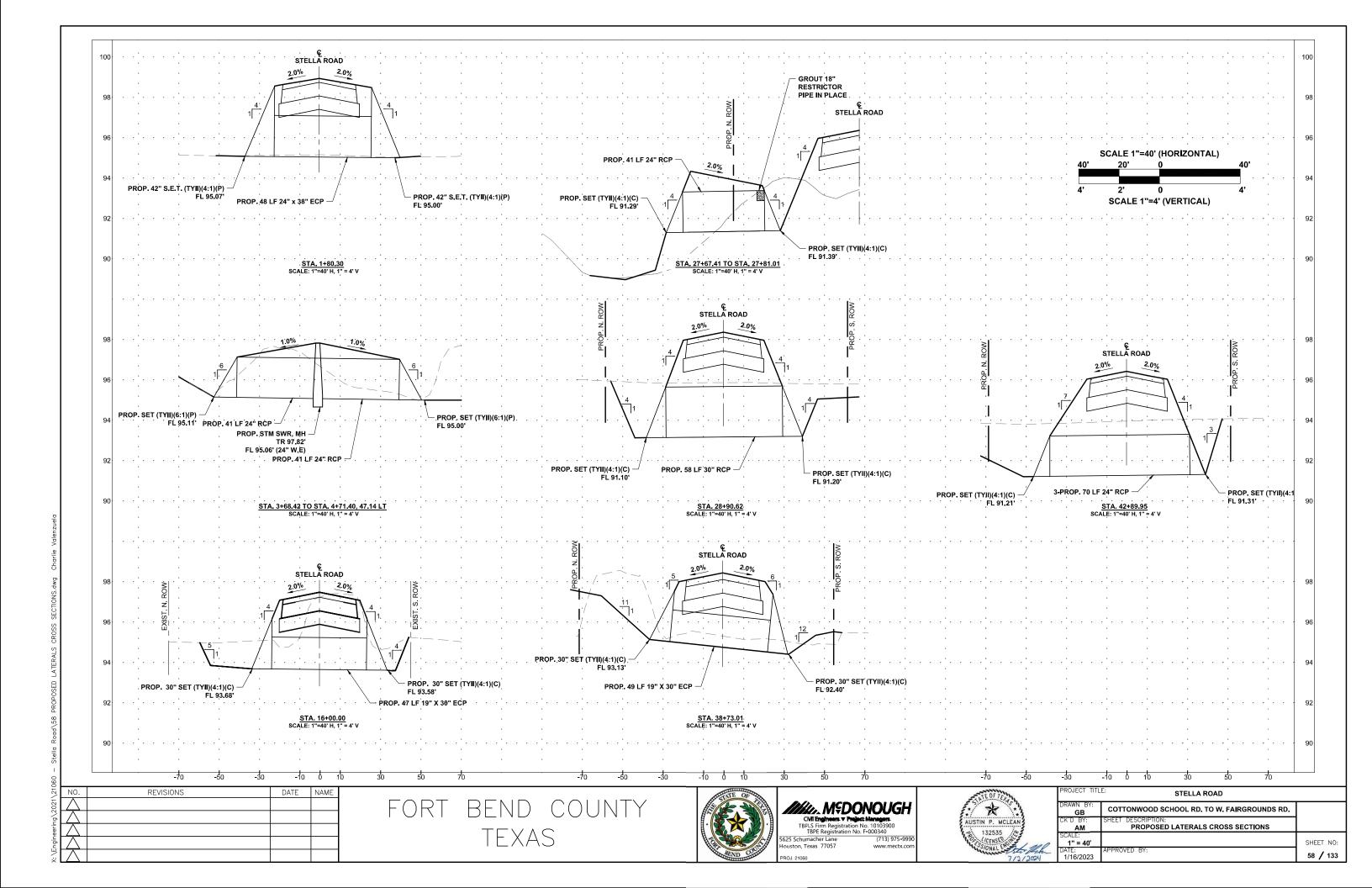
FORT BEND COUNTY TEXAS

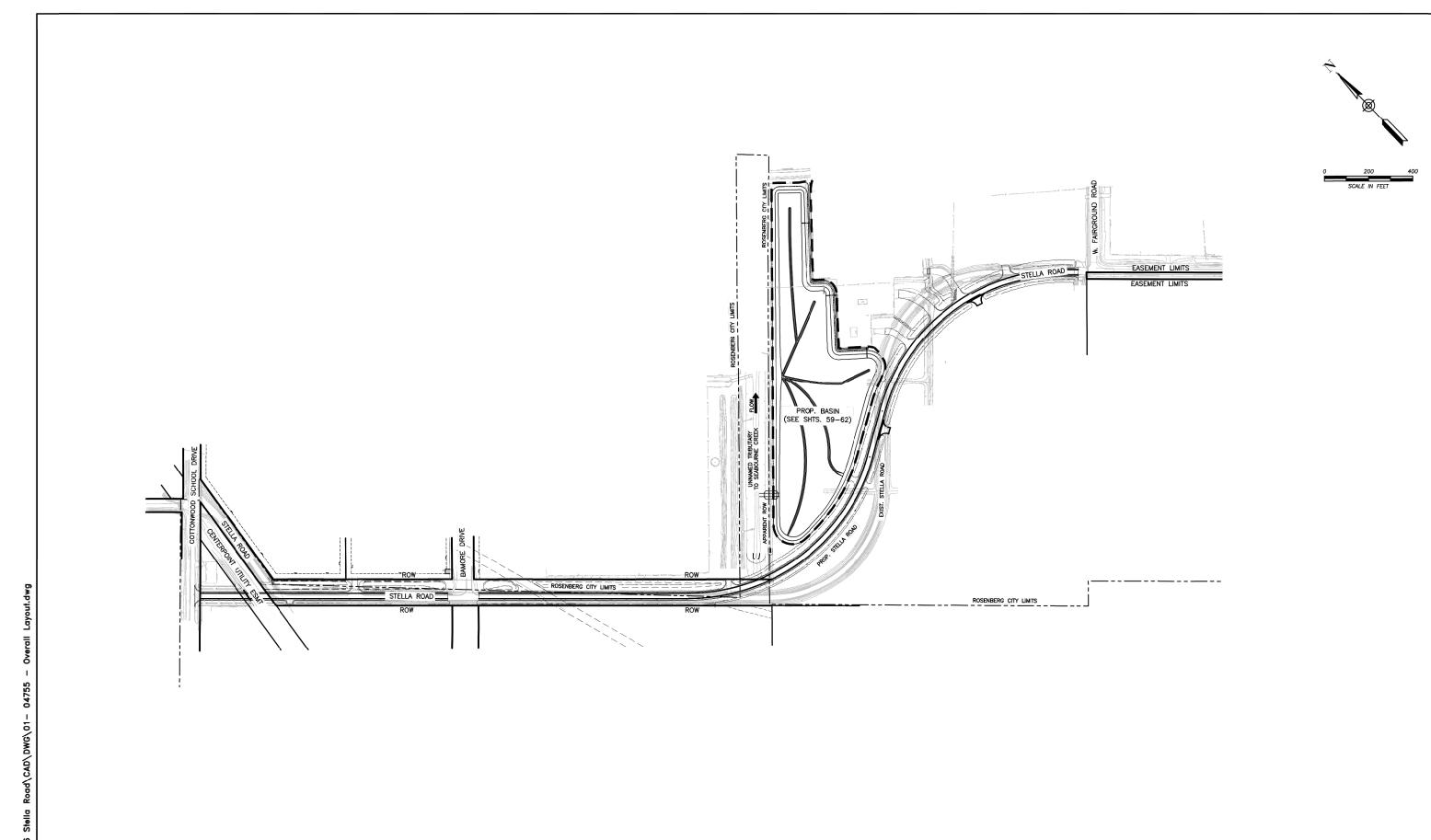




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The state of the s	USTIN P. MCLEAN	
N.	132535 (CENSE)	
	JS/ONAL ENG	2024

	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY:	SHEET DESCRIPTION: DRIVEWAY SUMMARY TABLE	
2	SCALE:		SHEET NO:
-	DATE: 1/16/2023	APPROVED BY:	57 / 133





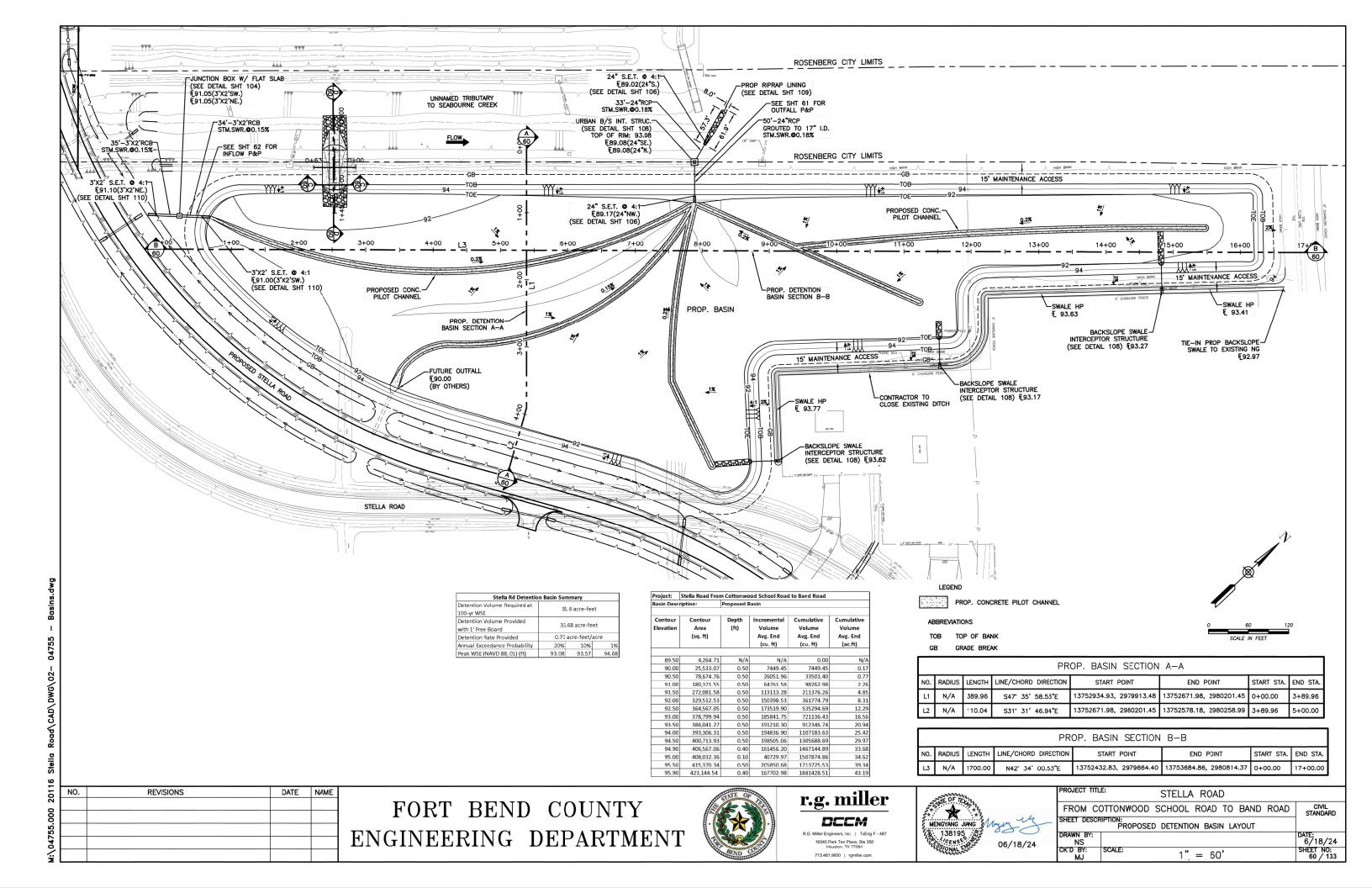
NO.	REVISIONS		DATE	NAME

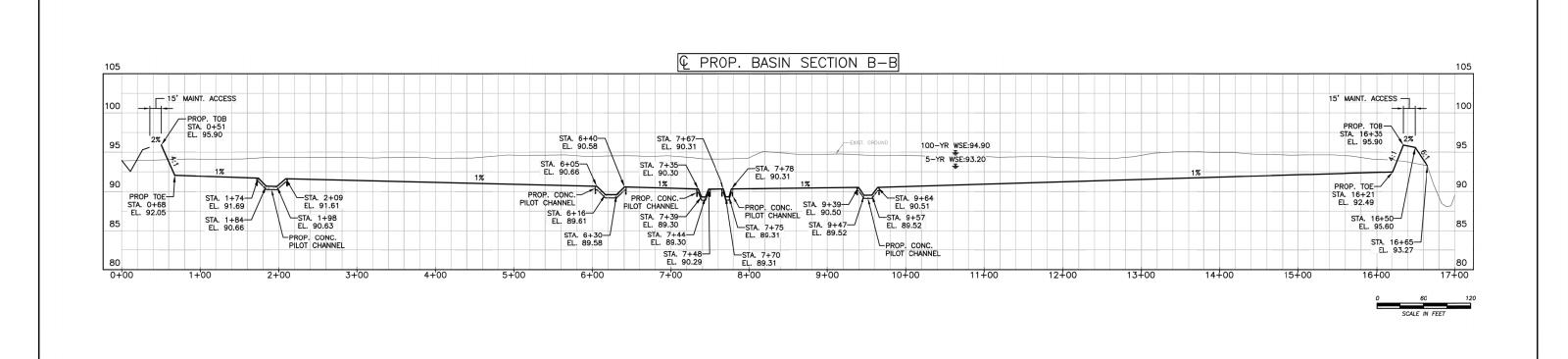


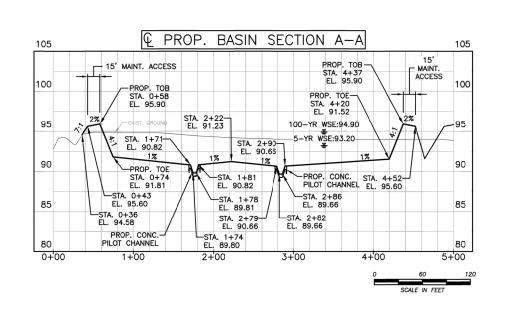


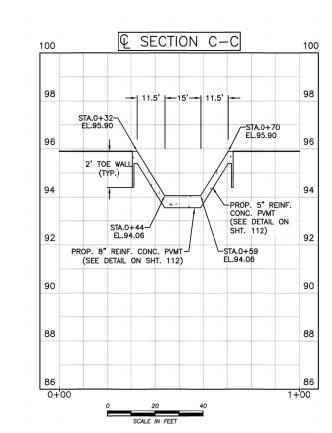
MENGYANG JIANG 3. 138195 SENST 06/18/24

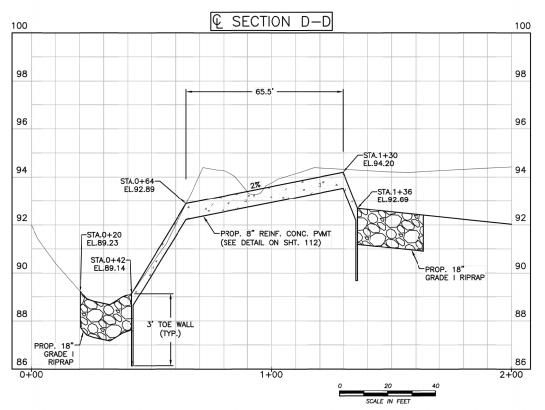
	PROJECT TITL	E: STELLA ROAD	
		OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
_	SHEET DESCR	IPTION: PROPOSED OVERALL DRAINAGE LAYOUT	
	DRAWN BY: NS		DATE: 6/18/24
	CK'D BY: MJ	1" = 400'	SHEET NO: 59 / 133











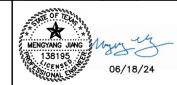
NO.	REVISIONS		DATE	NAME		

Road\CAD\DWG\02- 04755

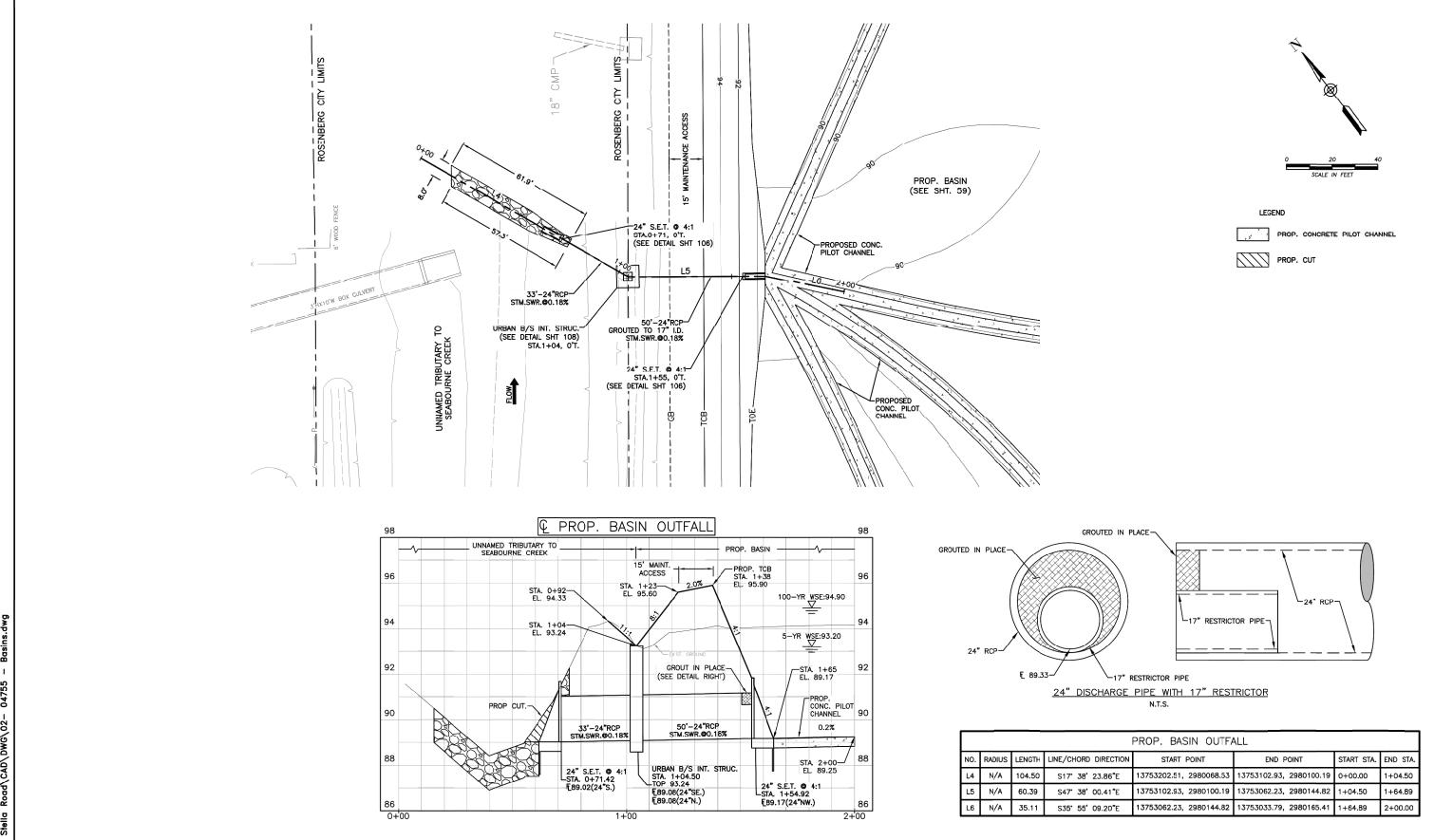
FORT BEND COUNTY ENGINEERING DEPARTMENT



r.g. miller



	PROJECT T	ITLE:	STELLA	ROAD			
		COTTONWOOD	SCHOOL	ROAD T	O BAND	ROAD	CIVIL STANDARD
-	SHEET DES		DETENTIO	N BASIN	SECTION		
	DRAWN BY: NS						DATE: 6/18/24
	CK'D BY: MJ	SCALE:	N/	/A			SHEET NO: 61 / 133



NO. REVISIONS DATE NAME

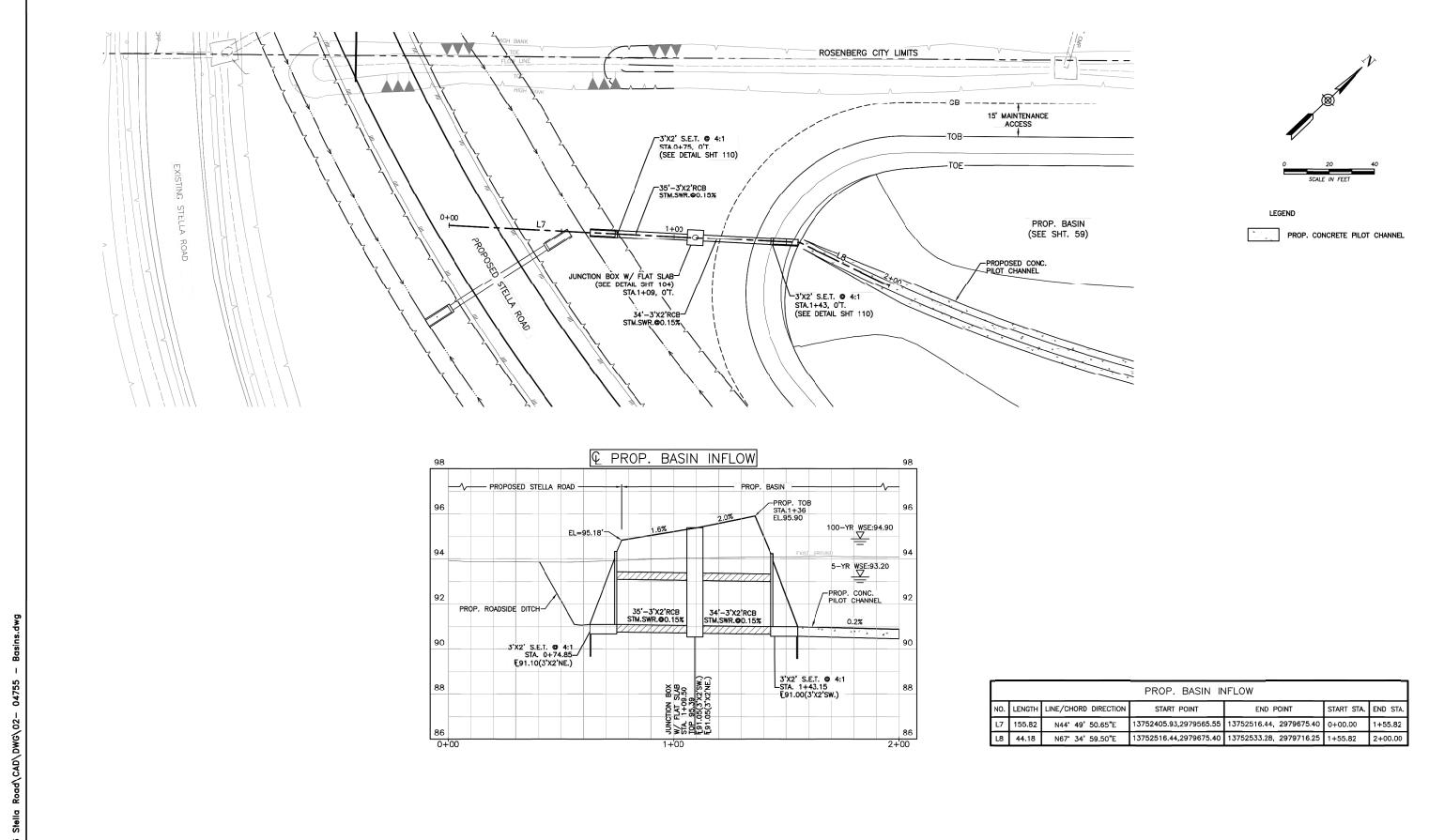
FORT BEND COUNTY ENGINEERING DEPARTMENT



r.g. miller



	PROJECT TITL	E: STELLA ROAD	
		OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
_	SHEET DESCR	RIPTION: PROPOSED DETENTION BASIN OUTFALL P&P	
	DRAWN BY: NS		DATE: 6/18/24
	CK'D BY: MJ	1" = 40'	SHEET NO: 62 / 133

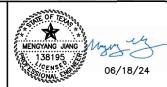


NO. REVISIONS DATE NAME

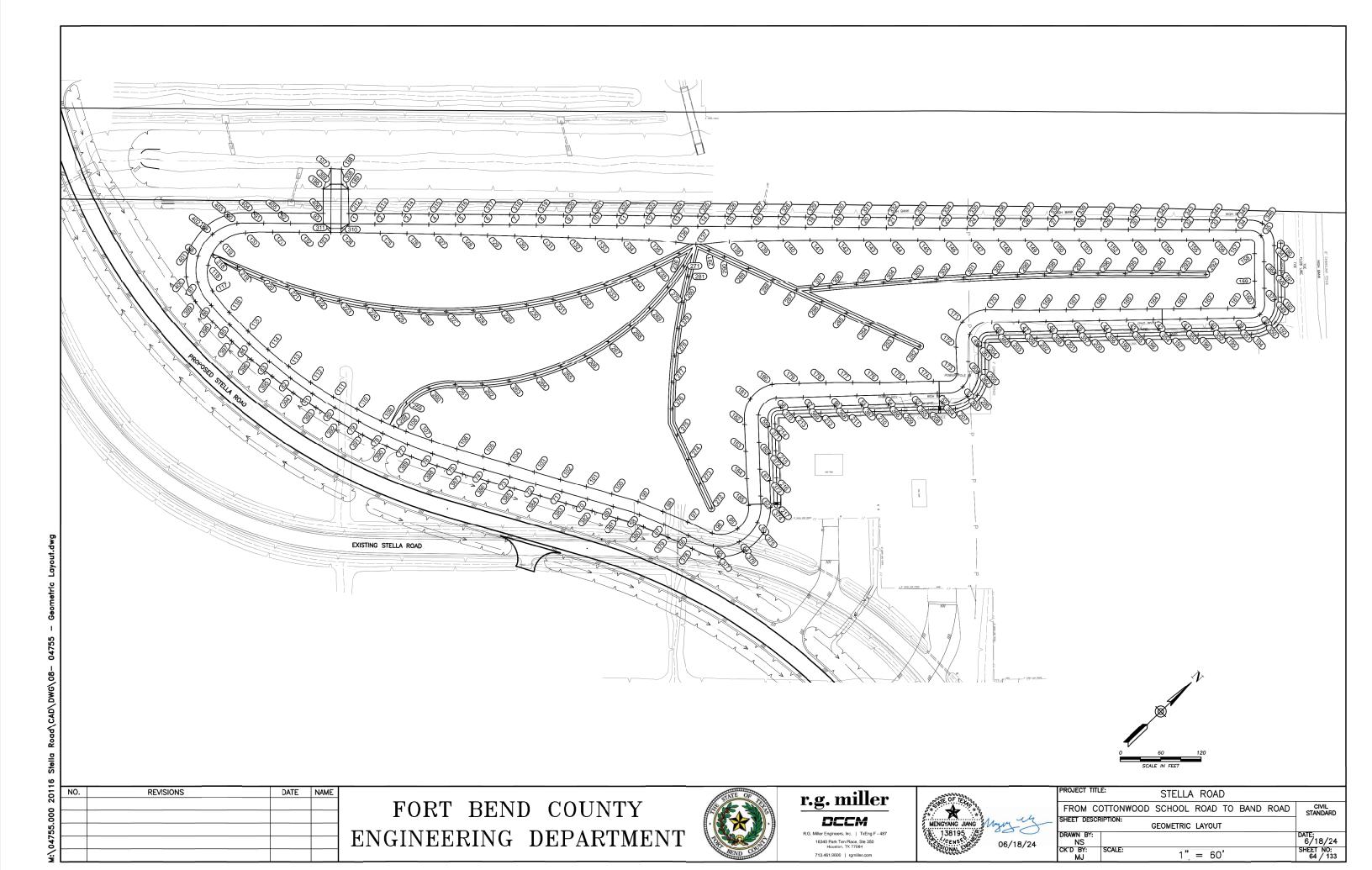
FORT BEND COUNTY ENGINEERING DEPARTMENT



r.g. miller



_			
	PROJECT TITL	^{E:} STELLA ROAD	
		OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
_	SHEET DESCR	RIPTION: PROPOSED DETENTION BASIN INFLOW P&P	
	DRAWN BY: NS		DATE: 6/18/24
	CK'D BY: MJ	1" = 40'	SHEET NO: 63 / 133



POINT NO.	ELEV.	NORTHING	EASTING	DES
1	95.90	13752698.93	2979777.03	WE
2	95.90	13752727.56	2979803.11	тс
3	95.90	13752757.13	2979830.05	TC
4	95.90	13752786.69	2979856.99	TC
•				-
5	95.90	13752816.26	2979883.93	TC
6	95.90	13752845.83	2979910.87	TC
7	95.90	13752875.40	2979937.81	TC
8	95.90	13752904.97	2979964.75	TC
9	95.90	13752934.53	2979991.69	TC
10	95.90	13752964.10	2980018.63	TC
11	95.90	13752993.67	2980045.57	TC
		13753023.24		-
12	95.90		2980072.50	TC
13	95.90	13753052.81	2980099.11	TC
14	95.90	13753082.38	2980126.38	TC
15	95.90	13753111.94	2980153.32	TC
16	95.90	13753141.51	2980180.26	TC
17	95.90	13753171.08	2980207.20	TC
18	95.90	13753200.65	2980234.14	TC
				-
19	95.90	13753230.22	2980261.08	TC
20	95.90	13753259.79	2980288.02	TC
21	95.90	13753289.35	2980314.96	TC
22	95.90	13753318.92	2980341.90	TC
23	95.90	13753348.49	2980368.84	TC
24	95.90	13753378.06	2980395.77	тс
25	95.90	13753407.63	2980422.71	TC
				⊢
26	95.90	13753437.20	2980449.65	TC
27	95.90	13753466.76	2980476.59	TC
28	95.90	13753496.33	2980503.53	тс
29	95.90	13753525.90	2980530.47	TC
30	95.90	13753555.47	2980557.41	TC
31	95.90	13753585.04	2980584.35	тс
32	95.90	13753614.60	2980611.29	TC
				_
33	95.90	13753644.17	2980638.23	TC
34	95.90	13753673.74	2980665.17	TC
35	95.90	13753696.44	2980696.14	TC
36	95.90	13753673.67	2980728.19	TC
37	95.90	13753647.23	2980758.20	TC
38	95.90	13753616.99	2980782.23	TC
39	95.90	13753584.46	2980760.48	TC
				_
40	95.90	13753554.23	2980734.29	TC
41	95.90	13753523.99	2980708.10	TC
42	95.90	13753493.76	2980681.91	TC
43	95.90	13753463.52	2980655.72	TC
44	95.90	13753433.29	2980629.53	TC
45	95.90	13753403.06	2980603.34	TC
46	95.90	13753372.82	2980577.15	TC
47	95.90	13753342.59	2980550.96	TC
48	95.90	13753312.36	2980524.77	TC
49	95.88	13753275.89	2980516.58	TC
50	95.90	13753247.55	2980544.45	тс
51	95.90	13753216.05	2980567.01	тс
52	95.90	13753181.71	2980549.20	тс
				_
53	95.90	13753151.20	2980523.33	TC
-				
54	95.90	13753120.70	2980497.46	TC
54 55	95.90 95.90	13753120.70 13753090.19	2980497.46 2980471.58	TC
				_
55	95.90	13753090.19	2980471.58	TC
55 56 57	95.90 95.90	13753090.19 13753059.69 13753029.18	2980471.58 2980445.71	TO
55 56 57 58	95.90 95.90 95.90 95.89	13753090.19 13753059.69 13753029.18 13752998.62	2980471.58 2980445.71 2980419.83 2980394.03	TO TO
55 56 57 58 59	95.90 95.90 95.90 95.89 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52	TC TC TC
55 56 57 58 59 60	95.90 95.90 95.90 95.89 95.90 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55 13752935.43	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52 2980425.80	TC TC TC TC
55 56 57 58 59	95.90 95.90 95.90 95.89 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52	TC TC TC
55 56 57 58 59 60	95.90 95.90 95.90 95.89 95.90 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55 13752935.43	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52 2980425.80	TC TC TC TC
55 56 57 58 59 60 61	95.90 95.90 95.89 95.90 95.90 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55 13752935.43 13752909.60	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52 2980425.80 2980456.34	TC TC TC TC
55 56 57 58 59 60 61 62	95.90 95.90 95.89 95.89 95.90 95.90 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55 13752935.43 13752909.60 13752883.77	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52 2980425.80 2980456.34 2980486.88	TC TC TC TC TC TC
55 56 57 58 59 60 61 62 63	95.90 95.90 95.90 95.89 95.90 95.90 95.90 95.90	13753090.19 13753059.69 13753029.18 13752998.62 13752961.55 13752935.43 13752909.60 1375283.77 13752854.04	2980471.58 2980445.71 2980419.83 2980394.03 2980395.52 2980425.80 2980456.34 2980486.88 2980512.98	TCC TCC TCC TCC TCC TCC TCC TCC TCC

		POINT TABLE					POINT TABLE		
NT NO.	ELEV.	NORTHING	EASTING	DESC.	POINT NO.	ELEV.	NORTHING	EASTING	C
1	95.90	13752698.93	2979777.03	WEIR	66	95.88	13752764.97	2980459.11	
2	95.90	13752727.56	2979803.11	тов	67	95.89	13752748.81	2980422.53	t
3	95.90	13752757.13	2979830.05	тов	68	95.88	13752731.21	2980386.61	t
4	95.90	13752786.69	2979856.99	TOB	69	95.90	13752710.77	2980352.26	+
					—	-			t
5	95.90	13752816.26	2979883.93	TOB	70	95.90	13752689.50	2980318.38	+
6	95.90	13752845.83	2979910.87	TOB	71	95.90	13752668.23	2980284.50	+
7	95.90	13752875.40	2979937.81	TOB	72	95.90	13752646.96	2980250.63	1
8	95.90	13752904.97	2979964.75	TOB	73	95.90	13752625.70	2980216.75	l
9	95.90	13752934.53	2979991.69	тов	74	95.90	13752604.43	2980182.87	Ī
10	95.90	13752964.10	2980018.63	TOB	75	95.90	13752583.17	2980148.99	Ī
11	95.90	13752993.67	2980045.57	тов	76	95.90	13752563.00	2980114.45	t
12	95.90	13753023.24	2980072.50	тов	77	95.90	13752544.62	2980078.93	t
						_			ł
13	95.90	13753052.81	2980099.11	TOB	78	95.90	13752528.06	2980042.52	+
14	95.90	13753082.38	2980126.38	тов	79	95.90	13752513.38	2980005.32	ļ
15	95.90	13753111.94	2980153.32	TOB	80	95.90	13752500.61	2979967.41	1
16	95.90	13753141.51	2980180.26	TOB	81	95.90	13752489.79	2979928.91	J
17	95.90	13753171.08	2980207.20	тов	82	95.90	13752480.94	2979889.91	İ
8	95.90	13753200.65	2980234.14	тов	83	95.90	13752474.08	2979850.50	İ
9	95.90	13753230.22	2980261.08	тов	84	95.90	13752469.24	2979810.80	t
						_			+
.0	95.90	13753259.79	2980288.02	TOB	85	95.90	13752466.42	2979770.91	
21	95.90	13753289.35	2980314.96	тов	86	95.90	13752465.63	2979730.92	+
2	95.90	13753318.92	2980341.90	TOB	87	95.90	13752472.97	2979692.03	1
23	95.90	13753348.49	2980368.84	TOB	88	95.90	13752499.71	2979663.02	
24	95.90	13753378.06	2980395.77	TOR	89	95.90	13752537.89	2979653.07	I
25	95.90	13753407.63	2980422.71	тов	90	95.90	13752575.39	2979665.36	Ī
26	95.90	13753437.20	2980449.65	тов	91	95.90	13752605.37	2979691.79	t
27	95.90	13753466.76	2980476.59	TOB	92	95.90	13752634.94	2979718.73	+
					93	-	13752670.85	2979751.45	+
28	95.90	13753496.33	2980503.53	тов		95.90			+
29	95.90	13753525.90	2980530.47	TOB	94	91.42	13752861.51	2980485.47	1
30	95.90	13753555.47	2980557.41	TOB	95	91.47	13752826.19	2980500.46	l
31	95.90	13753585.04	2980584.35	тов	96	91.36	13752793.60	2980479.96	J
32	95.90	13753614.60	2980611.29	тов	97	91.39	13752777.81	2980443.23	Ī
33	95.90	13753644.17	2980638.23	тов	98	91.59	13752760.59	2980407.13	İ
34	95.90	13753673.74	2980665.17	тов	99	91.59	13752742.70	2980371.36	t
						_			ł
5	95.90	13753696.44	2980696.14	TOB	100	91.62	13752721.64	2980337.37	ł
56	95.90	13753673.67	2980728.19	TOB	101	91.65	13752700.25	2980303.57	+
57	95.90	13753647.23	2980758.20	TOB	102	91.69	13752678.86	2980269.77	ļ
38	95.90	13753616.99	2980782.23	TOB	103	91.59	13752657.93	2980235.68	
39	95.90	13753584.46	2980760.48	тов	104	91.47	13752637.07	2980201.55	ľ
40	95.90	13753554.23	2980734.29	тов	105	91.39	13752616.08	2980167.50	ſ
41	95.90	13753523.99	2980708.10	тов	106	91.36	13752595.00	2980133.50	t
42	95.90	13753493.76	2980681.91	тов	107	91.36	13752575.16	2980098.78	t
43	95.90	13753463.52	2980655.72	TOB	108	91.67	13752575.10	2980063.64	t
_						-			+
44	95.90	13753433.29	2980629.53	TOB	109	92.00	13752538.77	2980027.57	+
45	95.90	13753403.06	2980603.34	TOB	110	92.30	13752523.42	2979990.64	1
46	95.90	13753372.82	2980577.15	TOB	111	92.59	13752510.11	2979952.93	1
47	95.90	13753342.59	2980550.96	тов	112	92.85	13752498.84	2979914.55	J
18	95.90	13753312.36	2980524.77	тов	113	92.90	13752490.40	2979875.47	I
·9	95.88	13753275.89	2980516.58	тов	114	92.84	13752484.42	2979835.92	t
50	95.90	13753247.55	2980544.45	тов	115	92.77	13752480.54	2979796.12	t
51	95.90	13753247.05	2980567.01	TOB	116	92.69	13752478.72	2979756.16	t
						_			ł
52	95.90	13753181.71	2980549.20	TOB	117	92.16	13752481.10	2979716.30	+
53	95.90	13753151.20	2980523.33	тов	118	91.41	13752502.10	2979683.31	1
14	95.90	13753120.70	2980497.46	TOB	119	91.57	13752539.33	2979670.49	l
55	95.90	13753090.19	2980471.58	TOB	120	92.33	13752575.96	2979684.32	ľ
56	95.90	13753059.69	2980445.71	тов	121	92.68	13752606.44	2979710.18	Ī
7	95.90	13753029.18	2980419.83	тов	122	92.71	13752636.08	2979737.05	t
8	95.89	13752998.62	2980394.03	тов	123	92.69	13752670.75	2979768.68	t
_						-			+
59	95.90	13752961.55	2980395.52	TOB	124	92.69	13752681.80	2979778.80	+
50	95.90	13752935.43	2980425.80	тов	125	92.60	13752724.51	2979818.16	1
	95.90	13752909.60	2980456.34	тов	126	92.52	13752753.83	2979845.36	1
61	95.90	13752883.77	2980486.88	тов	127	92.40	13752783.09	2979872.64	
61 62	95.90	13752854.04	2980512.98	тов	128	92.26	13752812.28	2979899.99	I
	95.90				129	92.10	13752841.41	2979927.40	Ť
52	95.90	13752814.94	2980516.53	TOB				2070027.40	
2		13752814.94 13752782.02	2980516.53 2980495.14	TOB	130	91.90	13752870.44	2979954.92	t

		POINT TABLE		
POINT NO.	ELEV.	NORTHING	EASTING	DESC
131	91.70	13752899.45	2979982.46	TOE
132	91.47	13752928.38	2980010.08	TOE
133	91.21	13752957.27	2980037.75	TOE
134	90.95	13752986.11	2980065.46	TOE
135	90.67	13753014.91	2980093.22	TOE
136	89.79	13753041.98	2980122.63	TOE
137	89.17	13753062.23	2980144.82	TOE
138	90.29	13753102.17	2980174.77	TOE
139	90.70	13753132.80	2980200.45	TOE
140	90.95	13753163.02	2980226.66	TOE
141	91.10	13753193.01	2980253.14	TOE
142	91.20	13753222.84	2980279.78	TOE
143	91.25	13753252.55	2980306.56	TOE
144	91.31	13753282.27	2980333.34	TOE
145	91.37	13753311.99	2980360.10	TOE
146	91.43	13753341.73	2980386.86	TOE
147	91.49	13753371.47	2980413.61	TOE
148	91.56	13753401.22	2980440.35	TOE
149	91.63	13753430.97	2980467.08	TOE
150	91.70	13753460.73	2980493.81	TOE
151	91.78	13753490.50	2980520.52	TOE
152	91.86	13753520.28	2980547.23	TOE
153	91.94	13753550.07	2980573.93	TOE
154	92.02	1.375.3579.86	2980600.61	TOF
155	92.11	13753609.67	2980627.30	TOE
156	92.19	13753639.45	2980654.00	TOE
157	92.40	13753669.57	2980680.32	TOE
158	92.46	13753671.23	2980710.13	TOE
159	92.40	13753644.63	2980740.00	TOE
160	92.54	13753617.51	2980768.76	TOE
161	92.27	13753587.09	2980743.55	TOE
162	92.17	13753557.14	2980717.05	TOE
163	92.09	13753527.10	2980690.63	TOE
164	92.01	13753497.07	2980664.21	TOE
165	91.94	13753467.04	2980637.79	TOE
166	91.85	13753437.03	2980611.34	TOE
167	91.77	13753407.02	2980584.89	TOE
168	91.68	13753377.02	2980558.44	TOE
169	91.44	13753347.41	2980531.55	TOE
170	91.13	13753317.91	2980504.53	TOE
1 71	91.02	13753280.29	2980494.99	TOE
172	91.27	13753247.78	2980516.52	TOE
173	91.62	13753221.95	2980547.05	TOE
174	91.44	13753189.11	2980532.11	TOE
175	91.42	13753158.65	2980506.18	TOE
176	91.39	13753128.25	2980480.20	TOE
177	91.07	13753098.57	2980453.38	TOE
178	90.84	13753068.67	2980426.81	TOE
179	90.80	13753038.28	2980400.81	TOE
180	90.77	13753007.50	2980375.30	TOE
181	90.78	13752969.02	2980369.31	TOE
182	90.85	13752937.32	2980392.29	TOE
183	90.93	13752911.73	2980423.03	TOE
184	91.06	13752886.30	2980453.90	TOE
185	91.42	13752861.60	2980485.36	TOE
186	95.82	13752705.50	2979713.25	WEIF
187	92.88	13752734.06	2979698.84	WEIF
188	92.82	13752744.72	2979709.42	WEIF
189	95.93	13752732.78	2979737.78	WEIF
190	95.00	13753710.26	2980724.48	SWAL
191	95.00	13753683.82	2980754.49	SWAL
192	95.00	13753657.38	2980784.51	SWAL
193	95.00	13753625.13	2980806.50	SWAL
194	95.00	13753587.77	2980796.42	SWAL

DOINT NO	G.C.	POINT T		DECC
POINT NO.	ELEV.	NORTHING	EASTING	DESC.
196	95.00	13753527.30	2980744.04	SWALE
197	95.00	13753497.07	2980717.85	SWALE
198	95.90	13753466.83	2980691.66	SWALE
199	95.90	13753436.60	2980665.47	SWALE
200	95.90	13753406.36	2980639.28	SWALE
201	95.90	13753376.13	2980613.09	SWALE
202	95.90	13753345.90	2980586.90	SWALE
203	95.90 95.90	13753315.66	2980560.71	SWALE
204	95.90 95.90	13753284.36	2980540.93	SWALE
		13753257.60	2980570.66	
206	95.00	13753224.18	2980591.11	SWALE
207	95.00	13753154.68	2980584.34	SWALE
209	95.90	13753154.68	2980533.19	SWALE
210	95.90	13753124.18	2980533.19	SWALE
		13753093.67		SWALE
211	95.90 95.90	13753063.17	2980481.44 2980455.57	SWALE
213	95.90	13753002.16	2980435.37	SWALE
214	95.90	13752971.61	2980429.70	SWALE
215	95.90	13752971.61	2980421.73	SWALE
216	95.90	13752945.78	2980452.27	SWALE
217	95.90	13752919.95	2980513.18	SWALE
217	95.90	13752594.27	2979677.26	PILOT CHANNEL
219	91.24	13752517.15	2979677.26	PILOT CHANNEL
220	91.03	13752549.56	2979714.74	PILOT CHANNEL
221	90.93	13752568.64	2979785.51	PILOT CHANNEL
222	90.80	13752589.55	2979819.60	PILOT CHANNEL
223	90.70	13752612.25	2979852.54	PILOT CHANNEL
224	90.59	13752636.67	2979884.21	PILOT CHANNEL
225	90.47	13752662.73	2979914.55	PILOT CHANNEL
226	90.37	13752690.37	2979943.45	PILOT CHANNEL
227	90.26	13752719.51	2979970.85	PILOT CHANNEL
228	90.15	13752750.06	2979996.66	PILOT CHANNEL
229	90.04	13752781.94	2980020.81	PILOT CHANNEL
230	89.92	13752815.06	2980043.23	PILOT CHANNEL
231	89.82	13752849.33	2980063.86	PILOT CHANNEL
232	89.71	13752884.64	2980082.64	PILOT CHANNEL
233	89.58	13752920.90	2980099.52	PILOT CHANNEL
234	89.49	13752958.01	2980114.44	PILOT CHANNEL
235	89.37	13752995.86	2980127.37	PILOT CHANNEL
236	89.25	13753034.34	2980138.27	PILOT CHANNEL
258	91.83	13752547.44	2980045.85	PILOT CHANNEL
259	91.24	13752584.75	2980032.39	PILOT CHANNEL
260	90.59	13752623.99	2980038.31	PILOT CHANNEL
261	90.28	13752656.82	2980060.52	PILOT CHANNEL
262	90.26	13752686.93	2980086.84	PILOT CHANNEL
263	90.23	13752719.58	2980109.90	PILOT CHANNEL
264	90.19	13752754.45	2980129.47	PILOT CHANNEL
265	90.13	13752791.15	2980145.34	PILOT CHANNEL
266	90.06	13752829.29	2980157.33	PILOT CHANNEL
267	89.98	13752868.47	2980165.31	PILOT CHANNEL
268	89.90	13752908.26	2980169.21	PILOT CHANNEL
269	89.80	13752948.24	2980168.97	PILOT CHANNEL
270	89.69	13752987.99	2980164.61	PILOT CHANNEL
271	89.53	13753027.07	2980156.17	PILOT CHANNEL
272	90.04	13752817.23	2980451.54	PILOT CHANNEL
273	89.96	13752828.90	2980413.28	PILOT CHANNEL
274	89.88	13752840.56	2980375.02	PILOT CHANNEL
275	89.80	13752852.22	2980336.76	PILOT CHANNEL
276	90.72	13752871.69	2980302.87	PILOT CHANNEL
	90.79	13752899.39	2980274.02	PILOT CHANNEL
277		13752928.39	2980246.48	PILOT CHANNEL
277 278	90.71			
	90.71	13752958.63	2980220.30	PILOT CHANNEL
278	-	13752958.63 13752990.04	2980220.30 2980195.53	PILOT CHANNEL

POINT NO.	ELEV.	POINT TO	EASTING	DESC.
282	89.90	13753208.97	2980479.72	PILOT CHANN
283	89.82	13753192.92	2980443.09	PILOT CHANN
284	89.74	13753176.87	2980406.45	PILOT CHANN
285	89.66	13753160.81	2980369.81	PILOT CHANN
286	89.58	13753144.76	2980333.17	PILOT CHANN
287	89.50	13753128.19	2980295.36	PILOT CHANN
288	89.42	13753112.65	2980259.90	PILOT CHANN
289	89.34	13753096.60	2980223.26	PILOT CHANN
290	89.26	13753080.55	2980186.62	PILOT CHANN
291	89.21	13753070.78	2980164.33	PILOT CHANN
292	90.72	13753596.68	2980683.97	PILOT CHANN
293	90.64	13753567.36	2980656.77	PILOT CHANN
294	90.57	13753537.83	2980629.79	PILOT CHANN
295	90.50	13753508.08	2980603.04	PILOT CHANN
296	90.40	13753478.13	2980576.53	PILOT CHANN
297	90.34	13753447.98	2980550.25	PILOT CHANN
298	90.25	13753417.62	2980524.20	PILOT CHANN
299	90.17	13753387.05	2980498.40	PILOT CHANN
300	90.10	13753356.29	2980472.83	PILOT CHANN
301	90.00	13753325.33	2980447.51	PILOT CHANN
302	89.93	13753294.17	2980422.43	PILOT CHANN
303	89.86	13753262.81	2980397.59	PILOT CHANN
304	89.76	13753231.26	2980373.00	PILOT CHANN
305	89.70	13753199.52	2980348.66	PILOT CHANN
306	89.61	13753167.60	2980324.56	PILOT CHANN
307	89.56	13753145.55	2980308.15	PILOT CHANN
308	92.80	13752730.12	2979725.55	WEIR
309	92.84	13752718.95	2979715.55	WEIR
310	92.70	13752685.82	2979774.37	WEIR
311	92.70	13752674.71	2979764.31	WEIR
312	95.93	13752708.73	2979765.67	GB
313	95.60	13752737.66	2979792.02	GB
314	95.60	13752767.23	2979818.96	GB
315	95.60	13752796.80	2979845.90	GB
316	95.60	13752826.36	2979872.84	GB
317	95.60	13752855.93	2979899.78	GB
318	95.60	13752885.50	2979926.72	GB
319	95.60	13752915.07	2979953.66	GB
320	95.60	13752944.64	2979980.60	GB
321	95.60	13752974.21	2980007.54	GB
322	95.60	13753003.77	2980034.48	GB
323	95.60	13753033.34	2980061.42	GB
324	95.60	13753062.91	2980088.36	GB
325	95.60	13753092.48	2980115.29	GB
326	95.60	13753122.05	2980142.23	GB
327	95.60	13753151.61	2980169.17	GB
328	95.60	13753181.18	2980196.11	GB
329	95.60	13753210.75	2980223.05	GB
330	95.60	13753240.32	2980249.99	GB
331	95.60	13753269.89	2980276.93	GB
332	95.60	13753299.46	2980303.87	GB
333	95.60	13753329.02	2980330.81	GB
334	95.60	13753358.59	2980357.75	GB
335	95.60	13753388.16	2980384.69	GB
336	95.60	13753417.73	2980411.63	GB
337	95.60	13753447.30	2980438.56	GB
338	95.60	13753476.87	2980465.50	GB
339	95.60	13753506.43	2980492.44	GB
340	95.60	13753536.00	2980519.38	GB
341	95.60	13753565.57	2980546.32	GB
- F1	95.60	13753595.14	2980573.26	GB
342	30.00	.0,00090.14	23003/3.20	
342 343	95 60	13753624 71	2980600 20	GR
343	95.60 95.60	13753624.71	2980600.20	GB
	95.60 95.60 95.60	13753624.71 13753654.28 13753683.84	2980600.20 2980627.14 2980654.08	GB GB

		POINT TABLE		
POINT NO.	ELEV.	NORTHING	EASTING	DESC.
347	95.60	13753700.34	2980720.61	GB
348	95.60	13753673.90	2980750.63	GB
349	95.60	13753647.45	2980780.64	GB
350	95.60	13753613.04	2980797.00	GB
351	95.60	13753579.74	2980776.23	GB
352	95.60	13753549.50	2980750.04	GB
353	95.90	13753519.27	2980723.85	GB
354	95.90	13753489.04	2980697.66	GB
355	95.90	13753458.80	2980671.47	GB
356	95.90	13753428.57	2980645.28	GB
357	95.90	13753398.34	2980619.09	GB
358	95.90	13753368.10	2980592.90	GB
	95.90	13753337.87		
359			2980566.71	CB
360	95.90	13753307.64	2980540.52	GB
361	95.90	13753274.41	2980537.04	GB
362	95.90	13753247.64	2980566.77	GB
363	95.60	13753212.10	2980582.06	GB
364	95.60	13753177.03	2980564.91	GB
365	95.90	13753146.53	2980539.03	GB
366	95.90	13753116.02	2980513.16	GB
367	95.90	13753085.52	2980487.29	GB
368	95.90	13753055.01	2980461.41	GB
369	95.90	13753024.51	2980435.54	GB
370	95.90	13752994.00	2980409.66	GB
371	95.90	13752961.75	2980417.91	GB
372	95.90	13752935.91	2980448.46	GB
373	95.90	13752910.08	2980479.00	GB
374	95.90	13752884.19	2980509.48	GB
375	95.60	13752850.55	2980530.24	GB
376	95.60	13752811.03	2980531.01	GB
377	95.60	13752776.60	2980511.59	GB
378	95.60	13752756.32	2980477.54	GB
379	95.60	13752740.65	2980440.74	GB
380	95.60	13752723.51	2980404.60	GB
381	95.60	13752704.02	2980369.71	GB
382	95.60	13752682.75	2980335.84	GB
383	95.60	13752661.48	2980301.96	GB
384	95.60	13752640.21	2980268.08	GB
385	95.60	13752618.94	2980234.20	GB
386	95.60	13752597.67	2980200.33	GB
387	95.60	13752576.41	2980166.45	GB
388	95.60	13752575.75	2980132.20	GB
389			2980096.97	GB
	95.60	13752536.82		
390	95.60	13752519.67	2980060.84	GB
391	95.60	13752504.34	2980023.89	GB
392	95.60	13752490.88	2979986.23	GB
393	95.60	13752479.31	2979947.95	GB
394	95.60	13752469.67	2979909.13	GB
395	95.60	13752461.97	2979869.88	GB
396	95.60	13752456.25	2979830.30	GB
397	95.60	13752452.51	2979790.48	GB
398	95.60	13752450.76	2979750.52	GB
399	95.60	13752451.65	2979710.57	GB
400	95.60	13752466.44	2979673.80	GB
401	95.60	13752496.30	2979647.73	GB
402	95.60	13752534.73	2979638.04	GB
403	95.60	13752573.37	2979646.83	GB
404		13752573.37	2979671.11	
	95.60			GB
405	95.60	13752634.51	2979698.05	GB
406	95.93	13752680.93	2979740.34	GB

NO.	REVISIONS	DATE	NAME

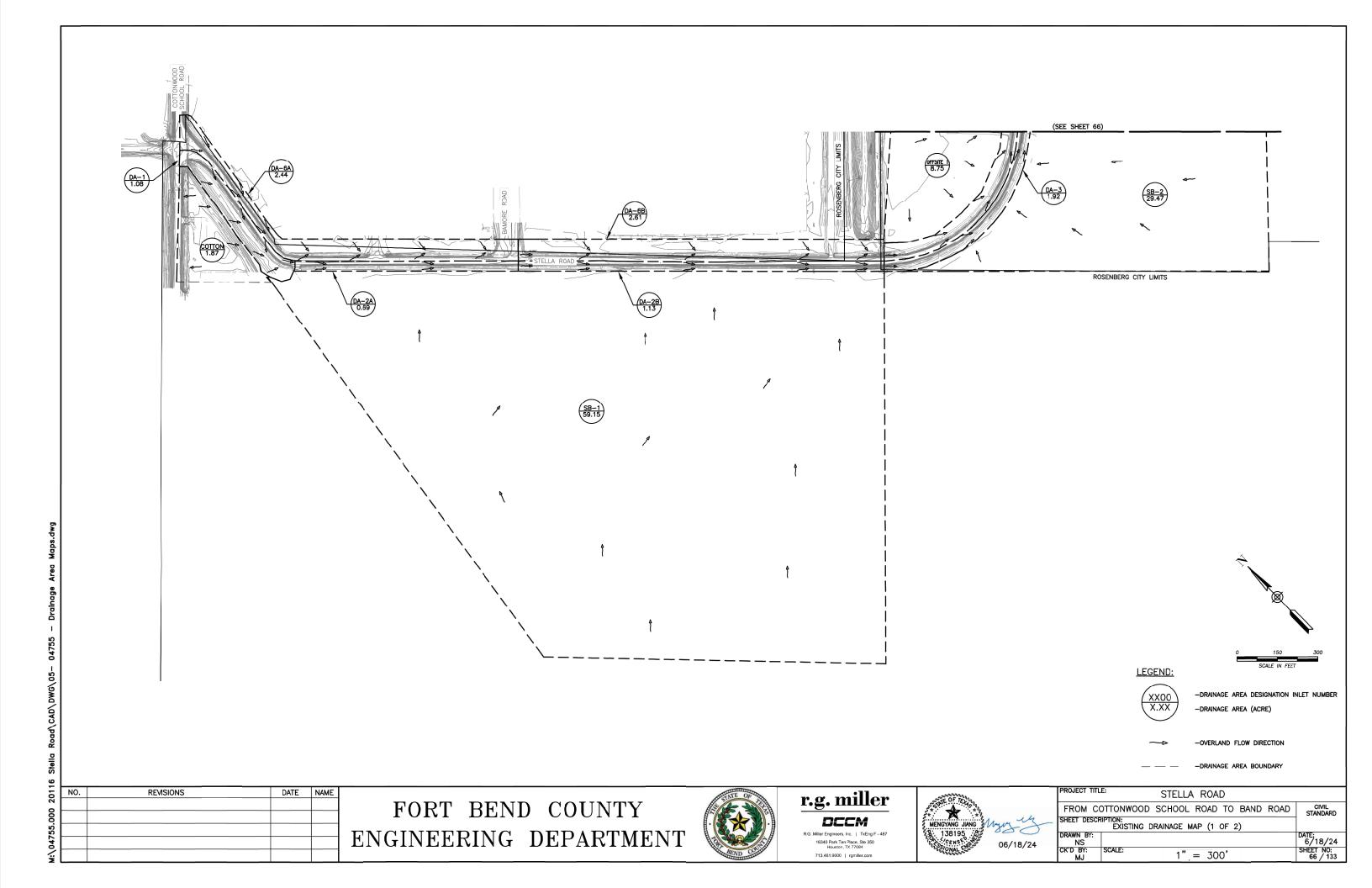


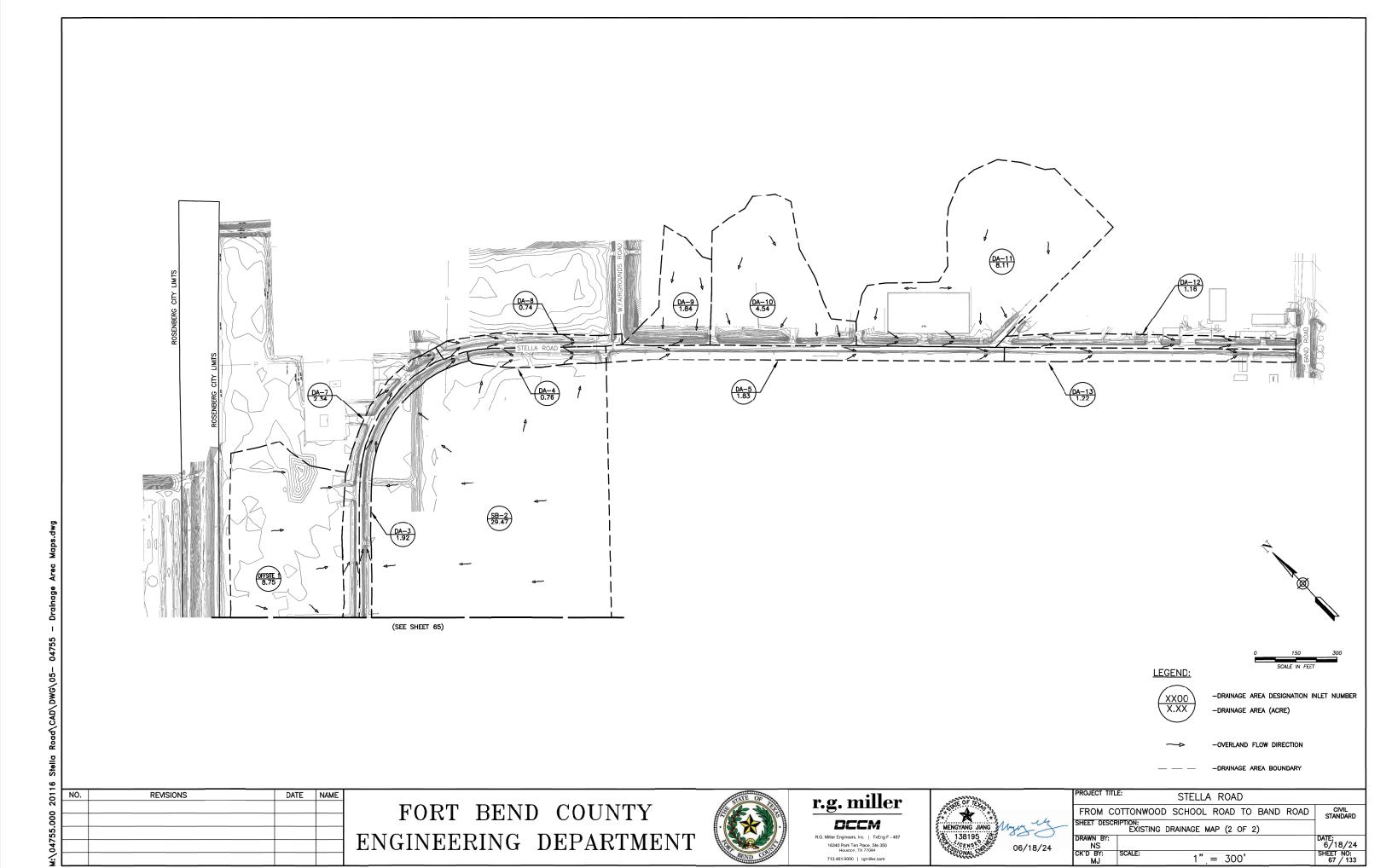


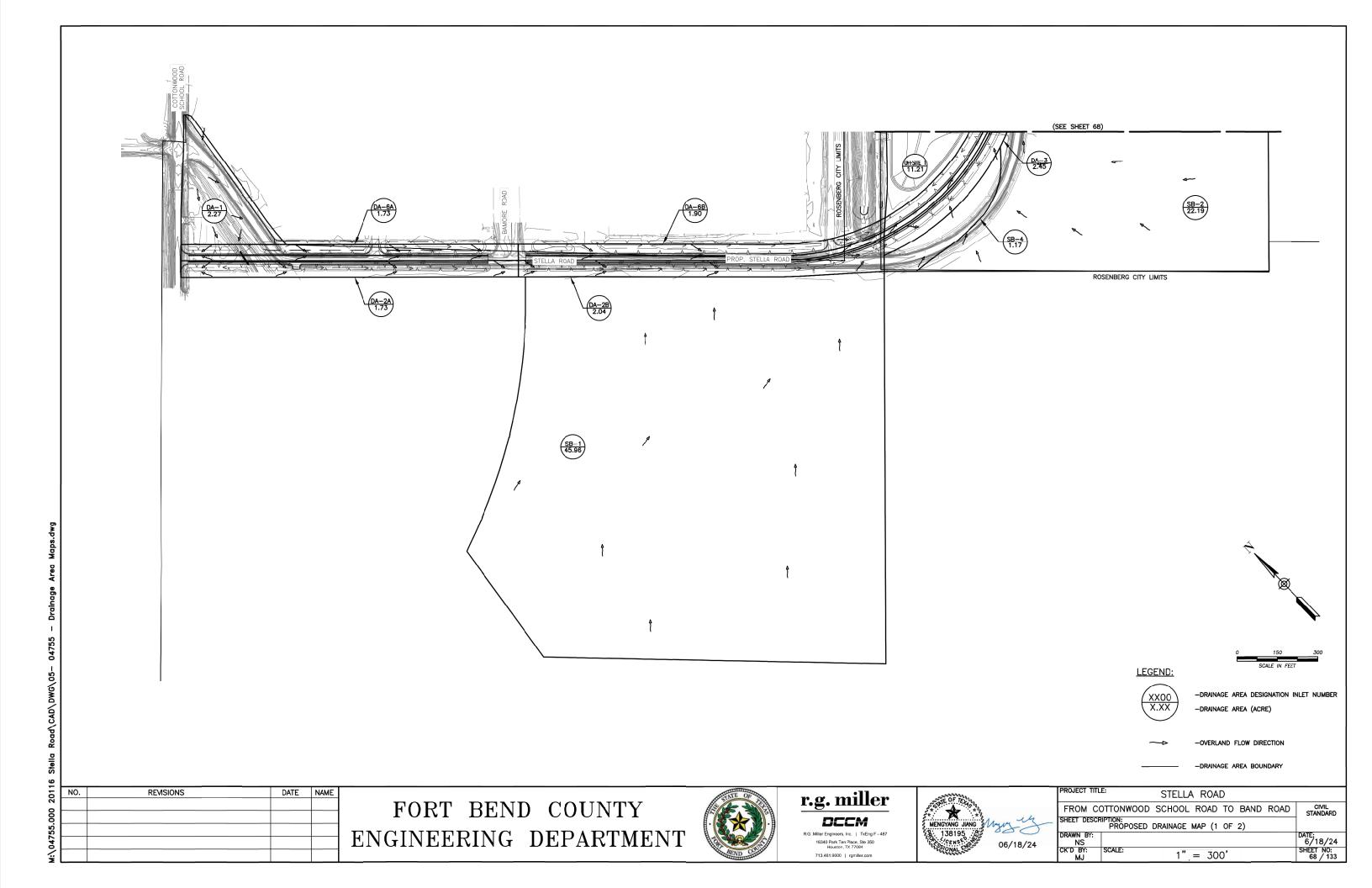
16340 Park Ten Place, Ste 350 Houston, TX 77084

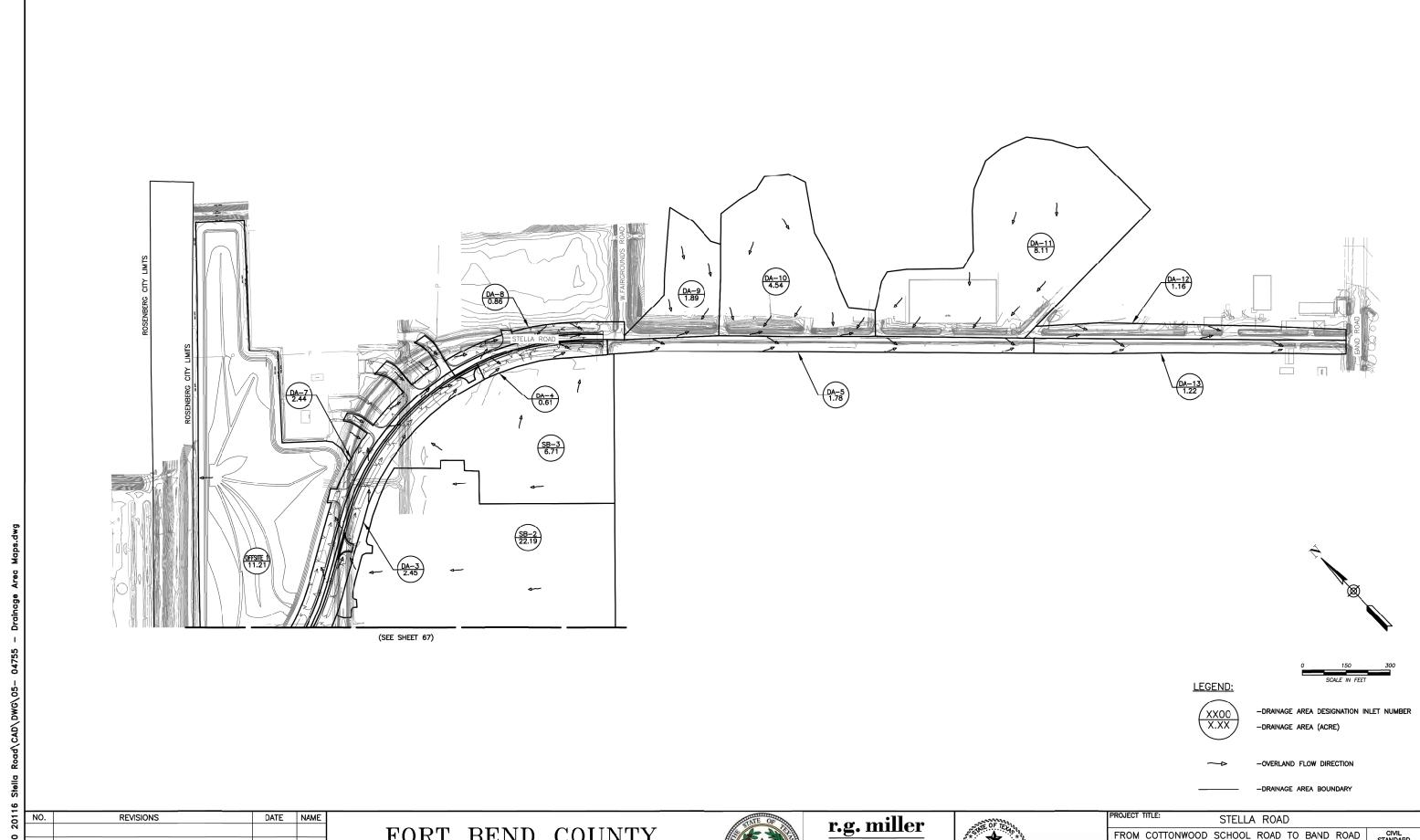


PROJECT TITL	E: STELLA ROAD	
	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: GEOMETRIC LAYOUT POINT DATA	
DRAWN BY: NS		DATE: 6/18/24
CK'D BY: MJ	SCALE: N/A	SHEET NO: 65 / 133















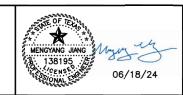
PROJECT TITL	^{E:} STELLA ROAD	
	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	RIPTION: PROPOSED DRAINAGE MAP (2 OF 2)	
DRAWN BY: NS		DATE: 6/18/24
CK'D BY: MJ	1" = 300'	SHEET NO: 69 / 133

							TIME OF		RATION CA Stella Road				AND METH	OD								
Sub	o-Area	_		DA 1	DA 2A	DA 2B	DA 3	DA 4	DA 5	DA 6A	DA 6B	DA 7	DA 8	DA 9	DA 10	DA 11	DA 12	DA 13	SB-1	SB-2	Cotton	Offsite 1
Basin Drainage Area	J-Area			DA_I	DA_ZA	DA_ZB	DA_3	DA_4	DA_3	DA_6A	DA_0B	DA_/	DA_8	DA_9	DA_10	DA_II	UA_12	DA_13	3D-1	3B-Z	Cotton	Offsite_1
Basiii Bi ailiage Alea	_	acres		1.08	0.69	1.13	1.92	0.76	1.83	2.44	2.61	2.34	0.74	1.84	4.54	8.11	1.16	1.22	59.15	29.47	1.87	8.75
Drainage Area		sq. mi.		0.002	0.001	0.002	0.003	0.001	0.003	0.004	0.004	0.004	0.001	0.003	0.007	0.013	0.002	0.002	0.092	0.046	0.003	0.014
Impervious Cover		39. 1111.		0.002	0.001	0.002	0.003	0.001	0.003	0.004	0.004	0.004	0.001	0.003	0.007	0.013	0.002	0.002	0.032	0.040	0.003	0.014
mpervious cover																						_
Description		с	,							Are	a of Land (d	icres) Beloi	nging to Ea	ch Develop	ment Cate	gory						
Road/Lake	W	0.85	100%	0.48	0.53	0.87	1.53	0.44	0.81	0.86	0.87	1.54	0.59	0.46	0.65	0.80	0.74	0.52	0.00	1.04	0.00	0.00
Commercial	С	0.75	85%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.38	0.00	7.30	0.20	0.03	0.00	0.00	0.25	0.00
Industrial	I	0.6	72%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.89	0.00	0.00	0.00	0.00	10.90	0.00	0.00
Single-Family	SF	0.5	66%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Residential larger Lot	LR	0.3	25%	0.00	0.00	0.00	0.00	0.00	0.00	0.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Open Space/Undeveloped	OS	0.3	0%	0.60	0.16	0.26	0.40	0.33	1.02	0.96	1.74	0.80	0.15	0.00	0.00	0.80	0.21	0.67	59.15	17.53	1.62	8.75
Runoff Coefficient				0.54	0.72	0.72	0.74	0.62	0.54	0.49	0.48	0.66	0.74	0.78	0.64	0.79	0.73	0.55	0.3	0.4	0.4	0.30
Impervious Area		acres		0.5	0.5	0.9	1.5	0.4	0.8	1.0	0.9	1.5	0.6	1.6	3.5	7.0	0.9	0.5	0.0	8.9	0.2	0.0
Impervious Cover		%		44.4%	76.4%	76.7%	79.7%	57.9%	44.3%	41.6%	33.3%	65.8%	79.7%	88.8%	76.0%	85.4%	78.4%	44.7%	0.0%	30.2%	11.4%	0.0%
										ne of Conce			_									
21.1					200				s Method (-										200.0
Distance		feet		50.0	30.0	30.0	0.0	50.0	0.0	40.0	65.0	20.0	30.0	0.0	0.0	0.0	0.0	0.0	300.0	300.0	77.8	300.0
Slope		percent		0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.07	0.01	0.05	0.05
Velocity		ft/sec		0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.18	0.08	0.16	0.16
Travel Time		minutes		3.78	2.27	2.27	0.00	3.78	0.00 Curve F - S	3.02	4.91	1.51	2.27	0.00	0.00	0.00	0.00	0.00	27.09	59.63	8.32	32.06
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2776.0	1222.0	0.0	653.0
Distance				0.00	0.11	0.11	0.00	0.00	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.01	0.07	0.05
Slope	_	percent ft/sec		0.00	0.11	0.11	0.00	0.00	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.01	0.07	0.03
Velocity Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	118.19	115.24	0.00	32.95
Traver Time		minutes		0.00	0.00	0.00			od Curve G		eas (Sheet		Jpland Gull		0.00	0.00	0.00	0.00	110.13	113.24	0.00	32.93
Distance		feet		50	0.0	0.0	10.0	25.0	0.0	0.0	0.0	25.0	25.0	341.0	370.0	233.0	0.0	0.0	0.0	0.0	0.0	0.0
Slope	_	percent		0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
Velocity		ft/sec		0.69	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51
Travel Time		minutes		1.22	0.00	0.00	0.32	0.81	0.00	0.00	0.00	0.81	0.81	11.06	12.00	7.56	0.00	0.00	0.00	0.00	0.00	0.00
											/Roadside											
Distance		feet		0.0	0.0	0.0	46.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Velocity		ft/sec		3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00
Travel Time		minutes		0.00	0.00	0.00	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
		_							Flo	w in Roads	ide Ditch							_	-			•
Distance		feet		689.0	820.0	1360.0	1728.0	550.0	1488.0	1400.0	1260.0	1047.0	712.0	162.0	506.0	1291.0	1030.0	1059.0	1879.0	712.0	270.0	0.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Travel Time		minutes		7.66	9.11	15.11	19.20	6.11	16.53	15.56	14.00	11.63	7.91	1.80	5.62	14.34	11.44	11.77	20.88	7.91	3.00	0.00
									Flo	w in Chan	nel/Ditch											
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
TC			minutes	12.65	11.38	17.38	19.78	10.70	16.53	18.58	18.91	13.95	10.99	12.86	17.62	21.90	11.44	11.77	166.15	182.79	11.32	65.02
TC			hours	0.21	0.19	0.29	0.33	0.18	0.28	0.31	0.32	0.23	0.18	0.21	0.29	0.36	0.19	0.20	2.77	3.05	0.19	1.08

NO.	REVISIONS	DATE	NAME
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PROJECT TITL	E: STELLA ROAD	•
	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: EXISTING DRAINAGE CALCULATIONS	
DRAWN BY: NS		DATE: 6/18/24
CK'D BY: MJ	SCALE: N/A	SHEET NO: 70 / 133

								TIME OF C	ONCENTRA	TION CALC	ULATIONS	USING THE	SCS UPLAI	ND METHO	D								
									Ste	ella Road &	Sheriff's T	raining Fac	ility										
Sub	-Area			DA_1	DA_2A	DA_2B	DA_3	DA_4	DA_5	DA_6A	DA_6B	DA_7	DA_8	DA_9	DA_10	DA_11	DA_12	DA_13	Offsite_1	SB-1	SB-2	SB-3	SB-4
										Basi	n Drainage	Area											
		acres		2.27	1.73	2.04	2.45	0.61	1.78	1.73	1.90	2.44	0.86	1.89	4.54	8.11	1.16	1.22	11.21	45.95	22.19	6.71	1.17
Drainage Area		sq. mi.		0.004	0.003	0.003	0.004	0.001	0.003	0.003	0.003	0.004	0.001	0.003	0.007	0.013	0.002	0.002	0.018	0.072	0.035	0.010	0.002
										Im	pervious C	over											
Description		с	,								Area o	f Land (acr	es) Belongi	ing to Each	Developm	ent Catego	ery						
Road/Lake	W	0.85	100%	0.00	1.21	1.30	1.61	0.43	0.81	1.46	1.90	1.74	0.58	0.46	0.65	0.80	0.74	0.52	9.67	0.00	0.00	0.00	0.00
Commercial	С	0.75	85%	0.21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.38	0.00	7.30	0.20	0.03	0.00	0.00	22.19	0.00	0.00
Industrial	- 1	0.6	72%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.89	0.00	0.00	0.00	0.00	0.00	0.00	5.94	0.00
Single-Family	SF	0.5	66%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Residential larger Lot	LR	0.3	25%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Open Space/Undeveloped	OS	0.3	0%	2.06	0.52	0.74	0.84	0.18	0.97	0.27	0.00	0.70	0.28	0.05	0.00	0.80	0.21	0.67	1.54	45.95	0.00	0.77	1.17
Runoff Coefficient	- 03	0.5	U70	0.3	0.52	0.74	0.64	0.18	0.97	0.27	0.00	0.70	0.28	0.05	0.6	0.80	0.21	0.67	0.8	0.3	0.00	0.77	0.3
Impervious Area		acres		0.2	1.2	1.3	1.6	0.4	0.8	1.5	1.9	1.7	0.6	1.6	3.5	7.0	0.9	0.5	9.7	0.0	18.9	4.3	0.0
Impervious Cover		%		7.9%	69.9%	63.7%	65.7%	70.5%	45.5%	84.4%	100.0%	71.3%	67.4%	86.4%	76.0%	86.4%	78.4%	44.7%	86.3%	0.0%	85.0%	63.7%	0.0%
								-	11-1		of Concen		· · ·										
-1.7										Method Cur				Areas									
Distance		feet		77.8	60.0	60.0	60.0	60.0	0.0	60.0	60.0	60.0	60.0			0.0	0.0	0.0	300.0	300.0	60.0	300.0	100.0
Slope		percent		0.05	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.07	0.10	0.10	0.10
Velocity		ft/sec		0.16	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.18	0.22	0.22	0.22
Travel Time		minutes		8.32	4.53	4.53	4.53	4.53	0.00	4.53	4.53	4.53	4.53	0.00	0.00	0.00	0.00	0.00	22.65	27.09	4.53	22.65	7.55
							SC	S Uplands	Method Cu	ırve F - Sha	llow Conce	entrated Flo	ow in Grass	sed Waterv	vay								
Distance		feet		521.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	140.0	2224.0	0.0	0.0	0.0
Slope		percent		0.05	0.11	0.11	0.00	0.00	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.10	0.07	0.00	0.00	0.00
Velocity		ft/sec		0.33	0.49	0.49	0.00	0.00	0.47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.47	0.39	0.00	0.00	0.00
Travel Time		minutes		26.29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.98	94.69	0.00	0.00	0.00
									ds Method	Curve G - F	aved Area			land Gullie									
Distance		feet		50	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	341.0	370.0	233.0	0.0	0.0	0.0	0.0	300.0	0.0	0.0
Slope		percent		0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
Velocity		ft/sec		0.69	0.51	0.12	0.51	0.12	0.51	0.51	0.12	0.51	0.12	0.51	0.12	0.12	0.12	0.51	0.12	0.51	0.12	0.12	0.51
Travel Time		minutes		1.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	11.06	12.00	7.56	0.00	0.00	0.00	0.00	9.73	0.00	0.00
Havel fille		minutes		1.22	0.00	0.00	0.00	0.00		low in Stor				11.00	12.00	7.30	0.00	0.00	0.00	0.00	3.73	0.00	0.00
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	738.0	0.0	0.0
		ft/sec		3.00	3.00	3.00			3.00		3.00		3.00	3.00	3.00			3.00	3.00		3.00	3.00	3.00
Velocity							3.00	3.00		3.00		3.00				3.00	3.00		_	3.00			
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.10	0.00	0.00
											in Roadsid												
Distance		feet		689.0	1253.0	1359.0	1786.0	446.0	1488.0	1253.0	1359.0	1563.0	627.0	162.0	506.0	1291.0	1030.0	1059.0	450.0	0.0	1732.0	906.0	500.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Travel Time		minutes		7.66	13.92	15.10	19.84	4.96	16.53	13.92	15.10	17.37	6.97	1.80	5.62	14.34	11.44	11.77	5.00	0.00	19.24	10.07	5.56
										Flow	in Channe	I/Ditch											
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	80.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.89
TC			minutes	43.48	18.45	19.63	24.38	9.49	16.53	18.45	19.63	21.90	11.50	12.86	17.62	21.90	11.44	11.77	32.63	121.77	37.60	32.72	14.00
TC			hours	0.72	0.31	0.33	0.41	0.16	0.28	0.31	0.33	0.36	0.19	0.21	0.29	0.36	0.19	0.20	0.54	2.03	0.63	0.55	0.23
			urs		2,00	0.00					5.55		0.20		5.25	2.00	0.20		5.54		0.103	5,55	

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PROJECT TIT	LE:	STELLA	ROA)			
	OTTONWOOD	SCHOOL	ROAD	ТО	BAND	ROAD	CIVIL STANDARD
SHEET DESC		DRAINAGE (CALCULA	ATION	IS		
DRAWN BY: NS							DATE: 6/18/24
CK'D BY: MJ	SCALE:	N/	Ά				SHEET NO: 71 / 133

 Name
 Node Name 0
 ft
 Elevation ft
 User Innovation
 Open From Node Name 0
 ft
 Elevation ft
 Hydrograph
 Ponding Type

 D1_US
 100-Year Storm
 D1_US
 98.33
 94.82
 0.25
 Allowed

 D1_DS
 100-Year Storm
 D1_DS
 98.11
 93.99
 0.25
 Allowed

 D2_US
 100-Year Storm
 D2_US
 98.11
 93.89
 0.17
 Allowed

 D2_DS
 100-rear Storm
 D2_DS
 98.11
 93.89

 D2_DS
 0.00 Year Storm
 D2_DS
 66.8
 91.2

 D131_US
 100-rear Storm
 D131_US
 96.8
 91.1

 Out_1
 100-rear Storm
 Out_1
 195.269
 89.274

 D3_DS
 100-rear Storm
 D4_US
 96.4
 92.7

 D4_US
 100-rear Storm
 D4_US
 96.3
 92.6

 D4_LUS
 100-rear Storm
 D4_US
 96.6
 92.4

 D9.1_US
 100-rear Storm
 D4.1_US
 96.6
 92.4

 D9.1_US
 100-rear Storm
 D4.1_US
 96.7
 91.31

 D9.1_US
 100-rear Storm
 D4.1_US
 96.7
 91.31

 D9.1_US
 100-rear Storm
 D8.0
 96.7
 91.31

 D6_DS
 100-Year Storm
 D6_DS
 96.5
 92.58

 D/_US
 100-Year Storm
 D/_US
 96.32
 92.73

 D16.1_US
 100-Year Storm
 D16.1_US
 96.6
 91.2

 D16.2_US
 96.6
 91.2
 96.6
 91.2
 D16.1_US 100-Year Storm D16.1_US 96.6 91.2 D16.2_DS 100-Year Storm D16.2_DS 95.39 91.1

> 100-Year Storm
> D12_DS
> 96.3
>
>
> 100-Year Storm
> D13_US
> 96.4
>
>
> 100-Year Storm
> D13_DS
> 96.8
> 100-Year Storm D14_US

D15_US 100-Year Storm D16_US 96.68 93.17
D16_US 100-Year Storm D16_US 96.7 91.72
D16_DS 100-Year Storm D16_DS 96.517 91.21
D17_US 100-Year Storm D16_DS 96.7 91.41
D17_US 100-Year Storm D17_US 96.7 91.41
D17_US 100-Year Storm D17_US 96.7 91.41 100-Year Storm D17_DS 96.6
 D18_US
 100-Year Storm
 D18_US
 96.5

 D18_DS
 100-Year Storm
 D18_DS
 96.6
 D19_US 100-Year Storm D19_US 96.31
 Out3.1
 100-Year Storm
 Out3.1
 95.407
 91.93

 Out_3
 100-Year Storm
 Out_3
 96.381
 90.113

 D14_DS
 100 Vear Storm
 D14_DS
 06.78
 02.27
 0.17

 D15_DS
 100 Vear Storm
 D15_DS
 96.7
 91.82

 D13_US.1
 100 Vear Storm
 D13_US.1
 96.7
 91.47

 D2_US.1
 100-Year Storm
 D2_US.1
 96.5
 91.37
 0.25

 D4.1_US.1
 100-Year Storm
 D4.1_US.1
 96.6
 91.44
 0.17

 Basin1_ptt2
 100-Year Storm
 Basin1_ptt2
 96
 89.2
 0.17

 Out_1.1
 100-Year Storm
 Out_1.1
 99
 08.93

Allowed

Allowed Allowed

Allowed Allowed Allowed

Allowed Allowed Allowed

Allowed

 Dashing Diput
 Loo-real storm
 Out_1.1
 90
 63-2

 Out_1.1
 100-real storm
 Out_1.1
 95
 60-93

 Out_1.2
 100-rear storm
 Out_1.1
 95.146
 89-15

 D7.1_US
 100-rear storm
 07.1_US
 95.725
 91.5

 D7_DS
 100-rear storm
 D7_DS
 96
 91.67

 D8_US
 100-rear storm
 D8_US
 96
 91.52

 D8_DS
 100-rear storm
 D8_DS
 95.949
 90.85

 Out_4
 100-rear storm
 Out_4
 95.838
 91.39

 Out_4
 100-rear storm
 Out_4
 95.838
 91.39

 Out_4
 100-rear storm
 D8_DS_0
 96.00
 91.14

 D26_US
 100-Year Storm
 D26_US
 95.09
 94.114

 D26.1_US
 100-Year Storm
 D26.1_US
 95.911
 89.26

 DZ7_US
 100-Year Storm
 DZ7_US
 95.09
 93.435

 DZ7_DS
 100-Year Storm
 DZ7_DS
 95
 92.4

 D28.1_US
 100-Year Storm
 D28.1_US
 95.971
 92.437
 0.17

Node503 100-Year Storm Node651 95.49 88.2

Node652 100-Year Storm Node652 93.38 89.1

Node652 100-Year Storm Node653 92.94 89.44 0.17

DV2N 100-Year Storm Node653 92.94 89.44 0.17

DW2N US 100-Year Storm DV2N 15.1 95.929 92.579 0.17

DW2N US 100-Year Storm DW2N US 98.81 95.3 0.17

 DW2N_US.1
 100 Year Storm
 DW2N_US.1
 99.36
 95.45
 0.17
 None

 D12_US.1
 100-Year Storm
 D12_US.1
 97.48
 93.68
 0
 Allowed

 D2B_US.1
 100-Year Storm
 D2B_US.1
 97.48
 93.58
 Allowed

DW1N_Out | 100-Year Storm | DW1N_Out | 100.5 | 95.07

 100-Year Storm
 D26_DS

 100-Year Storm
 D25_US

 D25_DS
 100-Year Storm
 D25_DS
 95.91

 D24_US
 100-Year Storm
 D24_US
 95.769

 D24_DS
 100-Year Storm
 D24_DS
 96.01

 D23_US
 100-Year Storm
 D23_US
 96.03

D23_US 100-rear storm D23_US 96.03
D23_US 100-rear storm D22_US 96.096
D22_US 100-Year Storm D22_US 95.94
D22_US 100-Year Storm D22_US 96.11
D21_US 100-Year Storm D21_US 97.803
D21_US 100-Year Storm D21_US 96.59
D20_US 100-Year Storm D21_US 96.59

 D28_DS
 100-Year Storm
 D28_DS
 95,22

 D29_US
 100-Year Storm
 D29_US
 95,213

 D29_US
 100-Year Storm
 D29_US
 95,213

 D29_US
 100-Year Storm
 D29_US
 95,213

 D30_US
 100-Year Storm
 D30_US
 95,187

 D30_US
 100-Year Storm
 D30_US
 95,187

 D30_US
 100-Year Storm
 D30_US
 93-167

 D30_DS
 100-Year Storm
 D30_DS
 94-939

 D31_US
 100-Year Storm
 D31_US
 94.9

 D31_US
 100-Year Storm
 D31_US
 95-52

 D28_US
 100-Year Storm
 D28_US
 95-691

D12_US

D26_DS D25_US

D4_D5 100-Year Storm D4_D5 96.7 91.53
D5_US 100-Year Storm D5_D5 96.7 91.63
D5_US 100-Year Storm D5_D5 96.6 91.99 0.17
D6_US 100-Year Storm D6_US 96.5 92.09

 100-Year Storm
 D10_DS
 98.1
 93.81

 100-Year Storm
 D11_US
 98
 93.86

 100-Year Storm
 D11_DS
 97.9
 93.91

 100-Year Storm
 D12_US
 97.54
 93.87
 0.17

(Spill Crest) Invert User Inflow

PROPOSED CULVERT LINK INPUT DATA

		Conduit Design	Diameter	Length			Upstream Invert Elevation	Downstream Invert	Natural Section Shape	Initial Water Surface	Vertical		Cross-section	Lett Channel	Right Channel	User- defined Conduit using	
Name	Link Name 0	Option 0	(Height) ft		Foughness 0	Shape 0	ft	Elevation ft	GLDB Reference 0	Slope 0		Factor ft	Number 0		Length ft		Roughness
1311.1	D1 D2 Cul	Circular	2	60	0.013	Circular	93.99	93.89		0.001	0	0	0	0	0	D	0.013
	D13 Out CUI	Circular	1.5	81	0.013	Circular	91.1	89.67		0.001	0	0	0	0	0	D	0.013
1322.1	D3_D4_Cul	Circular	2	80.2	0.013	Circular	92.7	92.6		0.001	0	0	0	0	0	D	0.013
1330.1	D4_D5_Cul	Circular	2	90	0.013	Circular	91.63	91.53		0.001	0	0	0	0	0	D	0.013
1334.1	D5_D6_Cul	Circular	2	91.5	0.013	Circular	92.09	91.99		0.001	0	0	0	0	0	D	0.013
1344.1	D7_D8_Cul	Circular	2	45	0.013	Circular	91.84	91.52		0.001	0	0	0	0	0	D	0.013
1344.2	D7_D8_Cul	Circular	2	45	0.013	Circular	91.67	91.57		0.001	0	0	0	0	0	D	0.013
1350.1	Rd_Cross3	Circular	2	42	0.013	Circular	91.31	91.21		0.001	0	0	0	0	0	D	0.013
1350.2	Rd_Cross3	Circular	2	42	0.013	Circular	91.31	91.21		0.001	0	0	0	0	0	D	0.013
1350.3	Rd_Cross3 D16.2 Out-2	Circular	2	42	0.013	Circular	91.31	91.21		0.001	0	0	0	0	0	D	0.013
1354.1		Circular	1.67	51	0.013	Circular	91.1	91		0.001	0	0	0	0	0	D	0.013
	D10_D11_Cul D12_D13_CUI	Circular	2	65.8	0.013	Circular	93.97	93.86		0.001	0	0	0	0	0	D	0.013
	D12_D13_C01	Circular	2	78	0.013	Circular	91.83	91.73 93.13		0.001	0	0	0	0	0	D	0.013
	D15_D14_Cul		2	79.1 71.1	0.013	Circular	93.3 91.41	91.21		0.001	0	0	0	0	0	0	0.013
	D17 D18 Cul	Circular Circular	2	61.7	0.013	Circular Circular	91.41	91.86		0.001	0	0	0	0	0	D	0.013
1389.1	Rd Cross4	Circular	2	83	0.013	Circular	92.89	92.83		0.001	0	0	0	0	0	D	0.013
1389.2	Rd_Cross4	Circular	2	83	0.013	Circular	92.87	92.65		0.001	0	0	0	0	0	D	0.013
	D22_D23_Cul	Circular	1	24	0.024	Circular	92.79	92.46		0.001	0	0	0	0	0	D	0.024
	D21_D22_Cul	Circular	3	81	0.024	Circular	88.49	87.83		0.001	0	0	0	0	0	D	0.024
	D14_D15_Cul	Circular	2	57	0.013	Circular	93.27	93.17		0.001	0	0	0	0	0	D	0.013
1444.1	D15_D16_Cul	Circular	2	70.6	0.013	Circular	91.82	91.72		0.001	0	0	0	0	0	D	0.013
1445.1	Rd_Cross2	Circular	2.5	44	0.013	Special	93.13	92.4		0.001	0	0	0	0	0	D	0.013
1487.1	Link533	Circular	2.5	100	0.013	Circular	91.1	91		0.001	0	0	0	0	0	D	0.013
1488.1	Link634	Circular	1.417	70	0.013	Circular	89.2	89.15		0.001	0	0	0	0	0	D	0.013
Coss_Cul	D10_D11_Cul																
	D14_D15_Cul																
	D12_D13_CUI																
	D13_D14_Cul																
	D15_D16_Cul																
	D16_D17_Cul										_					-	
CrossCul	CrossCul	Circular	2.5	64.7	0.014	Special	93.68	93.58		0.001	0	0	0	0	0	D	0.014
Cul_Cross2	D1_D2_Cul D3_D4_Cul										_			_			
	D17_D18_Cul										_						
Cul_Cross4	D5_D6_Cul																
Cul_Cross5	D4_D5_Cul																
D1	D1	Circular	0	80	0.014	Natural	94.82	93.99	Stella_A_S_XS_O240	0.001	0	0	0	80	80	D	0.014
D10	D10	Circular	0	382.88	0.014	Natural	95	93.97	Stella_A_N_XS_O2_60'	0.001	0	0	0	382.88	382.88	D	0.014
D11	D11	Circular	0	362.1	0.014	Natural	93.86	93.91	Stella_N_50RD	0.001	0	0	0	362.1	362.1	D	0.014
D11_D12_Cul	D11_D12_Cul	Circular	2	200	0.015	ectangula	93.91	93.87		0.001	0	0	0	0	0	D	0.015
D12.1	D12.1	Circular	0	115.8	0.014	Natural	93.87	93.68	Stella_N_50RD	0.001	0	0	0	115.8	115.8	D	0.014
D12.1.1.1	D12.1.1.1	Circular	0	818.9	0.014	Natural	92.64	91.83	Stella_N_50RD	0.001	0	0	0	818.9	818.9	D	0.014
D13	D13	Circular	0	109	0.014	Natural	91.73	91.47	Stella_A_N_XS_O2_40'	0.001	0	0	n	109	109	n	0.014
D13.1	D13.1	Circular	0	358.59	0.014	Natural	93.66	93.3	Stella_N_36_3ft	0.001	0	0	0	876.2	876.2	D	0.014
D13.1.1	D13.1.1	Circular	0	517.61	0.014	Natural	92.258	91.1	Stella_N_36_3ft	0.001	0	0	0	876.2	876.2	D	0.014
D13.2	D13.2	Circular	0	148.5	0.014	Natural	91.47	91.1	Stella_N_36	0.001	0	0	0	148.5	148.5	D	0.014
D14	D14	Circular	0	174.2	0.014	Natural	93.27	93.13	Stella_N_36_3ft	0.001	0	0	0	174.2	174.2	D	0.014
D15	D15	Circular	0	107.3 29.6	0.014	Natural	93.17 91.72	91.82 91.2	Stella_N_36_3ft Stella_N_36	0.001	0	0	0	107.3 29.6	107.3 29.6	D	0.014
D16.1	D16.1	Circular	0	32.3	0.014	Natural	91.72	91.2	Stella_N_36	0.001	0	0	0	32.3	32.3	D	0.014
D16.2	D16.2	Circular	0	118	0.014	Natural	91.2	91.1	D16.2_XS	0.001	0	0	0	118	118	D	0.014
D17	D17	Circular	0	167.8	0.014	Natural	91.86	91.41	Stella_N_36	0.001	0	0	0	157.8	167.8	D	0.014
D17_Out-2	D17_Out-2	Circular	0	115	0.014	Natural	93.06	92.8	D17_Out-2_XS	0.001	0	0	0	115	115	D	0.014
D18	D18	Circular	0	261.2	0.014	Natural	92.73	91.96	Stella_N_36_3ft	0.001	0	0	0	251.2	261.2	D	0.014
D19	D19	Circular	0	129	0.014	Natural	92.594	91.93	D19_XS	0.001	0	0	0	129	129	D	0.014
D2	D2	Circular	0	674	0.014	Natural	93.89	93.97	Stella_A_S_XS_O240	0.001	0	0	0	674	674	D	0.014
D2.1	D2.1	Circular	0	80	0.014	Natural	91.37	91.2	Stella_A_S_XS_O240	0.001	0	0	0	80	80	D	0.014
D2.2	D2.2	Circular	2	160	0.013	Circular	93.97	93.68	Stella_A_S_XS_O2_34'	0.001	0	0	0	79.123	79.123	D	0.013
D2.2.1	D2.2.1	Circular	0	142.9	0.014	Natural	93.68	93.58	Stella_A_S_XS_O240	0.001	0	0	0	142.9	142.9	D	0.014
D2.2.1.1	D2.2.1.1	Circular	0	1210.6	0.014	Natural	93.58	91.37	Stella_A_S_XS_O240	0.001	0	0	0	1210.6	1210.6	D	0.014
D20	D20	Circular	0	400	0.014	Natural	89.956	90.113	D20_XS	0.001	0	0	0	400	400	D	0.014
D20_D21_Cul		Circular	2.5	81	0.024	Circular	89.26	89.5	B04 ***	0.001	0	0	0	0	0	D	0.024
D21	D21	Circular	0	251	0.014	Natural	91.024	89.811	D21_XS	0.001	0	0	0	251	251	D	0.014
D22	D22	Circular	0	78	0.014	Natural	93.12	89.034	D22_XS	0.001	0	0	0	78	78	D	0.014
D23	D23	Circular	0	89	0.014	Natural	92.599	93.235	D23_XS	0.001	0	0	0	89	89	D	0.014
D23_D24_Cul D24	D23_D24_Cul D24	Circular	0	67 199	0.009	Circular Natural	92.01 93.06	91.12 92.591	D24 VS	0.001	0	0	0	199	199	D	0.009
D24_D25_Cul		Circular	1.5	80	0.014	Circular	93.06	92.591	D24_XS	0.001	0	0	0	199	199	D	0.014
D24_D25_Cui	D24_D25_Cui	Circular	0	179	0.013	Natural	92.38	93.038	D25_XS	0.001	0	0	0	179	179	D	0.013
D25_D26_Cul		Circular	1.25	37	0.014	Circular	92.811	92.85	DEJ_N3	0.001	0	0	0	0	0	D	0.014
D25_D26_Cui	D25_D26_Cui	Circular	0	132.5	0.012	Natural	94.114	92.814	D26_XS	0.001	0	0	0	0	0	D	0.012
D26.1	D26.1	Circular	0	25	0.014	Natural	92.782	93.082	D26.1_XS	0.001	0	n	0	25	25	n	0.014
D26.1_D28_Cult		Circular	2	82	0.024	Circular	90.6	89.34		0.001	0	0	0	0	0	D	0.024
D27	D27	Circular	0	36	0.014	Natural	93.435	92.97	D27_XS	0.001	0	0	0	0	0	D	0.014
D27_D28.1_Cu			2	63	0.024	Circular	92.4	92.44		0.001	0	0	0	0	0	D	0.024
D28	D28	Circular	0	15	0.014	Natural	92.157	92.437	D28_XS	0.001	0	0	0	15	15	D	0.014
	D28.1	Circular	0	463	0.014	Natural	92.437	92.092	D28.1_XS	0.001	0	0	0	463	463	D	0.014
D28.1		Circular	2	123	0.009	Circular	91.05	91.019		0.001	0	0	0	0	0	D	0.009
D28_D29_Cul								92.253	D20 VC	0.001	0	0	0	62	62	D	0.014
D28_D29_Cul D29	D29	Circular	0	62	0.014	Natural	92.47		D29_XS		_						
D28_D29_Cul D29 D29_D30_Cul	D29 D29_D30_Cul	Circular	2	122	0.009	Circular	91.51	91.42		0.001	0	0	0	0	0	D	0.009
D28_D29_Cul D29 D29_D30_Cul D3	D29 D29_D30_Cul D3	Circular Circular	2 0	122 197.91	0.009 0.014	Circular Natural	91.51 92.9	91.42 92.7	Stella_A_S_XS_O240	0.001 0.001	0	0	0	0 710.8	0 710.8	D D	0.009 0.014
D28_D29_Cul D29 D29_D30_Cul	D29 D29_D30_Cul	Circular	2	122	0.009	Circular	91.51	91.42		0.001	0	0	0	0	0	D	0.009

PROPOSED CULVERT LINK INPUT DATA

						Upstrea					ntal	Cross-			User-		
						m	Downstre	Natural			Distorti	section		Right	defined		
	Conduit	Diameter				Invert	am Invert	Section			on	Identifica		Channel	Conduit		
	Design	(Height)		Rough		Elevatio	Elevation	Shape GLDB	Initial Water Surface	Vertical	Factor	tion	Left Channel	Length	using	Rough	
Link Name 0	Option 0	ft	Length ft	ness 0	Shape 0	n ft	ft	Reference 0	Slope 0	Shift ft	ft	Number 0	Length ft	ft	GLDB	ness	Roughne
D30_D31_Cul	D30_D31_Cul	Circular	2	55	0.013	Circular	91.46	91.52		0.001	0	0	0	0	0	0	0.013
D31	D31	Circular	0	113	0.014	Natural	92.14	91.886	D31_XS	0.001	n	0	n	113	113	0	0.014
D4	D4	Circular	0	202.8	0.014	Natural	92.5	92.4	Stella_A_S_XS_O240	0.001	0	0	0	202.8	202.8	0	0.014
D4.1	D4.1	Circular	0	257.4	0.014	Natural	92.4	91.41	Stella_A_S_XS_O240	0.001	0	0	0	257.4	257.4	0	0.014
D4.1.1	D4.1.1	Circular	0	148.2	0.014	Natural	91.41	91.31	Stella_A_S_XS_O240	0.001	0	0	0	148.2	148.2	0	0.014
D4.2	D4.2	Circular	0	77.7	0.014	Natural	91.53	91.31	Stella_A_S_XS_O240	0.001	0	0	0	77.7	77.7	0	0.014
D5	D5	Circular	0	125.5	0.014	Natural	91.99	91.63	Stella_A_S_XS_O240	0.001	0	0	0	125.5	125.5	0	0.014
DG	DG	Circular	0	213	0.014	Natural	92.99	92.09	Stelle_A_S_XS_O240	0.001	0	0	0	213	213	0	0.014
D6_D7_Cul	D6_D7_Cul	Circular	0.03	82	0.013	Circular	92.58	92.73		0.001	0	0	0	0	0	0	0.013
D7	D7	Circular	0	1496.3	0.014	Natural	92.73	92.703	D7_XS	0.001	0	0	0	1496.31	1496.31	0	0.014
D7.1	D7.1	Circular	0	60	0.014	Natural	92.5	92.354	D7.1_XS	0.001	0	0	0	60	60	0	0.014
D7.2	D7.2	Circular	0	772.7	0.014	Natural	92.703	92.5	D7_XS	0.001	0	0	0	772.695	772.695	0	0.014
D8	D8	Circular	0	160	0.014	Natural	92.14	91.503	D8_XS	0.001	0	0	0	160	160	0	0.014
D9	D9	Circular	U	/4	0.014	Natural	91.503	91.392	D9_X2	0.001	U	U	U	/4	74	U	0.014
D_Co_OutL	D_Co_OutL	Circular	0	33	0.014	Natural	94.99	94.89	Cotton	0.001	0	0	0	33	33	0	0.014
D_CoWo	D_CoWo	Circular	3.167	50	0.015	Special	95.07	95		0.001	0	0	0	0	0	0	0.015
D_Out1.1	D_Out1.1	Circular	0	37.995	0.014	Natural	89.274	89.15	Out_1.1_XS	0.001	0	0	0	37.995	37.995	0	0.014
D_Out1.1.1	D_Out1.1.1	Circular	0	62.005	0.014	Natural	89.151	88.95	Out_1.1_XS	0.001	0	0	0	0	0	0	0.014
DW1_N	DW1_N	Circular	0	32.83	0.014	Natural	95.45	95.3	Stella_A_N_XS_O2_40'	0.001	0	0	0	32.83	32.83	0	0.014
DW1_N.1	DW1_N.1	Circular	0	170.42	0.014	Natural	95.45	95.07	Stella_A_N_XS_O2_40'	0.001	0	0	0	170.42	170.42	0	0.014
DW1_S	DW1_S	Circular	0	249.6	0.014	Natural	95.85	94.99	Stella_A_S_XS_O240	0.001	0	0	0	249.6	249.6	0	0.014
DW2_N	DW2_N	Circular	2	103	0.013	Circular	95.3	95	Stella_A_N_XS_O2_35'	0.001	0	0	0	102	102	0	0.013
DW2_S	DW2_S	Circular	0.3	53.8	0.04	rapezoida	98.16	98		0.001	0	0	0	0	0	0	0.04
L_Sheriff_Out	L_Sheriff_Out	Circular	1.5	177	0.015	ectangula	90.3	90		0.001	0	0	0	0	0	0	0.015
Link615	Link615	Circular	2.5	33	0.013	Circular	89.2	89.1		0.001	0	0	0	0	0	0	0.013
Link616	Link616	Circular	2.5	168	0.024	Circular	89.43	89.2		0.001	0	0	0	0	0	0	0.024
Link617	Link617	Circular	2.5	129	0.024	Circular	89.44	89.43		0.001	0	0	0	0	0	0	0.024
Link618	Link618	Circular	2.5	214	0.024	Circular	89.44	89.26		0.001	0	0	0	0	0	0	0.024
Out 2.2 RdCR	D16.2_Out-2																
Out_2.2L	Out_2.2L	Circular	0	33	0.014	Natural	87.31	87.28	D_Out_2.2	0.001	0	0	0	33	33	0	0.014
Overflow	Link711																
Overflow Weir	Link634																
Rd_Cross1	Rd_Cross1	Circular	2.5	46	0.013	Circular	91.2	91.1		0.001	0	0	0	0	0	0	0.013
Rd_Cross5	Rd_Cross5	Circular	2	41	0.024	Circular	91.16	90.85		0.001	0	0	0	0	0	0	0.024
RoadCross	D21_D22_Cul																
RoadCross2.1	D22_D23_Cul																1

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

REVISIONS DATE NAME NO.

FORT BEND COUNTY ENGINEERING DEPARTMENT





R.G. Miller Engineers, Inc. | TxEng F - 487 16340 Park Ten Place, Ste 350 Houston, TX 77084



	PROJECT TI	^{TLE:} STELLA ROAD	
		COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
	SHEET DESC	RIPTION: PROPOSED CULVERT INPUT DATA	
	DRAWN BY: NS		DATE: 6/18/24
	CK'D BY: MJ	SCALE: N/A	SHEET NO: 72 / 133
_	MJ_	N/A	72 / 13

EXISTING CULVERT NODE INPUT DATA

		Ground			
Name	Node Name 0	Elevation (Spill Crest) ft	Invert Elevation ft	User Inflow Hydrograph	Ponding Type
Node653	Node653	92.94	89.44	0.17	Allowed
Node652	Node652	93.38	89.43	0.17	Allowed
Node650	Node650	93.45	89.2		Allowed
D31_US	D31_US	94.9	91.52	0.17	Allowed
D30_DS	D30_DS	94.939	91.46	0.17	Allowed
D27_DS	D27_DS	95	92.4		Allowed
Out_1.1	Out_1.1	95	88.95		Allowed
D26_US	D26_US	95.09	94.114		Allowed
D27_US	D27_US	95.09	93.435	0.17	Allowed
D30 US	D30 US	95.187	91.42		Allowed
D29_DS	D29_DS	95.2	91.51		Allowed
D29_US	D29_US	95.213	91.019	0.17	Allowed
D28_DS	D28_DS	95.22	91.05		Allowed
Out_1	0ut_1	95.269	89.274		Allowed
Out_2	Out_2	95.28	87.31	0.17	Allowed
Out_2.2	Out_2.2	95.28	87.21	_	None
D16.2_DS	D16.2_DS	95.39	87.33		Allowed
Out3.1	Out3.1	95.407	91.93	0.17	Allowed
D31_DS	D31_DS	95.52	91.16	0.17	Allowed
D28_US	D28_US	95.691	90.6		Allowed
D25 US	D25 US	95.71	92.811		Allowed
D7.1_US	D7.1_US	95.725	92.5		Allowed
D24_US	D24_US	95.769	92.51		Allowed
Out_4	Out_4	95.838	91.39		Allowed
D25_DS	D25_DS	95.91	92.38		Allowed
D26.1_US	D26.1_US	95.911	89.26	0.17	Allowed
D26_D5	D26_D5	95.911	92.99	0.17	Allowed
D7_US.1	D7_US.1	95.928	92.578		Allowed
D22_US	D22_US	95.94	92.46		Allowed
D8_DS	D8_DS	95.949 95.971	90.85 92.437	0.17	Allowed
D28.1_US	D28.1_US			0.17	
D7 DS	D7 DS	96 96	91.67		Allowed
D8_US D24 DS	D8_US D24 DS	96.01	91.52 92.01		Allowed
D23_US	D23_US	96.03	89.2		Allowed
Node651	Node651	96.03	89.1		Allowed
			88.49		Allowed
D22_DS D20_US	D22_DS D20_U5	96.11 96.24	89.5		Allowed
D19_US	D19_US	96.31	92.594		Allowed
D7_US	D7_US	96.32	92.73		Allowed
D13_US	D13_US	96.366	92.84	0.17	Allowed
D18_US	D18_US	96.37	92.73	0.17	Allowed
Out 3	Out 3	96.381	90.113		Allowed
D6_DS	D6_DS	95.419	92.58		Allowed
D6_US	D6_US	96.43	92.89		Allowed
D15_US	D15_US	96.44	92.7		Allowed
D18_DS	D18_DS	96.451	94.389		Allowed
D23_DS	D23_DS	96.496	92.79		Allowed
D16_DS	D16_DS	95.517	92.73		Allowed
D3_DS	D3_DS	96.524	93.06		Allowed
D14_US	D14_US	96.56	91.9	0.17	Allowed
D5_US	D5_US	95.566	92.97		Allowed
D4.1_DS	D4.1_DS	96.58	90.7	_	Allowed
D16.1_US	D16.1_US	96.587	90.65		Allowed
D16_US	D16_US	95.587	92.42		Allowed
D21_DS	D21_DS	96.59	89.26		Allowed
D15_DS	D15_DS	96.59	92.1	0.17	Allowed
D17_US	D17_US	95.645	93.06		Allowed
D17_DS	D17_DS	95.645	92.25		Allowed
D5_DS	D5_DS	95.682	92.87		Allowed
D2_DS	D2_DS	96.756	91.45	0.17	Allowed
D3_US	D3_US	96.767	93.66		Allowed
D13.1_US	D13.1_US	96.78	91.1		Allowed
D14_DS	D14_DS	97.078	92.67		Allowed
D12_DS	D12_DS	97.098	93.2		Allowed
D11_DS	D11_DS	97.134	93.95		Allowed
D13_DS	D13_DS	97.435	92.89		Allowed
D4.1_US	D4.1_US	97.519	92.04		Allowed
D12_US	D12_US	97.625	93.91	0.17	Allowed
D4_US	D4_US	97.745	92.73		Allowed
D21_US	D21_US	97.803	87.73		Allowed
D11_US	D11_US	97.98	94.848		Allowed
D2B_US	D2B_US	98.785	93.671		Allowed
D4_DS	D4_DS	98.816	92.61		Allowed
D1_DS	D1_DS	100	95.26	0.17	Allowed
D2_US	D2_US	100	95	0.17	Allowed
D1_US	D1_US	100.113	95.748		Allowed
D10_DS	D10_DS	100.229	95.08	0.17	Allowed
D10_US	D10_US	100.278	95.18		Allowed

EXISTING CULVERT LINK INPUT DATA

	Conduit Design	Diameter (Height)		Roughness			Downstre am Invert Elevation	Natural Section Shape GLDB	Initial Water Surface	Vertical	Horizontal Distortion	Cross- section Identification	Left Channel	Right Channel	User- defined Conduit using	
	Option 0	ft	Length ft	0	Shape 0	ft	ft	Reference 0	Slope 0	Shift ft	Factor ft	Number 0	Length ft	Length ft	GLDB	Roughne
D7_D8_Cul	Circular	2	45	0.013	Circular	91.84	91.52		0.001	0	0	0	0	0	0	0.013
D7_D8_Cul	Circular	2	45	0.013	Circular	91.67	91.57		0.001	0	0	0	0	0	0	0.013
Rd Cross3	Circular	2.5	104	0.009	Circular	90.7	90.65		0.001	0	0	0	0	0	0	0.009
Rd_Cross3	Circular	2.5	104	0.009	Circular	90.75	90.77		0.001	0	0	0	0	0	0	0.009
Rd_Cross3	Circular	2.5	104 99	0.009	Circular	90.89 87.35	90.86 87.44		0.001	0	0	0	0	0	0	0.009
D16.2_Out-2 D16.2_Out-2	Circular	3	99	0.009	Circular	87.33	87.31		0.001	0	0	0	0	0	0	0.009
Rd_Cross4	Circular	2	83	0.009	Circular	92.89	92.83		0.001	0	0	0	0	0	0	0.003
Rd_Cross4	Circular	2	83	0.013	Circular	92.87	92.65		0.001	0	0	0	0	0	0	0.013
022 D23 Cul	Circular	1	24	0.024	Circular	92.79	92.46		0.001	0	0	0	0	0	0	0.024
	Circular	3	81	0.024	Circular	88.49	87.83		0.001	0	0	0	0	0	0	0.024
D1	Circular	0	550	0.014	Natural	95.748	95.33	D1_XS	0.001	0	0	0	550	550	0	0.014
D10	Circular	0	906	0.014	Natural	95.18	95.131	D10_XS	0.001	0	0	0	906	906	0	0.014
010_D11_Cul	Circular	1.25	60	0.013	Circular	95.08	94.89	D10_//3	0.001	0	0	0	0	0	0	0.013
D11	Circular	0	383	0.014	Natural	94.848	94.03	D11_XS	0.001	0	0	0	383	383	0	0.014
011 D12 Cul	Circular	2	131	0.015	Rectangula	93.95	93.91		0.001	0	0	0	0	0	0	0.015
D12	Circular	0	1106	0.014	Natural	94.356	93.829	D12_XS	0.001	0	0	0	1106	1106	0	0.014
012_D13_CUI	Circular	1.5	46	0.009	Circular	93.2	92.84	_	0.001	0	0	0	0	0	0	0.009
D13	Circular	0	170	0.014	Natural	93.675	91.296	D13 XS	0.001	0	0	0	170	170	0	0.014
D13.1	Circular	0	1174	0.014	Natural	92.601	93.268	D13.1_XS	0.001	0	0	0	1174	1174	0	0.014
013_D14_Cul	Circular	2	118	0.009	Circular	92.89	92.86		0.001	0	0	0	0	0	0	0.000
013_Out_CUI	Circular	3.5	81	0.009	Circular	91.1	89.67		0.001	0	0	0	0	0	0	0.009
D14	Circular	0	214	0.014	Natural	93.128	93.657	D14_XS	0.001	0	0	0	214	214	0	0.014
D14_D15_Cul	Circular	2	84	0.009	Circular	92.67	92.7		0.001	0	0	0	0	0	0	0.009
D15	Circular	0	158	0.014	Natural	93.458	93.301	D15_XS	0.001	0	0	0	158	158	0	0.014
015_D16_Cul	Circular	2	103	0.009	Circular	92.1	92.42		0.001	0	0	0	0	0	0	0.009
D16	Cliculai	0	37	0.014	Natural	93.185	91.898	D16_X3	0.001	0	0	0	37	37	0	0.014
D16.1	Circular	0	68	0.014	Natural	93.62	91.898	D16.1_XS	0.001	0	0	0	68	68	0	0.014
D16.2	Circular	0	43	0.014	Natural	91.626	91.024	D16.2_XS	0.001	0	0	0	43	43	0	0.014
16_D17_Cul	Circular	2	82	0.009	Circular	93.06	92.73		0.001	0	0	0	0	0	0	0.009
D17	Circular	0	218	0.014	Natural	94.236	94.284	D17_XS	0.001	0	0	0	218	218	0	0.014
17_D18_Cul	Circular	2.5	83	0.009	Circular	92.73	92.25		0.001	0	0	0	0	0	0	0.009
D1/_Out-2	Circular	U	115	0.014	Natural	94.463	92.8	D1/_Out-2_XS	0.001	U	U	U	115	115	U	U.U14
D18	Circular	0	278	0.014	Natural	94.389	94.028	D18_XS	0.001	0	0	0	278	278	0	0.014
D19	Circular	0	129	0.014	Natural	92.594	91.93	D19_XS	0.001	0	0	0	129	129	0	0.014
D1_D2_Cul	Circular	1.5	63.19	0.013	Circular	95.26	95		0.001	0	0	0	0	0	0	0.013
D2	Circular	0	806.98	0.014	Natural	95	94.259	D2_XS	0.001	0	0	0	806.98	806.98	0	0.014
D20	Circular	0	400	0.014	Natural	89.956	90.113	D20_XS	0.001	0	0	0	400	400	0	0.014
20_D21_Cul	Circular	2.5	81	0.024	Circular	89.26	89.5		0.001	0	0	0	0	0	0	0.024
D21	Circular	0	251	0.014	Natural	91.024	89.811	D21_XS	0.001	0	0	0	251	251	0	0.014
D22	Circular	0	78	0.014	Natural	93.12	89.034	D22_XS	0.001	0	0	0	78	78	0	0.014
D23	Circular	0	89	0.014	Natural	92.599	93.235	D23_XS	0.001	0	0	0	89	89	0	0.014
23_D24_Cul	Circular	1	67	0.009	Circular	92.01	91.12		0.001	0	0	0	0	0	0	0.009
D24	Circular	0	199	0.014	Natural	93.06	92.591	D24_XS	0.001	0	0	0	199	199	0	0.014
024_D25_Cul	Circular	1.5	80	0.013	Circular	92.38	92.51		0.001	0	0	0	0	0	0	0.013
D25	Circular	0	179	0.014	Natural	92.811	93.038	D25_XS	0.001	0	0	0	179	179	0	0.014
25_D26_Cul	Circular	1.25	37	0.012	Circular	92.99	92.85		0.001	0	0	0	0	0	0	0.012
D26	Circular	0	132.5	0.014	Natural	94.114	92.814	D26_XS	0.001	0	0	0	132.5	132.5	0	0.014
D26.1	Circular	0	25	0.014	Natural	92.782	93.082	D26.1_XS	0.001	0	0	0	25	25	0	0.014
26.1_D28_Cul	Circular	2	82	0.024	Circular	90.6	89.34		0.001	0	0	0	0	0	0	0.024
D27	Circular	0	36	0.014	Natural	93.435	92.97	D27_XS	0.001	0	0	0	36	36	0	0.014
27_D28.1_Cul	Circular	2	63	0.024	Circular	92.4	92.44		0.001	0	0	0	0	0	0	0.024
D28	Circular	0	15	0.014	Natural	92.157	92.437	D28_XS	0.001	0	0	0	15	15	0	0.014
D28.1	Circular	0	463	0.014	Natural	92.437	92.092	D28.1_XS	0.001	0	0	0	463	463	0	0.014
28_D29_Cul	Circular	2	123	0.009	Circular	91.05	91.019		0.001	0	0	0	0	0	0	0.009
D29	Circular	0	62	0.014	Natural	92.47	92.253	D29_XS	0.001	0	0	0	62	62	0	0.014
29_D30_Cul	Circular	2	122	0.009	Circular	91.51	91.42		0.001	0	0	0	0	0	0	0.009
D2B	Circular	0	1348.02	0.014	Natural	94.259	91.45	D2_XS	0.001	0	0	0	1348.02	1348.02	0	0.014
D3	Circular	0	1055	0.014	Natural	93.66	93.417	D3_XS	0.001	0	0	0	1055	1055	0	0.014
D30	Circular	0	232	0.014	Natural	92.458	92.21	D30_XS	0.001	0	0	0	232	232	0	0.014
30_D31_Cul	Circular	2	55	0.013	Circular	91.46	91.52		0.001	0	0	0	0	0	0	0.013
D31	Circular	0	113	0.014	Natural	92.14	91.886	D31_XS	0.001	0	0	0	113	113	0	0.014
D3_D4_Cul	Circular	2	33	0.009	Circular	93.06	92.73	D4 350	0.001	0	0	0	0	0	0	0.009
D4	Circular	0	213	0.014	Natural	93.38	93.154	D4_XS	0.001	0	0	0	213	213	0	0.014
D4.1	Circular	0	425	0.014	Natural	93.154	91.545	D4.1_XS	0.001	0	0	0	425	425	0	0.014
D4.2	Circular	0	173	0.014	Natural	91.545	93.781	D4.2_XS	0.001	0	0	0	173	173	0	0.014
D4_D5_Cul	Circular	2	82	0.009	Circular	92.61	92.97		0.001	0	0	0	0	0	0	0.009
D5 DF DC Cul	Circular	0	192	0.014	Natural	93.492	93.608	D5_XS	0.001	0	0	0	192	192	0	0.014
D5_D6_Cul	Circular	1.5	82	0.009	Circular	92.87	92.89	DC :::	0.001	0	0	0	0	0	0	0.009
D6	Circular	0	238	0.014	Natural	93.515	92.918	D6_XS	0.001	0	0	0	238	238	0	0.014
D6_D7_Cul	Circular	2	82	0.024	Circular	92.58	92.73		0.001	0	0	0	0	0	0	0.024
D7	Circular	0	1496.305	0.014	Natural	92.73	92.703	D7_XS	0.001	0	0	0		1496.305	0	0.014
D7.1	Circular	0	60	0.014	Natural	92.5	92.354	D7.1_XS	0.001	0	0	0	60	60	0	0.014
D7.2	Circular	0	772.695	0.014	Natural	92.703	92.5	D7_XS	0.001	0	0	0	772.695	772.695	0	0.014
D8	Circular	0	160	0.014	Natural	92.14	91.503	D8_XS	0.001	0	0	0	160	160	0	0.014
D9	Circular	0	74	0.014	Natural	91.503	91.392	D9_XS	0.001	0	0	0	74	74	0	0.014
D_Out1.1	Circular	0	100	0.014	Natural	89.274	88.95	D_Out1.1_XS	0.001	0	0	0	0	0	0	0.014
Link615	Circular	2.5	33	0.013	Circular	89.2	89.1		0.001	0	0	0	0	0	0	0.013
Link616	Circular	2.5	168	0.024	Circular	89.43	89.2		0.001	0	0	0	0	0	0	0.024
Link617	Circular	2.5	129	0.024	Circular	89.44	89.43		0.001	0	0	0	0	0	0	0.024
Link618	Circular	2.5	214	0.024	Circular	89.44	89.26		0.001	0	0	0	0	0	0	0.024
	Circular	0	33	0.014	Natural	87.31	87.28	D_Out_2.2	0.001	0	0	0	33	33	0	0.014
Out_2.2L	Circular	2.5	84	0.009	Circular	91.45	91.23		0.001	0	0	0	0	0	0	0.009
Rd_Cross1			84	0.009	Circular	92.04	91.9		0.001	0	0	0	0	0	0	0.009
Rd_Cross1 Rd_Cross2	Circular	1.5														
Rd_Cross1		2	41	0.024	Circular	91.16	90.85		0.001	0	0	0	0	0	0	0.024

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

2011	NO.	REVISIONS	DATE	NAME
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FORT BEND COUNTY
ENGINEERING DEPARTMENT







	PROJECT TITI	^{LE:} STELLA ROAD	
		OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
-	SHEET DESC	RIPTION: EXISTING CULVERT INPUT DATA	
	DRAWN BY: NS		DATE: 6/18/24
	CK'D BY: MJ	SCALE: N/A	SHEET NO: 73 / 133

PROPOSED CULVERT NODE 100-YR RESULT

Name Basin1_Opt2	Scenario 100-Year Storm	Node Name 0 Basin1_Opt2	Max Water Elevation ft 94.825	Max Volume ft^3 1381275.708	Node Inflow ft^ 656456.154
D10_DS	100-Year Storm	D10_DS	96.711	36.456	0
D10_US	100-Year Storm	D10_US	96.711	21.501	100815.313
D11_DS	100-Year Storm	D11_DS	96.731	35.451	0
D11_US	100-Year Storm	D11_US	96.731	36.078	0
D12 DS D12_US	100-Year Storm 100-Year Storm	D12 DS D12_US	95.214 96.734	42.529 35.99	0 114980.557
D12_03	100-Year Storm	D12_03	96,734	38.377	0
D12_US.1.1	100-Year Storm	D12_US.1.1	95.215	32.351	0
D13.1_US	100-Year Storm	D13.1_US	95.214	51.698	0
D13_DS	100-Year Storm	D13_DS	96.086	35.004	0
D13_DS.1	100-Year Storm	D13_DS.1	95.214	37.147	0
D13_DS.2	100-Year Storm	D13_DS.2	96.086	30.481	0
D13_US	100-Year Storm	D13_US	95.214	43.782	0
D13_US.1	100-Year Storm	D13_US.1	95.214	47.049	0
D14_DS	100-Year Storm	D14_DS	96.084	35.365	138723.595
D14_US D15_DS	100-Year Storm 100-Year Storm	D14_US D15_DS	96.085	37.139 52.117	0
D15_US	100-Year Storm	D15_US	95.968	35.158	0
D16.1_US	100-Year Storm	D16.1_US	95.768	57.399	0
D16.2_DS	100-Year Storm	D16.2_DS	95.766	2338.4	0
D16_DS	100-Year Storm	D16_DS	95.768	57.274	0
D16_US	100-Year Storm	D16_US	95.768	50.865	0
מח"וזח	100-Year Storm	מת"/דמ	95.77	49.136	Ü
D17_US	100-Year Storm	D17_US	95.77	54.791	0
D18_DS	100-Year Storm	D18_DS	95.771	38.215	47104.329
D18_US	100-Year Storm	D18_US	95.771	47.89	0
D19_US D1_DS	100-Year Storm 100-Year Storm	D19_US D1_DS	93.39 97.141	10.006 39.594	0
D1_US	100-Year Storm	D1_US	97.141	29.166	0
D20_US	100-Year Storm	D20_US	92.192	33.832	107948.85
D21_DS	100-Year Storm	D21_DS	93.901	59.078	0
D21_US	100-Year Storm	D21_US	94.124	80.417	0
D22_DS	100-Year Storm	D22_DS	94.93	83.779	0
D22_US	100-Year Storm	D22_US	95.245	34.997	0
D23_DS	100-Year Storm	D23_DS	95.265	31.176	0
D23_US	100-Year Storm	D23_US	95.285	76.736	261742.407
D24_DS D24_US	100-Year Storm 100-Year Storm	D24_DS D24_US	95.211 95.211	40.225 33.956	0
D25_DS	100-Year Storm	D25_DS	95.218	35.657	0
D25 US	100-Year Storm	D25 US	95.218	30.245	0
D26.1_US	100-Year Storm	D26.1_US	95.288	75.752	0
D26_DS	100-Year Storm	D26_DS	95.288	28.88	0
D26_US	100-Year Storm	D26_US	95.288	1108.697	0
D27_DS	100-Year Storm	D27_DS	94.739	29.393	0
D27_US	100-Year Storm	D27_US	94.739	16.387	0
D28_DS	100-Year Storm	D28_DS	94.725	28 918 46.186	66905 239
D28_D3	100-Year Storm	D28_US	94.723	52.024	0
D29_DS	100-Year Storm	D29_DS	94.347	35.646	0
D29_US	100-Year Storm	D29_US	94.353	41.896	0
D2_DS	100-Year Storm	D2_DS	96.718	69.344	2221285.356
D2_US	100 Year Storm	D2_US	97.14	40.842	98049.784
D2_US.1	100-Year Storm	D2_US.1	96.718	1285.002	0
D2_US.2	100-Year Storm	D2_US.2	97.137	39.8	107282.869
D2B_US	100-Year Storm	D2B_US	96.737	38.413	115393.415
D2B_US.1 D30 DS	100-Year Storm 100-Year Storm	D2B_US.1 D30 DS	96.734 93.885	39.636 30.477	115393.415
D30_D3	100-Year Storm	D30_D3	93.973	32.087	0
D31_DS	100-Year Storm	D31_DS	93.342	27.413	0
D31_US	100-Year Storm	D31_US	93.536	25.335	0
D3_DS	100-Year Storm	D3_DS	96.259	44.717	137463.164
D3_DS.1	100-Year Storm	D3_DS.1	96.716	1191.691	57717.88
D3_US	100-Year Storm	D3_US	96.277	42.439	0
D4.1_DS	100-Year Storm	D4.1_DS	96.1	60.185	0
D4.1_US	100-Year Storm	D4.1_US	96.109	46.608	0
D4.1_US.1	100-Year Storm	D4.1_US.1	96.102 96.1	58.964	379091.294
D4_DS D4_US	100-Year Storm	D4_DS		57.421 44.347	0
D4_0S D5 DS	100-Year Storm 100-Year Storm	D4_US D5_DS	96.129 96.1	51.649	36543.347
D5_US	100-Year Storm	D5_US	96.1	56.172	0
D6_DS	100-Year Storm	D6_DS	96.1	44.237	0
D6_US	100-Year Storm	D6_US	96.1	50.394	0
D7.1_US	100-Year Storm	D7.1_US	93.303	10.095	0
D7_DS	100-Year Storm	D7_DS	93.124	18.272	0
D7_US	100-Year Storm	D7_US	93.848	14.05	101348.761
D7_US.1 D8_DS	100-Year Storm	D7_US.1 D8_DS	93.846 92.833	15.92 24.914	66303.446 0
D8_US	100-Year Storm 100-Year Storm	D8_US	93.097	19.813	0
D_Co_Out	100-Year Storm	D_Co_Out	95.556	8.37	0
DW1N_Out	100-Year Storm	DW1N_Out	96.03	12.068	0
DW1S_Out	100-Year Storm	DW1S_Out	95.915	11.628	0
DW2N_US	100-Year Storm	DW2N_US	96.663	17.132	0
DW2N_US.1	100-Year Storm	DW2N_US.1	96.647	15.038 8.487	113943.611
DW2S_DS	100-Year Storm	DW2S_DS	95.915		0
Node650 Node651	100-Year Storm 100-Year Storm	Node650 Node651	89.2 89.1	0	0
Node651	100-Year Storm	Node652	95.449	34623.146	0
Node653	100-Year Storm	Node653	95.853	87123.784	476578.977
Out3.1	100-Year Storm	Out3.1	92.426	6.233	0
Out_1	100-Year Storm	Out_1	90.374	13.827	0
Out_1.1	100-Year Storm	Out_1.1	89.757	10.138	0.041
Out_1.2	100-Year Storm	Out_1.2	90.349	15.069	0
Out_2 Out_2.2	100-Year Storm 100-Year Storm	Out_2 Out_2.2	88.679 88.211	17.206 12.582	0
Out_3	100-Year Storm	Out_3	91.257	14.399	0.021
	100-Year Storm	Out_4	92.279	11.173	0.004
Out_4	100 ICai Stollii				

PROPOSED CULVERT NODE 10-YR RESULT

Name	Scenario	Node Name 0	Max Water Elevation ft	Max Volume ft^3	Node Inflow fi
Basin1_Opt2	10-Year Storm	Basin1_Opt2	93.648	912239.284	334337.348
D10_DS	10-Year Storm	D10_DS	95.974	27.195	0
D10_US	10-Year Storm	D10_US	95.979	12.302	51371.947
D11_DS	10-Year Storm	D11_DS	95.921	25.264	0
D11_US	10-Year Storm	D11_US	95.921	25.901	0
D12_DS	10-Year Storm	D12_DS	93.899	26.002	0
D12 US	10-Year Storm	D12_US	95.912	25.659	59597.205
D12 US.1	10-Year Storm	D12_US.1	95.912	28.042	0
D12_US.1.1	10-Year Storm	D12_US.1.1	93.899	15.824	0
D13.1_US	10-Year Storm		93.899	35.173	0
		D13.1_US			_
D13_DS	10-Year Storm	D13_DS	94.658	20.076	0
D13_DS.1	10-Year Storm	D13_DS.1	93.899	20.622	0
D13_DS.2	10-Year Storm	D13_DS.2	94.658	12.537	0
D13_US	10-Year Storm	D13_US	93.899	27.257	0
D13_US.1	10-Year Storm	D13_US.1	93.899	30.524	0
D14_DS	10-Year Storm	D14_DS	94.657	17.424	69840.797
D14_US	10-Year Storm	D14_US	94.657	19.194	0
D15_DS	10-Vear Storm	D15_DS	94.613	35.094	0
D15_US	10-Year Storm	D15_US	94.614	18.15	0
D16.1_US	10-Year Storm	D16.1_US	94.585	42.532	0
D16.2_DS	10-Year Storm	D16.2_DS	94.583	43.771	0
			94.585	42.408	0
D16_DS	10-Year Storm	D16_DS			
D16_US	10-Year Storm	D16_US	94.585	36	0
D17_D3	10-Year Storm	D17_D3	94.580	34.254	0
D17_US	10-Year Storm	D17_US	94.586	39.908	0
D18_DS	10-Year Storm	D18_DS	94.587	23.334	24905.078
D18_US	10-Year Storm	D18_US	94.587	33.008	0
D19_US	10-Year Storm	D19_US	93.246	8.192	0
D1_DS	10-Year Storm	D1_DS	96.494	31.468	0
D1_US	10-Year Storm	D1_US	96.494	21.03/	U
D20_US	10-Year Storm	D20_US	91.824	29.201	55417.52
			92.485	40.531	0
D21_DS	10-Year Storm	D21_DS			
D21_US	10-Year Storm	D21_US	93.012	66.372	0
D22_DS	10-Year Storm	D22_DS	93.291	60.327	0
D22_US	10-Year Storm	D22_US	93.752	16.234	0
D23_DS	10-Year Storm	D23_DS	94.849	25.873	0
D23_US	10-Year Storm	D23_US	94.87	71.252	131418.505
D24_DS	10-Year Storm	D24_DS	94.832	35.467	0
D24_US	10-Year Storm	D24_US	94.833	29.189	0
D24_03	10-Year Storm	D25_DS	94.834	30.837	0
D25_DS	10-Year Storm		94.834	25.422	0
		D25_US		70.051	0
D26.1_US	10-Year Storm	D26.1_US	94.835		
D26_DS	10-Year Storm	D26_DS	94.835	23.185	0
D26_US	10-Year Storm	D26_US	94.835	9.056	0
D27_DS	10-Year Storm	D27_DS	94.313	24.039	0
D27_US	10-Year Storm	D27_US	94.313	11.034	0
D28 1_US	10-Year Storm	D28 1_US	94 312	23 565	33545 764
D28_DS	10-Year Storm	D28_DS	94.282	40.613	0
D28_US	10-Year Storm	D28_US	94.316	46.695	0
D28_03 D29 DS	10-Year Storm	D28_03	94.003	31.329	0
D29_US	10-Year Storm	D29_US	94.014	37.64	0
D2_DS	10-Year Storm	D2_DS	95.419	53.019	990486.574
D2_US	10-Year Storm	D2_US	96.493	32.709	48665.824
D2_US.1	10-Year Storm	D2_US.1	95.42	50.896	0
D2_US.2	10-Year Storm	D2_US.2	96.489	31.657	107284.182
D2B_US	10-Year Storm	D2B_US	95.907	27.98	56584.135
D2B_US.1	10-Year Storm	D2B_US.1	95.858	28.63	115395.149
D30_DS	10-Year Storm	D30_DS	93.624	27.189	0
D30_US	10-Year Storm	D30_US	93.755	29.336	0
D31_DS	10-Year Storm	D31_DS	93.062	23.906	0
D31_US	10-Year Storm	D31_US	93.374	23.3	0
D3_DS	10-Year Storm	D3_DS	94.735	25.572	68524.407
D3_DS.1	10-Year Storm	D3_DS.1	95.419	40.327	25563.083
D3_US	10-Year Storm	D3_US	94.739	23.109	0
D4.1_DS	10-Year Storm	D4.1_DS	94.648	41.944	0
D4.1_US	10-Year Storm	D4.1_US	94.658	28.373	0
D4.1_US.1	10-Year Storm	D4.1_US.1	94.655	40.774	187624.549
D4_DS	10-Year Storm	D4_DS	94.648	39.181	0
D4_US	10-Year Storm	D4_US	94.666	25.961	0
D5_DS	10-Year Storm	D5_DS	94.649	33.412	17763.76
D5_D3	10-Year Storm	D5_US	94.649	37.935	0
D6_DS	10-Year Storm	D6_DS	94.649	25.999	0
D6_US	10-Year Storm	D6_US	94.649	32.156	0
D7.1_US	10-Year Storm	D7.1_US	93.106	7.612	0
D7_DS	10-Year Storm	D7_DS	92.886	15.282	0
D7_US	10-Year Storm	D7_US	93.63	11.31	49266.88
D7_US.1	10-Year Storm	D7_US.1	93.632	13.23	32824.989
D8_DS	10-Year Storm	D8_DS	92.692	23.147	0
D8_US	10-Year Storm	D8_US	92.874	17.018	0
D_Co_Out	10-Year Storm	D_Co_Out	95.207	3,984	0
DW1N Out	10-Year Storm	DW1N_Out	95.536	5.86	0
_ ** *!*_Out	10-Year Storm	DW1N_Out	95.536	6.245	0
DIMES OF					
	10-Year Storm	DW2N_US	96	8.797	0
DW2N_US	10-Year Storm	DW2N_US.1	96.072	7.821	51122.415
DW2N_US DW2N_US.1		DW2S_DS	95.24	0	0
DW2N_US	10-Year Storm		89.2	0	0
DW2N_US.1		Node650		0	0
DW2N_US DW2N_US.1 DW2S_DS	10-Year Storm	Node650 Node651	89.1		
DW2N_US.1 DW2N_DS.1 DW2S_DS Node650	10-Year Storm 10-Year Storm		89.1 95.032	21145.097	0
DW2N_US DW2N_US.1 DW2S_DS Node650 Node651	10-Year Storm 10-Year Storm 10-Year Storm	Node651		21145.097	
DW2N_US.1 DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653	95.032 95.221	21145.097 43967.968	242745.684
DW2N_US.1 DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1	95.032 95.221 92.335	21145.097 43967.968 5.088	242745.684
DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1 Out_1	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1 Out_1	95.032 95.221 92.335 90.102	21145.097 43967.968 5.088 10.4	242745.684 0 0
DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1 Out_1 Out_1.1	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1 Out_1 Out_1.1	95.032 95.221 92.335 90.102 89.502	21145.097 43967.968 5.088 10.4 6.948	242745.684 0 0 0
DW2N_US DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1 Out_1 Out_1.1 Out_1.2	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1 Out_1 Out_1.1 Out_1.2	95.032 95.221 92.335 90.102 89.502 90.04	21145.097 43967.968 5.088 10.4 6.948 11.189	242745.684 0 0 0 0
DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1 Out_1 Out_1.1	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1 Out_1 Out_1.1	95.032 95.221 92.335 90.102 89.502	21145.097 43967.968 5.088 10.4 6.948	242745.684 0 0 0
DW2N_US DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1 Out_1 Out_1.1 Out_1.2	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1 Out_1 Out_1.1 Out_1.2	95.032 95.221 92.335 90.102 89.502 90.04	21145.097 43967.968 5.088 10.4 6.948 11.189	242745.684 0 0 0 0
DW2N_US DW2N_US.1 DW2S_DS Node650 Node651 Node652 Node653 Out3.1 Out_1 Out_1.1 Out_1.2 Out_2	10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm 10-Year Storm	Node651 Node652 Node653 Out3.1 Out_1 Out_1.1 Out_1.2 Out_2	95.032 95.221 92.335 90.102 89.502 90.04 88.295	21145.097 43967.968 5.088 10.4 6.948 11.189 12.372	242745.684 0 0 0 0 0

PROPOSED CULVERT NODE 5-YR RESULT

Name	Scenario	Node Name 0	Max Water Elevation ft	Max Volume ft^3	Node Infl
Basin1_Opt2	5-Year Storm	Basin1_Opt2	93.134	714598.167	264979.8
D10_DS	5-Year Storm	D10_DS	95.844	25.562	0
D10_US	5-Year Storm	D10_US	95.85	10.681	40327.0
D11_DS	5-Year Storm	D11_DS	95.804	23.806	0
D11_US	5-Year Storm	D11_US	95.805	24.44	0
D12_D5	5 Year Storm	D12_D5	93.555	21.674	
D12_US	5-Year Storm	D12_US	95.798	24.228	47774.6
D12_US.1	5-Year Storm	D12_US.1	95.798	26.612	0
D12_US.1.1	5-Year Storm	D12_US.1.1	93.555	11.497	0
D13.1_US	5-Year Storm	D13.1_US	93.554	30.84	0
D13_DS	5-Year Storm	D13_DS	94.329	15.941	0
D13_D3.1	5-Year Storm	D13_D3.1	93.554	16.289	0
D13_DS.2	5-Year Storm	D13_DS.2	94.329	8.405	0
D13_US	5-Year Storm	D13_US	93.554	22.926	0
D13_US.1	5-Year Storm	D13_US.1	93.554	26.191	
D14_DS	5-Year Storm 5-Year Storm	D14_DS	94.328	13.29	54121.4
D14_U\$ D15_DS	5-Year Storm	D14_US D15_DS	94.328 94.273	15.055 30.828	0
D15_US	5-Year Storm	D15_US	94.277	13.91	0
D16.1_US	5-Year Storm	D16.1_US	94.256	38.407	0
D16.2_DS	5-Year Storm	D16.2_DS	94.255	39.641	0
		D16.2_D3	94.257		0
D16_DS D16_US	5-Year Storm 5-Year Storm	D16_US	94.256	38.283 31.875	0
D17_DS	5-Year Storm	D17_DS	94.258	30.135	0
D17_US	5-Year Storm	D17_US	94.258	35.785	0
D17_03	5-Year Storm	D18_DS	94.259	19.212	21002.8
D18_US	5-Year Storm	D18_US	94.259	28.886	0
D19_US	5-Year Storm	D19_US	93.201	7.63	0
D1_DS	5-Year Storm	D1_DS	96.36	29.782	0
D1_US	5-Year Storm	D1_US	96.36	19.351	0
D20_US	5-Year Storm	D20_US	91.686	27.473	43206.3
D21_DS	5-Year Storm	D21_DS	92.085	35.499	0
D21_US	5-Year Storm	D21_US	92.782	63.49	0
D22_DS	5-Year Storm	D22_DS	92.963	56.206	0
D22_US	5-Year Storm	D22_US	93.687	15.42	0
D23_DS	5-Year Storm	D23_DS	94.784	25.057	0
D23_US	5-Year Storm	D23_US	94.8	70.369	103999.1
D24_DS	5-Year Storm	D24_DS	94.719	34.047	0
D24_US	5-Year Storm	D24_US	94.719	27.764	0
D25_DS	5-Year Storm	D25_DS	94.718	29.384	0
D25_US	5 Vear Storm	D25_US	94.718	23.966	0
D26.1_US	5-Year Storm	D26.1_US	94.707	68.448	0
D26_DS	5-Year Storm	D26_DS	94.707	21.577	0
D26_US	5-Year Storm	D26_US	94.707	7.453	0
D27_DS	5-Year Storm	D27_DS	94.218	22.849	0
D27_US	5-Year Storm	D27_US	94.219	9.853	0
D28.1_U3	5-Year Storm	D28.1_U3	94.215	22.349	27303.58
D28_DS	5-Year Storm	D28_DS	94.177	39.298	0
D28_US	5-Year Storm	D28_US	94.22	45.488	0
D29_DS	5-Year Storm	D29_DS	93.925	30.349	0
D29_US	5-Year Storm	D29_US	93.938	36.681	0
D2_DS	5-Year Storm	D2_DS	95.042	48.276	735952.3
D2_US	5-Year Storm	D2_US	96.359	31.022	38528.54
D2_US.1	5-Year Storm	D2_US.1	95.045	46.175	0
D2_US.2	5-Year Storm	D2_US.2	96.355	29.967	107284.3
D2B_US	5-Year Storm	D2B_US	95.813	26.801	44644.8
D2B_US.1	5-Year Storm	D2B_US.1	95.759	27.378	115396.0
D30_DS	5-Year Storm	D30_DS	93.567	26.473	0
D30_US	5-Year Storm	D30_US	93.706	28.73	0
D31_DS	5-Year Storm	D31_DS	92.991	23.011	0
D31_US	5-Year Storm	D31_US	93.339	22.858	0
D3_DS	5-Year Storm	D3_DS	94.382	21.14	53040.64
D3_DS.1	5-Year Storm	D3_DS.1	95.042	35.586	19019.5
D3_US	5-Year Storm	D3_US	94.382	18.628	0
D4.1_DS	5-Year Storm	D4.1_DS	94.314	37.754	0
D4.1_US	5-Year Storm	D4.1_US	94.328	24.229	0
D4.1_US.1	5-Year Storm	D4.1_US.1	94.323	36.608	146708.4
D4_DS	5-Year Storm	D4_DS	94.315	34.991	0
D4_US	5-Year Storm	D4_US	94.338	21.84	0
D5_DS	5-Year Storm	D5_DS	94.316	29.224	13681.3
D5_US	5-Year Storm	D5_US	94.316	33.748	0
D6_DS	5-Year Storm	D6_DS	94.316	21.814	0
D6_US	5-Year Storm	D6_US	94.316	27.97	0
D7.1_US	5-Year Storm	D7.1_US	93.054	6.965	0
D7_DS	5-Year Storm	D7_DS	92.82	14.452	20502.6
D7_US D7_US.1	5-Year Storm	D7_US D7_US.1	93.566	10.507	38583.63
	5-Year Storm		93.567	12.415	24185.09
D8_DS	5-Year Storm	D8_DS	92.654	22.668	0
D8_US	5-Year Storm	D8_US	92.812	16.229	0
D_Co_Out	5-Year Storm	D_Co_Out	95.156 95.473	3.341	0
DW1N_Out	5-Year Storm	DW1N_Out	95.473 95.429	5.063	0
DW1S_Out	5-Year Storm	DW1S_Out	95.429	5.511 7.344	0
DW2N_US	5-Year Storm	DW2N_US	96.004		38101.1
DW2N_US.1	5-Year Storm	DW2N_US.1		6.957	
DW2S_DS Node650	5-Year Storm	DW2S_DS Node650	95.85	0	0
Node650	5-Year Storm	Node650	89.2 89.1	0	
Node651	5-Year Storm	Node651 Node652		18151 529	0
Node652 Node653	5-Year Storm		94.91	18151.529 35657.035	192104.6
Node653	5-Year Storm 5-Year Storm	Node653	95.035	35657.035 4.647	
Out 1	5-Year Storm 5-Year Storm	Out3.1	92.3 90.07	10.003	0
Out_1		Out_1			0
Out_1.1	5-Year Storm	Out_1.1	89.483	6.701	
Out_1.2	5-Year Storm	Out_1.2	90.008	10.779	0
Out_2	5-Year Storm	Out_2	88.267	12.022	0
Out 2.2	5-Year Storm	Out_2.2	87.872	8.316	
Out_2.2		Out 2	90 919	10 112	0.00
Out_2.2 Out_3 Out_4	5-Year Storm 5-Year Storm	Out_3 Out_4	90.918	9.516	0.06

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

NO. REVISIONS DATE NAME

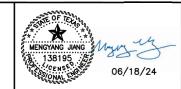
- Drainage Area Maps.dwg

Road\CAD\DWG\05- 04755

FORT BEND COUNTY ENGINEERING DEPARTMENT







PROJECT TITL	E: STELLA ROAD	
	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	RIPTION: PROPOSED CULVERT NODE RESULTS	
DRAWN BY: NS		DATE: 6/18/24
CK'D BY: MJ	SCALE: N/A	SHEET NO: 74 / 133

PROPOSED CULVERT LINK 10-YR RESULT

Teach Security Description Descripti							Max X-	Max	Maximum Water	Ma
1912 1974 1976 1918 1918 1976 7946 91489 1918				Node Name 0	Node Name 0	cfs	Area ft^2	ft/s	ft	E1
1919.1. 10 10 10 10 10 10 10	1317.1	10-Year Storm	D13_Out_CUI	D13.1_US	Out_1	13.936	1.796	7.945	93.899	
1941. 1974 1975	1330.1	10-Year Storm	D4_D5_Cul	D5_US	D4_DS	1.125	3.291	0.591	94.649	
1992 1974 1986 1965 1981 1981 1988 3448 1992 1981 1988 3448 1992 1981 1988 3448 1992 1981 1988 3448 1992 1981 1988 3448 1992 1981 1988 3448 1992 1981 1988 3448 1992 1988 3448 1992 1993 1988 3448 1992 1993 1988 3448 1992 1993	1344.1	10 Voor Storm	D7_D8_Cul	D7_D5	DB_US	1.41	1.723	0.019	92.886	F
1992 1974 1974 1986 1962 1994	1350.1	10-Year Storm	Rd_Cross3	D4.1_DS	D16.1_US	6.107	3.293	1.938	94.648	E
1991 19-Per Stem 19-00 10-10	1350.3	10-Year Storm	Rd_Cross3	D4.1_DS	D16.1_US	6.107	3.293	1.938	94.648	L
1971 10 10 10 10 10 10 10				D10_DS			3.149	1.29	95.974	L
1892.1 10-rear Storm 10-7,013,Cul 2012, 10-15 2012, 10-15 2013 2019 94-507 189-22 10-rear Storm 82, custod 07,015 1019,015 226-65 12013 2019 94-507 189-22 10-rear Storm 102,012,013 072,015 072	1372.1	10-Year Storm				-1.552		-1.168		
1892 15 15 16 16 17 18 18 18 18 18 18 18					D17_DS					L
1442.1 1.0 Pere 200m 0.0 1.0 2.0 1.0										H
1444.1 10 10 10 10 10 10 10	1414.1	10-Year Storm	D21 D22 Cul	D22 DS	D21 US	16.273	7.378	2.293	93.291	
1485.1 104-res from 1014-09 1013.1 56 861-1, 0921 912.13 5.114 8.855 99.859 99.66 1488.1 1047-095 90.172 10.172 10.172 99.66		10-Year Storm	D14_D15_Cu1		D15_US					
Const. Call 10-Year Shorm D1, D1, Call D1, D5 D1, D5 D1, D5 D D 0 0 0 0 0 0 0 0										
Cons. Cul. In-Vers-Somm D14,D15, Cul. D14,D15 D11,US D										-9
Const. Cult. 1 Veres Sterm 101, 015, 016, 017, 018, 016, 018 0 0 0 0 0 0 0 0 0	Cross_Cul	10-Year Storm	D14_D15_Cul	D14_DS	D15_US					4
Const. Cut 10-Year Storm 101, D17, Cut 101, D5 102, D5 103, D6 103, 00 104, 000, 009, 099 102, Cut 102, Cut 101, D5 102, D5 102, D6 103, D6 103, D7 103, D										-3
Current Curr	Cross_Cul4		D16_D17_Cul		D16_DS					-5
Cui_Coossi Interest Seem Dif_Dif_Coi Dif_US Dif_US Dif_DS Dif	Cul_Cross	10-Year Storm	D1_D2_Cul	D1_DS	D2_US	0	0	0	-9.00E+99	-3
DI Di Pera Storm Di Di Di Di Di Di Di D	Cul_Cross3	10-Year Storm	D17_D13_Cul	D18_US	D17_DS	0	0	0	-9.00E+99	-3
D10	Cul_Cross5	10-Year Storm	D4_D5_Cul	D5_US	D4_DS	0	0	0	-9.00E+99	-
Description Description	D10	10-Year Storm	D10	D10_US	D10_DS	6.69	29.54	0.407	95.979	F
Dig 1 Pear Storm Dig D	D11_D12_Cul	10-Year Storm	D11_D12_Cul	D11_DS	D12_US	-4.323	10.047	-0.558	95.921	E
D13.1.1 10 Fear-Storm D13.1. D13.05.2 D13.05 -0.07.2 10.28 -0.194 94.638 D13.1.1 D13.2.1 D13.2.1 D13.2.2 10 Fear-Storm D13.2 D13.2.5 D13.1.05 -0.595 42.707 0.058 93.899 93.899 0.101 D10 Fear-Storm D14 D14.05 D14.05 D13.2.0 3.346 -0.356 93.899 93.899 0.101 D10 Fear-Storm D14 D14.05 D14.05 D15.2.0 5.03 13.373 0.977 94.657 94.657 0.101				D12_US.1.1	D12_DS					F
D12.2 10-Year Storm D14 D14, D5 D14, D5 D14, D5 D14, D5 D14, D5 D14, D5 D14, D5 D14, D5 D14, D5 D14, D5 D14, D15 D14, D15 D14, D15 D14, D15 D14, D15 D15, D15 D15, D15 D15, D15 D15, D15, D15, D15, D15, D15, D15, D15,		10-Year Storm		D13_DS.2	D13_DS	-0.721	10.28	-0.194	94.658	
D15	D13.2	10-Year Storm	D13.2	D13_US.1	D13.1_US	-6.479	33.466	-0.326	93.899	F
Display	D15	10-Year Storm	D 15	D15_U3	D15_D5	3.927	15.525	3.964	94.614	F
D17	D16.1	10-Year Storm	D161	D16_DS	D16.1_US	-3.615	49.883	0.226	94.585	F
D18	D17	10-Year Storm	D17	D17_DS	D17_US	1.598	41.806	0.432	94.586	F
D21 16/Fear Storm D21 D2_US D2_US D3_US D3_FeA P3_US D3_FeS P3_US D3_US D18	10-Year Storm	D18	D18_DS	D18_US	3.673	22.149	0.64	94.587	F	
D22.2 10-Year Storm D2.1 D2, US.2 D28, US.2 D28, US.2 D28, US.2 D38, US.2	D2	10-Year Storm	D2	D2_US	D2_US.2	3.743	26.947	0.246	96.493	F
D22_L11_L1 10/Year Storm D22_L12_L1_D28_US_1 D22_US_1_L28_E1_2 25.512_L1_376_ 95.858_L20_D20_D21_L01_L01_L01_Year Storm D20_D20_D21_L01_D20_D20_D21_L01_D14_Was Storm D20_D20_D21_L01_D20_D20_D21_L01_D14_Was Storm D21_D21_D21_D20_D20_D21_L01_D20_D20_D21_L01_D20_D20_D21_L01_D14_Was Storm D21_D21_D21_D20_D20_D21_L01_D20_D20_D21_L01_D20_D20_D21_D20_D20_D21_D20_D20_D20_D20_D20_D20_D20_D20_D20_D20		10-Year Storm	D2.2	D2_US.2	D2B_US	10.643	3.29	3.368		F
D2012_LOL 16-Year-Storm D20_D2_LOL D21_DS D20_DS 15-99 S.09 S.23 92.485	D2.2.1.1	10-Year Storm	D2.2.1.1	D2B_US.1	D2_US.1	28.821	25.512	1.376	95.858	F
D22	D20_D21_Cul	10-Year Storm	D20_D21_Cul	D21_DS	D20_US	15.991	5.09	3.263	92.485	F
1922 10 10 10 10 10 10 10	D22	10-Year Storm	D22	D22_US	D22_DS	16.505	11.397	1.486	93.752	F
1025_DZ_S_CUI_ 104*ea*Storm 024_025_CUI_ 025_DS_ 024_US1.12_1848	D23_D24_Cul	10-Year Storm	D23_D24_Cul	D24_DS	D23_US	-2.551	0.82	-3.201	94.832	Ė
D25_D26_CUI 10*Vear*5torm D26_D26_D26_CUI D26_D5 D26_D5 0.1413 1.282 -1.144 9.4835 D26_D26_L1 10*Vear*5torm D26_D26_D26_D5 D26_D5 -1.465 6.665 6.565 6.03 9.4835 D26_L1 D26_	D24_D25_Cul	10-Year Storm	D24_D25_Cul	D25_DS	D24_US	-1.12	1.848	-0.632	94.834	F
D26.1 10/ear storm D26.1 D26.1 U5 D26.0 S -1.453 46.549 0.137 94.835 D26.1 D27 D	D25_D26_Cul	10-Year Storm	D25_D25_Cu1	D26_DS	D25_US	-1.413	1.282	-1.144	94.835	F
D27	D26.1	10-Year Storm	D26.1	D26.1_US	D26_DS	-1.455	46.549	-0.137	94.835	F
D28	D27	10-Year Storm	D27	D27_US	D27_DS	-0.544	23.266	0.084	94.313	F
Description Description	D28	10-Year Storm	D28	D28_US	D28.1_US	8.343	20.199	0.601	94.316	Ė
D39 D30 Cul 10/ear storm D3	D28_D29_Cul	10-Year Storm	D28_D29_Cul	D28_DS	D29_US	8.767	3.293	2.78	94.282	F
D31 10 Fear Storm D31 D3_DS.1 D2_DS 5.1292 80.744 D.24 9.544-01 D30_D31_CUI 10 Fear Storm D30_D31_CUI D30_DS D30_DS S30_DS S.754 11.529 0.79 9.384-01 D30_D31_CUI D30_DS D31_US S.756 S.754 12.59 0.79 9.384-01 D31_DS D31_US D31_DS S.756 S.756 S.234 2.707 9.364-01 D31_DS D31_US D31_DS S.756	D29_D30_Cul	10-Year Storm	D29_D3)_Cul	D29_DS	D30_US	8.759	3.292	2.763	94.003	F
1930 D11 Cul 16-Year Storm 1930 D31 Cul 1930 D5 1931 U5 18-Year Storm 1931 D11 U5 1931 D5 1931 D5 18-Year Storm 1941 D41 D41 U5 1941	D3.1	10-Year Storm	D3.1	D3_DS.1	D2_DS	-1.929	80.744	0.24	9.54E+01	9
D4	D30_D31_Cul	10-Year Storm	D30_D31_Cul	D30_DS	D31_US	8.756	3.234	2.707	9.36E+01	9
D4.1.1 10-Year-Storm D4.1.0 D4.1.0.5 D4.1.0.5 20.4.4 43.19 0.929 94.655	D4	10-Year Storm	D4	D4_US	D4.1_US	6.724	17.418	0.804	9.47E+01	9
Description	D4.1.1	10-Year Storm	D4.1.1	D4.1_US.1	D4.1_DS	20.43	43.19	0.929	94.655	9
Delta Delt	D5	10-Year Storm	D5	D5_DS	D5_US	1.099	33.165	0.493	9.46E+01	9
D7.1 10-Year-Storm D7.1 D7.1_US D7_DS 2.75.1 2.74 1.005 9.3.106 D7.2 D7_US D7_DS 2.75.1 2.74 1.005 9.3.106 D7.2 D7_US D7_US D7_US 2.786 7.184 0.404 9.36.32 D8 10-Year-Storm D8 D8_US D8_DS 2.742 3.191 0.86 9.2.874 D9 D8_DS 0.014 11.27 6.519 1.729 9.2.692 D.C.O.O.L. 1.005 0	D6_D7_Cul	10-Year Storm	D6_D7_Cul	D6_DS	D7_US	0.001	0.001	0.511	9.46E+01	9
D8	D7.1	10-Year Storm	D7.1	D7.1_US	D7_DS	2.751	2.74	1.005	93.106	9
D_CO_Out.	D8	10-Year Storm	D8	D8_US	D8_DS	2.742	3.191	0.86	92.874	F
December December	D_Co_OutL	10-Year Storm	D_Co_OutL	DW15_Out	D_Co_Out	1.186	1.168	1.016	95.487	F
DW2_N_N 16/Yes*Storm DW1_N DW2_N_U.S 1372 1.568 1.095 9.618+01	D_Out1.1	10-Year Storm	D_0ut1.1	Out_1	Out_1.2	13.936	13.999	1.001	90.102	
DW1_5 10-Year Storm DW1_5 DW2_5 DW3_5 W1_N	10-Year Storm	DW1_N	DW2N_US.1	DW2N_US	1.372	1.608	1.095	9.61E+01	9	
DW2_S 16-Year Storm DW2_S DW2_S DV_S DV	DW1_S	10-Year Storm	DW1_S	DW2S_DS	DW1S_Out	0	0.101	0	-9.00E+99	-
Link615 10-Year Storm Link615 Node650 Node651 0 0 0 9.006499	DW2_S	10-Year Storm	DW2_S	DW2S_DS	D1_US	0	0	0	-9.00E+99	-9
Unis617 10-Year Storm Unis617 Node653 Node652 7.856 5.143 1.857 95.221	Link615	10-Year Storm	Link615	Node650	Node651	0	0	0	-9.00E+99	
Out.2 ABCK 16/yas/Storm D162_0xt_2 D162_DS Out.2 0 0 0 0.9006499	Link617	10-Year Storm	Link617	Node653	Node652	7.856	5.143	1.587	95.221	9
Overflow 1 eVrex storm LinK71 03 US 03 05.1 0 0 0 0006+00 Deurflow Web 1 eVrex storm LinK64 4 8xin1, 07t2 0 - 1, 2 0 0 0 9004499 Rg_Cross1 1 eVrex storm Rg_Cross5 0 2 DS 0 13.1_US 38.67 5.13 7.859 95.419 Rg_Cross5 1 leVrex storm Rg_Cross5 0 19.0 0 0 0.906499 - RoadCross 1 eVrex storm 20,22 Cul 102.2 DS 0 0 0 0 0 0.906499	Out2.2RdCR	10-Year Storm	D16.2_0ut-2	D16.2_DS	Out_2	0	0	0	-9.00E+99	- 2
Rd_Cross1 16-Year Storm Rd_Cross1 D2_DS 013.1_US 38.67 5.13 7.859 95.419 Rd_Cross5 16-Year Storm Rd_Cross5 03.1_DS 08.05 8.768 3.077 2.858 93.062 RoadCross1 16-Year Storm 021_02Z_GU 022_US 021_US 0 0 0 -9.06+99 -	Overflow	10-Year Storm	Link711	D3_US	D3_DS.1	0	0	0	0.00E+00	(
RoadCross 10-Year Storm D21_D22_Cul D22_DS D21_US 0 0 0 -9.00E+99 -	Rd_Cross1	10-Year Storm	Rd_Cross1	D2_DS	D13.1_US	38.67	5.13	7.859	95.419	-
RoadCross2.1 10-Year Storm D22_D23_Cul D23_DS D22_US 13.447 0 0 -9.00E+99 -	RoadCross	10-Year Storm	D21_D22_Cul	D22_DS	D21_US	0	0	0	-9.00E+99	-3

PROPOSED CULVERT LINK 5-YR RESULT

Name 1311.1	Scenario 5-Year Storm	Link Name 0	upstream Node Name 0 D1_DS	Downstream Node Name 0	Max How cfs -0.583	Max X- sectional Area ft^2 3.291	Max velocity ft/s -0.216	Maximum Water Elevation (US) ft 96.36	Maximum Water Elevation (DS) ft 96.359
1317.1	5-Year Storm	D13_Out_CUI	D13.1_US	Out_1	13.078	1.796	7.505	93.554	90.823
1322.1 1330.1	5-Year Storm 5-Year Storm	D3_D4_Cul D4_D5_Cul	D3_DS D5_US	D4_US D4_DS	6.496 1.135	2.827 3.291	2.467 0.608	94.382 94.316	94.338
1334.1	5-Year Storm	D5_D6_Cul	D6_US	D5_DS	-1.538	3.291	-0.586	94.316	94.316
1544.1	5-Year Storm	D7_D8_Cul D7_D8_Cul	D7_D5 D7_DS	D8_US	1.141	1.592	0.717	92.82	92.812
1350.1	5-Year Storm	Rd_Cross3	D4.1_DS	D16.1_US	1.069 5.558	3.292	1.765	94.314	94.256
1350.2	5-Year Storm	Rd_Cross3	D4.1_DS	D16.1_US	5.558	3.292	1.765	94.314	94.256
1350.3 1354.1	5-Year Storm 5-Year Storm	Rd_Cross3 D16.2_Out-2	D4.1_DS D16.2_DS	D16.1_US Out_2	5.558 15.326	3.292 2.232	1.765 7.017	94.314 94.255	94.256
1359.1	5-Year Storm	D10_D11_Cul	D10_DS	D11_US	3.536	3.062	1.16	95.844	95.805
1367.1	5-Year Storm 5-Year Storm	D12_D13_CUI D13_D14_Cul	D12_D5	D13_US	-5.201 -1.171	2.993	-2.265 -1.137	93.555 94.329	93.554
1381.1	5-Year Storm	D16_D17_Cul	D17_US	D16_DS	-2.856	3.284	-0.907	94.258	94.257
1385.1	5-Year Storm	D17_D18_Cul	D18_US	D17_DS D19_US	1.83	3.291	0.93	94.259	94.258
1389.1 1389.2	5-Year Storm 5-Year Storm	Rd_Cross4 Rd_Cross4	D7_US D7_US	D19_US	2.287	0.895	1.926 2.417	93.566 93.566	93.283 93.201
1410.1	5-Year Storm	D22_D23_Cul	D23_DS	D22_US	3.124	0.816	3.967	94.784	93.687
1414.1	5-Year Storm 5-Year Storm	D21_D22_Cul D14_D15_Cul	D22_D6 D14_DS	D21_US D15_US	3.693	7.279	2.689	94.328	94.277
1444.1	5-Year Storm	D15_D16_Cul	D15_DS	D16_US	2.88	3.292	1.262	94.273	94.257
1445.1 1487.1	5-Year Storm 5-Year Storm	Rd_Cross2 Link633	D14_US D13.1_US	D4.1_US Basin1 Opt2	6.652 19.128	2.8 4.799	2.394 4.045	94.328 93.554	94.328
1488.1	5-Year Storm	Link634	Basin1_Opt2	Out_1.2	11.511	1.549	7.504	93.134	90.008
Coss_Cul	5-Year Storm	D10_D11_Cul	D10_DS	D11_US	0	0	0	-9.00E+99	-9.00E+9
Cross_Cul Cross_Cul1	5-Year Storm 5-Year Storm	D14_D15_Cul D12_D13_CUI	D14_DS D12_DS	D15_IIS D13_US	0	0	0	-9.00E+99	-9.00E+9
Cross_Cul2.1	5-Year Storm	D13_D14_Cul	D13_DS	D14_US	0	0	0	-9.00E+99	-9.00E+9
Cross_Cul3 Cross_Cul4	5-Year Storm 5-Year Storm	D15_D16_Cul D16_D17_Cul	D15_DS D17_US	D16_US D16_DS	0	0	0	-9.00E+99 -9.00E+99	-9.00E+9
CrossCul	5-Year Storm	CrossCul	D12_US.1	D2B_US.1	5.422	3.307	1.64	95.798	95.759
Cul_Cross	5-Year Storm	D1_D2_Cul	D1_DS	D2_US	0	0	0	-9.00E+99	-9.00E+9
Cul Cross2 Cul_Cross3	5-Year Storm 5-Year Storm	D3 D4 Cul D17_D18_Cul	D3 DS D18_US	D4 US D17_DS	0	0	0	-9.00E+99 -9.00E+99	-9.00E+9
Cul_Cross4	5-Year Storm	D5_D6_Cul	D6_US	D5_DS	0	0	0	-9.00E+99	-9.00E+9
Cul_Cross5 D1	5-Year Storm 5-Year Storm	D4_D5_Cul D1	D5_US D1_US	D4_DS D1_DS	-0.742	0 21.136	0.089	-9.00E+99 96.36	-9.00E+9
D10	5-Year Storm	D10	D10_US	D10_DS	5.95	25.367	0.752	95.85	95.844
D11 D11 D12 Cul	5-Year Storm	D11 D11 D12 Cul	D11_US D11_DS	D11_DS D12_US	3.432 -4.358	62.686 9.489	0.068 -0.605	95.805 95.804	95.804
D12.1	5-Year Storm 5-Year Storm	D12.1	D12_US	D12_US.1	5.544	9.489 62.998	0.114	95.798	95.798
D12.1.1.1	5-Year Storm	D12.1.1.1	D12_US.1.1	D12_DS	-2.492	51.12	-0.095	93.555	93.555
D13 D13.1	5-Year Storm 5-Year Storm	D13 D13.1	D13_US D13_DS.2	D13_US.1	-5.299 -0.43	16.978 5.84	-0.491 -0.168	93.554 94.329	93.554 94.329
D13.1.1	5-Year Storm	D13.1.1	D13_DS.1	D13.1_US	-0.828	32.629	-0.054	93.554	93.554
D13.2 D14	5-Year Storm 5-Year Storm	D13.2 D14	D13_US.1 D14_DS	D13.1_US D14_US	-5.539 4.333	25.65 8.201	-0.312 0.947	93.554 94.328	93.554
D15	5-Year Storm	D15	D15_US	D15_DS	3.529	9.955	4.818	94.277	94.273
D16	5-Year Storm	D16	D16_US D16_DS	D16.1_US	2.67	29.429	3.024	94.257 94.257	94.256
D16.1 D16.2	5-Year Storm 5-Year Storm	D16.1 D16.2	D16_US	D16.1_US D16.2_DS	-2.995 15.437	40.685 75.916	0.256	94.257	94.256 94.255
D17	5-Year Storm	D17	D17_DS	D17_US	1.48	33.893	0.508	94.258	94.258
D17_Out-2 D18	5-Year Storm 5-Year Storm	D17_Out-2 D18	D17_US D18_DS	Out_2 D18 US	3.3	31.93 15.364	0.731	94.258 94.259	92.8 94.259
D19	5-Year Storm	D19	D19_US	Out3.1	4.01	3.085	1.3	93.201	92.3
D2 D2.1	5-Year Storm 5-Year Storm	D2 D2.1	D2_US D2_US.1	D2_US.2 D2_DS	3.108 19.887	24.231 61.327	0.225	96.359 95.045	96.355 95.042
D2.2	5-Year Storm	D2.2	D2_US.2	D2B_US	10.372	3.287	3.178	96.355	95.813
D2.2.1 D2.2.1.1	5-Year Storm 5-Year Storm	D2.2.1 D2.2.1.1	D2B_US D2B_US.1	D2B_US.1 D2_US.1	13.957 26.972	18.289 22.158	0.862 1.39	95.813 95.759	95.759 95.045
D20	5-Year Storm	D20	D20_US	Out_3	13.626	11.472	1.188	91.686	90.918
D20_D21_Cul D21	5-Year Storm 5-Year Storm	D20_D21_Cul D21	D21_D5 D21_US	D20_U5 D21_DS	12.846	5.083 7.299	2.527 1.835	92.085 92.782	91.686
D22	5-Year Storm	D22	D21_US	D21_D3	13.127	9.993	1.332	93.687	92.963
D23	5-Year Storm	D23	D23_US	D23_DS	13.132	30.457	0.431	94.8	94.784
D23_D24_Cul D24	5-Year Storm 5-Year Storm	D23_D24_Cul D24	D24_DS D24_US	D23_US D24_DS	-2.566 -1.623	0.82 32.72	-3.22 0.49	94.719 94.719	94.8
024_D25_Cul	5-Year Storm	D24_D25_Cul	D25_DS	D24_US	-1.126	1.848	-0.636	94.718	94.719
D25 D25 D26 Cul	5-Year Storm 5-Year Storm	D25 D25 D26 Cul	D25_U5 D26_DS	D25_DS D25_US	-1.28 -1.441	19.478	-0.221 1.578	94.718 94.707	94.718 94.718
D26	5-Year Storm	D25 D26 Cui	D26_US	D26.1_US	-0.738	50.979	0.031	94.707	94.707
D26.1	5-Year Storm 5-Year Storm	D26.1	D26.1_US D28 US	D26_DS	-1.486	38.363	-5.798	94.707	94.707
D26.1_D28_Cul D27	5-Year Storm 5-Year Storm	D26.1_D28_Cul D27	D28_US D27_US	D26.1_US D27 DS	-8.145 -0.392	3.28 19.784	-2.567 0.08	94.22 94.219	94.707
D27_D28.1_Cul	5-Year Storm	D27_D28.1_Cul	D27_DS	D28.1_US	-0.377	2.991	-0.404	94.218	94.216
D28 D28.1	5-Year Storm 5-Year Storm	D28 D28.1	D28_US D28.1_US	D28.1_US D28 DS	9.137 9.125	18.457 26.893	1.378 0.472	94.22 94.216	94.216
D28_D29_Cul	5-Year Storm	D28_D29_Cul	D28_DS	D29_US	8.288	3.293	2.629	94.177	93.938
D29 D29_D30_Cul	5-Year Storm 5-Year Storm	D29 D29_D30_Cul	D29_US D29_DS	D29_DS D30_US	8.283 8.28	15.309 3.292	0.605 2.547	93.938 93.925	93.925 93.706
D3	5-Year Storm	D29_D30_Cui	D29_DS D3_US	D30_DS	-0.514	11.071	0.105	94.382	94.382
D3.1	5-Year Storm	D3.1	D3_DS.1	D2_DS	1.276	53.252	0.201	95.042	95.042
D30 D30_D31_Cul	5-Year Storm 5-Year Storm	D30 D30_D31_Cul	D30_US D30_DS	D30_DS D31_US	8.273	10.838 3.199	0.787 2.586	93.706 93.567	93.567
D31	5-Year Storm	D31	D31_US	D31_DS	8.273	6.212	1.34	93.339	92.991
D4 D4.1	5-Year Storm 5-Year Storm	D4 D4.1	D4_US D4.1 US	D4.1_US D4.1_US.1	6.237 8.298	12.379 16.791	0.832	94.338 94.328	94.328
D4.1.1	5-Year Storm	D4.1.1	D4.1_US.1	D4.1_DS	18.585	34.18	0.983	94.323	94.314
D4.2 D5	5-Year Storm 5-Year Storm	D4.2 D5	D4_DS D5_DS	D4.1_DS	1.303	34.819	0.314	94.315	94.314
D5 D6	5-Year Storm 5-Year Storm	D5 D6	D5_DS D6_DS	D5_US D6_US	1.236 -0.642	25.697 18.547	0.512 0.251	94.316 94.316	94.316 94.316
D6_D7_Cul	5-Year Storm 5-Year Storm	D6_D7_Cul	D6_DS D7_US	D7_US D7_US.1	0	0.001	0.429	94.316 93.566	93.566
D7 D7.1	5-Year Storm 5-Year Storm	D7 D7.1	D7_US D7.1_US	D7_US.1 D7_DS	0.293	6.57 2.337	0.071	93.566	93.567
D7.2	5-Year Storm	D7.2	D7_US.1	D7.1_US	2.321	6.232	0.373	93.567	93.054
D8	5-Year Storm 5-Year Storm	D8 D9	D8_US D8_DS	D8_DS Out 4	2.205	2.873 6.128	0.769 1.671	92.812 92.654	92.654
D_Co_OutL	5-Year Storm	D_Co_OutL	DW1S_Out	D_Co_Out	0.843	0.919	0.917	95.429	95.156
D_CoWo D_Out1.1	5-Year Storm 5-Year Storm	D_CoWo D Out1.1	DW1N_Out Out_1	DW1S_Out	0.843 13.078	0.675	1.25 0.994	95.473 90.07	95.429
D_Out1.1 D_Out1.1.1	5-Year Storm 5-Year Storm	D_Out1.1.1	Out_1 Out_1.2	Out_1.2 Out_1.1	24.303	13.207	1.758	90.07	90.008 89.483
DW1_N	5-Year Storm	DW1_N	DW2N_US.1	DW2N_US	1.243	1.24	1.095	9.60E+01	9.59E+0
DW1_N.1 DW1 S	5-Year Storm 5-Year Storm	DW1_N.1 DW1 S	DW2N_US.1 DW2S_DS	DW1N_Out DW1S Out	0.845	1.172 0.078	0.721	96.004 -9.00E+99	95.473 -9.00E+5
DW2_N	5-Year Storm	DW2_N	DW2N_US	D10_US	1.229	0.816	2.292	9.59E+01	9.59E+0
DW2_S	5-Year Storm 5-Year Storm	DW2_5	DW25_DS	D1_US	0	0	0	-9.00E+99	-9.00E+9
Emergency Overflow L_Sheriff_Out	5-Year Storm 5-Year Storm	Link634 L_Sheriff_Out	Basin1_Opt2 Sheriff_Outfall	Out_1.2 Basin1_Opt2	57.745	7.567	3.839	-9.00E+99 93.877	-9.00E+9
Link615	5-Year Storm	Link615	Node650	Node651	0	0	0	-9.00E+99	-9.00E+9
Link616 Link617	5-Year Storm 5-Year Storm	Link616 Link617	Node652 Node653	D23_US Node652	5.923 6.337	5.135 5.139	1.198	9.49E+01 95.035	9.48E+0
Link618	5-Year Storm	Link618	Node653	D26.1_US	9.135	5.136	1.847	95.035	94.707
Out2.2RdCR Out_2.2L	5-Year Storm	D16.2_Out-2	D16.2_DS	Out_2	15 226	7 076	1 022	-9.00E+99	-9.00E+9
	5-Year Storm	Out_2.2L	Out_2	Out_2.2	15.326	7.976	1.922	8.83E+01 0.00E+00	8.79E+0 0.00E+0
Overflow	5-Year Storm	Link711	D3_US	D3_DS.1					
	5-Year Storm 5-Year Storm 5-Year Storm	Rd_Cross1 Rd_Cross5	D3_US D2_DS D31_DS	D3_D5.1 D13.1_US D8_DS	36.6 8.28	5.102 3.01	7.479 2.756	95.042	93.554 92.654

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

REVISIONS DATE NAME NO.

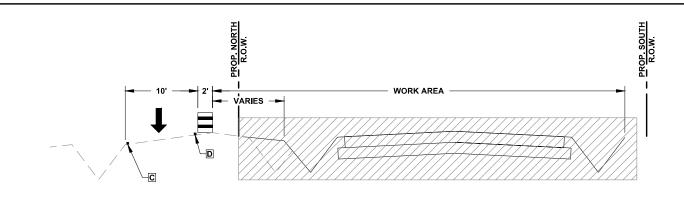
FORT BEND COUNTY ENGINEERING DEPARTMENT



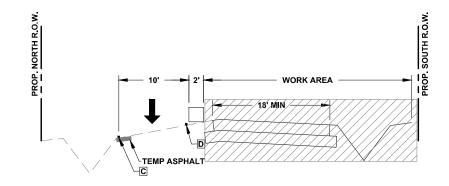




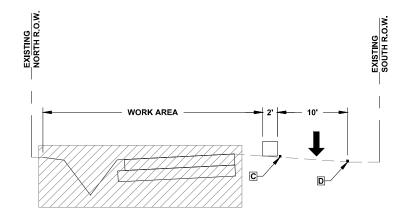
ROJECT II	TLE:	STELLA ROAD)			
FROM (COTTONWOOD	SCHOOL ROAD	ТО	BAND	ROAD	CIVIL STANDARD
SHEET DESC		CULVERT LINK RES	SULT	s		
RAWN BY: NS						DATE: 6/18/24
K'D BY: MJ	SCALE:	N/A				SHEET NO: 75 / 133



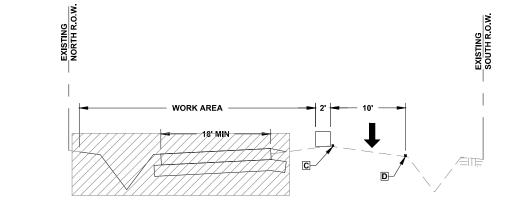
PHASE 1 STEP 1 STA. 38+70 TO STA. 45+50



PHASE 1 STEP 1 STA. 46+75 TO END



PHASE 1 STEP 2 STA. 4+70 TO STA. 6+30



NORTH R.O.W		SOUTH R.O.W.
PROP	WORK AREA	VARIES — 10'

PHASE 1 STEP 1

STA. 6+30 TO STA. 26+00

PHASE 1 STEP 1 STA. 26+00 TO STA. 35+60

NO.	REVISIONS	DATE	NAME	
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FORT BEND COUNTY TEXAS







PROJECT TITL		
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY:	SHEET DESCRIPTION:	
AM	TCP TYPICAL SECTION PHASE 1	
SCALE:		
N.T.S.		SHEET NO:
DATE: 1/16/2023	APPROVED BY:	76 / 133

LEGEND

EXIST. R.O.W.
PROP. R.O.W.
PROP. TRAFFIC FLOW
EXIST. TRAFFIC FLOW

CONSTRUCTION PHASE 1

CONSTRUCTION PHASE 2

PORT CTB LOW PROFILE (LPCB) TY 1

PORT CTB LOW PROFILE (LPCB) TY 2 TYPE III BARRICADE

CHANNELIZING DEVICE

4" WHITE SOLID REMOVABLE

4" YELLOW SOLID REMOVABLE

4" WHITE SOLID NON-REMOVABLE

4" YELLOW SOLID NON-REMOVABLE

TEMP ASPHALT

24" WHITE SOLID

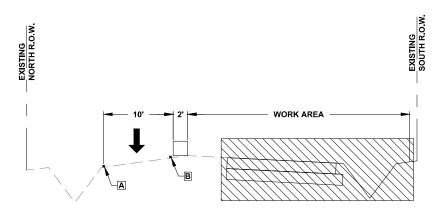
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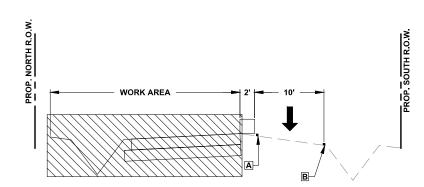
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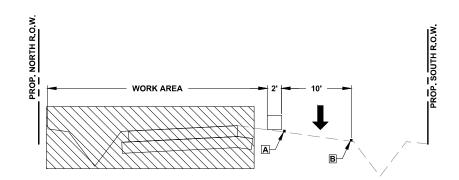
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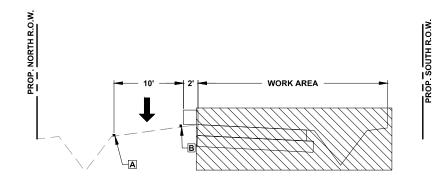
PHASE 2 STEP 1 STA. 4+70 TO STA. 13+08 STA. 14+48 TO STA. 26+00



PHASE 2 STEP 1 STA. 46+75 TO END



PHASE 2 STEP 1 STA. 36+50 TO STA. 38+70



PHASE 2 STEP 2 STA. 35+60 TO STA. 38+14

LEGEND

EXIST. R.O.W.

– – – PROP. F

PROP. TRAFFIC FLOW

EXIST. TRAFFIC FLOW

CONSTRUCTION PHASE 1

CONSTRUCTION PHASE 2

PORT CTB LOW PROFILE (LPCB) TY 1

PORT CTB LOW PROFILE

В

(LPCB) TY 2

TYPE III BARRICADE

CHANNELIZING DEVICE



TEMP ASPHALT

Α

С

4" WHITE SOLID NON-REMOVABLE

4" YELLOW SOLID REMOVABLE

4" WHITE SOLID REMOVABLE

D

4" YELLOW SOLID NON-REMOVABLE

E

24" WHITE SOLID

	NO.	REVISIONS	DATE	NAME	
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FORT BEND COUNTY TEXAS







PROJECT TITL	PROJECT TITLE: STELLA ROAD					
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.					
CK'D BY:	SHEET DESCRIPTION: TCP TYPICAL SECTION PHASE 2					
SCALE: N.T.S.		SHEET NO:				
DATE: 1/16/2023	APPROVED BY:	77 / 133				

TRAFFIC CONTROL PLAN NARRATIVE:

PHASE 1, STEP 1:

- SET LOW PROFILE CONCRETE BARRIER AND INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS SHOWN IN THE TRAFFIC CONTROL PLAN.
- 2) CONSTRUCT PAVEMENT, STORM SEWER AND DITCHES FROM STA 0+00 TO 4+70 (ROW TO ROW), AND 6+00 TO END (PROPOSED POADWAY)
- 3) CONSTRUCT TEMPORARY ASPHALT PAVEMENT AT THE LOCATION SHOWN.

PHASE 1, STEP 2:

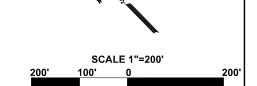
- SET LOW PROFILE CONCRETE BARRIER AND INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS SHOWN IN THE TRAFFIC CONTROL PLAN.
- 2) CONSTRUCT PAVEMENT, STORM SEWER AND DITCHES FROM STA 4+70 TO 6+00 (ROW TO ROW)

PHASE 2, STEP 1:

- SET LOW PROFILE CONCRETE BARRIER AND INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS SHOWN IN THE TRAFFIC CONTROL PLAN.
- CONSTRUCT PAVEMENT, STORM SEWER AND DITCHES FROM STA 6+00 TO END (EXISTING ROADWAY).
- 3) CONSTRUCT TEMPORARY ASPHALT PAVEMENT AT THE LOCATION SHOWN.

PHASE 2, STEP 2:

- SET LOW PROFILE CONCRETE BARRIER AND INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS SHOWN IN THE TRAFFIC CONTROL PLAN.
- CONSTRUCT PAVEMENT, STORM SEWER AND DITCHES FROM STA 35+60 TO 38+20 (SOUTH SIDE OF PROPOSED ROADWAY).



LEGEND

_____ PROP. R.O.W.

— - - — EXIST. R.O.W.

PROP. TRAFFIC FLOW

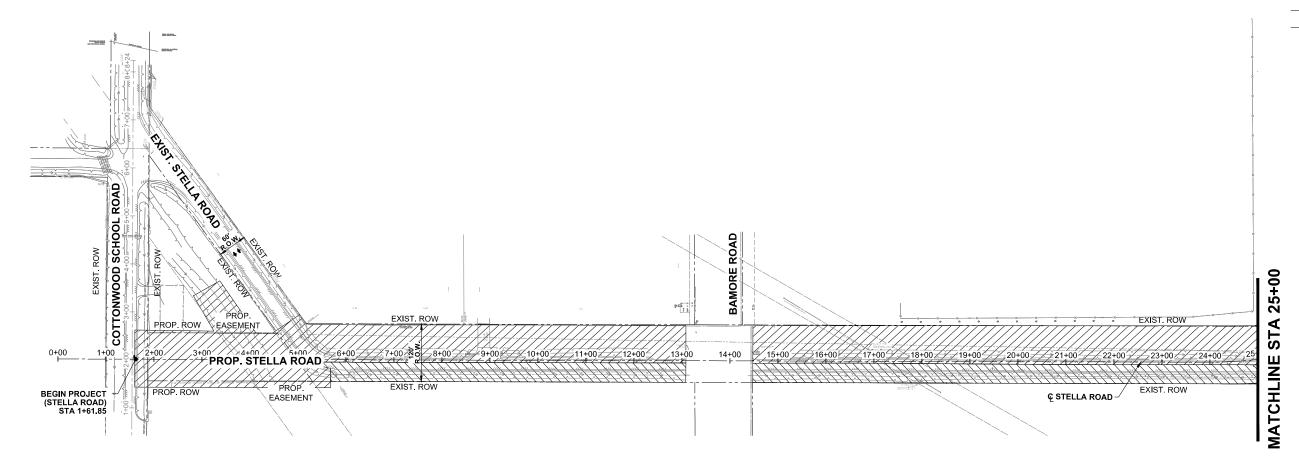
EXIST. TRAFFIC FLOW

CONSTRUCTION PHASE 1

CONSTRUCTION PHASE 2

GENERAL NOTES:

- MAINTAIN ACCESS WITH CRUSHED LIMESTONE TO ALL RESIDENTS AND BUSINESSES. SEE TYPICAL DRIVEWAY ACCESS DETAIL.
- CONTRACTOR TO MAINTAIN DRAINAGE DURING ALL PHASES OF CONSTRUCTION.
- 3) SEE SHEET 85 FOR EASTBOUND TRAFFIC DETOUR.



NO. REVISIONS DATE NAME

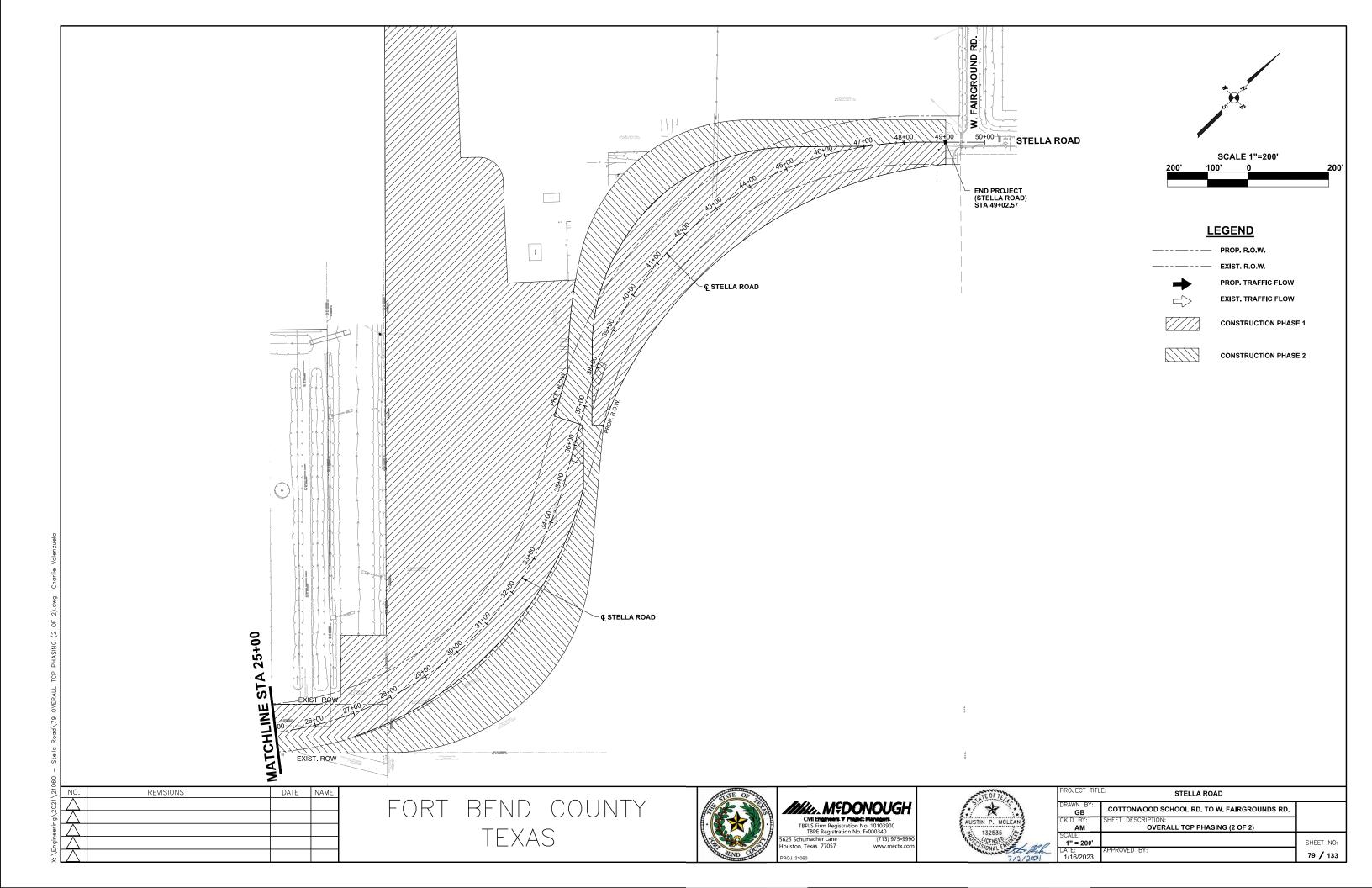
FORT BEND COUNTY TEXAS

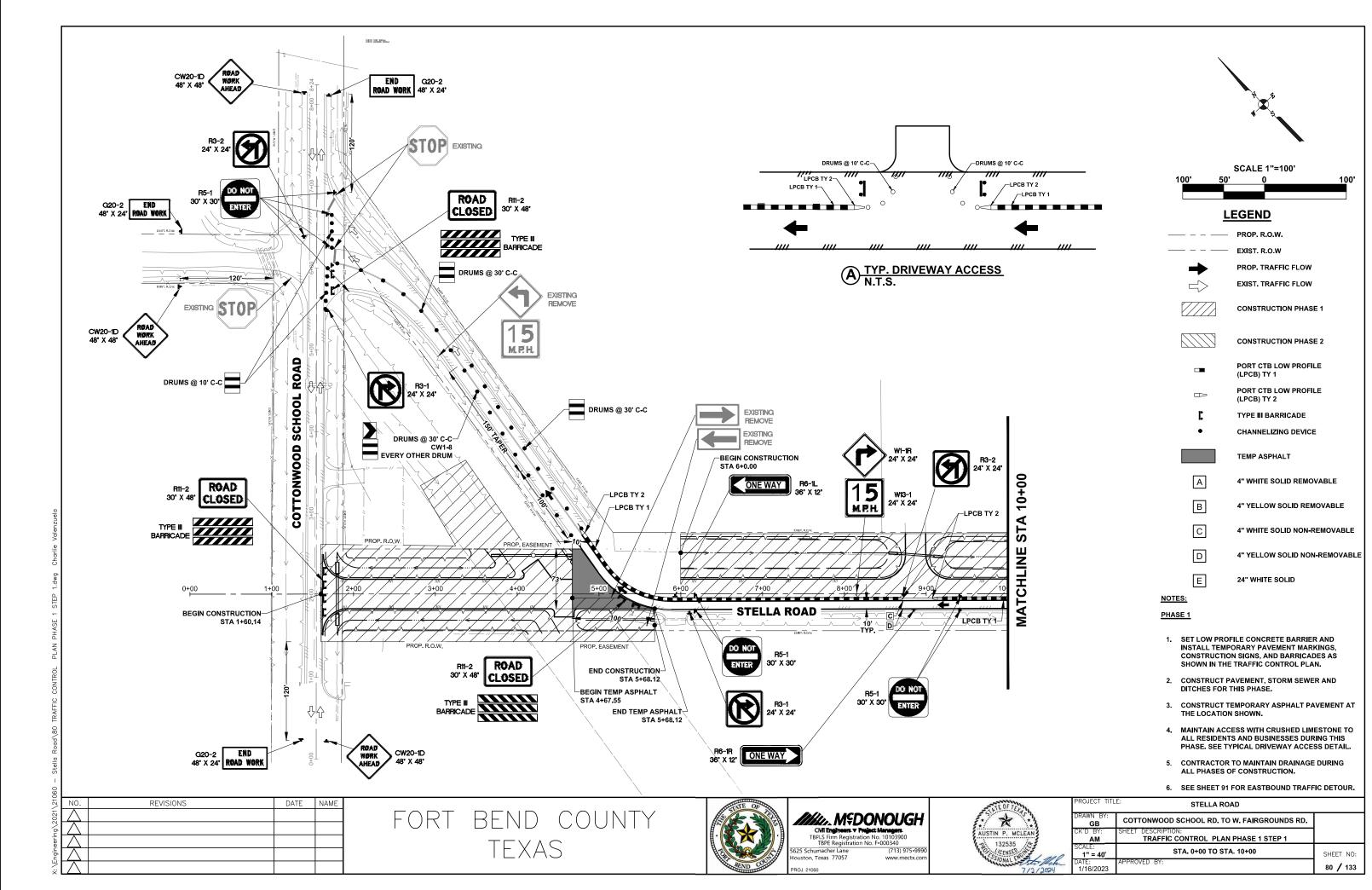


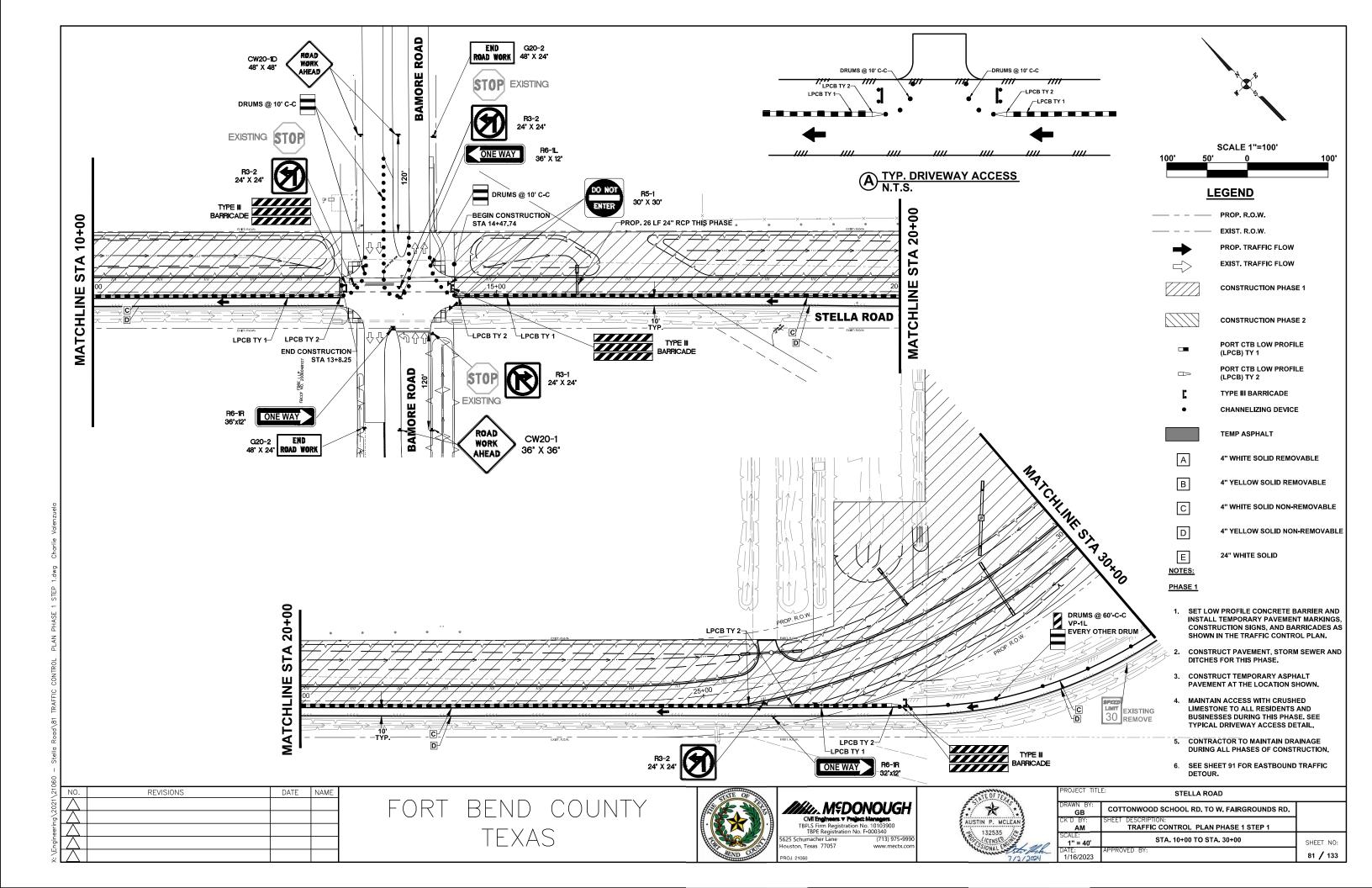


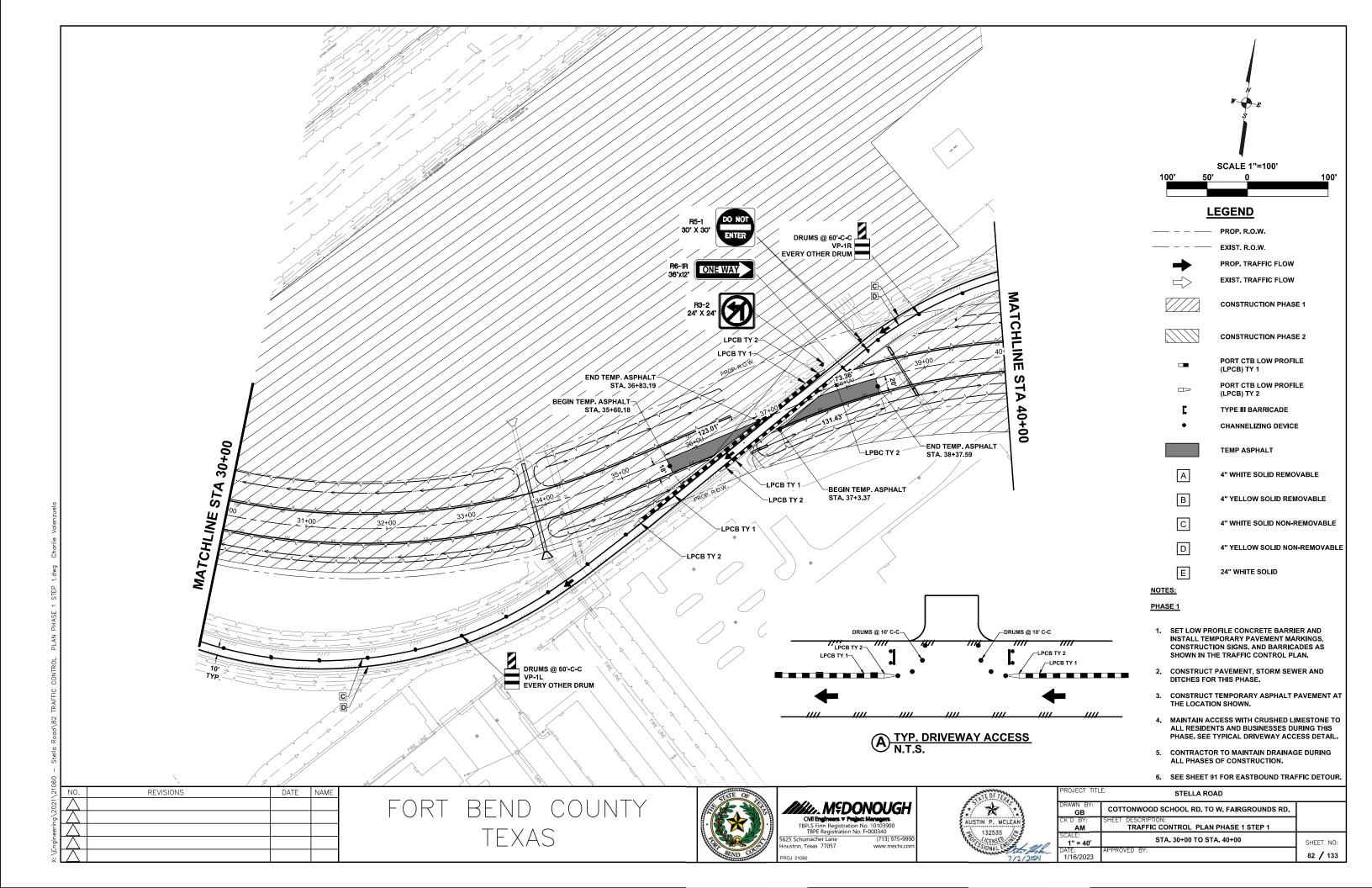


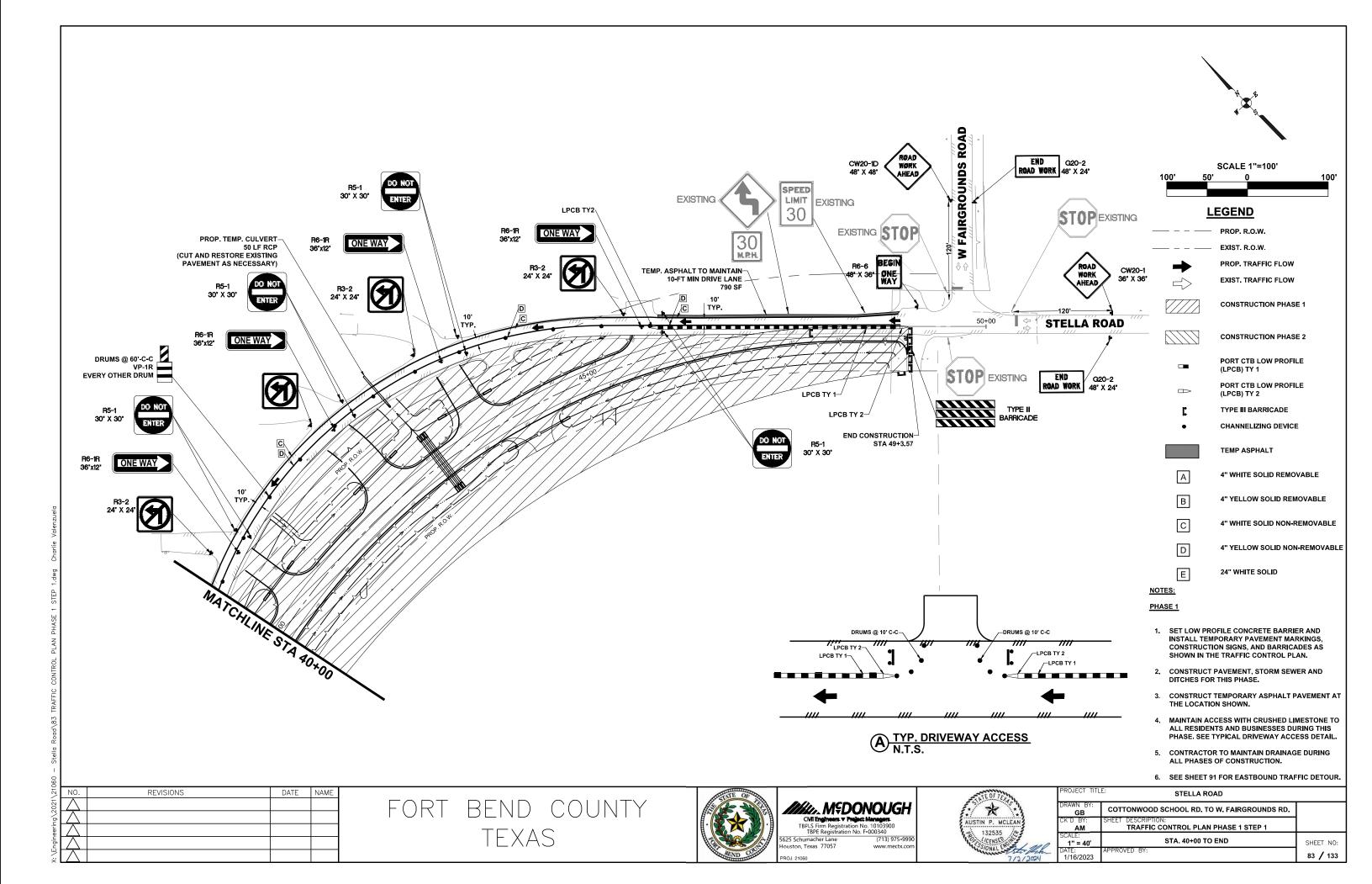
	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY:	SHEET DESCRIPTION: OVERALL TCP PHASING (1 OF 2)	
,	SCALE: 1" = 200'		SHEET NO:
1	DATE: 1/16/2023	APPROVED BY:	78 / 133

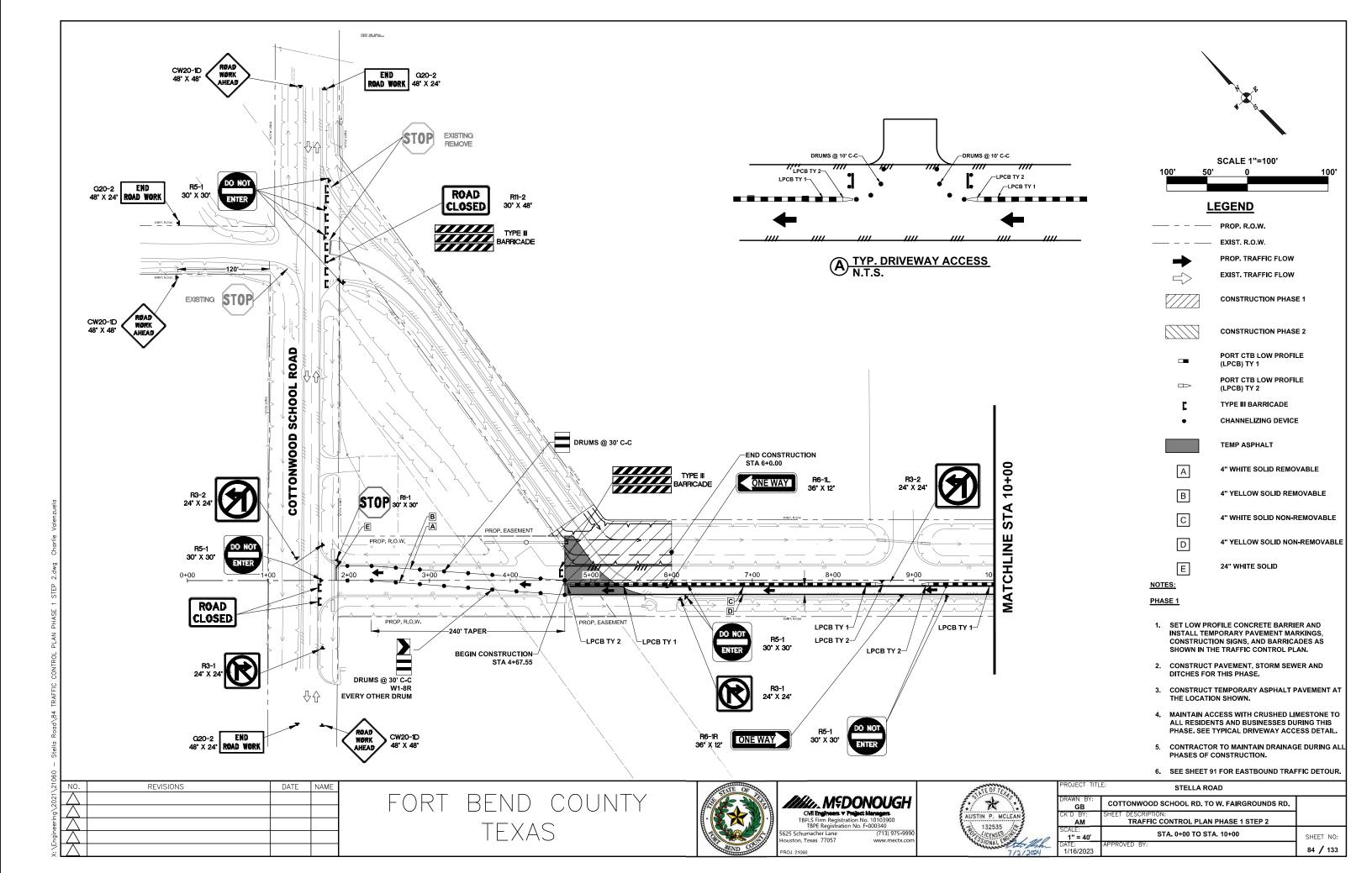


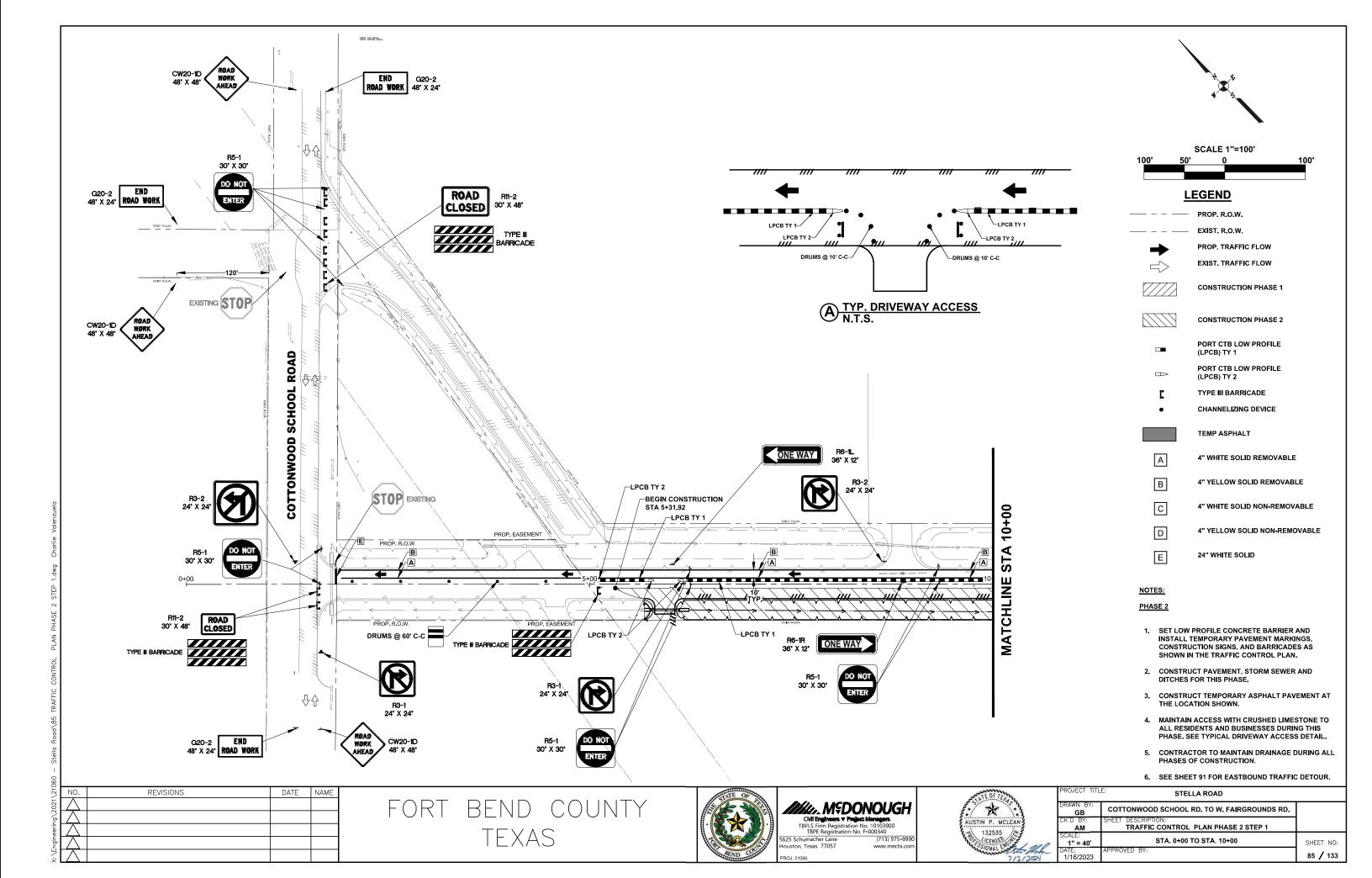


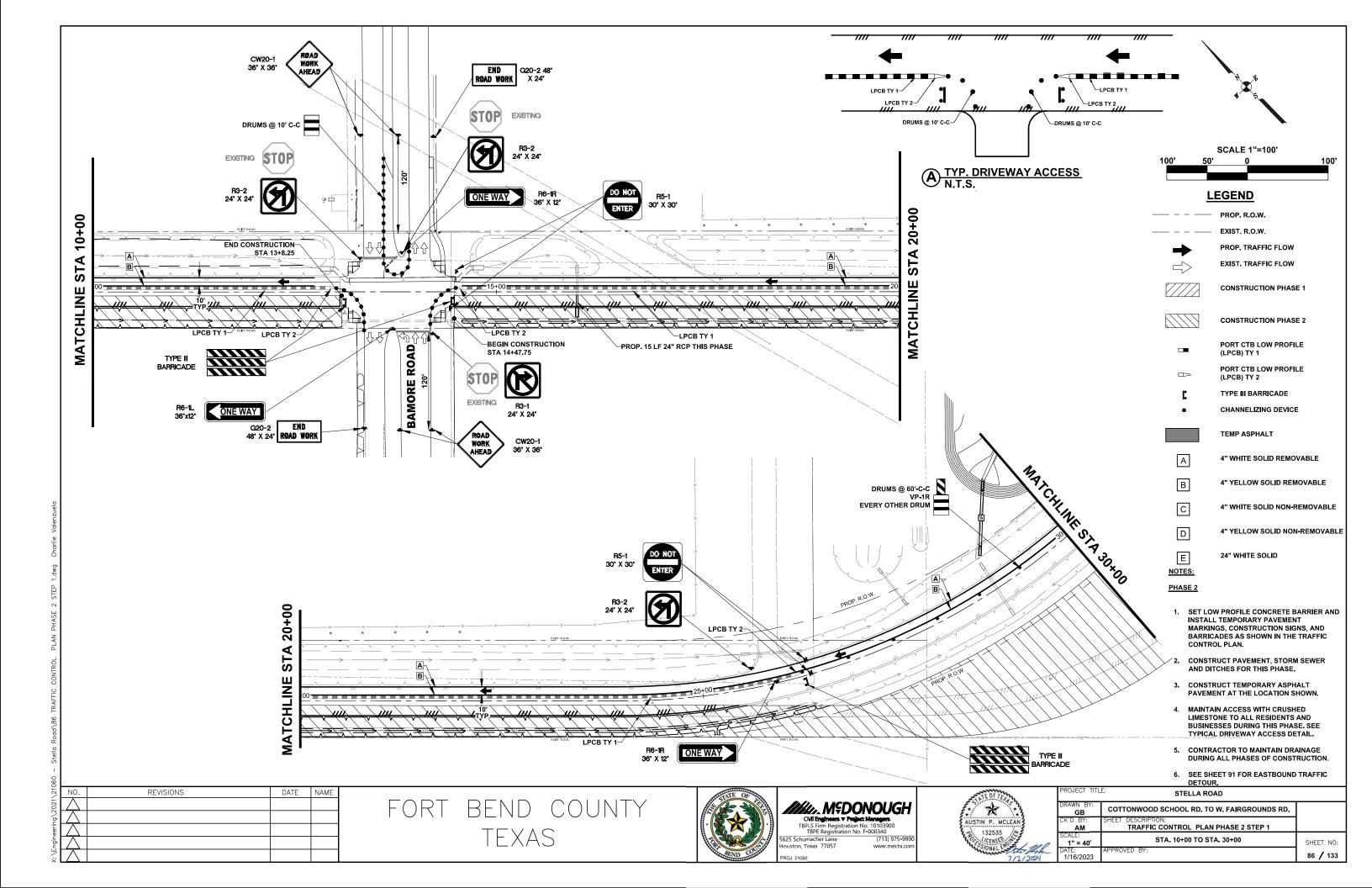


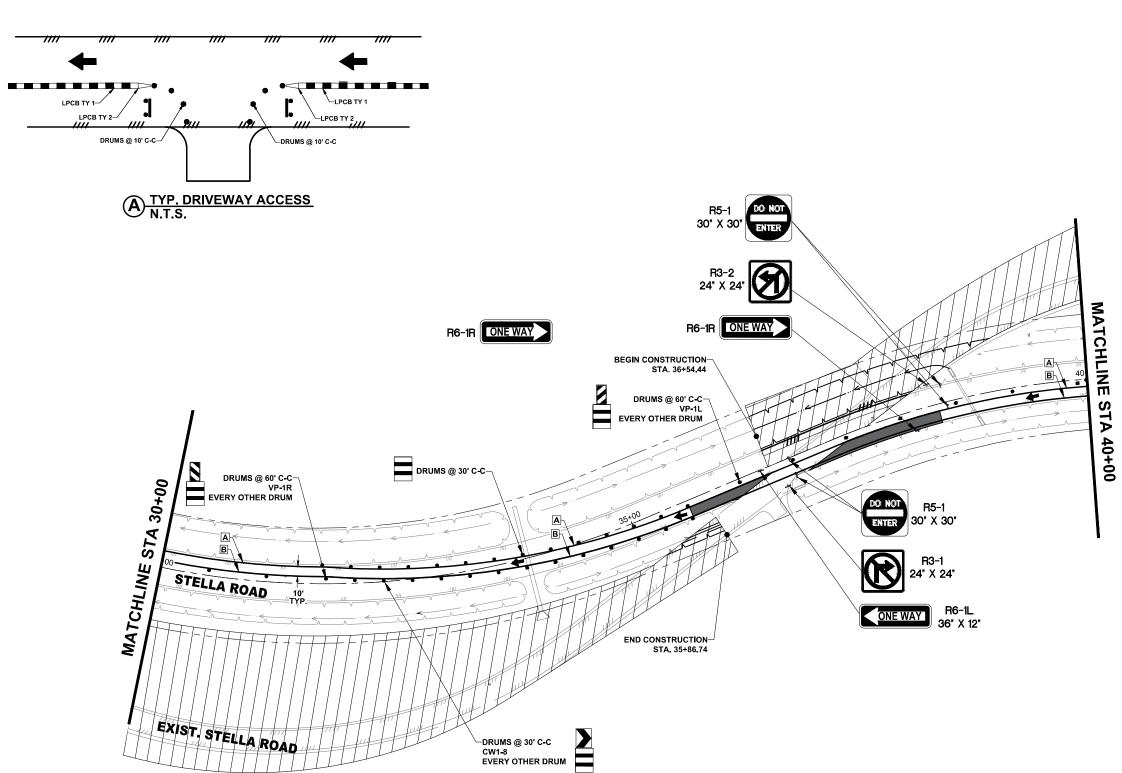


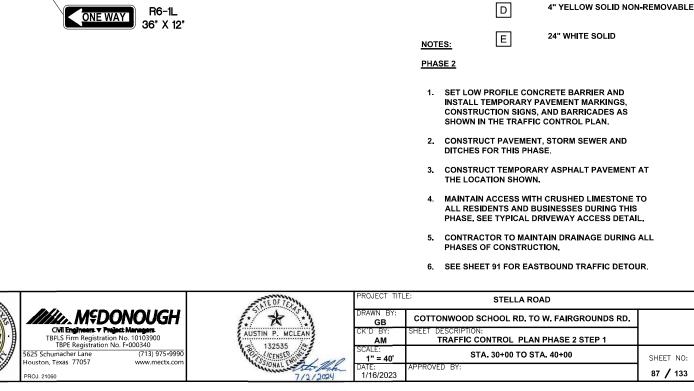












REVISIONS DATE NAME

FORT BEND COUNTY TEXAS



PROJECT TITL	E: STELLA ROAD	
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY:	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 2 STEP 1	
SCALE: 1" = 40'	STA. 30+00 TO STA. 40+00	SHEET NO:
DATE: 1/16/2023	APPROVED BY:	87 / 133

SCALE 1"=100'

CONSTRUCTION PHASE 1

CONSTRUCTION PHASE 2

PORT CTB LOW PROFILE

PORT CTB LOW PROFILE

4" WHITE SOLID REMOVABLE

4" YELLOW SOLID REMOVABLE

4" WHITE SOLID NON-REMOVABLE

TYPE III BARRICADE **CHANNELIZING DEVICE**

TEMP ASPHALT

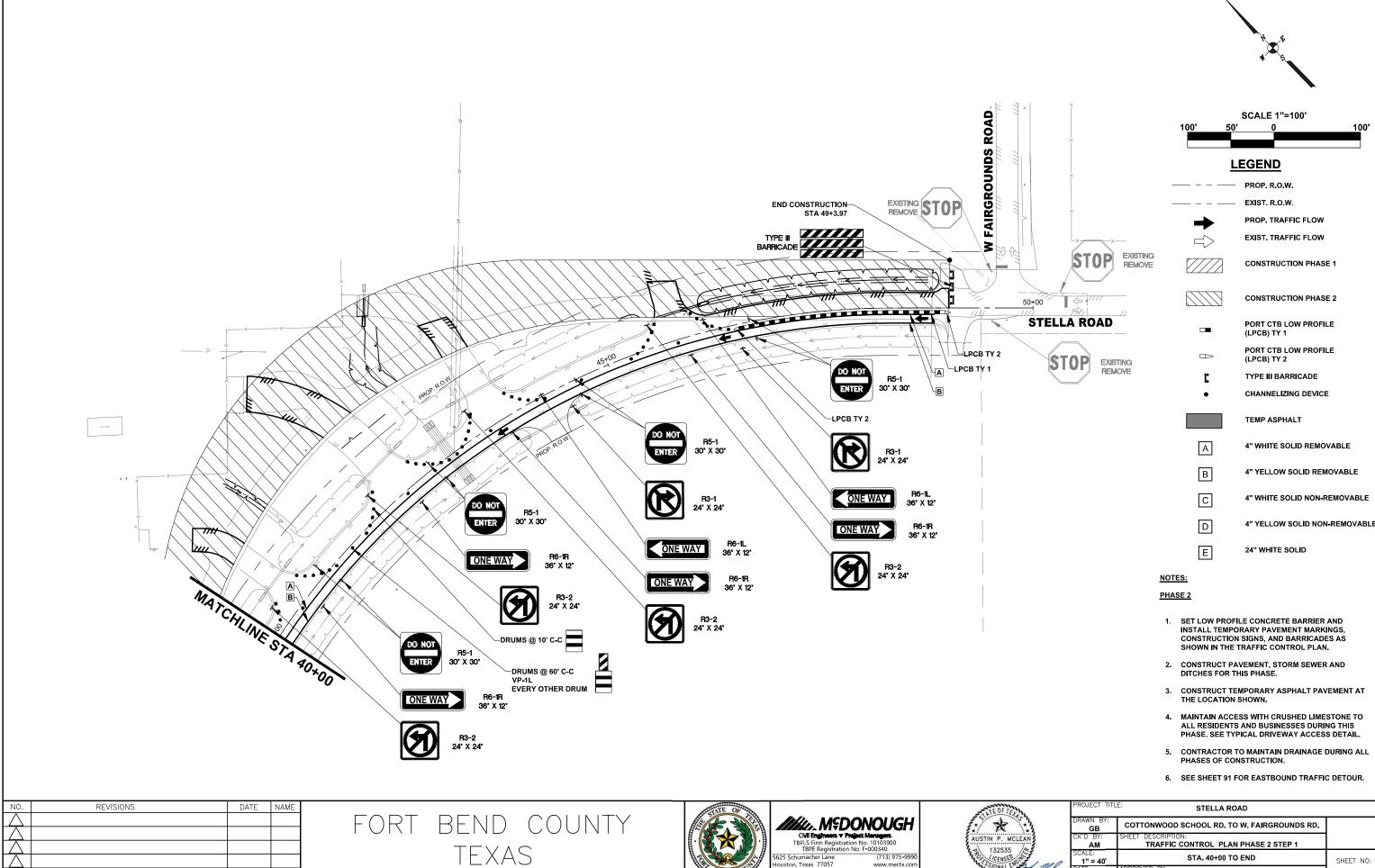
В

С

(LPCB) TY 1

(LPCB) TY 2

LEGEND PROP. R.O.W. EXIST. R.O.W. PROP. TRAFFIC FLOW EXIST. TRAFFIC FLOW 100



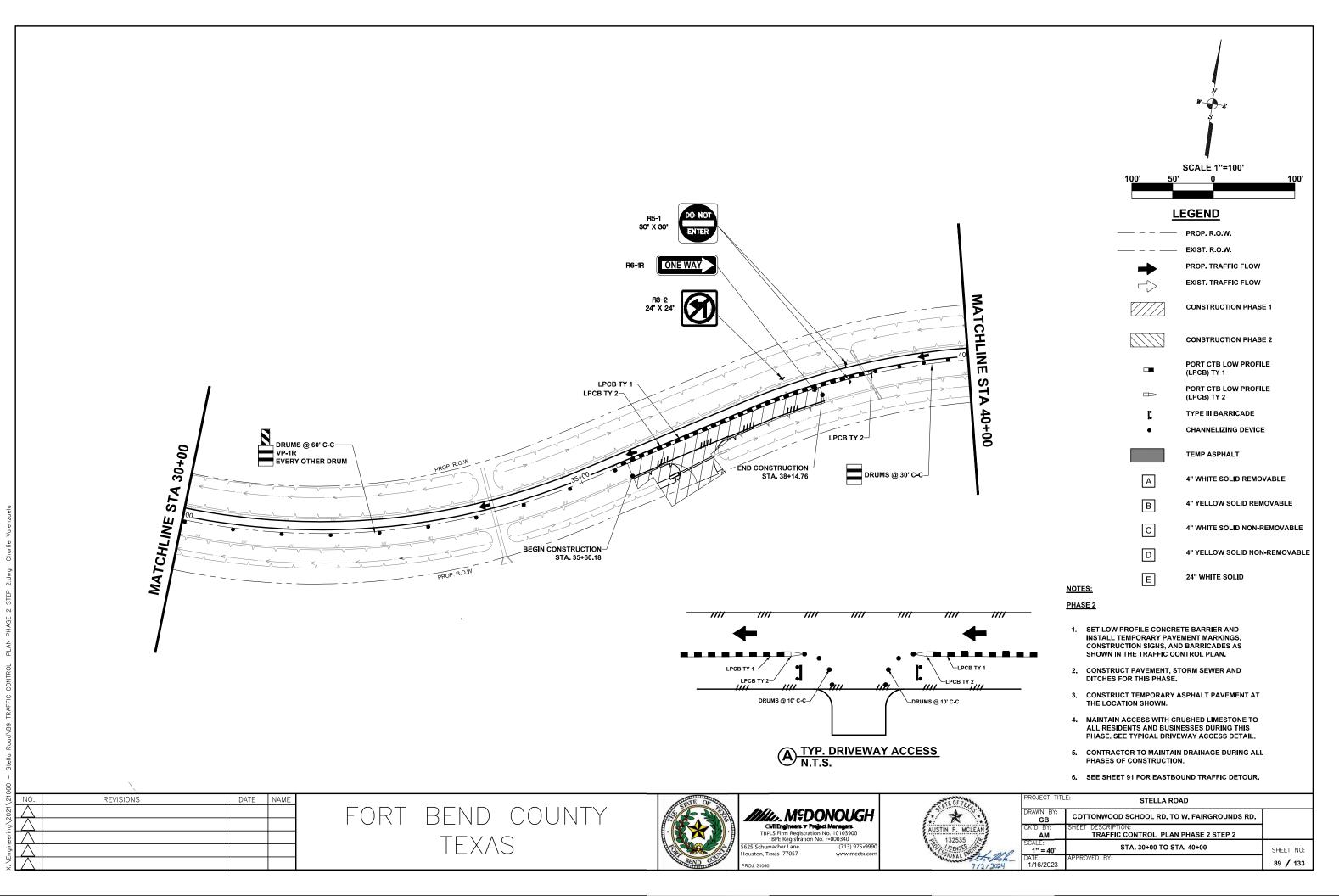
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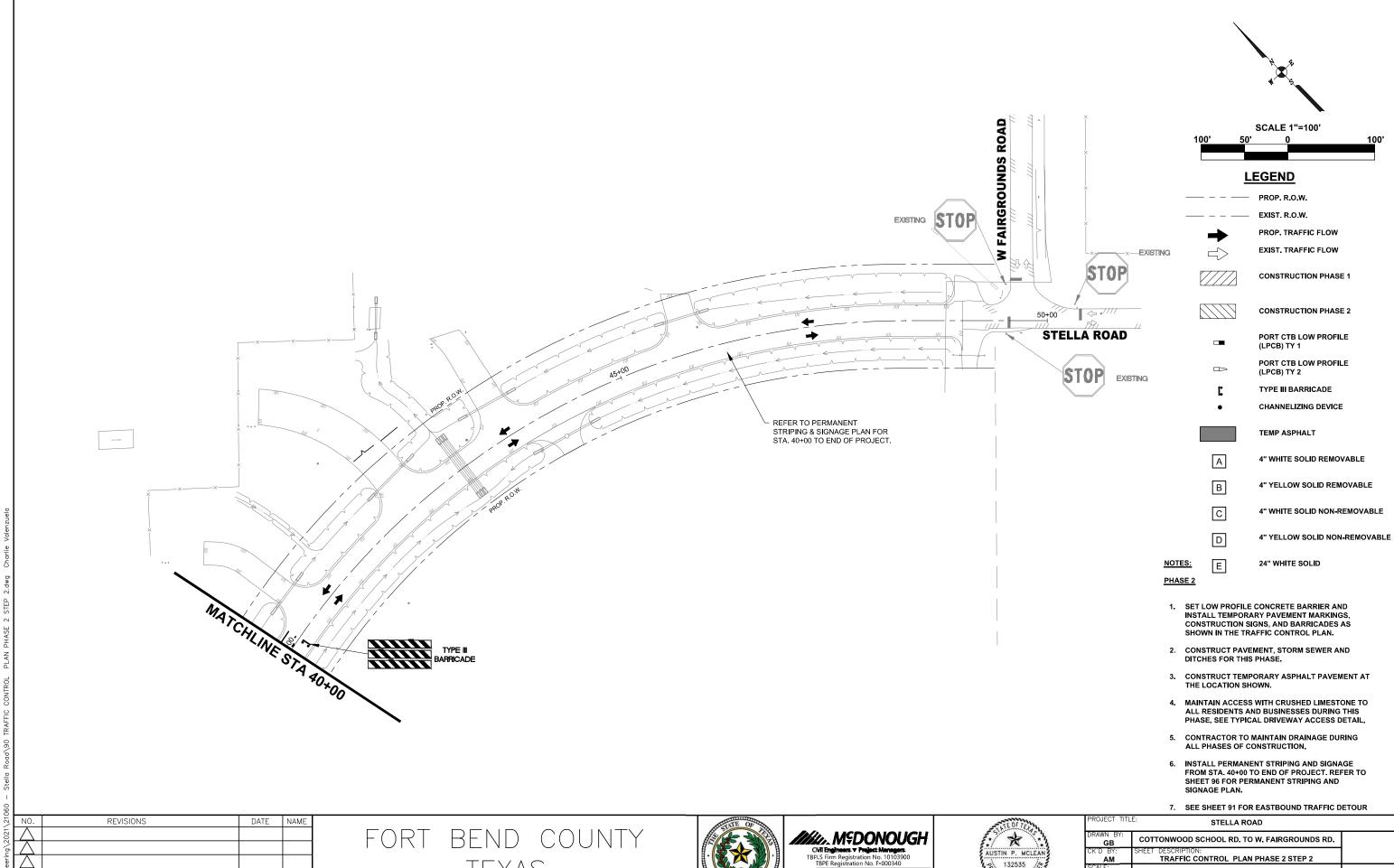
STA. 40+00 TO END

1/16/2023

SHEET NO:

88 / 133





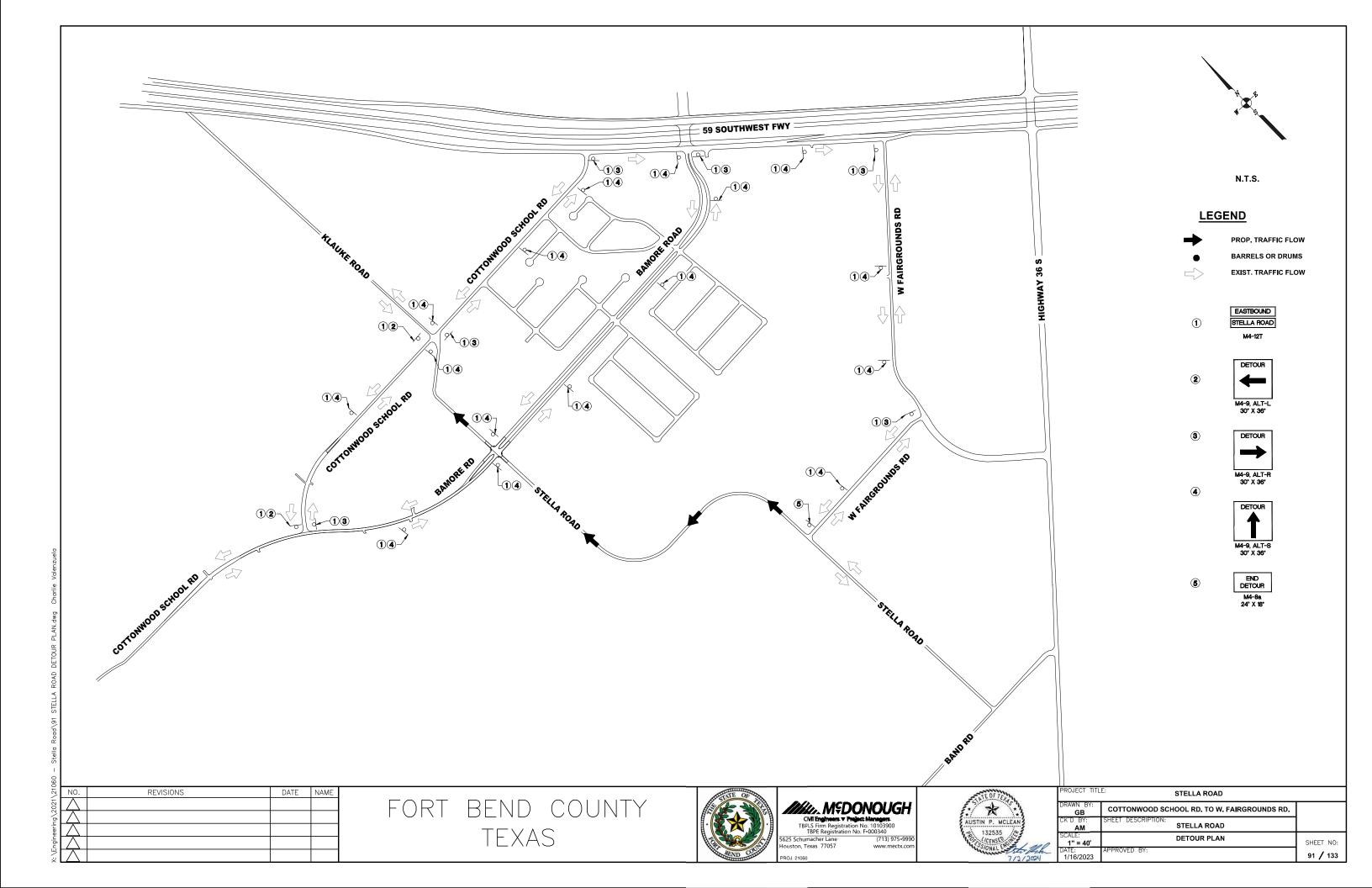
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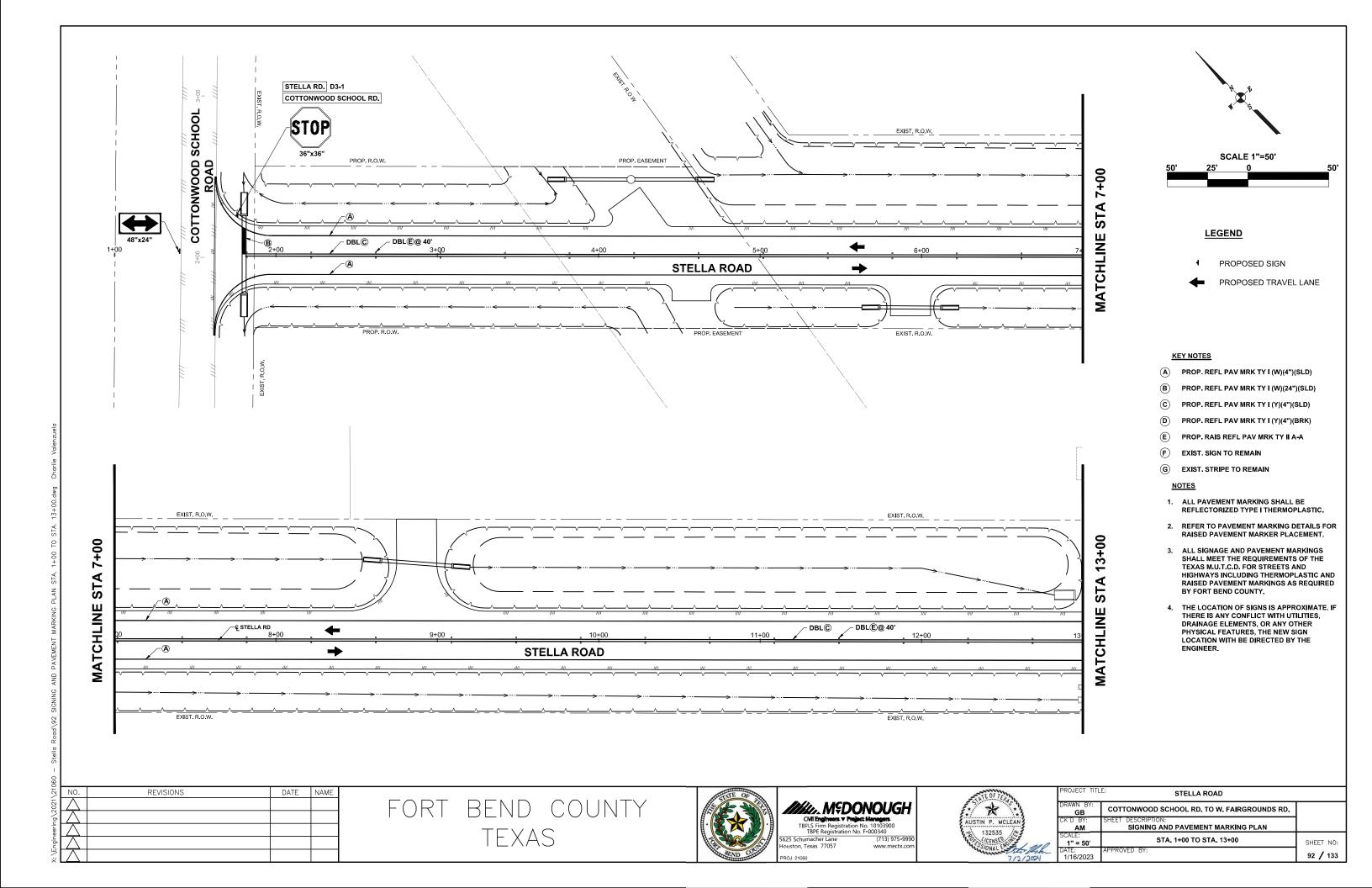


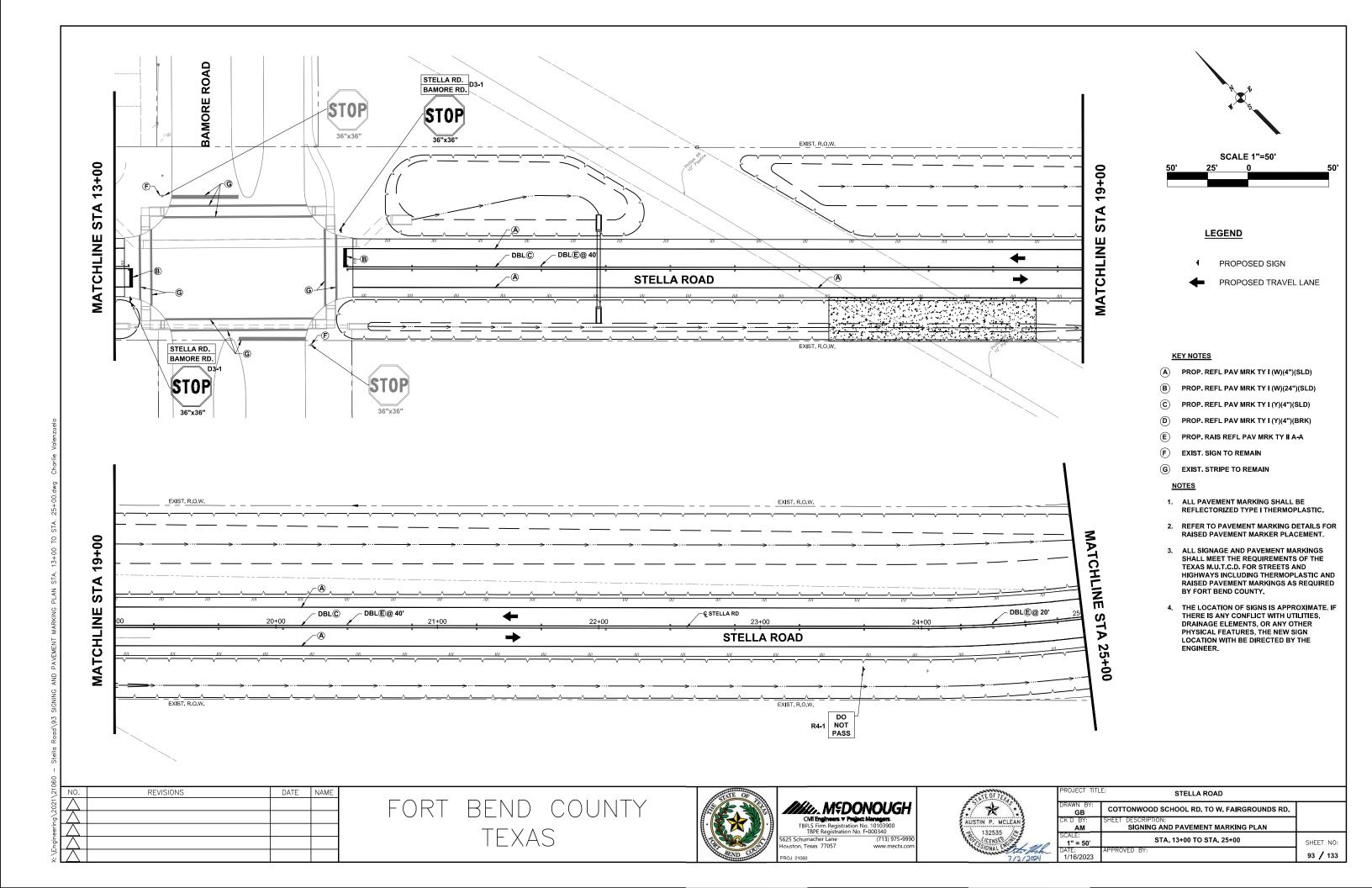


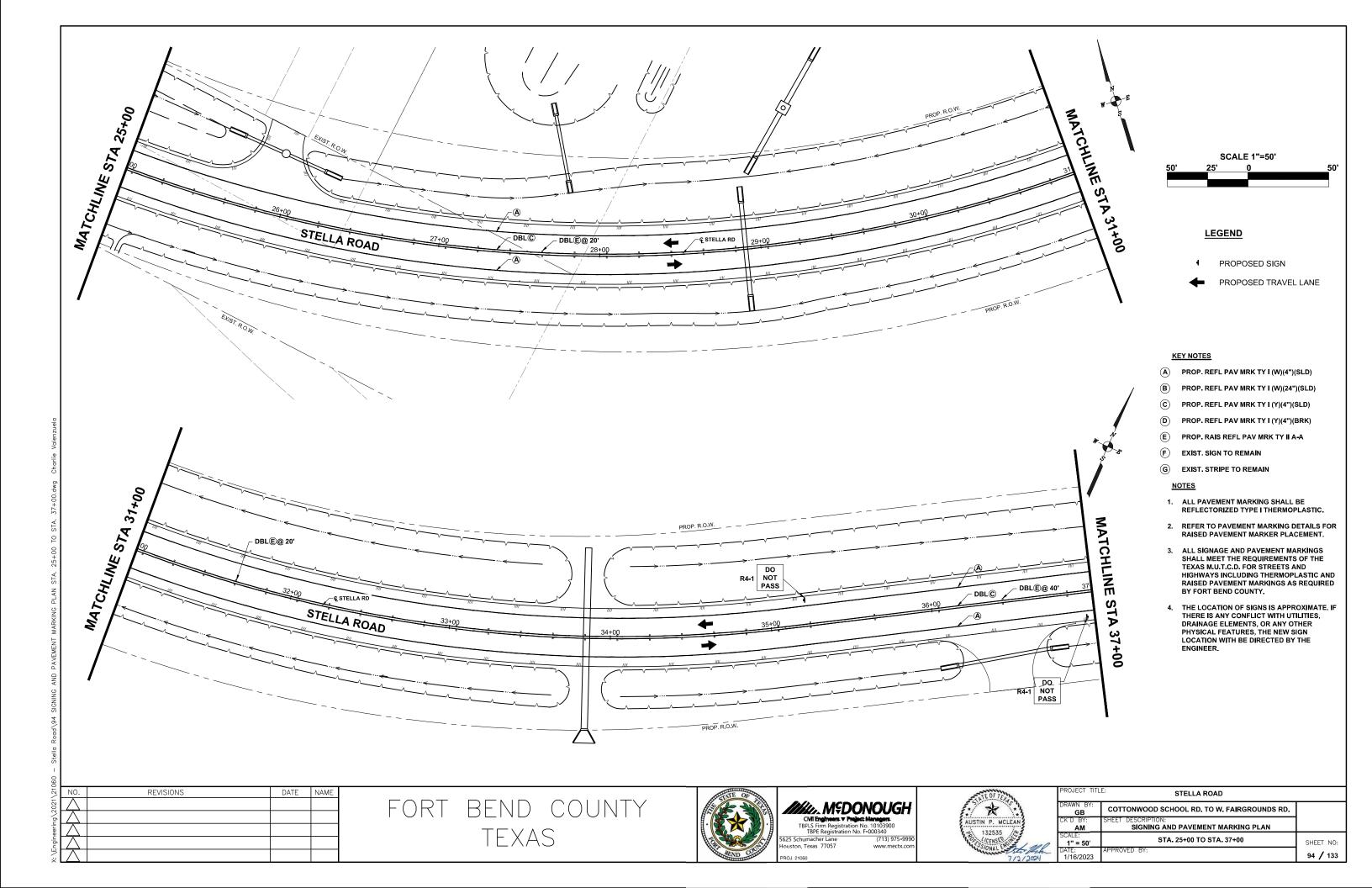


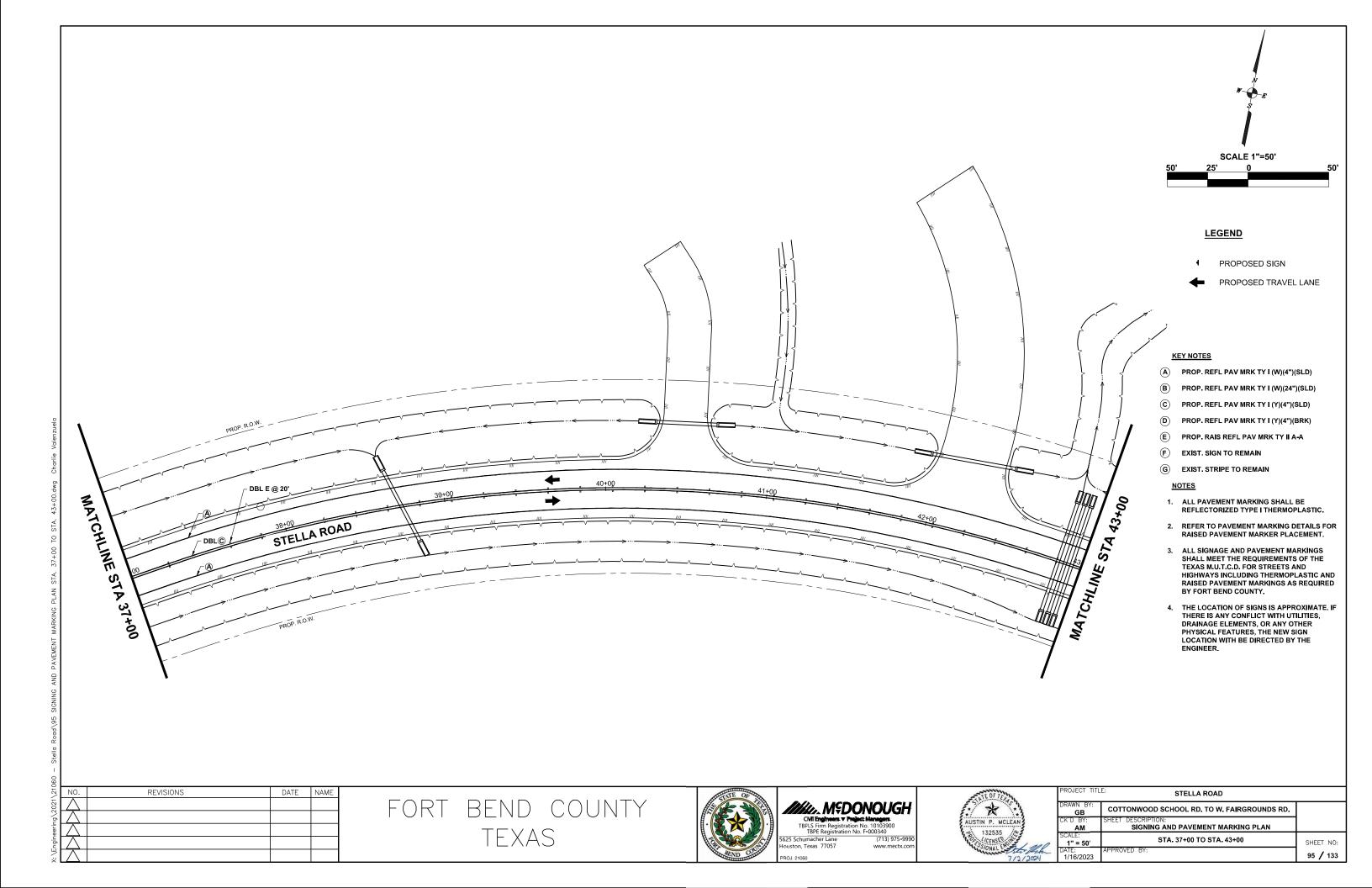
STA. 40+00 TO END SHEET NO: 1" = 40 1/16/2023 90 / 133

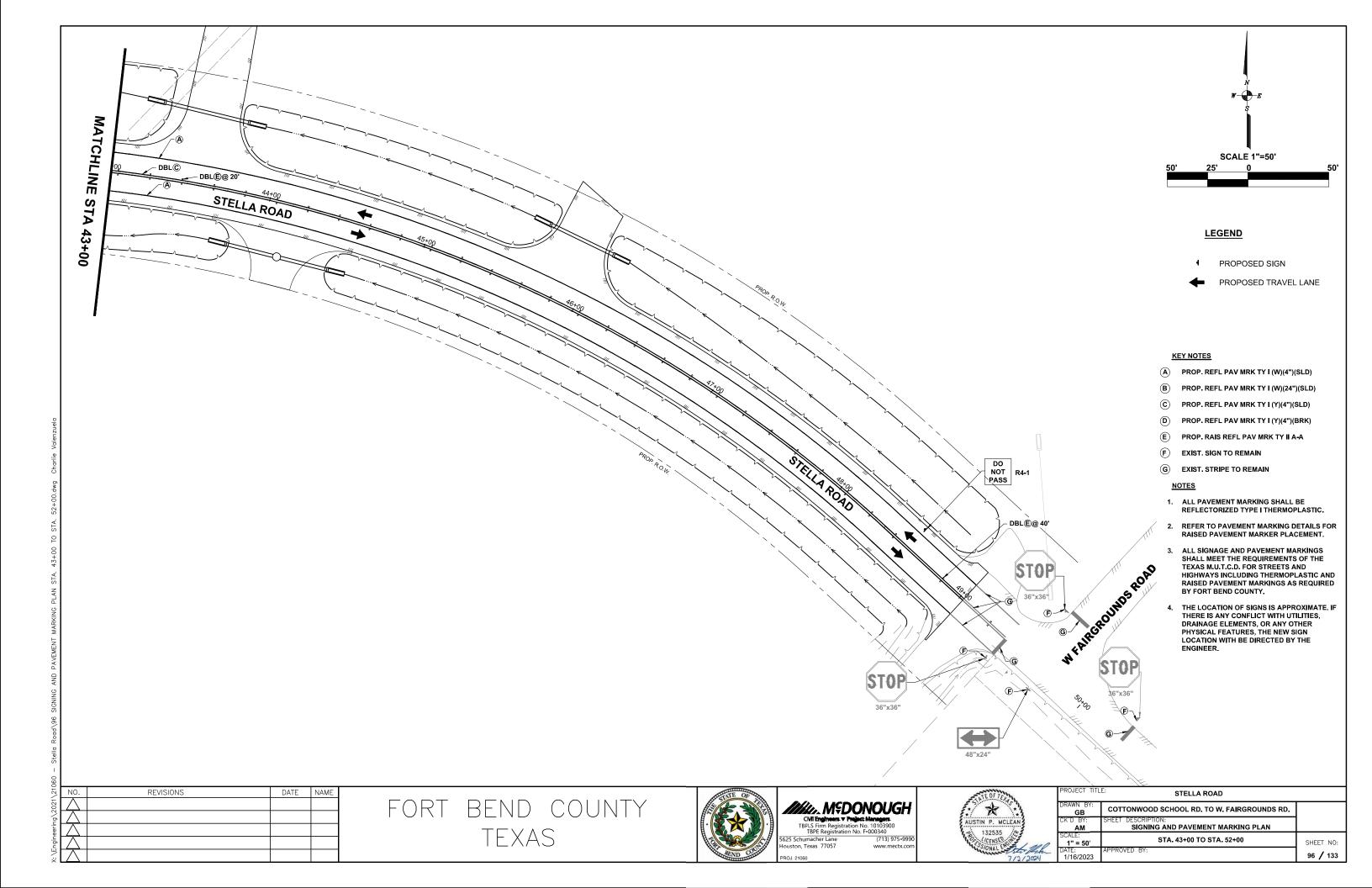


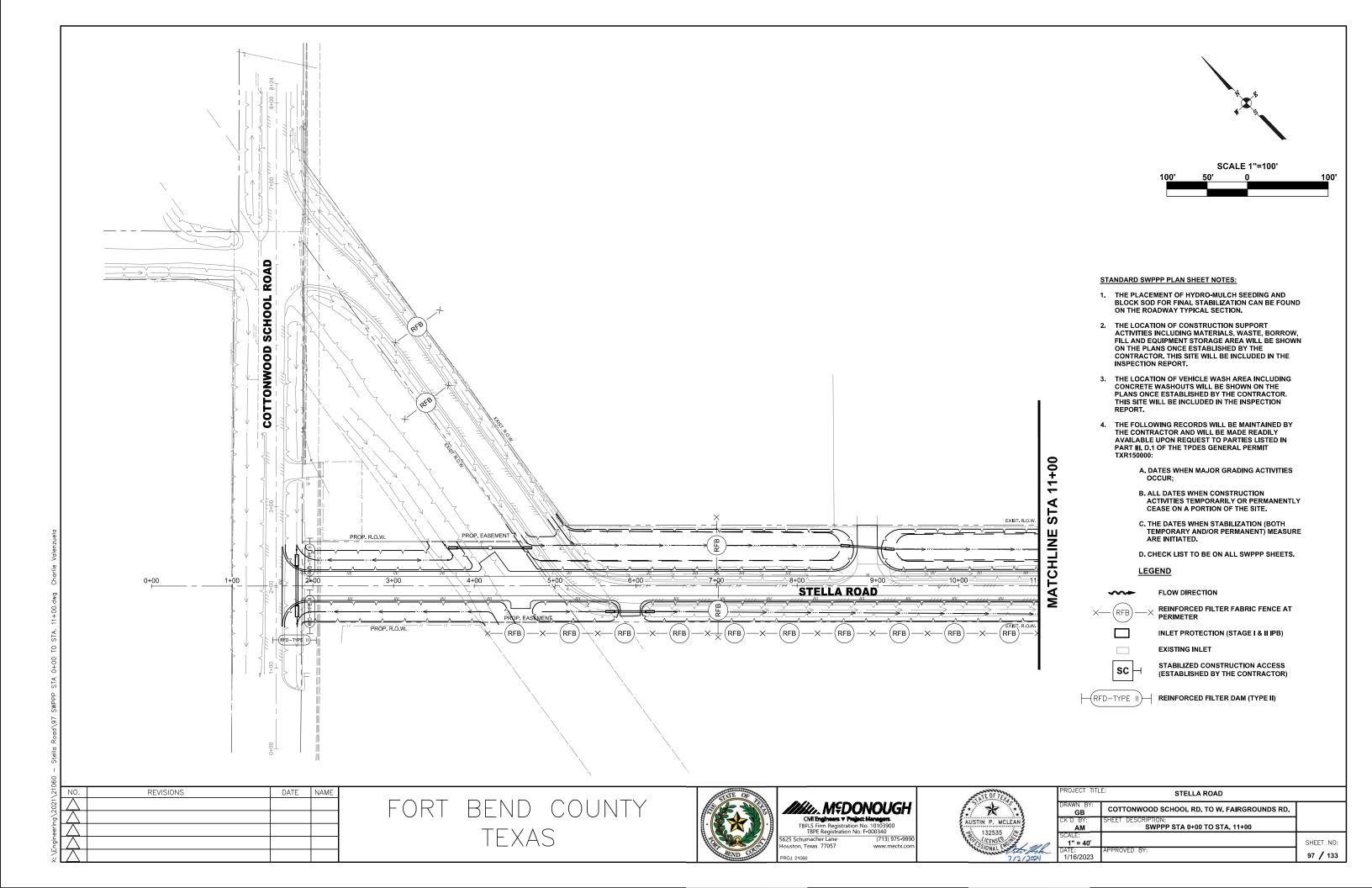


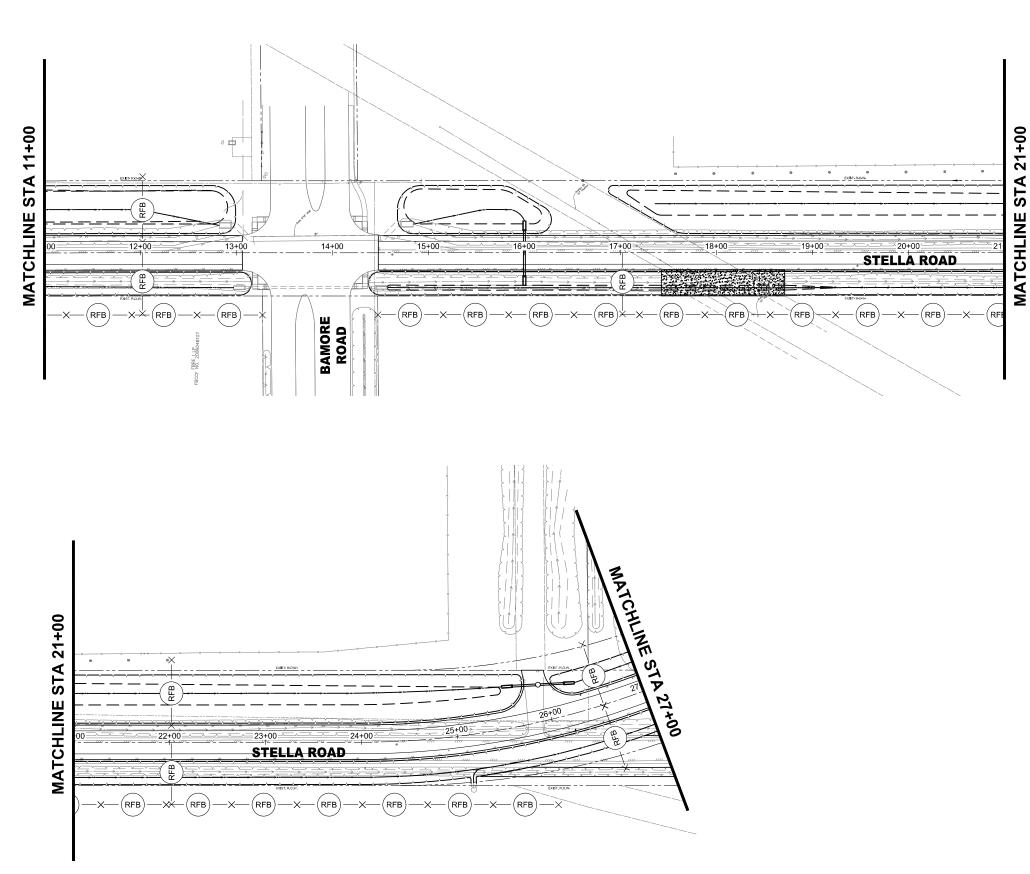






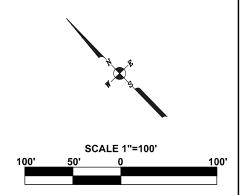






REVISIONS

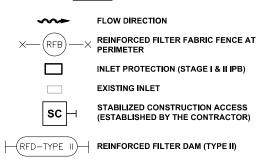
DATE NAME



STANDARD SWPPP PLAN SHEET NOTES:

- THE PLACEMENT OF HYDRO-MULCH SEEDING AND BLOCK SOD FOR FINAL STABILIZATION CAN BE FOUND ON THE ROADWAY TYPICAL SECTION.
- 2. THE LOCATION OF CONSTRUCTION SUPPORT ACTIVITIES INCLUDING MATERIALS, WASTE, BORROW, FILL AND EQUIPMENT STORAGE AREA WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 3. THE LOCATION OF VEHICLE WASH AREA INCLUDING CONCRETE WASHOUTS WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 4. THE FOLLOWING RECORDS WILL BE MAINTAINED BY THE CONTRACTOR AND WILL BE MADE READILY AVAILABLE UPON REQUEST TO PARTIES LISTED IN PART III. D.1 OF THE TPDES GENERAL PERMIT
 - A. DATES WHEN MAJOR GRADING ACTIVITIES OCCUR;
 - B. ALL DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE.
 - C. THE DATES WHEN STABILIZATION (BOTH TEMPORARY AND/OR PERMANENT) MEASURE ARE INITIATED.
 - D. CHECK LIST TO BE ON ALL SWPPP SHEETS.

LEGEND



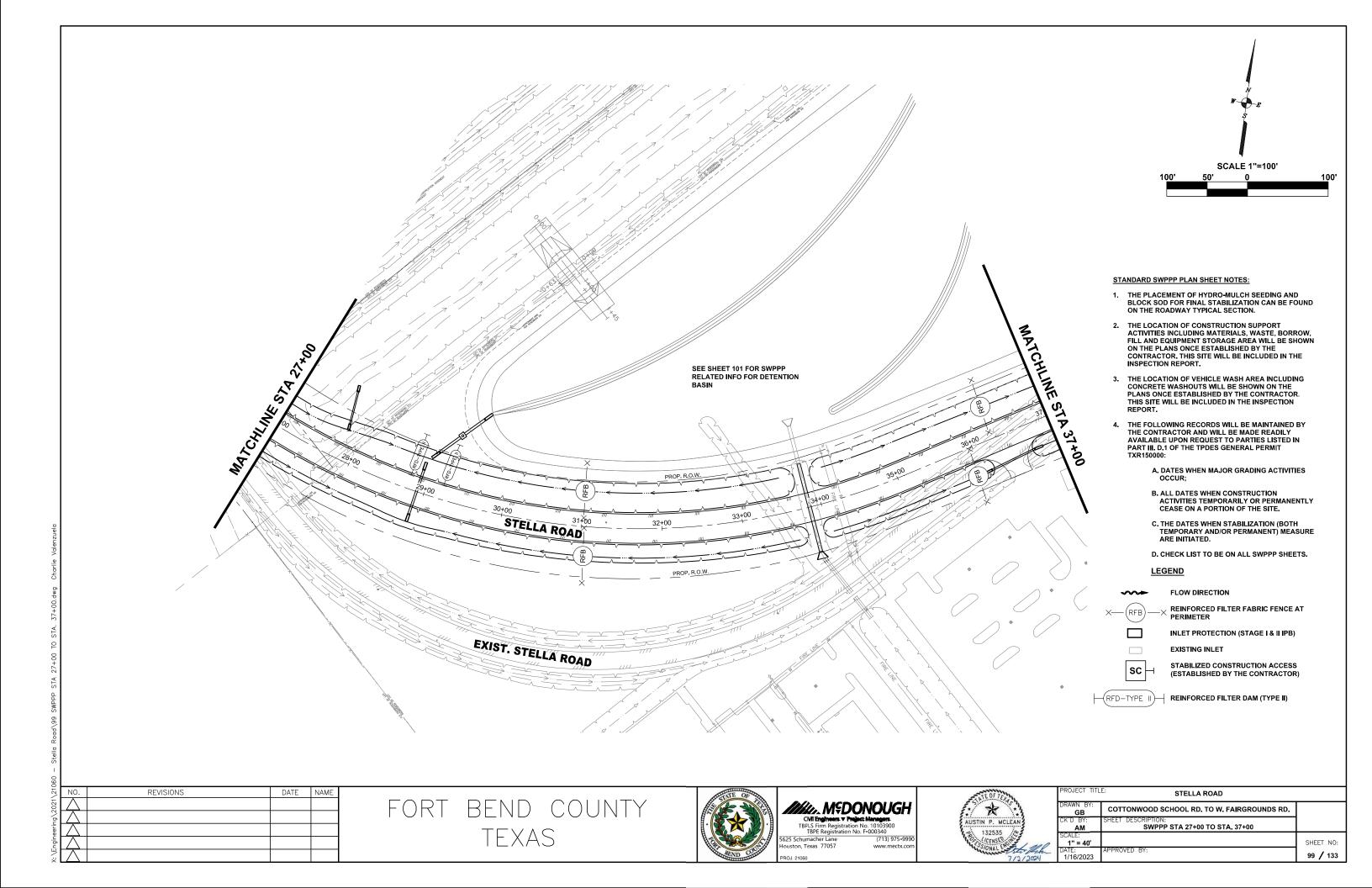


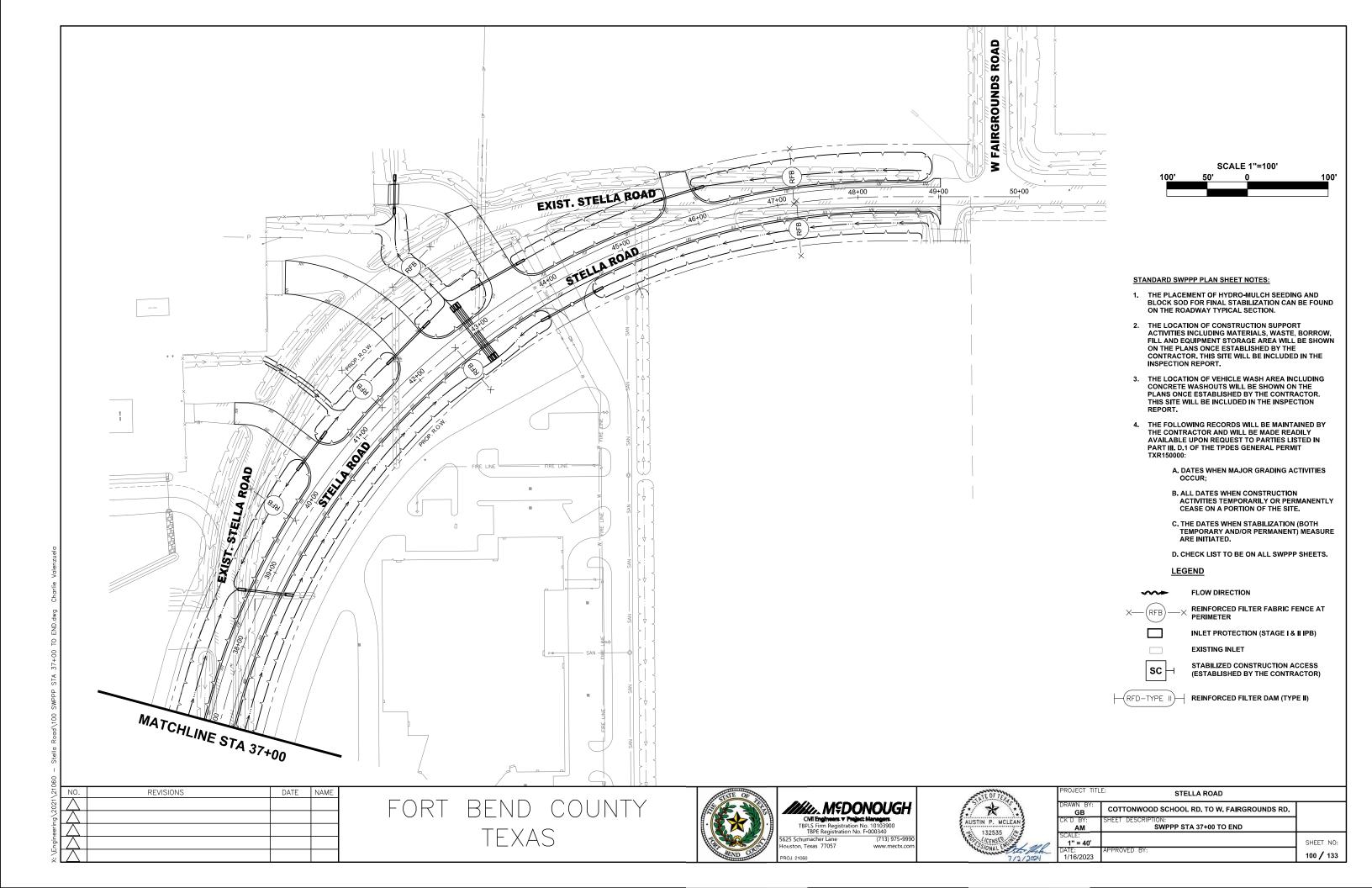
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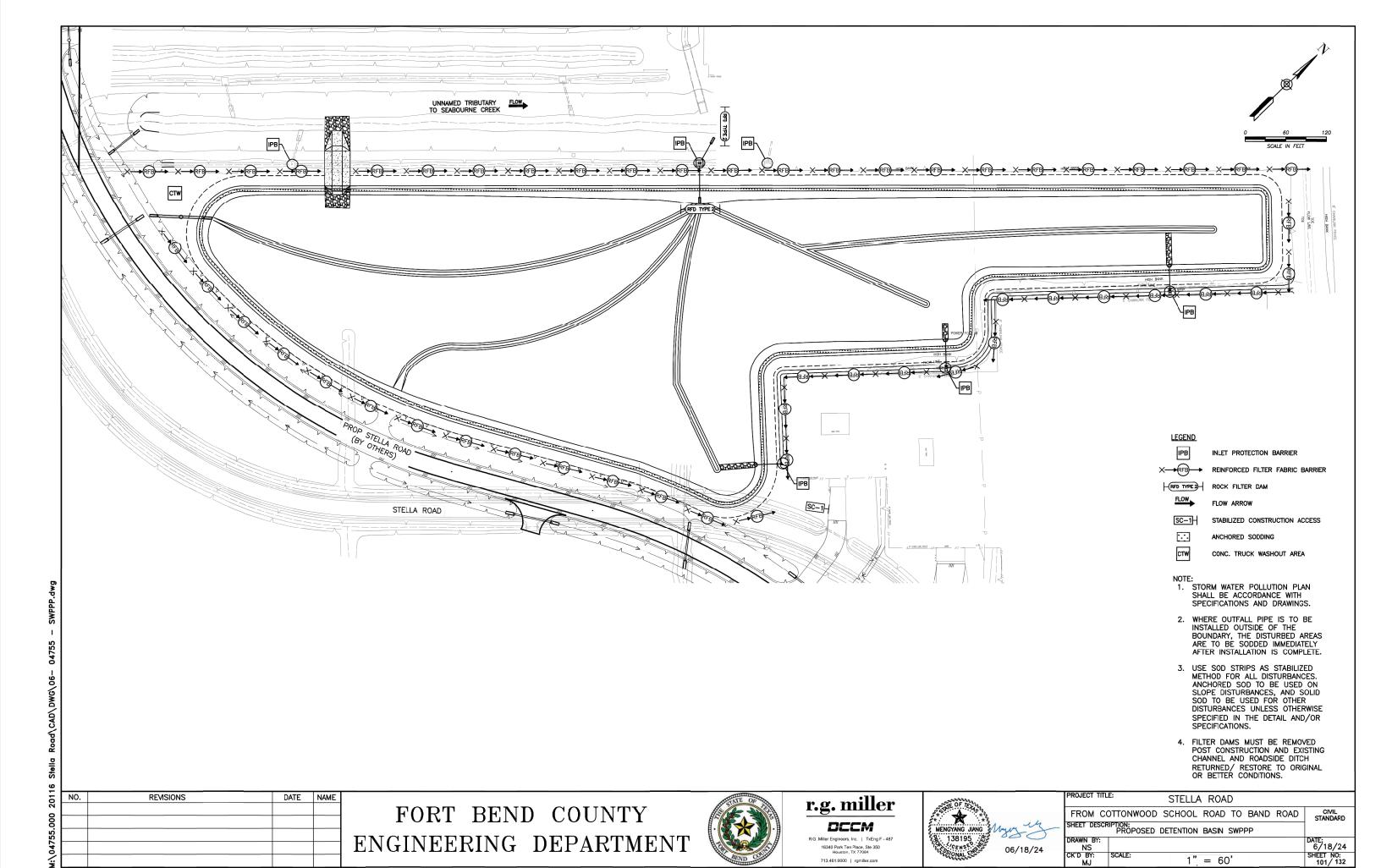


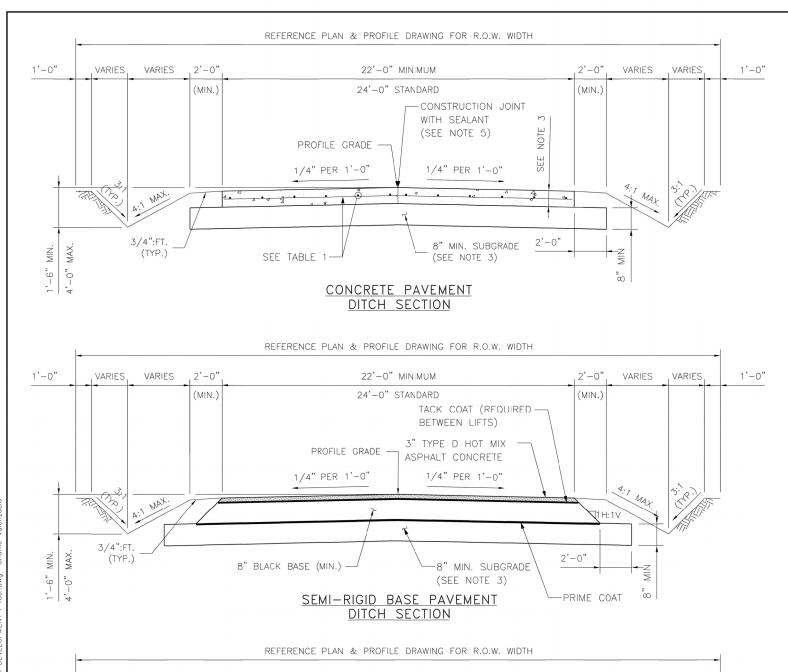


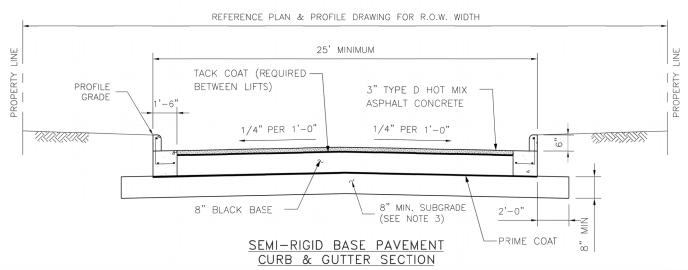
PROJECT TITLE: STELLA ROAD		
DRAWN BY: GB COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.		
CK'D BY:	SHEET DESCRIPTION: SWPPP STA 11+00 TO STA. 27+00	
SCALE: 1" = 40'		SHEET NO:
DATE: 1/16/2023	APPROVED BY:	98 / 133

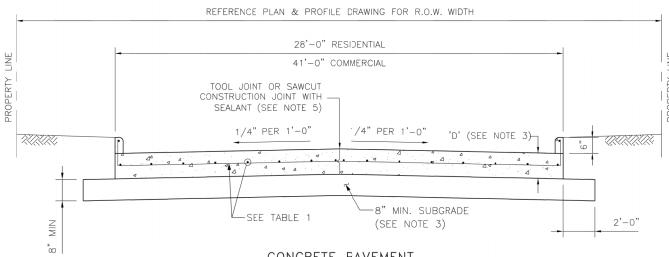












CONCRETE PAVEMENT CURB & GUTTER SECTION (PARABOLIC CROWN IS AN ACCEPTABLE OPTION)

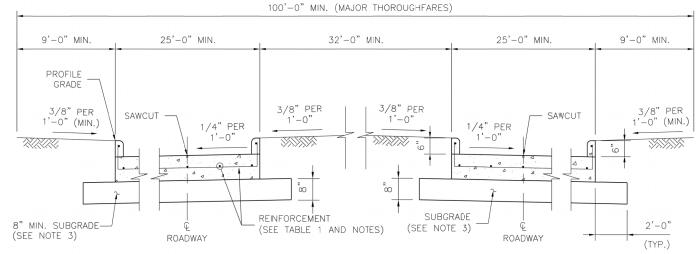


TABLE 1 (CONSTRUCTION JOINT DOWELS)

DOWEL SIZE	PAVEMENT DEPTH
#4 BAR	< 6"
#5 BAR	6" ≤ D < 9"
#6 BAR	≥9"

NOTES:

- 1. PAVEMENT SECTIONS SHOWN ARE INTENDED FOR DEVELOPMENT PROJECTS AND NOT FOR PUBLIC PROJECTS, WHERE WIDTH OF R.O.W. MAY VARY.
- 2. PAVEMENT SECTIONS SHALL BE LOCATED IN CENTER OF R.O.W. 3. SUBGRADE TREATMENT AND PAVEMENT THICKNESS AS DESIGNATED IN PLANS
- 4. REFERENCE CONSTRUCTION JOINT DETAIL ON THE STANDARD CIVIL DRAWING "CONCRETE PAVEMENT DETAILS - SHEET 1 OF 2" FOR JOINT AND SEALANT REQUIREMENTS.
- 5. NO TRAFFIC ON CONCRETE PAVEMENT FOR 7 DAYS AND COMPRESSIVE STRENGTH OF 3,500 psi HAS BEEN REACHED.
- 6. ALL CONSTRUCTION JOINTS SHALL BE SEALED

TYPICAL SECTIONS FOR MAJOR THOROUGHFARES

HORIZONTAL SCALE: 1"=3'-0" VERTICAL SCALE: 1"=1'-6"

SLAB AND REBAR NOTES:

- TYPICAL SLAB THICKNESS D=8"
- 2. TYPICAL REBAR SIZE AND SPACING ARE: a. #4 BAR @ 18" C-C LONGITUDINAL b. #4 BAR @ 18" C-C TRANSVERSE
- 3. REBAR SIZE FOR PAVEMENT LESS THAN 8" THICK a. #4 BAR @ 24" C-C LONGITUD NAL b. #4 BAR @ 24" C-C TRANSVERSE
- 4. REBAR SHALL NOT BE PLACED WITHIN 3" FROM THE EDGE OF PAVEMENT.
- 5. TYPICAL STABILIZED SUBGRADE THICKNESS IS 8 INCHES.
- 6. FOR HEAVY INDUSTRIAL TRAFFIC, SLAB THICKNESS AND REBAR SIZE AND SPACING WILL BE AS PER GEOTECHNICAL RECOMMENDATION.
- ALL BENT BARS SHALL BE GRADE 40 STEEL, ALL OTHER SHALL BE GRADE 60.
- 8. MINIMUM LAP SPLICE 16".
- . LAP SPLICES SHOULD BE ON ALTERNATING BARS, ADJACENT LAP SPLICES ARE NOT ACCEPTABLE.

DATE REVISIONS NAME ORIGINAL STANDARD ISSUED 3-1-22

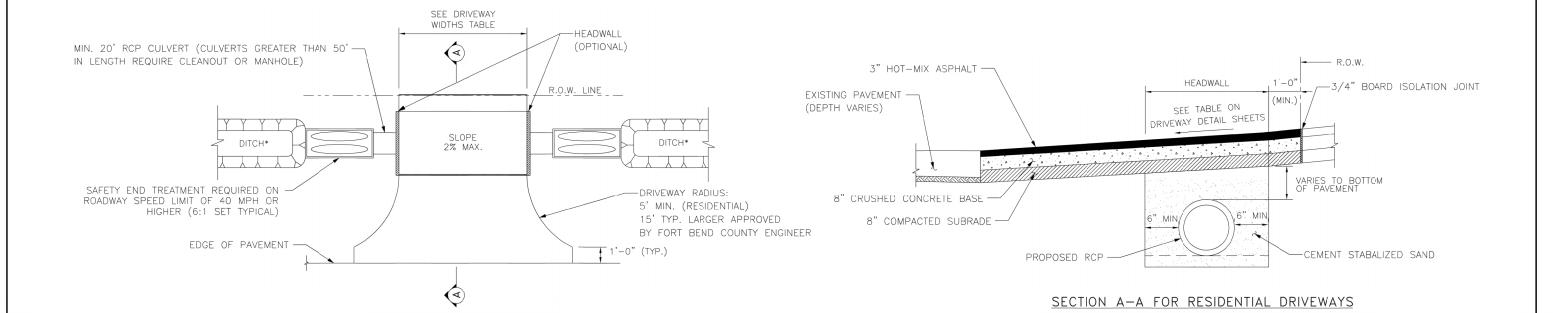






	PROJECT TITL	E:
	DRAWN BY: GB	COTTONW
	CK'D BY: AM	SHEET DESCR FB(
1	SCALE: 1" = 40'	
~	DATE: 1/16/2023	APPROVED B

DJECT TITL	E: STELLA ROAD	
WN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
D BY: AM	SHEET DESCRIPTION: FBCED TYPICAL PAVEMENT SECTIONS	
LE: " = 40'	FOR DEVELOPMENT PROJ.	SHEET NO:
E: 16/2023	APPROVED BY:	102 / 133



MINIMUM RADII REQUIREMENTS - DRIVEWAYS

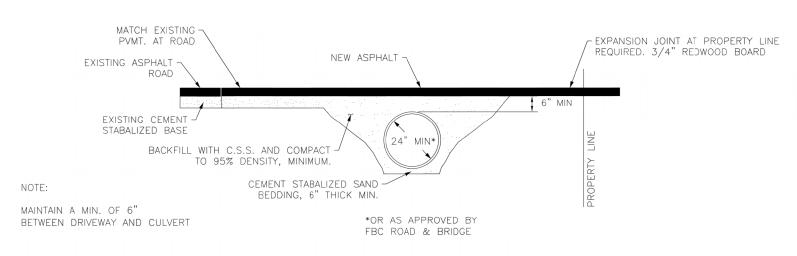
OPEN DITCH DRIVEWAY
**DITCH IS TO HAVE 4:1 SLOPE

	LOCAL	COLLECTOR	MAJOR
RESIDENTIAL	5'	5'	
COMMERCIAL	10'	10'	25'

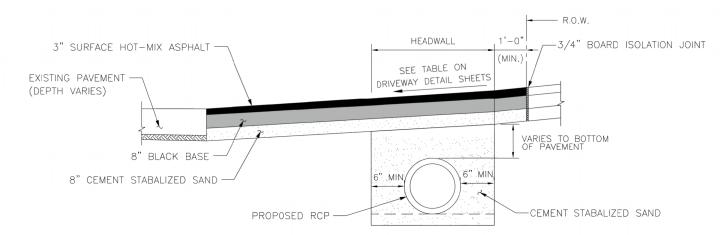
DRIVEWAY WIDTHS*

	MINIMUM	MAXIMUM
RESIDENTIAL	10'	25'
COMMERCIAL	20'	40'

*DRIVEWAY WIDTHS ARE MEASURED AT THE ROW LINE



ASPHALT APRON DETAIL - DRIVEWAY PROFILE FOR CULVERT DRAINAGE



SECTION A-A FOR COMMERCIAL DRIVEWAYS

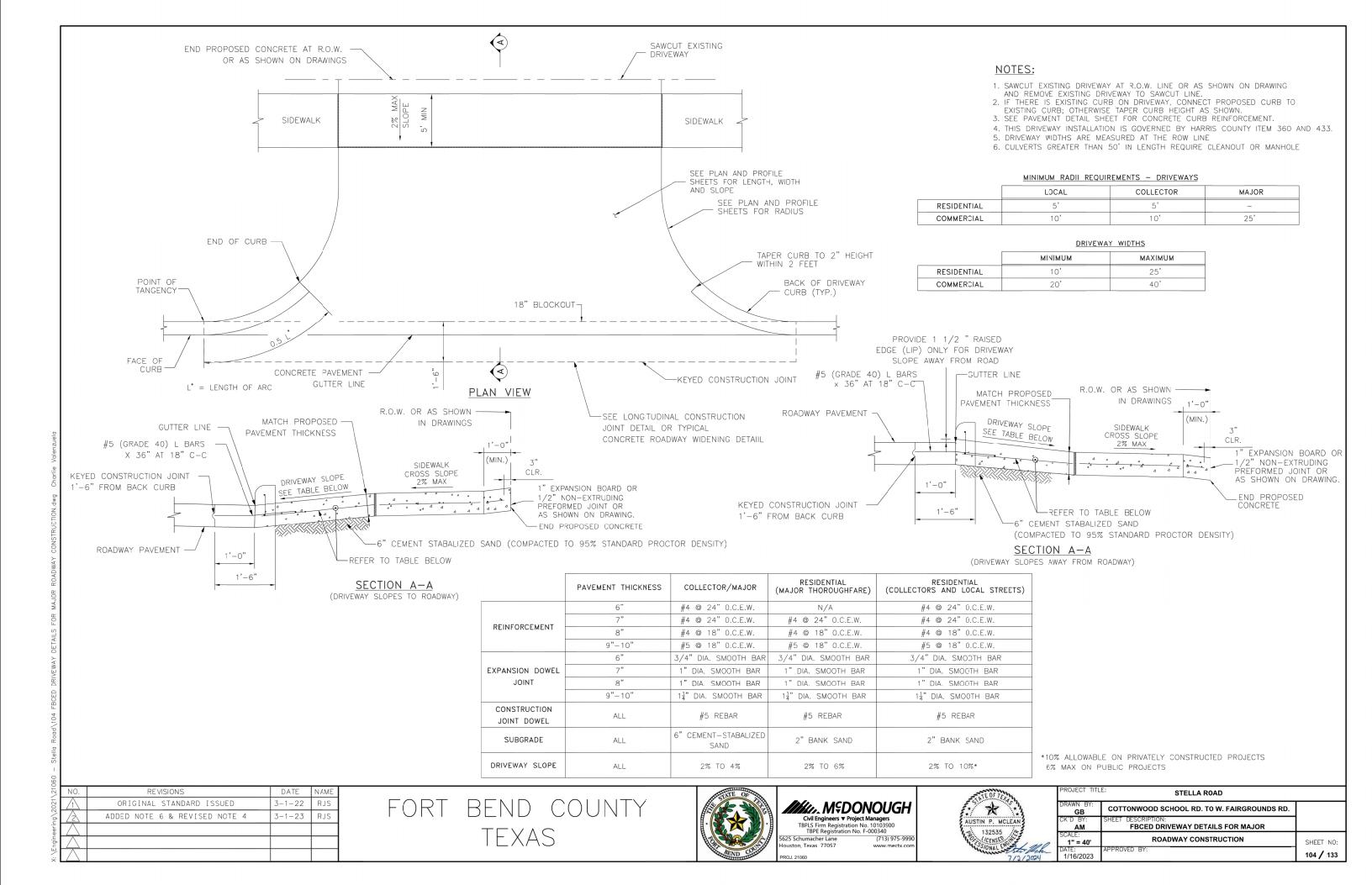
NO.	REVISIONS	DATE	NAME
1	ORIGINAL STANDARD ISSUED	3-1-22	RJS
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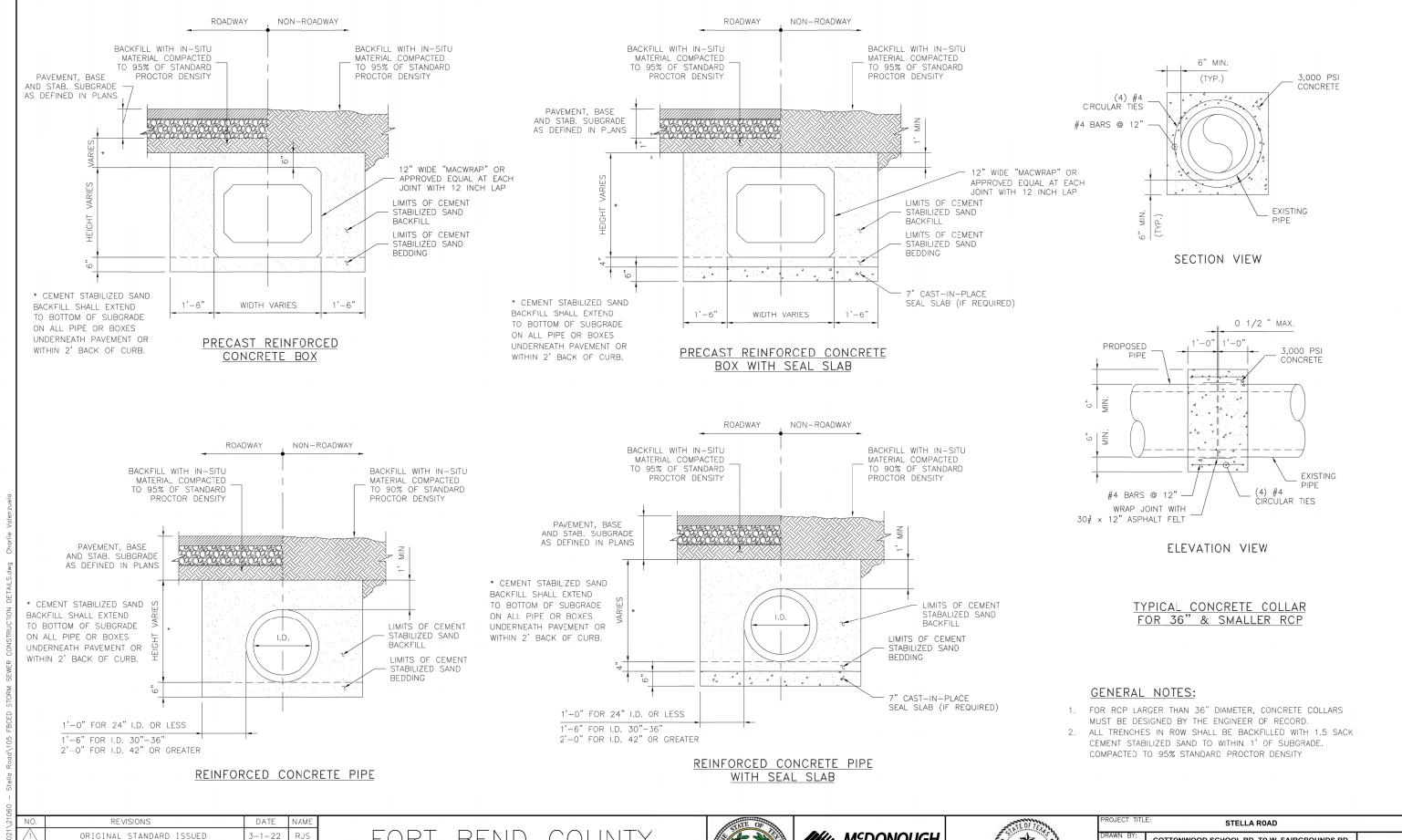




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AUSTIN P. MCLEAN	С
3 132535 A	S
7/2/2024	D,

PROJECT TITL	PROJECT TITLE: STELLA ROAD				
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.				
CK'D BY:	SHEET DESCRIPTION:				
AM	FBCED ASPHALT DRIVEWAY DETAILS				
SCALE:					
1" = 40'		SHEET NO:			
DATE: 1/16/2023	APPROVED BY:	103 / 133			



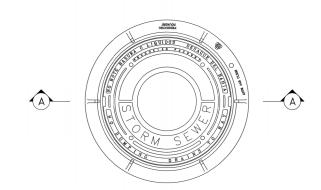






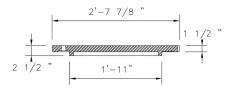


	PROJECT TITLE: STELLA ROAD		
	DRAWN BY: GB COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.		
CK'D BY: AM		SHEET DESCRIPTION: FBCED STORM SEWER CONSTRUCTION DETAILS	
	SCALE: 1" = 40'		SHEET NO:
	DATE: 1/16/2023	APPROVED BY:	105 / 133

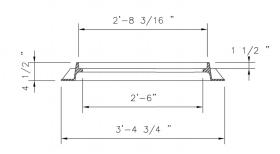


PLAN VIEW FRAME AND COVER

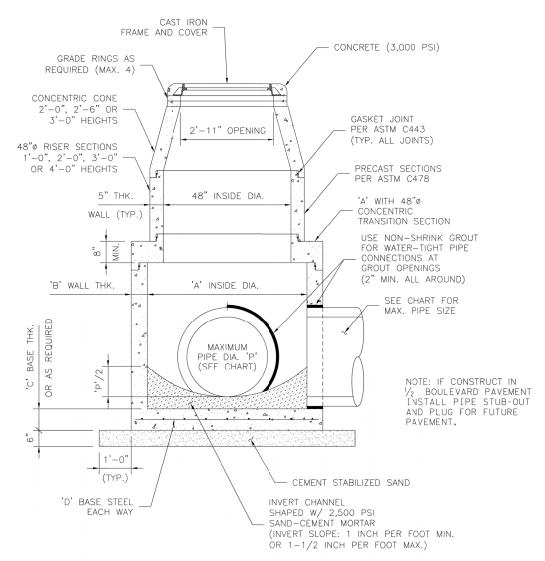
NOTE: IF PROJECT IS WITHIN A CITY ETJ OR CITY LIMITS, USE CITY'S STD MANHOLE COVER



COVER SECTION A-A SCALE: 1" = 1'-0"

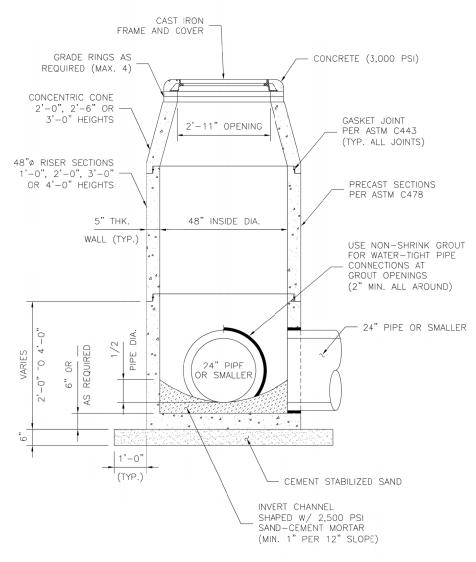


FRAME SECTION A-A



PRECAST CONCENTRIC MANHOLE FOR PIPE SIZES GREATER THAN 24"

MAXIMUM PIPE DIA. 'P'	INSIDE DIA. 'A'	WALL THICKNESS 'B'	BASE THICKNESS 'C'	BASE STEEL 'D'
30"	5'-0"	6"	8"	#5 @ 8"
42"	6'-0"	7"	8"	#5 @ 8"
54"	7'-0"	8"	10"	#6 @ 12" (2 LAYERS)
60"	8'-0"	9"	10"	#6 @ 12" (2 LAYERS)



48"Ø PRECAST CONCENTRIC MANHOLE FOR PIPE SIZES 24" OR SMALLER

GENERAL NOTES:

- 1. CONSTRUCTION AND MATERIALS SHALL MEET REQUIREMENTS OF ITEM 471 "PRECAST CONCRETE MANHOLES".
 2. CONCRETE FOR MANHOLE: MINIMUM 4,000 PSI IN 28 DAYS
- 3. HS-20 LOADING; MANHOLE DESIGN SHALL MEET OR EXCEED
- ASTM C478 REQUIREMENTS.
 4. GASKET JOINT: PER ASTM C443
- 5. FRAME AND COVER SHALL BE EAST JORDAN IRON WORKS
- MODEL V-1420 OR APPROVED EQUAL.

 5. SHOP DRAWINGS WITH MANUFACTURER'S CERTIFICATION SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL.

	NO.	REVISIONS	DATE	NAME	Г
	1	ORIGINAL STANDARD ISSUED	3-1-22	RJS	ı
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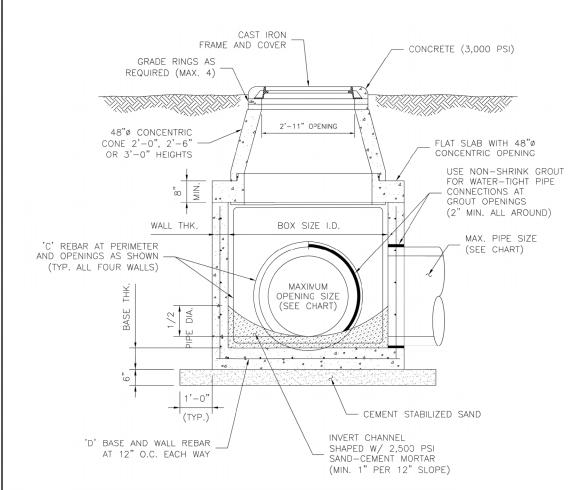




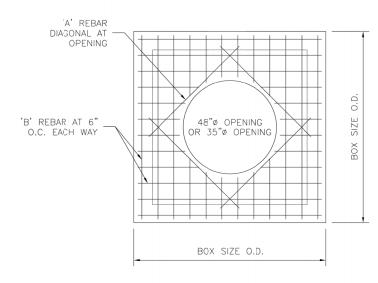


	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB		
	CK'D BY: AM	SHEET DESCRIPTION: FBCED PRECAST CONCRETE STORM	
1	SCALE: 1" = 40'	SEWER MANHOLE DETAILS	SHEET NO:
-	DATE: 1/16/2023	APPROVED BY:	106 / 133

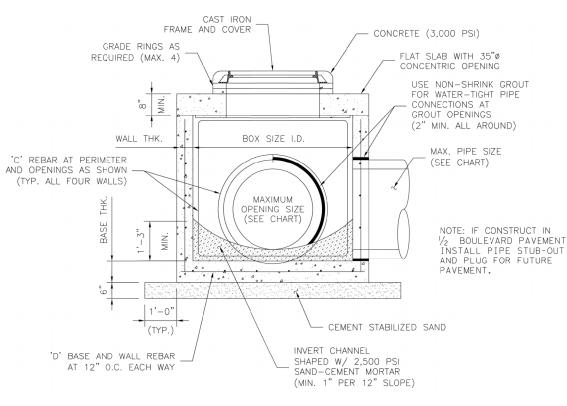
* FOR 7'X7' AND 8'X8' BOX SIZE: TWO LAYERS OF STEEL REQUIRED. (FOR DEPTHS GREATER THAN 15')



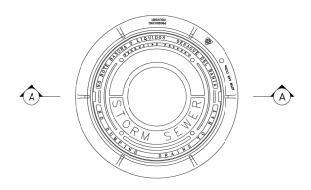
JUNCTION BOX/MANHOLE WITH CONCENTRIC CONE SCALE: 1''=1'-6'



PLAN VIEW FLAT SLAB WITH OPENING

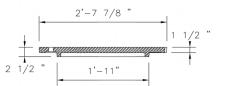


JUNCTION BOX/MANHOLE WITH FLAT SLAB SCALE: 1"=1'-6"

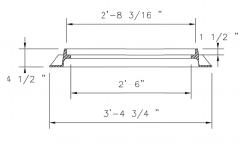


PLAN VIEW FRAME AND COVER SCALE: 1"=1'-0

NOTE: IF PROJECT IS WITHIN A CITY ETJ USE CITY'S STD MANHOLE COYER



COVER SECTION A-A SCALE: 1"=1'-0"



FRAME SECTION A-A SCALE: 1"=1'-0"

GENERAL NOTES:

- 1. CONSTRUCTION AND MATERIALS SHALL MEET REQUIREMENTS
 OF ITEM 471 "PRECAST CONCRETE MANHOLES".
 2. CONCRETE FOR JUNCTION BOX: MINIMUM 4,000 PSI IN 28 DAYS
 3. HS—20 LOADING; MANHOLE DESIGN SHALL MEET OR EXCEED
- ASTM C478 AND ASTM C913 REQUIREMENTS.
- 4. JOINT SEALANT: RAM-NEK GASKET MA'ERIAL 5. FRAME AND COVER SHALL BE EAST JORDAN IRON WORKS MODEL V-1420 CR APPROVED EQUAL.
- 6. SHOP DRAWINGS WITH MANUFACTURER'S CERTIFICATION SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL.

1	NO.	REVISIONS	DATE	NAME	Γ
1	\triangle	ORIGINAL STANDARD ISSUED	3-1-22	RJS	
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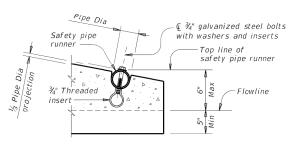
	PROJECT TITL	E:
à.	DRAWN BY: GB	сот
3	CK'D BY:	SHEET
7	SCALE:	
- Mich	1" = 40' DATE: 1/16/2023	APPRO
1 due 7		

PROJECT TITL	PROJECT TITLE: STELLA ROAD					
DRAWN BY: GB						
CK'D BY: AM	SHEET DESCRIPTION: FBCED JUNCTION BOX AND MANHOLE DETAILS					
SCALE: 1" = 40'		SHEET NO:				
DATE: 1/16/2023	APPROVED BY:	107 / 133				

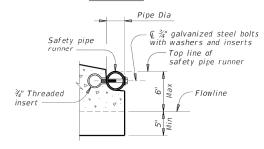
Unit length (varies) Eq Spa at 24" Max Safety Pipe (if required) **Safety Safety** PLAN(Showing bell end connection.) - Satety pipe runner (Typ) (if required) step slope Top face of safety end treatment 1 Flowline LONGITUDINAL ELEVATION (Showing bell end connection.) 6" 5 Reinforcing to have

Pipe Dia Safety pipe runner -Ǿ" galvanized steel bolts with washers and inserts ¾" Threaded

INSTALLATION DETAIL FOR SAFETY PIPE RUNNERS



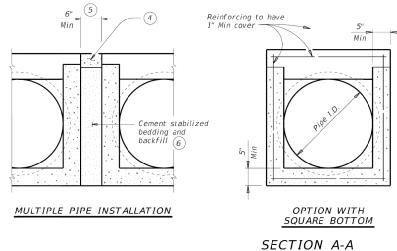
OPTION A

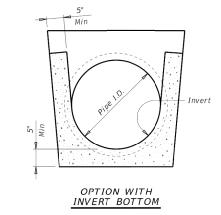


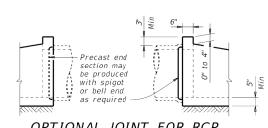
OPTION B

END DETAILS FOR INSTALLATION OF SAFETY PIPE RUNNERS

(If required)







OPTIONAL JOINT FOR RCP

(Showing joint between RCP and

REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS

Pipe		TP Wall		Min	Pipe Runners Required		Required Pipe Runner Size			
I.D.	Thickness	Thickness	"D"		Single Pipe	Multiple Pipe	Nominal Dia.	0.D.	I.D.	
12"	2"	1.15"	17.00"	6:1	4' - 9"	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
15"	2 1/4"	1.30"	20.50"	6:1	6' - 5"	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
18"	2 ½"	1.60"	24.00"	6:1	8' - 0"	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
24"	ייכ	1.95"	J1.00"	G:1	11' - 3"	No	Yes, for > 2 pipes	ייכ STD	3.500"	2.060"
30"	3 1/2"	2.65"	38.50"	6:1	14' - 8"	No	Yes	4" STD	4.500"	4.026"
36"	4"	2.75"	45.50"	6:1	17' - 11"	Yes	Yes	4" STD	4.500"	4.026"
42"	4 1/2"	2.7"	52.50"	6:1	21' - 2"	Yes	Yes	4" STD	4.500"	4.026"

- $^{(1)}$ Dimension "U" is based on reinforced concrete pipe (RCP) meeting the requirements of ASIM C-/6, Class III, (RCP Wall "B" thickness). Adjust "D" for any other wall thickness used. For thermoplastic pipe (TP) take into account the annular space requirements for grouted connections.
- (2) Slope as shown elsewhere in the plans. Slope of 6:1 or flatter is required for vehicle safety.
- Toewall to be used only when dimension is shown elsewhere in the plans.
- (4) Fill the top 4" of void between precast end treatments with concrete riprap. Concrete riprap is considered subsidiary to the Item 467, "Safety End Treatment".
- $^{\left(5\right)}$ Adjust clear distance between pipes to provide for the minimum distance between safety end treatments.
- $\stackrel{oldsymbol{(6)}}{}$ Provide cement stabilized bedding and backfill in accordance with the Item 400, "Excavation and Backfill for Structures". Bedding and backfill is considered subsidiary to the Item 467, "Safety End Treatment". When concrete riprap is specified around the safety end treatment, backfill as directed by Engineer.
- (2) Thermoplastic pipe wall thickness may vary. Adjust accordingly. Thermoplastic pipe requires the safety end treatments to have a bell end for grouted connections.

GENERAL NOTES:
Precast safety end treatment for reinforced concrete pipe (RCP), and thermoplastic pipe (TP) may be used for TYPE II end treatment as specified in Item "Safety End Treatment".
When precast safety end treatment is used as a Contractor's alternate

to mitered RCP, riprap will not be required unless noted otherwise on

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Manufacture this product in accordance with Item 467, "Safety End Treatment" except as noted below:

A. Provide minimum reinforcing of #4 at 6" (Grade 40) or #4 at 9" (Grade 60) each way or 6"x6" - D12 x D12 or 5"x5" - D10 x D10 welded wire reinforcement (WWR). B. For precast (steel formed) sections, provide Class "C" concrete (f'c = 3,600 psi).

At the option and expense of the Contractor the next larger size of

At the option and expense or the Contractor the next larger size of safety end treatment may be furnished; as long as the "D" dimension cast is that of the required size of pipe.

Pipe runners are designed for a traversing load of 10,000 Lbs at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981. Provide pipe runners meeting the requirements of ASTM A53 (Type E or S, Grade B), ASTM A500 (Grade B), or API 5LX52.

Galvanize all steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the proceedings. with the specifications.

Connect RCP using the Optional Joint for RCP detail shown or in accordance with Item 464, "Reinforced Concrete Pipe". Connect TP by grouting. See Pipe and Box Grouted Connections (PBGC) standard for grouted connections with TP and precast safety end treatment.

Texas Department of Transportation

SHEET NO:

108 / 133

PRECAST SAFETY END TREATMENT TYPE II ~ PARALLEL DRAINAGE

PSET-SP

FILE:	psetspss-21.dgn	DN: RL	V	CK: KLR	DV/:	JTR		CK:	GAF
©TxD0T	February 2020	CONT	SECT	JOB			HIG	HWA	
12-21: /	REVISIONS Added 42" TP								
		DIST		COUNTY				SHEE	T NO.

REVISIONS NAME TXDOT REVISED STANDARD ISSUE 12/21







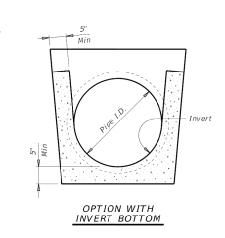
	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	Г
	CK'D BY:	SHEET DESCRIPTION: TXDOT PRECAST S.E.TTYII-PARALLEL	
1	SCALE: 1" = 40'	DRAINAGE (PSET-SP)	Γ
-	DATE: 1/16/2023	APPROVED BY:	

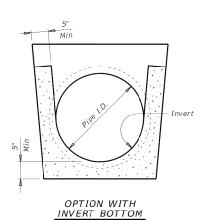
REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS Single Pipe Multiple Pipes RCP Wal. Min Wall Pipe I.D. Length Pipe Thickness hicknes "D" Slope of Uni Skew Runners Skew Runners (8) 3:1 2' - 11 12" 1.15" 17.00" 4:1 3' - 6" ≤ 45° No ≤ 45° 6:1 4' - 9" J:1 J' - 8" 15" 2 1/4" 1.30" 20.50" 4:1 4' - 7'' ≤ 45° No ≤ 45° 6:1 6' - 5' 3:1 4' - 6" 18" 2 1/2" 1.60" 24.00 4:1 5' - 8'' ≤ 45 NO ≤ 45° 6:1 8' - 0" 3:1 6' - 2" = 30° 24" 4:1 7' - 10' ≤ 45° 1.95" 31.00" No > 30° 6:1 11' - 3" 3:1 7' - 10' $= 15^{\circ}$ No = 15° 30" 4:1 3 ½" 2.65" 38.50 10' - 1" > 15° > 15° Yes 6:1 14' - 8" 3:1 9' - 5" = 0° No 36" 2.75" 45.50" 4:1 12' - 3" $\geq 0^{\circ}$ > 00 Yes 6:1 17' - 11' 3:1 11' - 1' 42" 2.7" 4 1/2" 52.50" 4:1 14' - 5" ≥ 0° Yes ≥ 0° 6:1 21' - 2" Pipe support cradie welded € ¾" galvanized steel ų̃ Safety bolt and nut with washer pipe runner to support post Flowline ¾" Threaded insert © Pipe support post (post to be same diameter as safety pipe runner and `¾" qalvanized steel bolts with fitted in a formed pocket) washers and END DETAIL FOR INSTALLATION OF SAFETY PIPE RUNNERS OPTIONAL JOINT FOR RCP (5) Reinforcement to have 1" Min cover Min

Cement stabilized

bedding and backfill (7

MULTIPLE PIPE INSTALLATION







Unit length (varies)

(if required) —

- See Detail "A'

Pocket is to be formed to fit 0.D. of pipe support post

PLAN

(Showing bell end connection.)

LONGITUDINAL ELEVATION

(Showing bell end connection.)

Safety

if safety pipe runners are used.-

Safety pipe runner

have an O.D. of - 1⁄4" to 5⁄8" less

than the I.D. of

the safety pipe

OPTION A

Pipe Dia

Cross pipe

Top face of safety end treatment

Optional casting

line for toewall

DETAIL A

(If required)

- C ¾" galvanized steel bolts with washers and inserts

- End of payment for pipe

7" Max 1'-0"_

step slope

1

Safety pipe runner length 6

SAFETY PIPE RUNNER **DIMENSIONS**

Max Safety	Required Pipe Runner Size				
Pipe Runner Length	Pipe Size	Pipe O.D.	Pipe I.D.		
11' - 2"	3" STD	3.500"	3.068"		
15' - 6"	3 ½" 51 D	4.000"	3.548"		
20' - 10"	4" STD	4.500"	4.026"		
35' - 4"	5" STD	5.563"	5.047"		

- (1) Dimension "D" is based on reinforced concrete pipe (RCP) meeting the requirements of ASTM C-76, Class III, (RCP Wall "B" thickness). Adjust "D" for any other wall thickness used. For thermoplastic pipe (TP) take into account the annular space requirements for arouted connections.
- 2 Slope as shown elsewhere in plans. Slope of 3:1 or flatter is required for vehicle safety.
- $\widehat{\mbox{\it 3}}$ Toewall to be used only when dimension is shown elsewhere in the plans.
- $\stackrel{\textstyle \textcircled{4}}{}$ Fill the top 4" of void between precast end treatments with concrete riprap. Concrete riprap is considered subsidiary to the Item 467, "Safety End Treatment".
- $\stackrel{\textstyle \frown}{}$ Adjust clear distance between pipes to provide for the minimum distance between safety end
- 6 Measured along slope.

Safety

pipe runner

1/4

Cross pipe to

be same size

as safety pipe runner or

large

OPTION B

- 7 Provide cement stabilized bedding and backfill in accordance with the Item 400, "Excavation and Backfill for Structures". Bedding and backfill is considered subsidiary to the Item 467, "Safety End Treatment". When concrete riprap is specified around the safety end treatment, backfill as directed by Engineer
- ${f 8}$ Thermoplastic pipe wall thickness may vary. Adjust accordingly. Thermoplastic pipe requires the safety end treatments to have a bell end for grouted connections.

GENERAL NOTES:

Precast safety end treatment for reinforced concrete pipe (RCP), and thermoplastic pipe (TP) may be used for TYPE II end treatment as specified in Item "Safety End Treatment". When precast safety end treatment is used as a Contractor's alternate

to mitered RCP, riprap will not be required unless noted otherwise on

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Manufacture this product in accordance with Item 467, "Safety End

Treatment" except as noted below:

A. Provide minimum reinforcing of #4 at 6" (Grade 40)

or #4 at 9" (Grade 60) each way or 6"x6" - D12 x D12 or 5"x5" - D10 x D10 welded wire reinforcement (WWR).

B. For precast (steel formed) sections, provide Class "C" concrete

(f'c = 3,600 psi). At the option and expense of the Contractor, the next larger size of

At the option and expense or the Contractor, the next larger size or safety end treatment may be furnished as long as the "D" dimension cast is that of the required size of pipe.

Pipe runners are designed for a traversing load of 1,800 Lbs at yield as recommended by Research Report 280-1, "Safety Treatment of Roadside Cross-Drainage Structures", Texas Transportation Institute, March 1981.

Provide safety pipe runners, cross pipes, pipe support posts, and pipe stubs meeting the requirements of ASTM A53 (Type E or 5, Grade B), ASTM A500 (Grade B), or API SLX52.

Galvanize all steel components excent reinforcing steel after fabrication.

Galvanize all steel components except reinforcing steel after fabrication Repair galvanizing damaged during transport or construction in accordance with the specifications.

Connect RCP using the Optional Joint for RCP detail shown or in accordance with Item 464 "Reinforced Concrete Pipe". Connect TP by grouting. See Pipe and Box Grouted Connections (PBGC) standard for grouted connections with TP and precast safety end treatment.



PRECAST SAFETY END TREATMENT TYPE II ~ CROSS DRAINAGE

PSET-SC

FILE:	psetscss-21.dgn	DN: RL	N	ck: KLR	DW:	JTR	CK: GA
©TxD0T	February 2020	CONT	SECT	JOB		F	IIGHWAY
REVISIONS 12-21: Added 42" TP							
		DIST		COUNTY			SHEET N

REVISIONS NAME 12/21 TXDOT REVISED STANDARD ISSUE RI W

FORT BEND COUNTY TEXAS

OPTION WITH

SQUARE BOTTOM

SECTION A-A

Pipe

No

No

NO

No

Yes

No

Yes

Yes

Yes

Precast end

section may

with spigot

or bell end

(Showing joint between RCP and

precast safety end treatment)



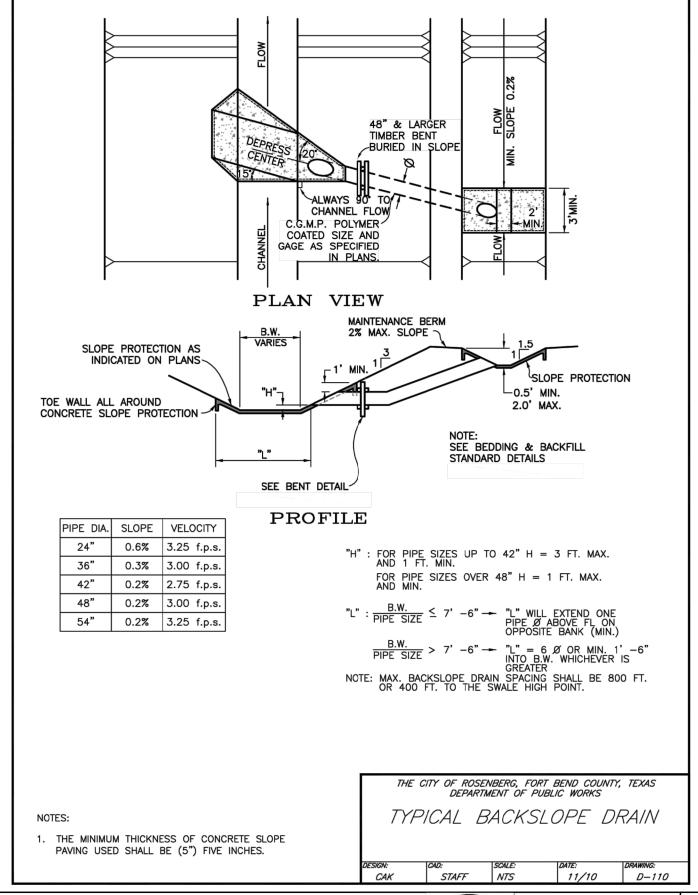




L	PROJECT TITL	E: STELLA ROAD
43.	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W.
	CK'D BY:	SHEET DESCRIPTION: TXDOT PRECAST S.E.TT
1 110	SCALE: 1" = 40'	DRAINAGE (PSET-S
1/2024	DATE: 1/16/2023	APPROVED BY:
1/2024	1/16/2023	

D. TO W. FAIRGROUNDS RD. S.E.T.-TYII-CROSS E (PSET-SC)

SHEET NO: 109 / 133



NO.	REVISIONS	DATE	NAME
Λ	ORIGINAL STANDARD ISSUED	3-1-22	RJS

Details.dwg

Road\CAD\DWG\07- 04755

FORT BEND COUNTY ENGINEERING DEPARTMENT

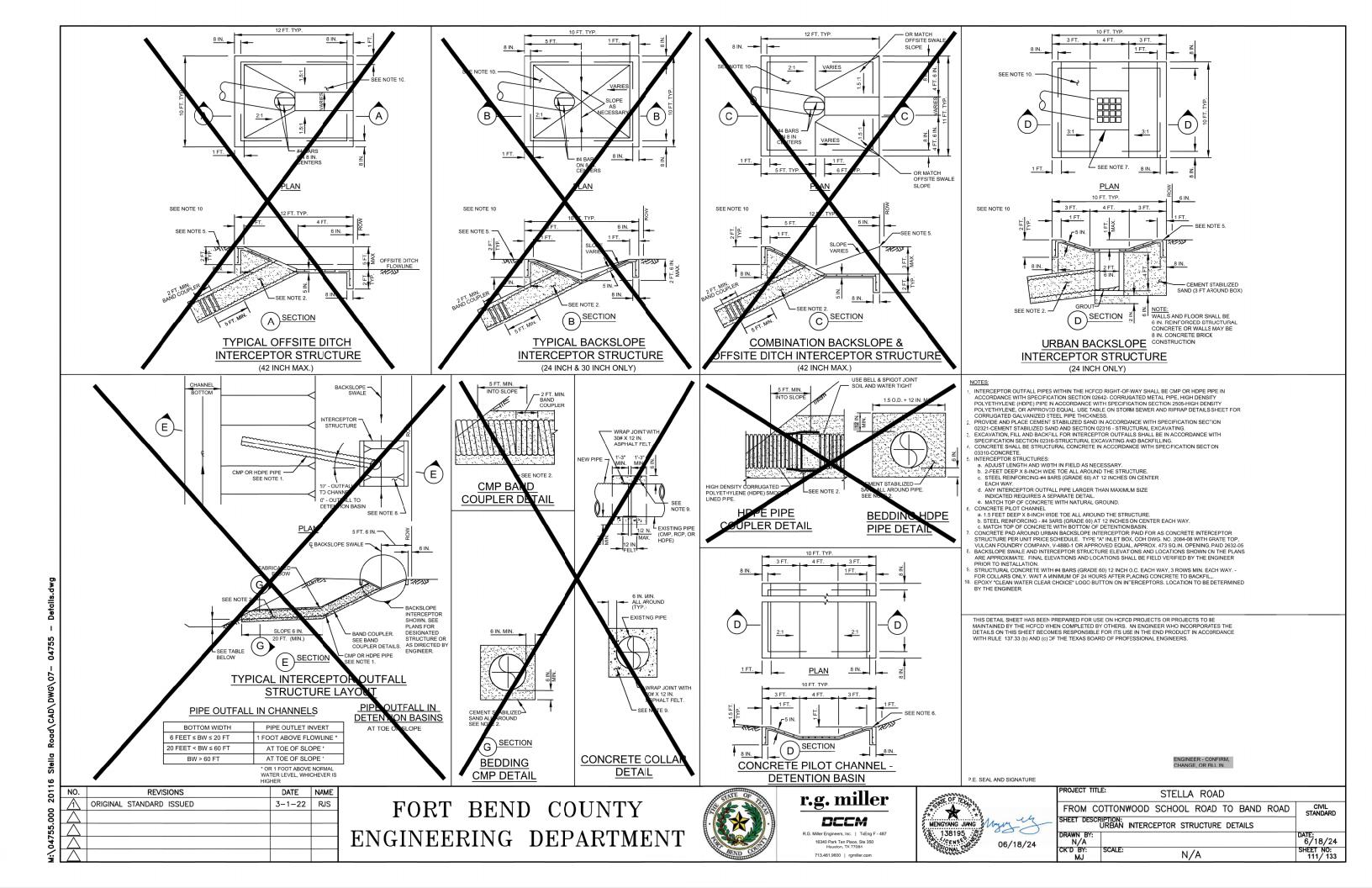


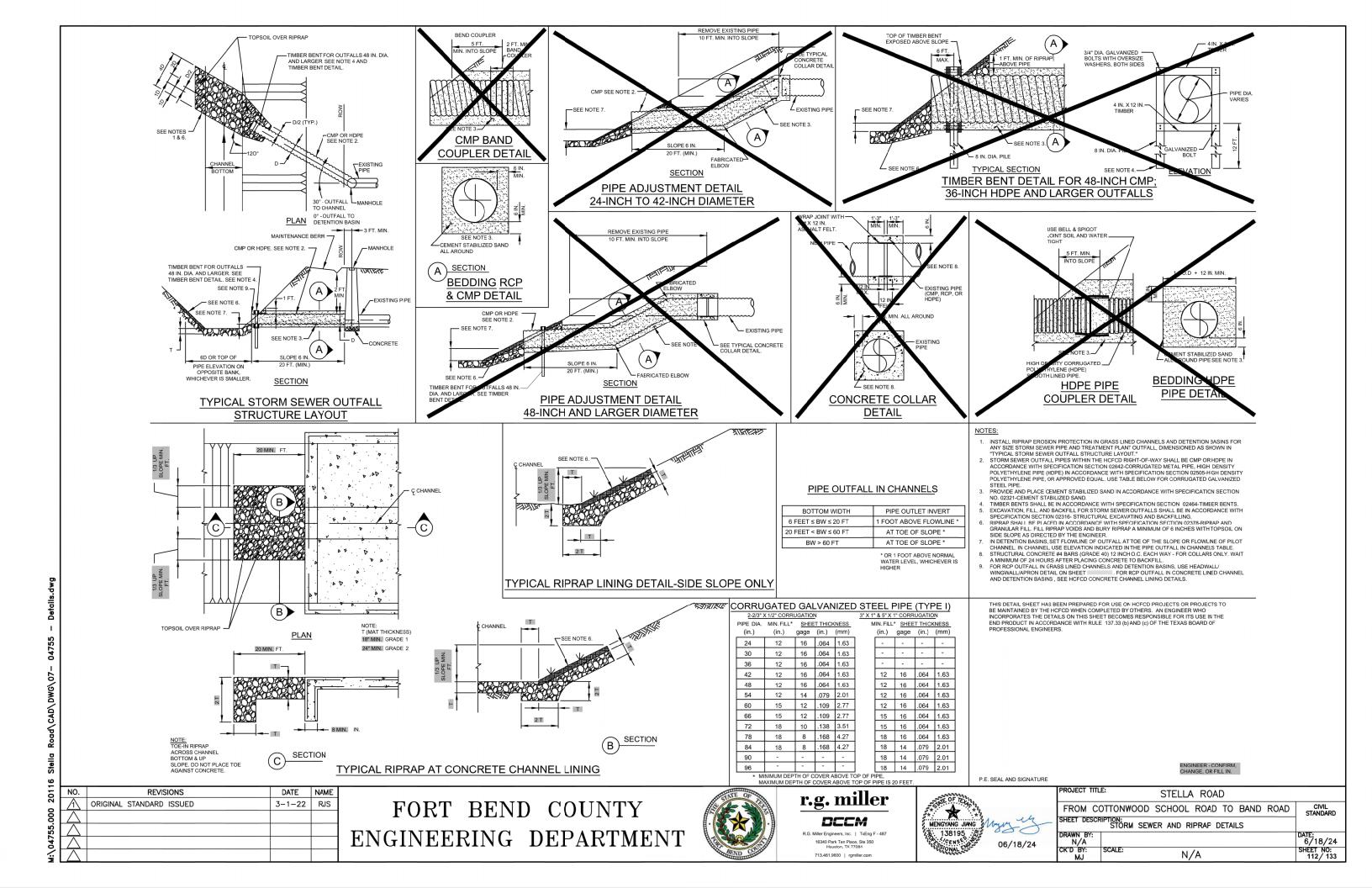
r.g. miller

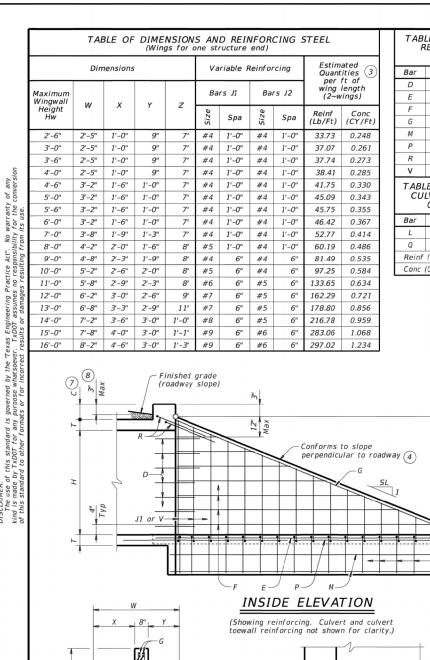
R.G. Miller Engineers, Inc. | TxEng F - 487 16340 Park Ten Place, Sle 350 Houston, TX 77084 713.461.9600 | romiller.com

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CENS 06/18/24

PROJECT TITL	E: STELLA ROAD	
	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR		
DRAWN BY: N/A		DATE: 6/18/24
CK'D BY: MJ	SCALE: N/A	SHEET NO: 110/133







Permissible

WINGWALL

TABLE OF WINGWALL REINFORCING WING DIMENSION FORMULAS: (2~wings) (All values are in feet.) Bar Size No. Spa #5 Lw = (Hw - 0.333') (SL)#4 ~ 1'-0" Ltw = (N)(S) + (N + 1)(U)#6 #4 For precast culverts: Ltw = (N) (2U + S) + (N - 1) (0.5')P #4 ~ 1'-0" R #5 6 ~ V #4

Details.

Length of wings based on SL:1

slope along

FOOTING AND TOEWALL

Toe of

TABLE OF ESTIMATED CULVERT TOEWALL

QUANTITIES					
Bar	Size	No.	Spa		
L	#4	~	1'-6"		
Q	#4	1	~		
Reinf (Lb/Ft)			2.45		
			0.037		

Total Wingwall Area (two wings ~ SF) = (Hw + 0.333') (Lw)

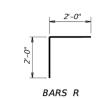
Hw = Height of wingwall SL:1 = Side slope ratio (horizontal:1 vertical) Lw = Length of wingwall

PLAN

(Showing dimensions.)

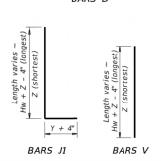
= Number of culvert spans

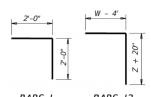
See applicable box culvert standard sheet for H, S, T, and U values.



4'-0"

BARS D





BARS L BARS J2

6 At Contractor's option, culvert toewall may be ended flush with wingwall toewall. Adjust reinforcing O" Min to 5'-O" Max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail or curbs taller than 1'-0, refer to the Extended Curb Details (ECD) standard sheet. For structures with T631 or T631& T631LS bridge rail, refer to the Mounting Details for T631& T631LS Rails (T631-CM) standard sheet. Refer to the Box Culvert Rail Mounting Details (RAC) standard sheet for structures with bridge

8 For rehicle safety, the following requirements must be met:

1) Extend Bars P 3'-0" minimum into bottom slab of

Adjust as necessary to maintain 1 1#2" clear cover and 4" minimum between bars.

(5) When shown elsewhere on the plans, construct

Quantities shown are based on an average wing height for two wings (one structure end). To determine total

5" deep concrete riprap. Payment for riprap is as required by Item 432, "Riprap". Unless otherwise shown on the plans or directed by the Engineer,

provide a 6" wide by 1'-6" deep reinforced concrete toewall along all edges of the riprap adjacent to natural ground; reinforce the toewall by

extending typical riprap reinforcing into the toewall; and extend construction joints or grooved joints oriented in the direction of flow across the full

distance of the riprap at intervals of approximately 20'

When such riprap is provided, the culvert toewall shown in SECTION B-B will not be required.

quantities for two wings, multiply the tabulated values

4 Recommended values of side slope are: 2:1, 3:1, 4:1, and 6:1.

For structures without bridge rail, construct curbs no more than 3" above finished grade.
 For structures with bridge rail, construct curbs flush with finished erade.

with finished grade. Reduce curb heights, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.

MATERIAL NOTES:

rail other than T631 or T631LS.

Provide Class C concrete (fc=3,600 psi). Provide Grade 60 reinforcing steel.

Provide galvanized reinforcing steel if required elsewhere in the plans.
In riprap concrete, synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL)

may be used in lieu of steel reinforcing unless noted otherwise.

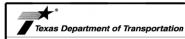
GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

When structure is founded on solid rock, depth of toewalls for culverts and wingwalls may be reduced or eliminated as directed by the Engineer.

See Box Culvert Supplement (BCS) standard sheet for additional dimensions and information. The quantities for concrete and reinforcing steel

resulting from the formulas given on this sheet are for Contractor's information only.



Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing dimensions are out-to-out of bars.

CONCRETE WINGWALLS WITH STRAIGHT WINGS FOR 0° SKEW BOX CULVERTS

SW-O

FILE:	sw-Ustae-20.agn	DN: GA	-	CK: CAI	DW:	I XDUI	CK: I XDOI
©TxD0T	February 2020	CONT	SECT	JOB		F	IIGHWAY
	REVISIONS						
		DIST		COUNT			SHEET NO.

NO. REVISIONS DATE NAME

Const joint

Wingwall toewall 6"

SECTION A-A

Details

04755

FORT BEND COUNTY ENGINEERING DEPARTMENT

CORNER DETAILS



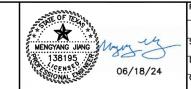


-Culvert bottom slab reinforcing

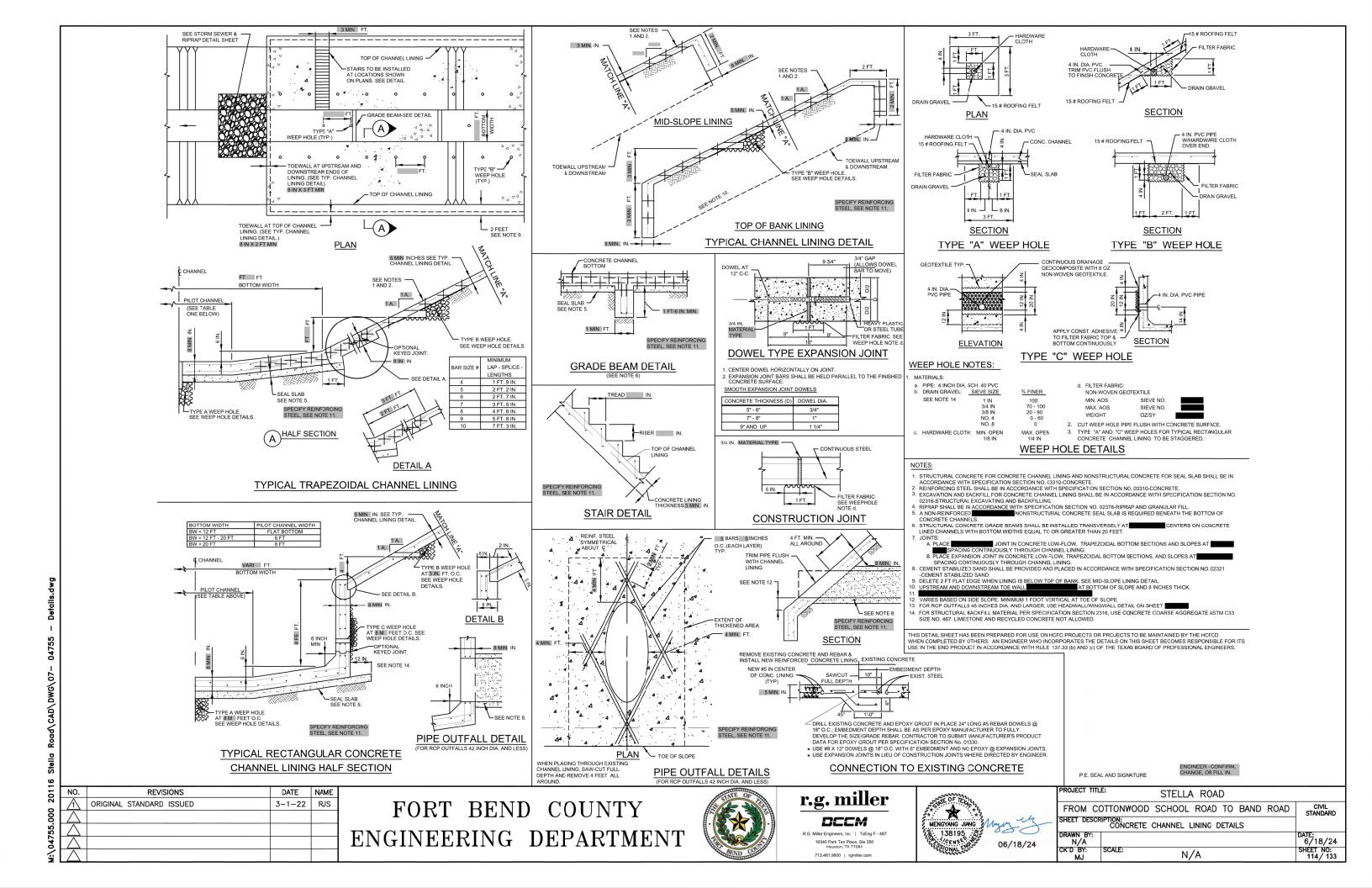
Culvert toewall 6"

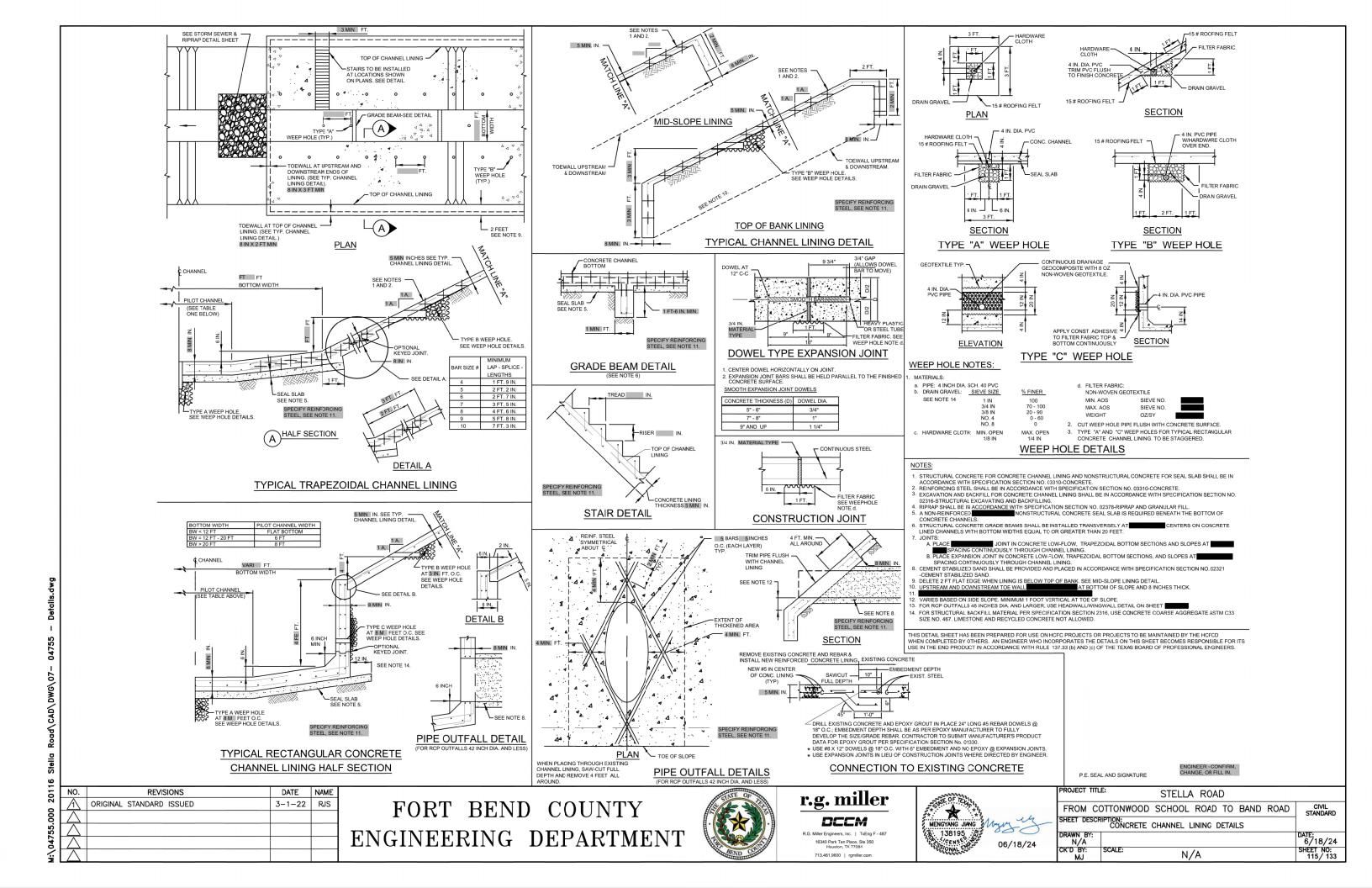
SECTION B-B (5)

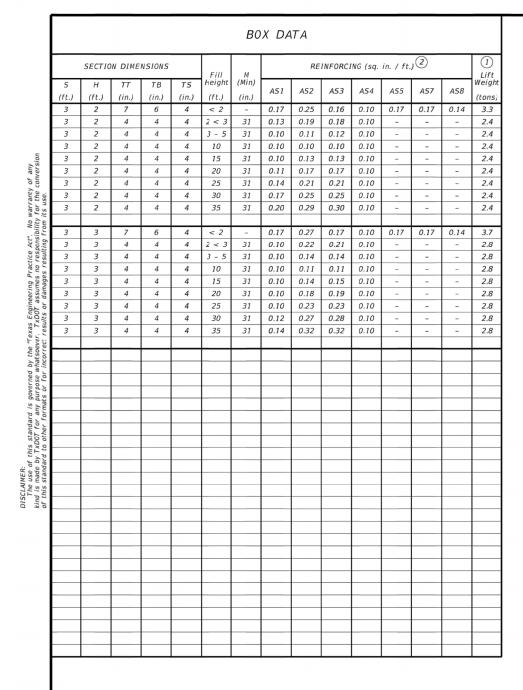
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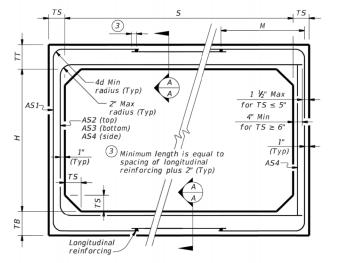


ROJECT TITL	^{E:} STELLA ROAD	
	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
HEET DESCR		
RAWN BY: N/A		DATE: 6/18/24
K'D BY: MJ	SCALE: N/A	SHEET NO: 113 / 133









FILL HEIGHT 2 FT AND GREATER

½" Min (Typ)

2" Max (Tvp) Longitudinal reinforcemer

CORNER OPTION "A"

AS3 (bottom)

6" Min_

3 Outer cage

CORNER OPTION "B"

A53 (bottom)

CORNER OPTION "A"

4d Min

oted otherwise)

Тур)

radius (Typ)

radius (Typ)

-AS8

CORNER OPTION "B"

-AS3

for TS ≤

4" Min

for $TS \ge 6$

-AS1

FILL HEIGHT LESS THAN 2 FT

4 Length is equal to spacing of longitudinal reinforcing plus 2". (10" Min) (Typ)

-AS2 -AS7

circumferential reinforcement at groove end. SECTION A-A

(Showing top and bottom slab joint reinforcement.,

MATERIAL NOTES:

Provide 0.03 sq. in./ft. minimum longitudinal reinforcement at each face in slabs and walls. This minimum requirement may be met by the transverse wires when wire mesh reinforcement is used.

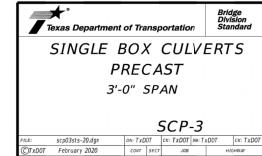
Provide Class H concrete (f'c = 5,000 psi).

GENERAL NOTES:
Designs shown conform to ASTN C1577. Refer to ASTM C1577 for information or details not shown.

See Box Culverts Precast Miscellaneous Details (SCP-ND) standard sheet for details and notes not shown.

In lieu of furnishing the designs shown on this sheet, the contractor may furnish an alternate design that is equal to or exceeds the box design for the design fill height in the table. Submit shop plans for alternate designs in accordance with them. with Item "Precast Concrete Structural Members (Fabrication)".

HL93 LOADING



1) For box length = 8'-0''

2 AS1 thru AS4, AS7 and AS8 are minimum required areas of reinforcement per linear foot of box length. AS5 is minimum required area of reinforcement per linear foot of box width.

DATE NAME NO. REVISIONS ORIGINAL STANDARD ISSUED 3-1-22 RJS

FORT BEND COUNTY ENGINEERING DEPARTMENT



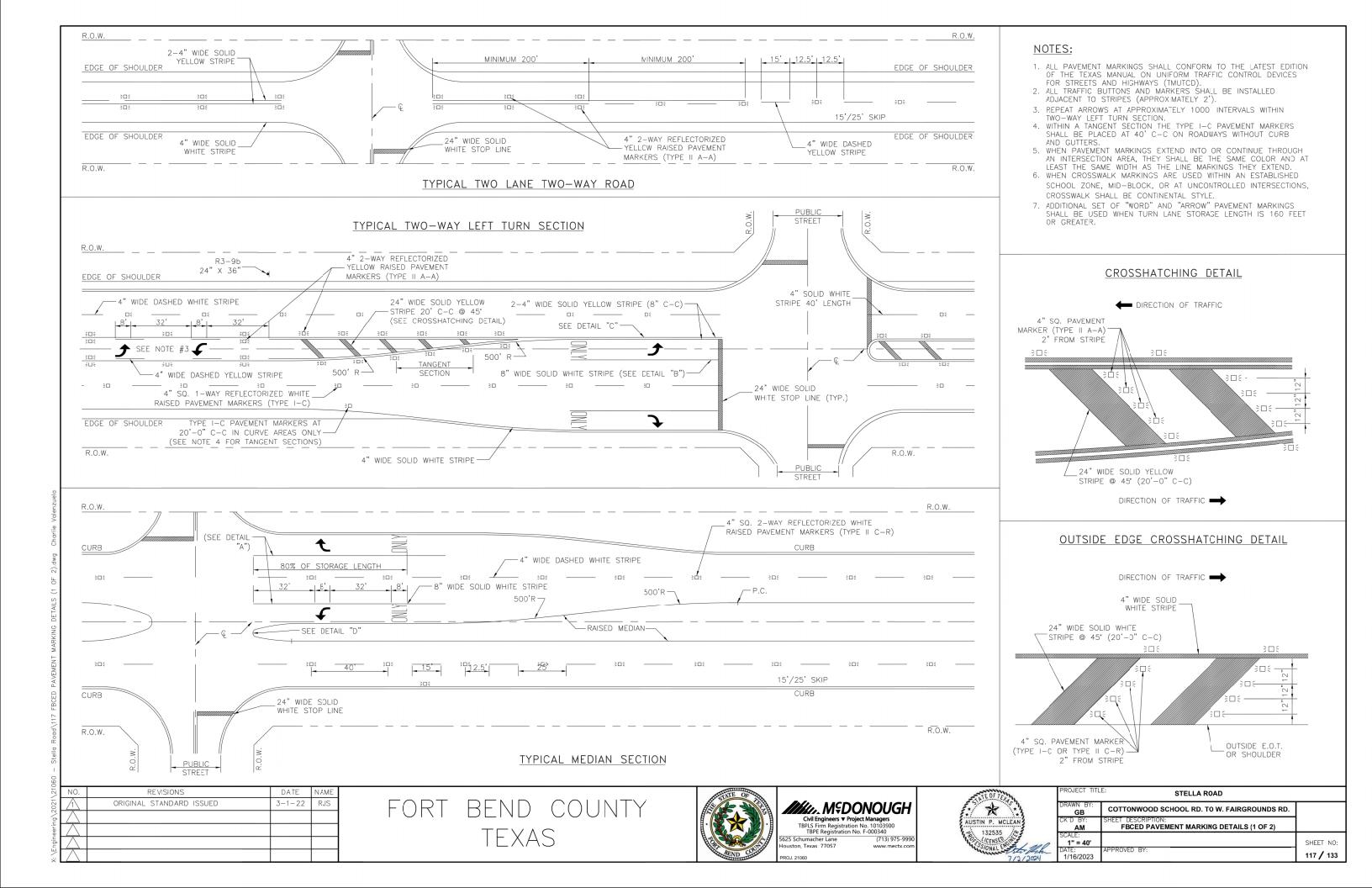
r.g. miller DCCM

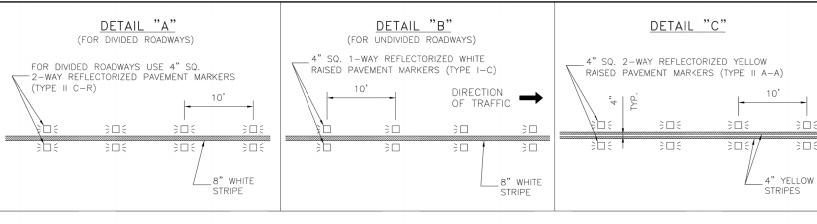
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CIVIL STANDARD
ATE: 6/18/24
SHEET NO: 116 / 133

04755





PAVEMENT MARKER LEGEND

DESCRIPTION

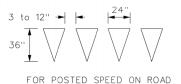
4" x 4" REFLECTORIZED }□€ RAISED PAVEMENT MARKER

SYMBOL

INDICATED DIRECTION OF TRAFFIC FLOW

YIELD LINE DETAILS

FOR POSTED SPEED ON ROAD BEING MARKED EQUAL TO OR LESS THAN 40 MPH.



BEING MARKED EQUAL TO OR GREATER THAN 45 MPH

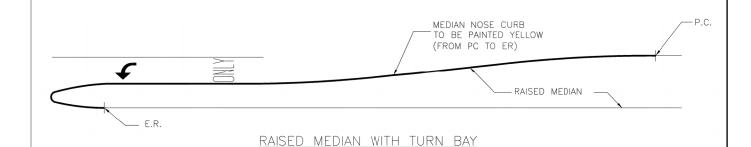
DETAIL "D"

* PAINT FROM THE BACK OF CURB TO THE GUTTER LINE

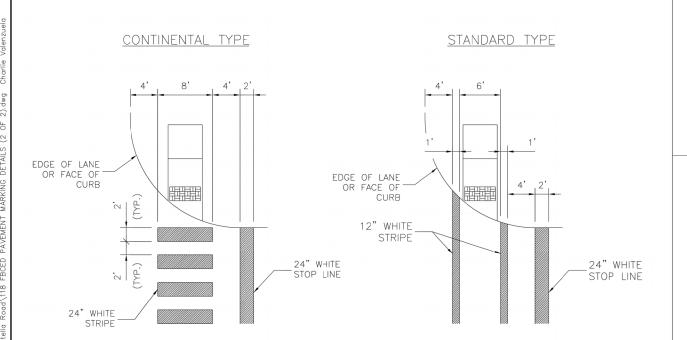
YELLOW PAINT



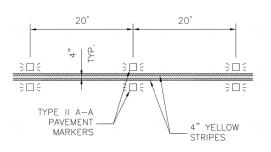
TYPICAL CURB SECTION



TYPICAL CROSSWALK PLACEMENT



CENTER LINE DETAIL CURVE SECTION



CENTER LINE DETAIL

TANGENT SECTION

40'

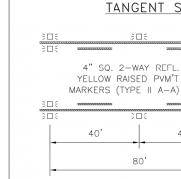
4" YELLOW

40'

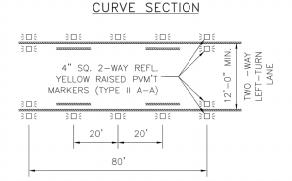
TYPE II A-A PAVEMENT

MARKERS

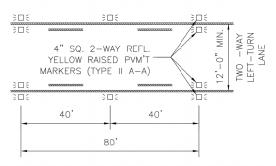
TYPICAL MEDIAN NOSE

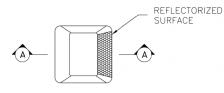


TWO-WAY LEFT TURN



TWO-WAY LEFT TURN TANGENT SECTION

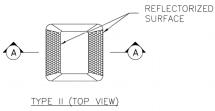


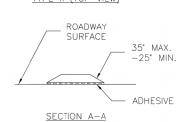


RAISED PAVEMENT

MARKERS

TYPE I (TOP VIEW)





DATE NAME REVISIONS ORIGINAL STANDARD ISSUED 3-1-22

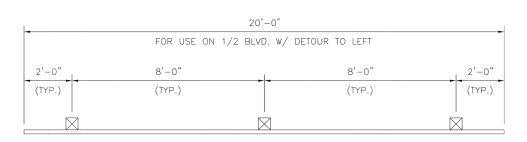




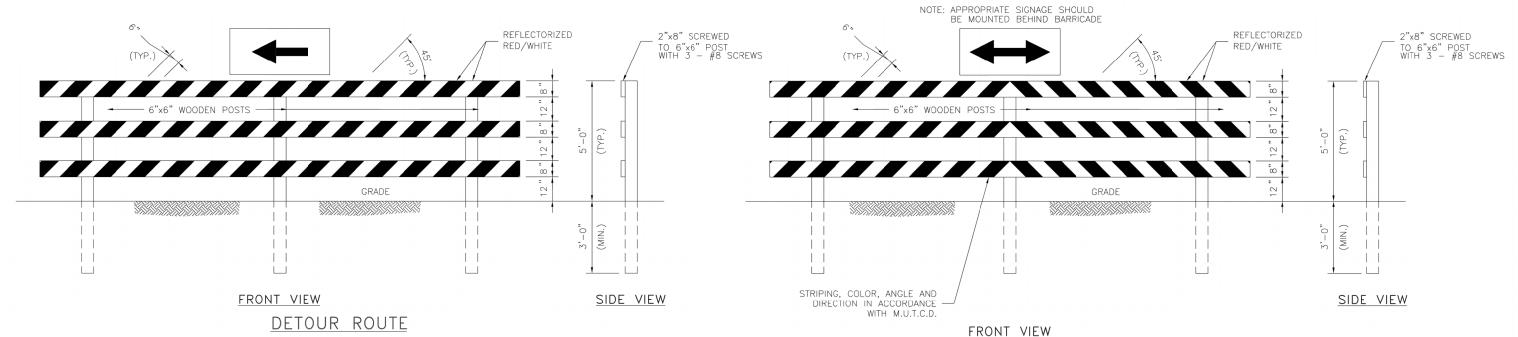


	PROJECT
	DRAWN B
	CK'D BY:
1 110	SCALE: 1" = 40
-11/2	DATE:

PROJECT TITL	E: STELLA ROAD	
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY: AM	SHEET DESCRIPTION: FBCED PAVEMENT MARKING DETAILS (2 OF 2)	
SCALE: 1" = 40'		SHEET NO:
DATE: 1/16/2023	APPROVED BY:	118 / 133

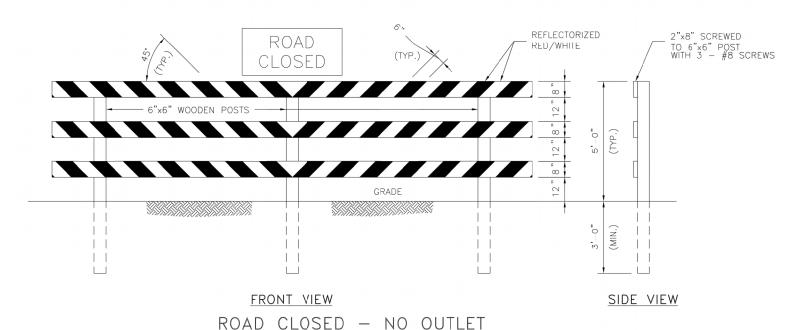


PLAN VIEW



DETOUR ROUTE

T-INTERSECTION



APPLICATION: PERMANENT AND SEMI-PERMANENT CLOSURE OF ROADWAY OR ROADWAY TERMINATION

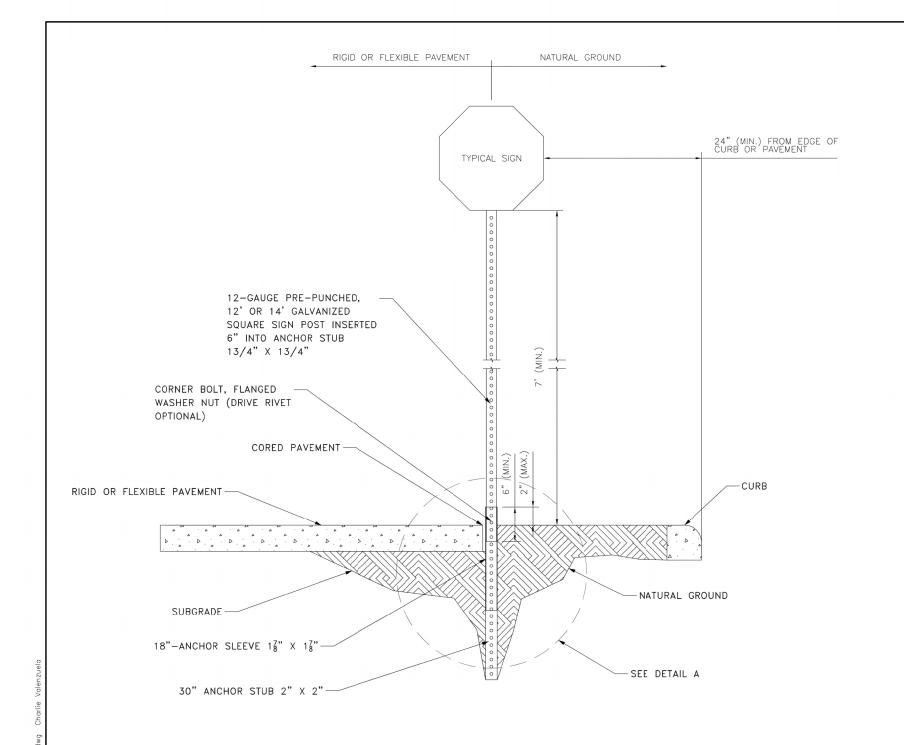
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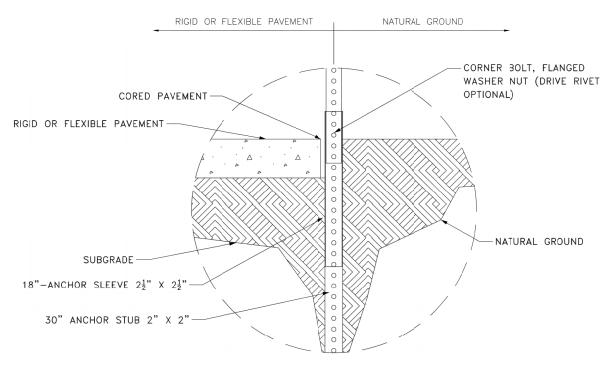






PROJECT TITL		
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY: AM	SHEET DESCRIPTION: FBCED TYPE III BARRICADE DETAILS	
SCALE: 1" = 40'		SHEET NO:
DATE: 1/16/2023	APPROVED BY:	119 / 133





NOTES

 THE CROSS SECTION OF ALL MEMBERS SHALL BE SQUARE TUBE FORMED OF 12 GAUGE AND MANUFACTURED FROM HOT-GALVANIZED STEEL

TYPICAL GROUND SIGN INSTALLATION
DETAIL A

- 2. THE TELESCOPE BREAKAWAY SYSTEM OR "SYSTEM" IS DEFINED AS FOLLOW:
 - A MINIMUM 30" ANCHOR STUB;
 - 18" ANCHOR SLEEVE.
- 3. DRIVE THE SYSTEM TOGETHER MAKING SURE THE HOLES ARE ALIGNED.
- 1. THE SYSTEM IS TO BE DRIVEN INTO NATURAL GROUND EXPOSED SUBGRADE UNTIL ONLY 1 TO 2 INCHES ARE LEFT EXPOSED.
- 5 ATTACH THE SIGN TO AN 1 3/4" SQUARE POST AT THE DESIRED HEIGHT, SUCH THAT IT MEETS THE MINIMUM VERTICAL CLEARANCE.
- 6. SIGNS ARE FASTENED TO THE POST BY USING DRIVE RIVETS OR BOLTS.
- 7. INSERT THE SIGN POST APPROXIMATELY 6 TO 8 INCHES INTO THE ANCHOR BASE.
- 8. BOLT THE SIGN POST TO THE ANCHOR ASSEMBLY WITH A CORNER BOLT.
- 9. WHEN INSTALLING IN RIGID OR FLEXIBLE PAVEMENT, USE A CORING MACHINE TO EXPOSE THE SUBGRADE MATERIAL AND INSTALL THE SYSTEM.

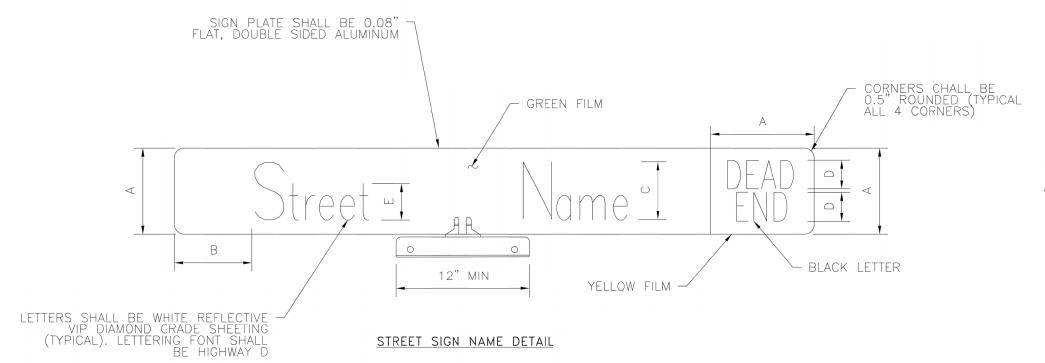
	NO.	REVISIONS	DATE	NAME
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	PROJECT TITLE: STELLA ROAD		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: FBCED TYPICAL GROUND SIGN INSTALLATION	
1	SCALE: 1" = 40'		SHEET NO:
-	DATE: 1/16/2023	APPROVED BY:	120 / 133

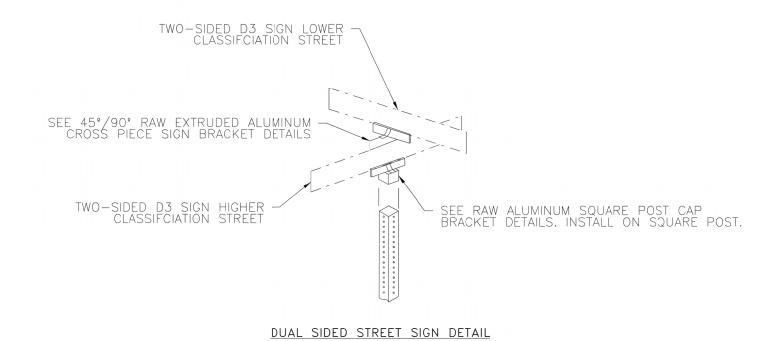


DIMENSION SCHEDULE

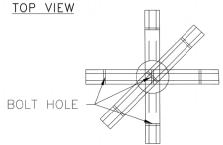
2" 6" 3.5" 4.5"

NOTES:

ALL STREET BLADES SHALL BE 9" IN HEIGHT



45°/90° RAW EXTRUDED ALUMINUM CROSS PIECE SIGN BRACKET DETAILS RAW ALUMINUM SQUARE POST CAP BRACKET DETAILS



FRONT VIEW

13" SQUARE POST

SIDE VIEW



SIDE VIEW



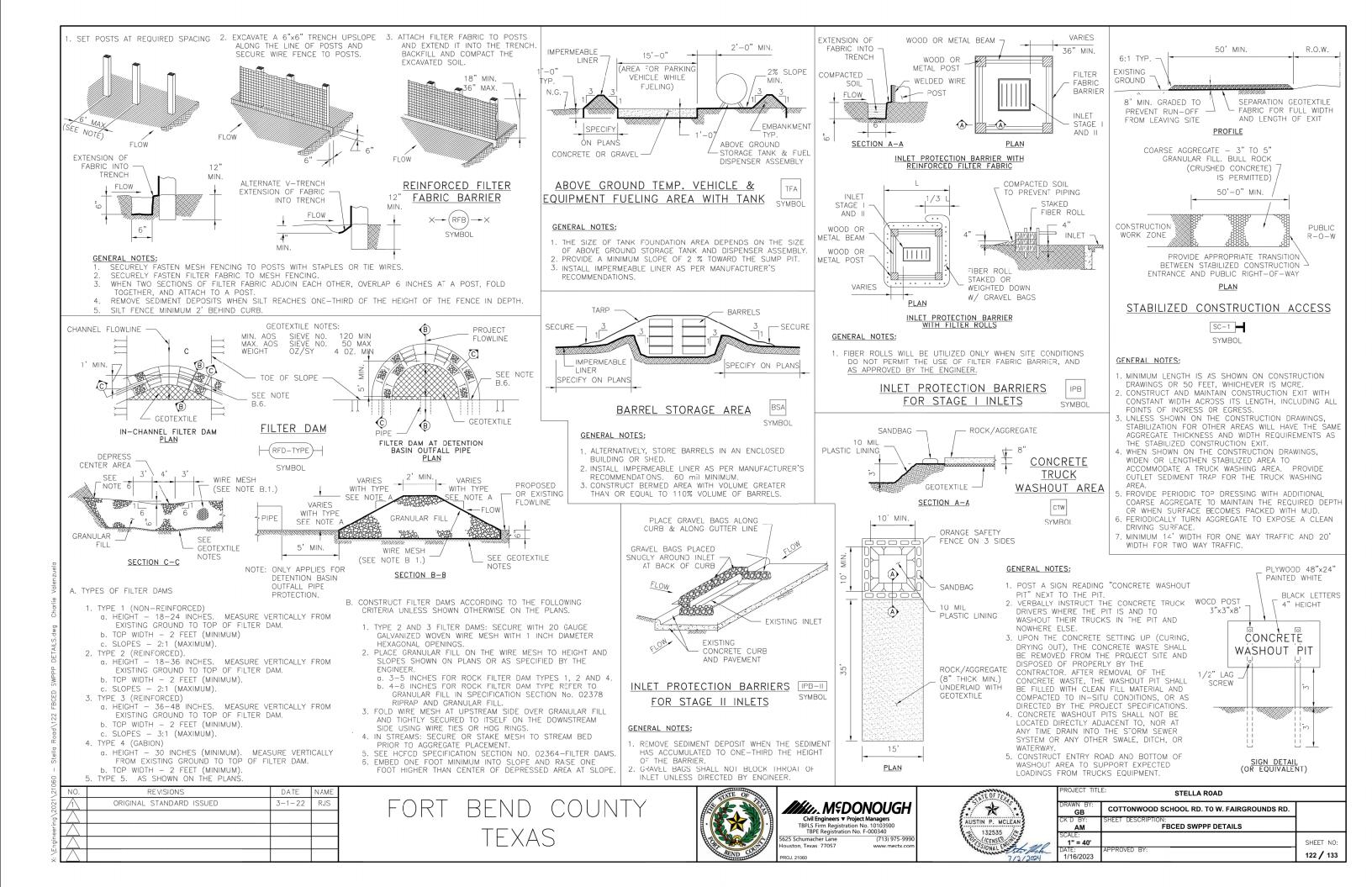
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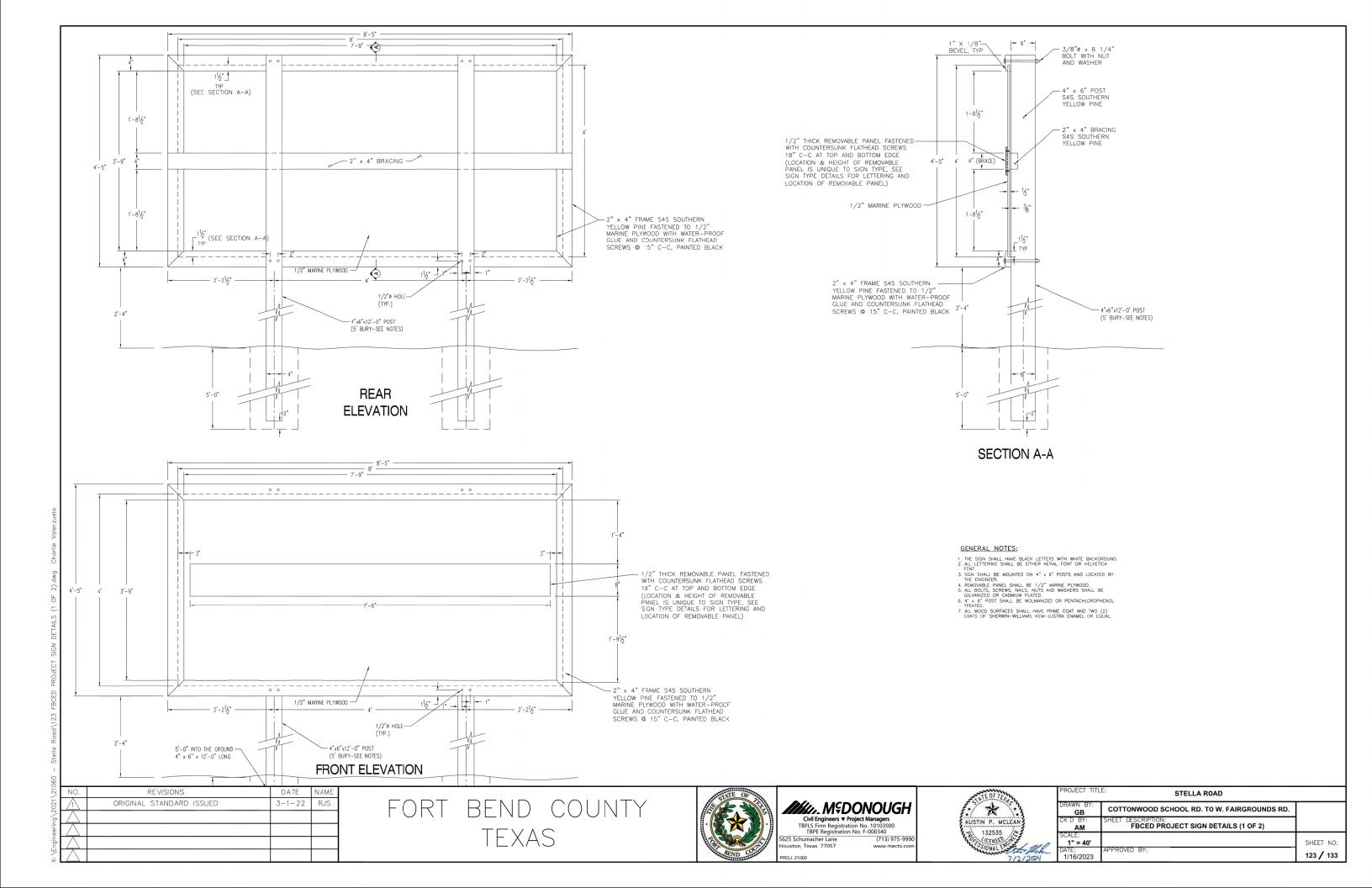


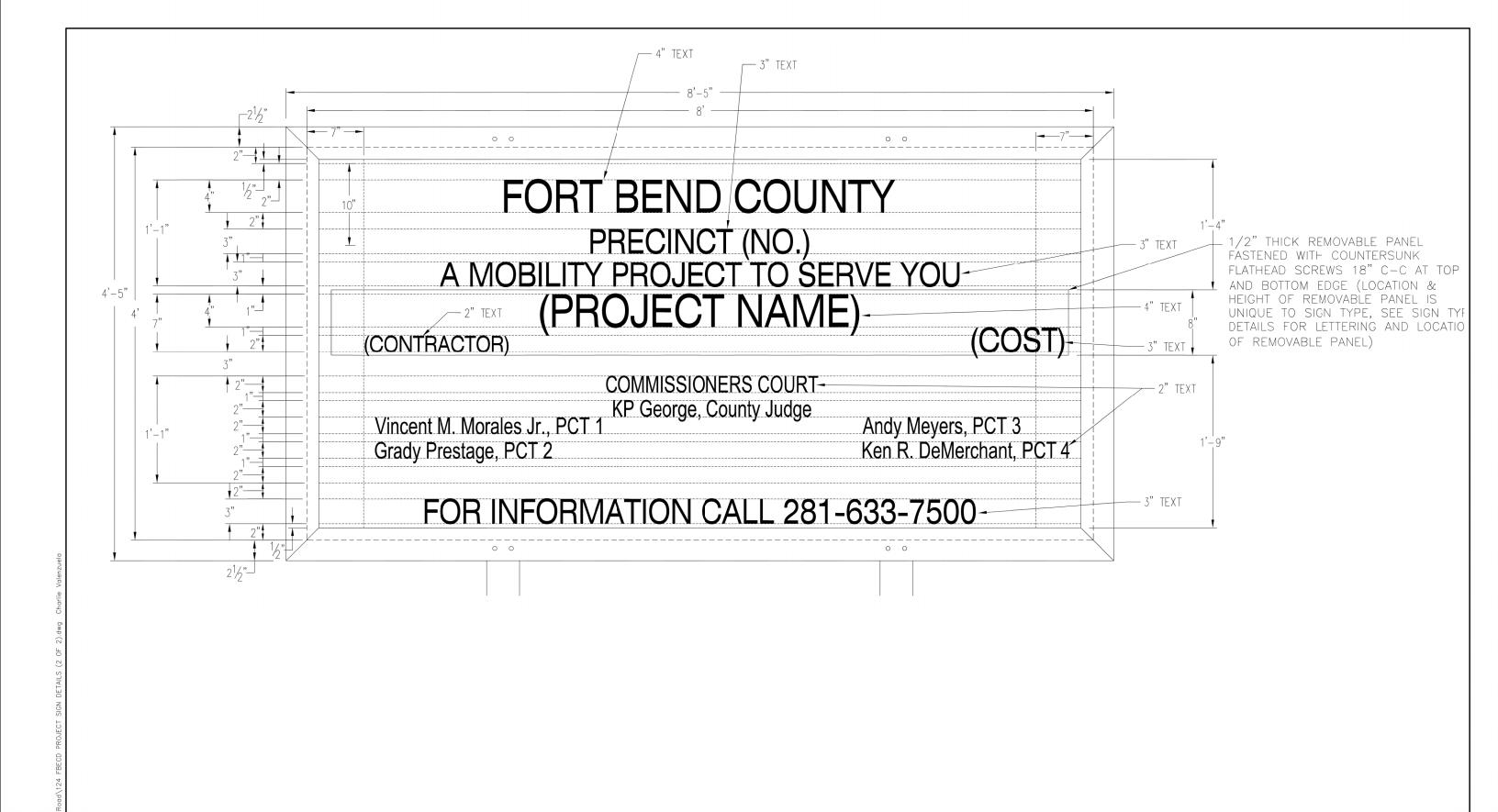




	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: FBCED STREET SIGN NAME DETAILS	
110	SCALE: 1" = 40'		SHEET NO:
24 24	DATE: 1/16/2023	APPROVED BY:	121 / 133







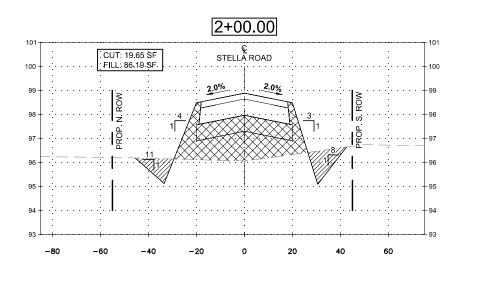
NO.	REVISIONS	DATE	NAME
\triangle	ORIGINAL STANDARD ISSUED	3-1-22	RJS
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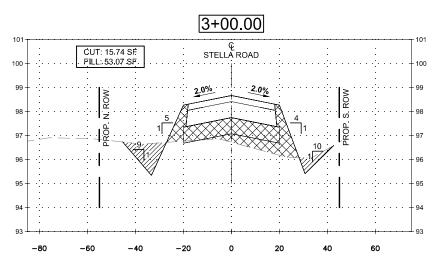


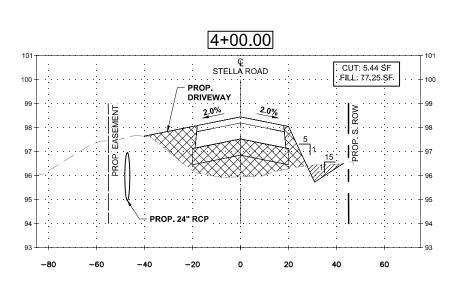




_	PROJECT TITL	F	
	PROJECT TITL		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: FBECD PROJECT SIGN DETAILS (2 OF 2)	
	SCALE: 1" = 40'		SHEET NO:
	DATE: 1/16/2023	APPROVED BY:	124 / 133

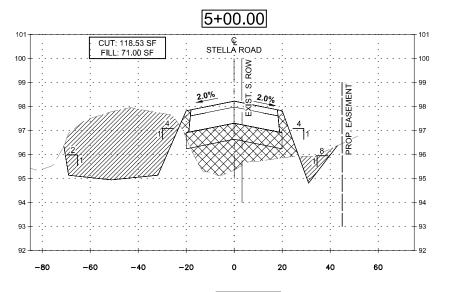


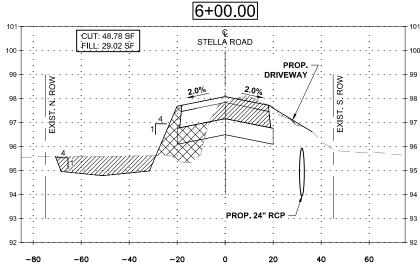


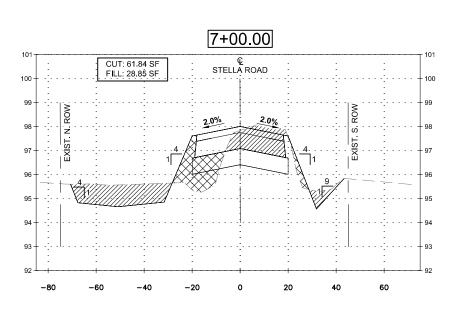


DATE NAME

REVISIONS







FORT BEND COUNTY TEXAS







_			
PROJECT TITL	PROJECT TITLE: STELLA ROAD		
DRAWN BY: GB	I COTTONWOOD SCHOOL BD TOW EXIDEDOLINDS DD I		
CK'D BY:	SHEET DESCRIPTION: CROSS SECTIONS		
SCALE: 1" = 20'	2+00 TO 7+00	SHEET NO:	
DATE: 1/16/2023	APPROVED BY:	125 / 133	

2' 0 4'
SCALE 1"=4' (VERTICAL)

LEGEND



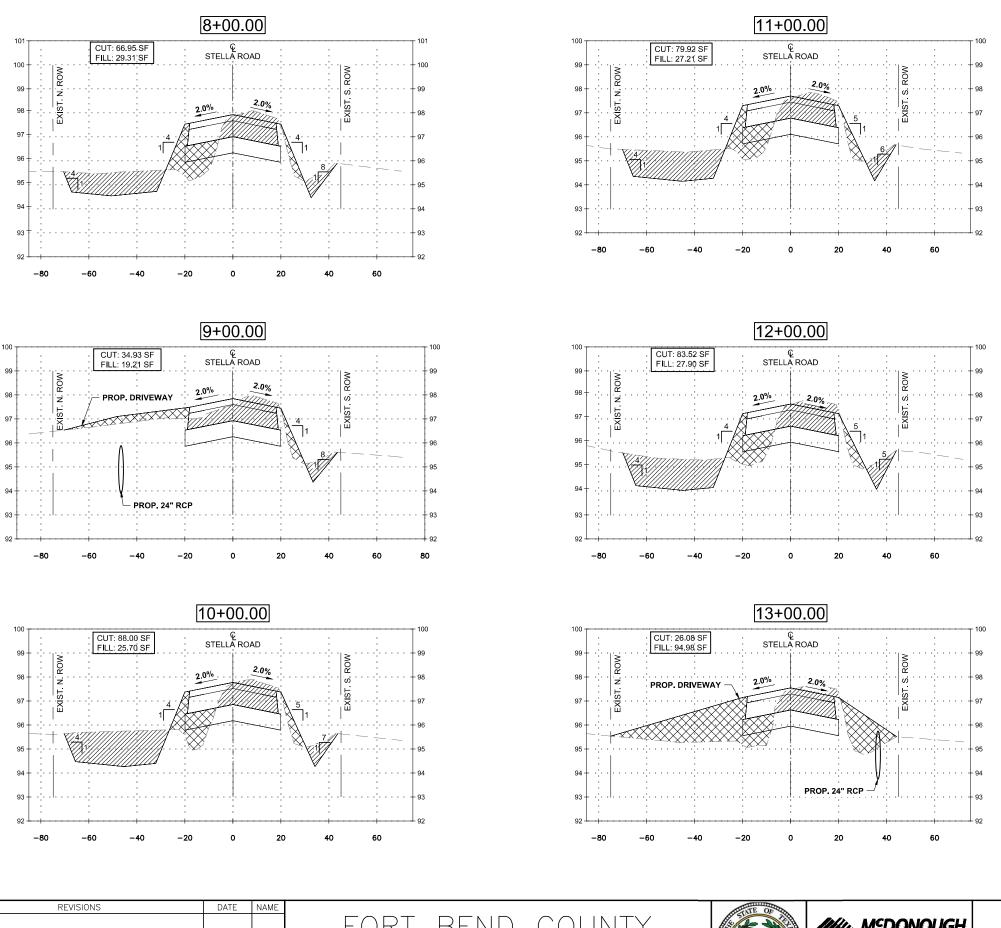
FILL

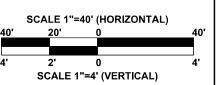


CUT

NOTES

- ALL EXISTING UTILITIES ARE SHOWN
 APPROXIMATELY IN BOTH VERTICAL AND
 HORIZONTAL LOCATION. CONTRACTOR IS
 RESPONSIBLE FOR FIELD VERIFYING ALL
 UTILITIES PRIOR TO CONSTRUCTION.
- 2) REGRADE FILLED DITCHES AND DEMOLISHED ROAD AREA TOWARDS STELLA ROAD PROP ROADSIDE SWALES







FILL



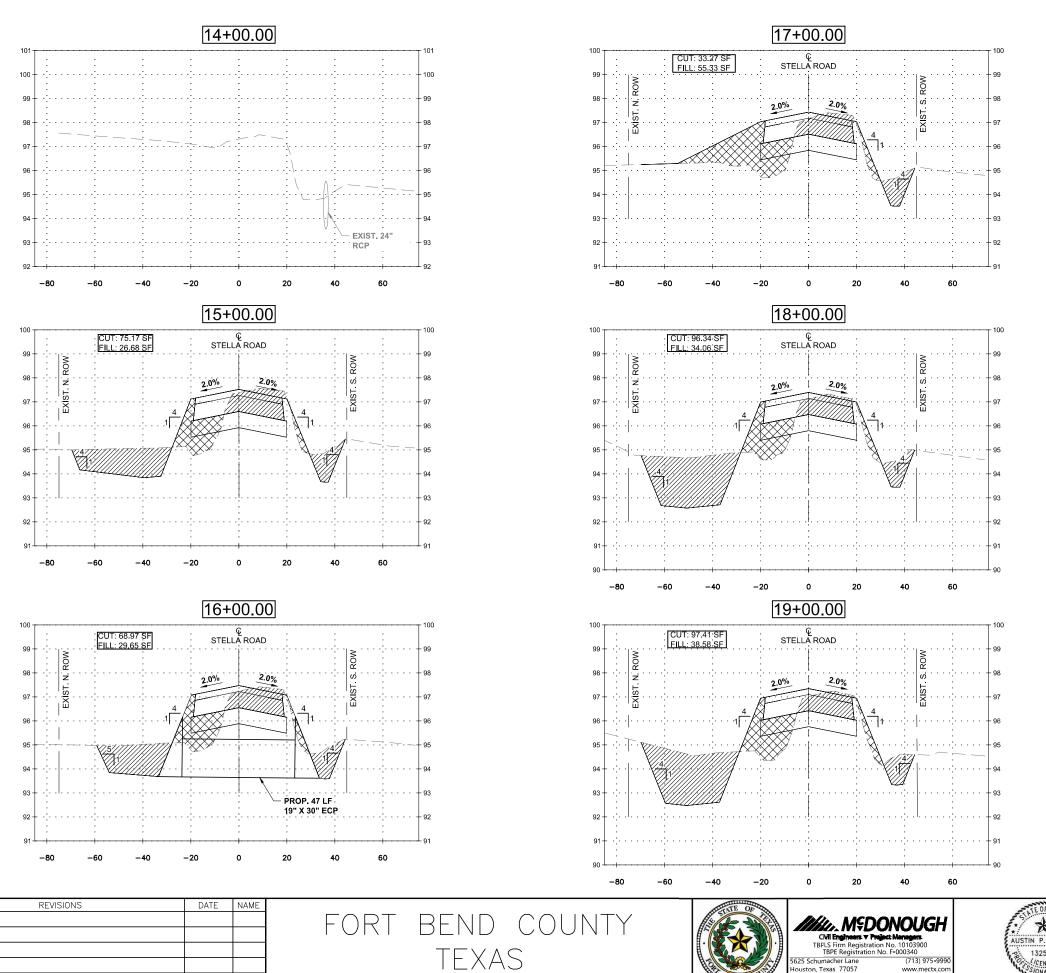
CUT

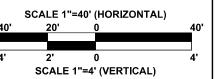






PROJECT TITL	E: STELLA ROAD		
DRAWN BY: GB	COTTONWOOD SCHOOL DD TOW EXIDEDOLINDS DD I		
CK'D BY:	SHEET DESCRIPTION: CROSS SECTIONS		
SCALE: 1" = 40'	8+00 TO 13+00	SHEET NO:	
DATE: 1/16/2023	APPROVED BY:	126 / 133	





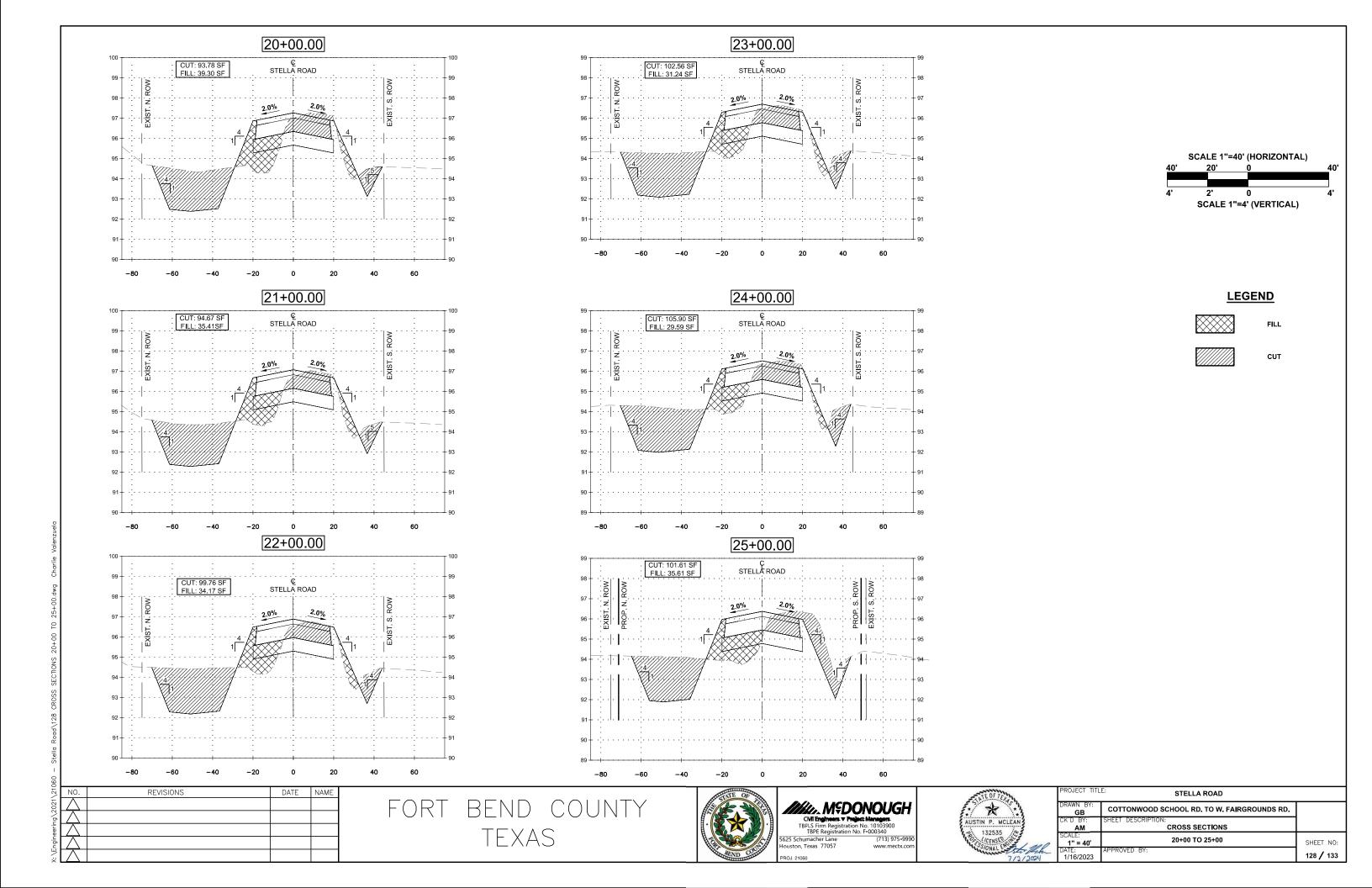


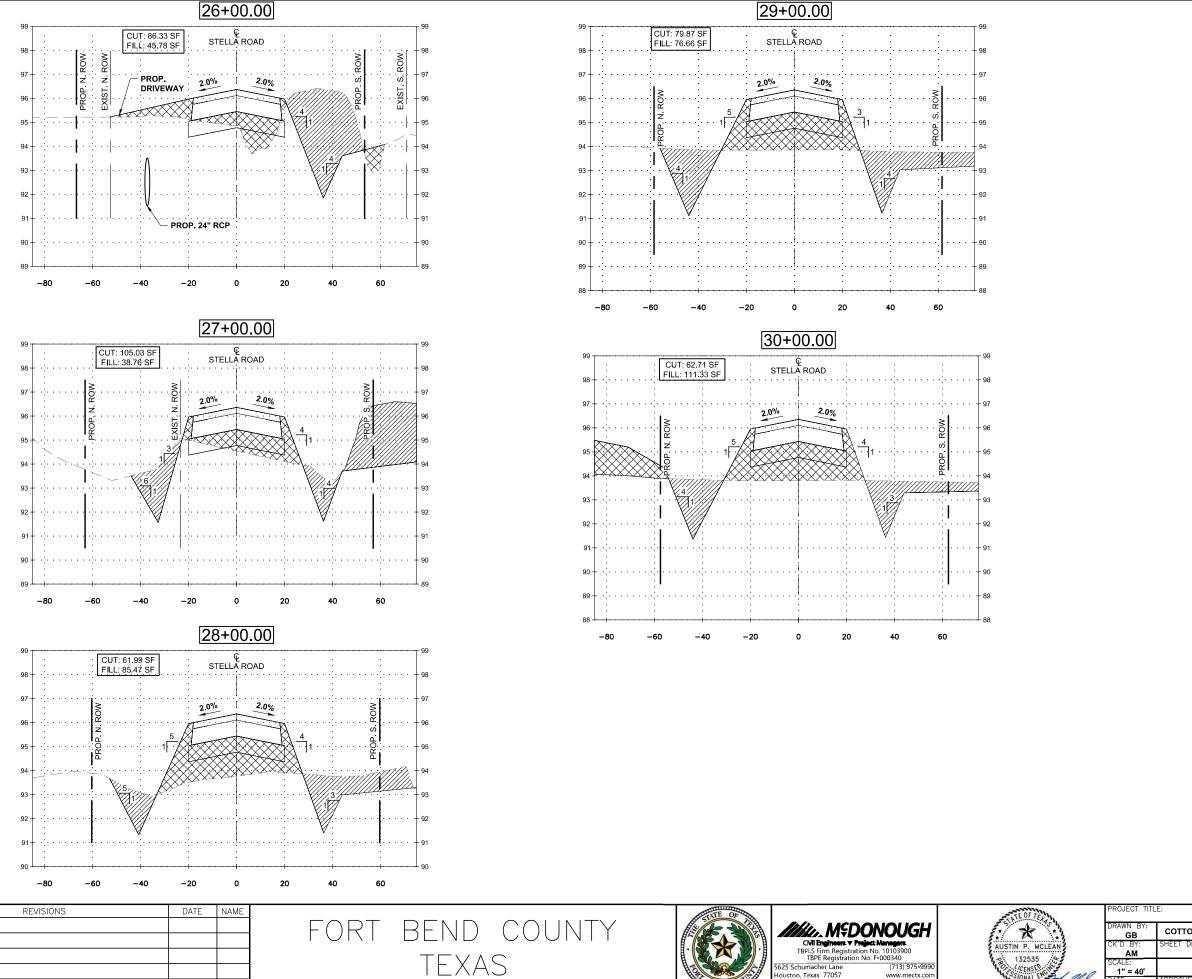
FILL



SHEET NO: 127 / 133

TE OF TEX	PROJECT TITL	E: STELLA ROAD
* 1	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS F
N P. MCLEAN	CK'D BY:	SHEET DESCRIPTION: CROSS SECTIONS
132535 (CENSCO	SCALE: 1" = 40'	14+00 TO 19+00
7/2/2024	DATE: 1/16/2023	APPROVED BY:









PROJECT TITL				
ORAWN BY: COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.				
CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS			
SCALE: 1" = 40'	26+00 TO 31+00	SHEET NO:		
DATE: 1/16/2023	APPROVED BY:	129 / 133		

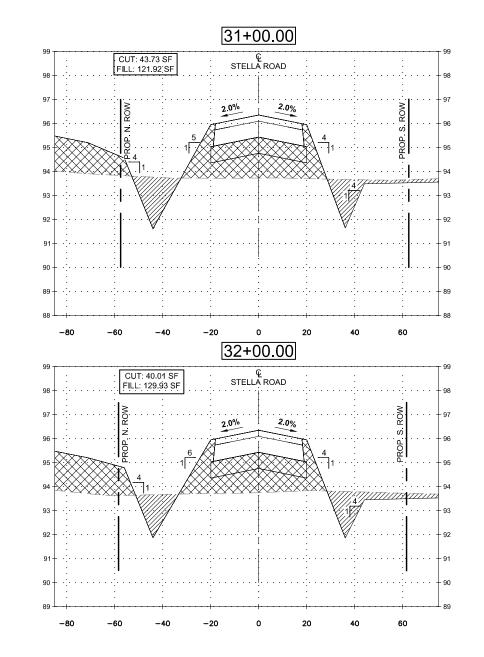
SCALE 1"=40' (HORIZONTAL)

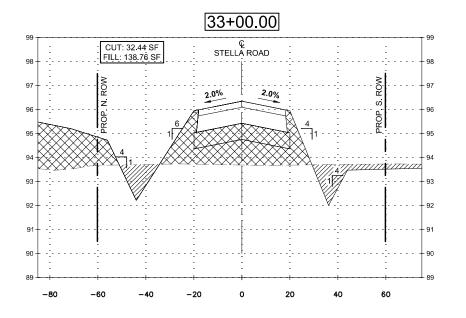
SCALE 1"=4' (VERTICAL)

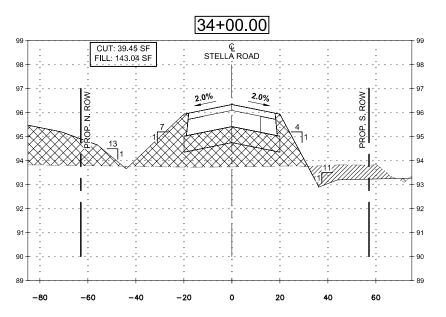
LEGEND

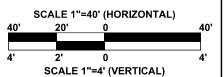
FILL

CUT











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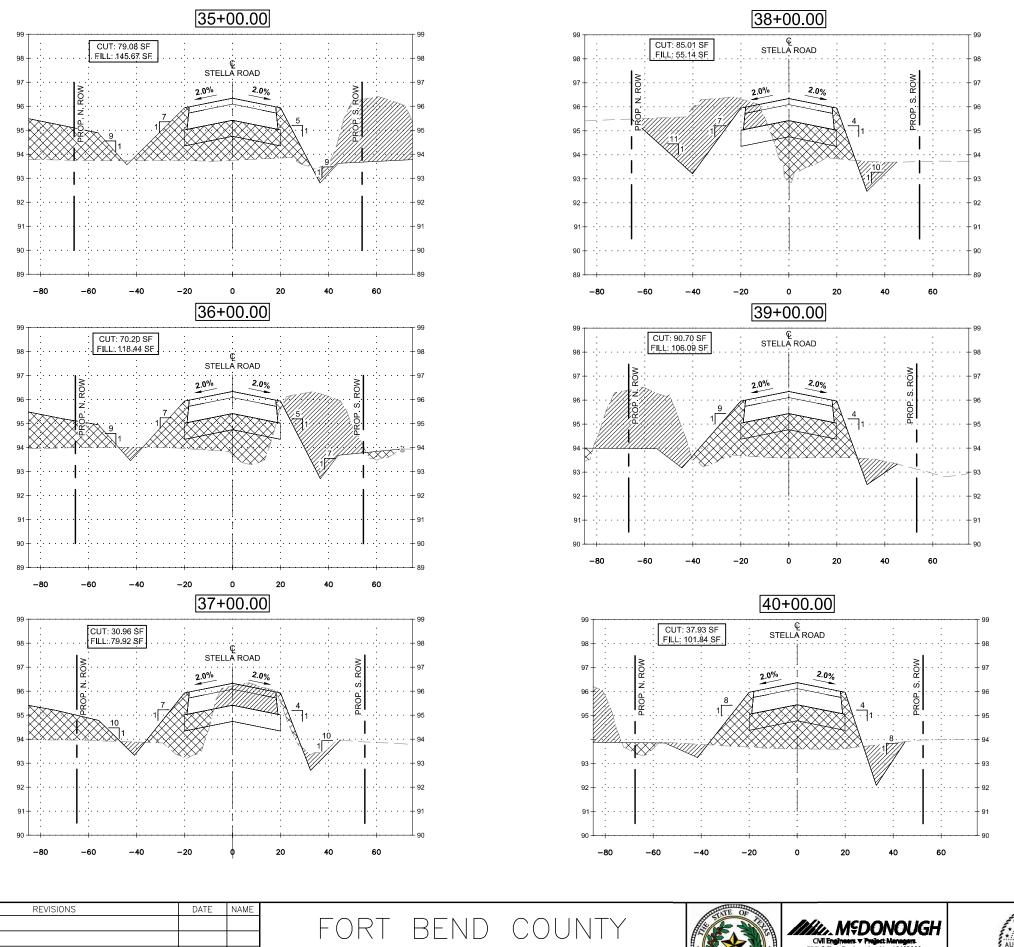
NO. REVISIONS DATE NAME

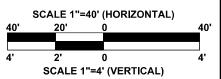






PROJECT TITL		
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY:	SHEET DESCRIPTION: CROSS SECTIONS	
SCALE: 1" = 40'	32+00 TO 35+00	SHEET NO:
DATE: 1/16/2023	APPROVED BY:	130 / 133







FILL



CUT

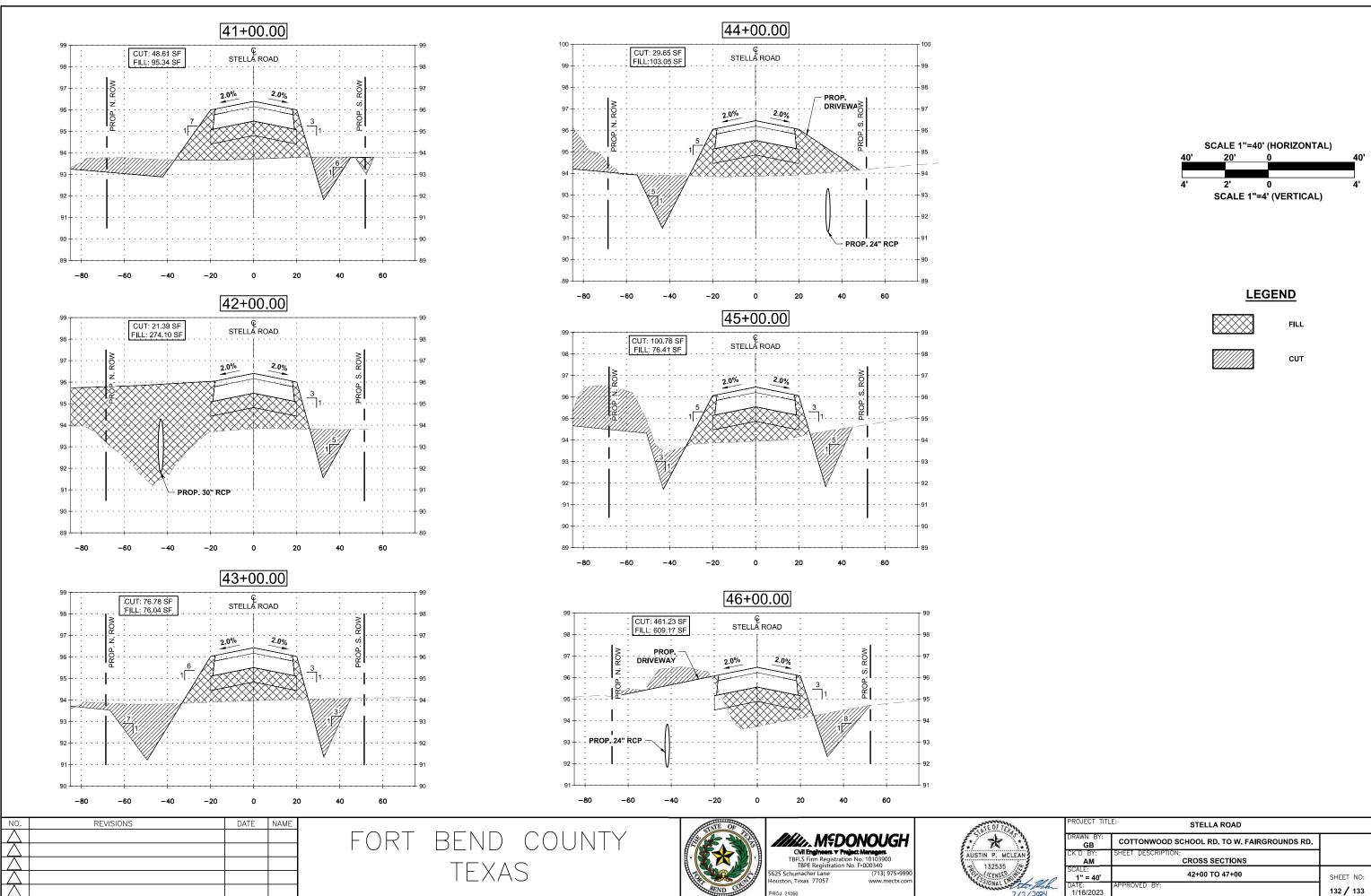
TEXAS



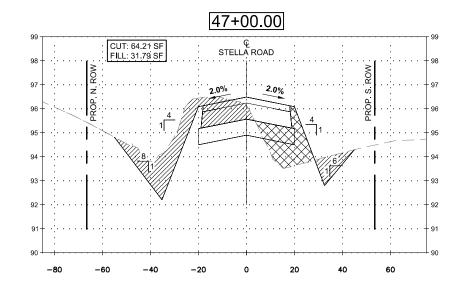


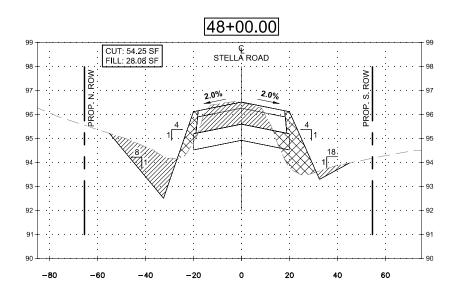


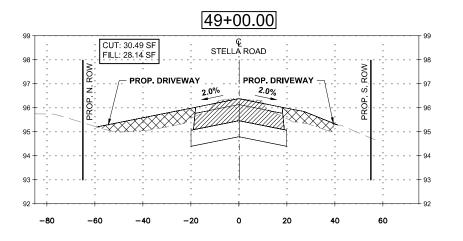
ROJECT TITL		
RAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
K'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
CALE: 1" = 40'	36+00 TO 41+00	SHEET NO:
ATE: 1/16/2023	APPROVED BY:	131 / 133

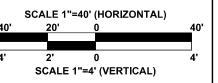


021\21060 — Stella Road\132 CROSS SECTIONS 42+00 TO 47+00.dwg Charlie Valen:











FILL



CUT

NO.	REVISIONS	DATE	NAME	
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$\overline{\Lambda}$				







PROJECT TITL	E: STELLA ROAD	
DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY:	SHEET DESCRIPTION: CROSS SECTIONS	
SCALE: 1" = 40'	47+00 TO 49+00	SHEET NO:
DATE: 1/16/2023	APPROVED BY:	133 / 133