Fort Bend County, Texas Invitation for Bid



Construction of Stella Road from Cottonwood School Road to W. Fairgrounds Road for Fort Bend County Mobility Bond Project No. 20116 BID 24-076

SUBMIT BIDS TO:

Fort Bend County Purchasing Department Travis Annex 301 Jackson, Suite 201 Richmond, TX 77469

Note: All correspondence must include the term "Purchasing Department" in address to assist in proper delivery

SUBMIT NO LATER THAN:

Tuesday, August 20, 2024 2:00 PM (Central)

LABEL ENVELOPE:

BID 24-076 Construction of Stella Road

ALL BIDS MUST BE RECEIVED IN AND TIME/DATE STAMPED BY THE PURCHASING OFFICE OF FORT BEND COUNTY ON OR BEFORE THE SPECIFIED TIME/DATE STATED ABOVE.

BIDS RECEIVED AS REQUIRED WILL THEN BE OPENED AND PUBLICLY READ.

BIDS RECEIVED AFTER THE SPECIFIED TIME, WILL BE RETURNED UNOPENED.

Results will not be given by phone. Results will be provided to bidder in writing after Commissioners Court award. Requests for information must be in writing and directed to: LeAnn Cernoch Senior Buyer LeAnn.Cernoch@fortbendcountytx.gov

Vendor Responsibilities:

- Download and complete any addendums. (Addendums will be posted on the Fort Bend County website no Later than 48 hours prior to bid opening)
- Submit response in accordance with requirements stated on the cover of this document.
- > DO NOT submit responses via email or fax.



Jaime Kovar

COUNTY PURCHASING AGENT Fort Bend County, Texas

Vendor Information

Purchasing Agent				Office (28	1) 341-8640
Legal Company Name					
(top line of W9)					
Business Name					
(if different from legal name)	<u>)</u>				
	Corporation/LLC		Partnership	Age in Bu	usiness?
Type of Business	Sole Proprietor/Ind	lividual	Tax Exempt		
Federal ID # or S.S. #			SAM.gov Unique Entity ID #		
SAM.gov					
CAGE / NCAGE					
Publicly Traded Business	NoYe	s Ticker Sy	mbol		
Remittance Address					
City/State/Zip					
Physical Address					
City/State/Zip					
Phone Number					
E-mail					
Contact Person					
Check all that apply to the company listed above and provide certification number.	DBE-Disadvantaged Business Enterp SBE-Small Business Enterprise HUB-Texas Historically Underutilize WBE-Women's Business Enterprise _	ed Business	Certification # Certification # Certification # Certification #	_	Exp Date
Company's gross annual	<\$500,000	\$500,000)-\$4,999,999	·	
receipts	\$5,000,000-\$16,999,999	\$17,000,0	000-\$22,399,999	>\$22,400,0	00
NAICs codes (Please enter all that apply)					
Signature of Authorized Representative					
Printed Name					
Title					
Date					

THIS FORM MUST BE SUBMITTED WITH THE SOLICITATION RESPONSE

1.0 GENERAL REQUIREMENTS:

- 1.1 Read this entire document carefully. Follow all instructions. You are responsible for fulfilling all requirements and specifications. Be sure you understand them.
- 1.2 General Requirements apply to all advertised bids; however, these may be superseded, whole or in part, by the scope, special requirements, specifications, special specifications or other data contained herein.
- 1.3 Governing Law: Bidder is advised that these requirements shall be fully governed by the laws of the State of Texas and that Fort Bend County may request and rely on advice, decisions and opinions of the Attorney General of Texas and the County Attorney concerning any portion of these requirements.
- 1.4 Bid Form Completion: Fill out, sign, and return to the Fort Bend County Purchasing Department one (1) complete bid form. An authorized representative of the bidder must sign the Contract Sheet. The Contract will be binding only when signed by the County Judge, Fort Bend County and a purchase order authorizing the item(s) desired has been issued. The use of corrective fluid is not acceptable and may result in the disqualification of bid. If an error is made, the bidder must draw a line through error and initial each change.
- 1.5 Bid Returns: Bidders must return all completed bids to the Fort Bend County Purchasing Department at 301 Jackson, Suite 201 Richmond Texas no later than 2:00 P.M. on the date specified. <u>Late bids will not be accepted</u>. Bids must be submitted in a sealed envelope, addressed as follows: Fort Bend County Purchasing Agent, Travis Annex, 301 Jackson, Suite 201 Richmond, Texas 77469.
- Addenda: No interpretation of the meaning of the drawings, specifications or 1.6 other bid documents will be made to any bidder orally. All requests for such interpretations must be made in writing addressed to LeAnn Cernoch, Senior Buyer, 301, Jackson, Suite 201, Richmond, Texas, 77469, E-mail: LeAnn.Cernoch@fortbendcountytx.gov. Any and all interpretations and any supplemental instructions will be in the form of written addenda to the contract documents which will be posted on Fort Bend County's website. Addenda will ONLY be issued by the Fort Bend County Purchasing Agent. It is the sole responsibility of each bidder to insure receipt of any and all addenda. All addenda issued will become part of the contract documents. Bidders must sign and include it in the returned bid package. Deadline for submission of questions and/or clarification is no later than Tuesday, August 13, 2024 at 9:30AM (central) Requests received after the deadline will not be responded to due to the time constraints of this bid process.
- 1.7 References: All bidders must submit, **WITH BID**, at least three (3) references from clients for whom a project similar to that specified herein has been

successfully accomplished. References must include clients name, contact person and telephone number.

- 1.8 Bid Bond: All bidders must submit, **WITH BID**, a cashier's check or certified check for at least five percent (5%) of the total bid price, payable to the order of Fort Bend County, or a Bid Bond in the same amount issued by a surety, acceptable to Fort Bend County, authorized to do business in the State of Texas, as a guarantee that the Bidder will do the work described herein at the rates stated herein. Unsuccessful bidder's Cashier's Check or Certified Check will be returned only after a written request to do so have been received in the Office of the Fort Bend County Purchasing Agent.
- 1.9 Material Safety Data Sheets: Under the "Hazardous Communication Act", commonly known as the "Texas Right to Know Act", a bidder must provide to Fort Bend County and using departments, with each delivery, material safety data sheets, which are, applicable to hazardous substances defined in the Act. Bidders are obligated to maintain a current, updated file in the Fort Bend County Purchasing Department. Failure of the bidder to maintain such a file will be cause to reject any bid applying thereto.
- 1.10 Pricing: Prices for all goods and/or services shall be firm for the duration of this Contract and shall be stated on the bid sheet. Prices shall be all inclusive. No price changes, additions, or subsequent qualifications will be honored during the course of the Contract. All prices must be written in ink or typewritten. If there are any additional charges of any kind, other than those mentioned above, specified or unspecified, bidder MUST indicate the items required and attendant costs or forfeit the right to payment for such items.
- 1.11 Term Contracts: If the Contract is intended to cover a specific time period, said time will be given in the specifications under scope.
- 1.12 Recycled Materials: Fort Bend County encourages the use of products made of recycled materials and shall give preference in purchasing to products made of recycled materials if the products meet applicable specifications as to quantity and quality. Fort Bend County will be the sole judge in determining product preference application.
- 1.13 Evaluation: Evaluation shall be used as a determinant as to which bid items or services are the most efficient and/or most economical for Fort Bend County. It shall be based on all factors which have a bearing on price and performance of the items in the user environment. All bids are subject to tabulation by the Fort Bend County Purchasing Department and recommendation to Fort Bend County Commissioners Court. Compliance with all bid requirements, delivery and needs of the using department are considerations in evaluating bids. Pricing is NOT the only criteria for making a recommendation. The Fort Bend County Purchasing Department reserves the right to contact any bidder, at any time, to clarify, verify or request information with regard to any bid.

- 1.14 Disqualification of Bidder: Upon signing this bid document, a bidder offering to sell supplies, materials, services, or equipment to Fort Bend County certifies that the bidder has not violated the antitrust laws of this state codified in section 15.01, et seq., Business & Commerce Code, or the federal antitrust laws, and has not communicated directly or indirectly the bid made to any competitor or any other person engaged in such line of business. Any or all bids may be rejected if Fort Bend County believes that collusion exists among the bidders. Bids in which the prices are obviously unbalanced may be rejected. If multiple bids are submitted by a bidder and after the bids are opened, one of the bids is withdrawn, the result will be that all of the bids submitted by that bidder will be withdrawn; however, nothing herein prohibits a vendor from submitting multiple bids for different products or services.
- 1.15 Awards: Fort Bend County reserves the right to award this Contract on the basis of lowest and best bid in accordance with the laws of the State of Texas, to waive any formality or irregularity, to make awards to more than one bidder, to reject any or all bids. In the event the lowest dollar bidder meeting specifications is not awarded a contract, the bidder may appear before the Commissioners Court and present evidence concerning its responsibility.
- 1.16 Contract Obligation: Fort Bend County Commissioners Court must award the Contract and the County Judge or other person authorized by the Fort Bend County Commissioners Court must sign the Contract before it becomes binding on Fort Bend County or the bidders. Department heads are not authorized to sign agreements for Fort Bend County. Binding agreements shall remain in effect until all products and/or services covered by this purchase have been satisfactorily delivered and accepted.

2.0 SCOPE:

It is the intent of Fort Bend County to contract with one (1) vendor for all materials, supplies, equipment, tools, services, labor and supervision necessary to complete the Construction of Stella Road from Cottonwood School Road to W. Fairgrounds Road, hereinafter referred to as the "Project," as specified herein.

2.1 *Work* means the procurement, delivery and proper construction and/or installation of all materials and facilities and associated appurtenances necessary to fulfill the winning bidder's obligations (hereinafter the "Contractor") under the Contract as awarded for the Project specified herein, including the coordination and administration of all services necessary for Contractor, and/or its agents and/or subcontractors, to fulfill Contractor's obligations under the Contract.

3.0 PRE-BID CONFERENCE:

A pre-bid conference will be conducted on Tuesday, August 6, 2024 at 9:30 AM (CST). The

pre-bid conference will be held at the Fort Bend County Purchasing Department located in the Travis Annex at 301 Jackson, Suite 201, Richmond, Texas 77469. All bidders are encouraged to attend.

4.0 LIQUIDATED DAMAGES:

The County and the Contractor recognize the delays, expense, and difficulties involved in proving in a legal or arbitration proceeding the actual loss suffered by the County if the work is not complete on time. Accordingly, instead of requiring any such proof, the County and the Contractor agree that as liquidated damages for delay (but not as a penalty) the Contractor shall pay the County \$1,500.00 for each day that expires after the time specified herein for completion until the Work is complete, unless contract time has been adjusted by extension of time approved by Commissioner's Court.

The Contractor will be placed on one (1) year probation if liquidated damages are accrued. During the probation period, if the Contractor accrues liquidated damages on another project, they will be disqualified from being awarded any County work for two (2) years.

5.0 COMPLETION TIME & PAYMENT:

- 5.1 Fort Bend County shall pay the Contractor in current funds for the Contractor's performance of the Contract the contract sum, as stated herein, after receipt of notice to proceed and a purchase order issued by the Fort Bend County Purchasing Agent.
- 5.2 Based upon Applications for payment submitted to the County Auditor, Fort Bend County shall make progress payments on account of the contract sum to the Contractor as provided below and elsewhere in the contract documents.
 - 5.2.1 The period covered by each application for payment shall be one calendar month ending on the last day of the month.
 - 5.2.2 Provided a customary, accurate and complete application for payment is received by the County Auditor not later than the 15th day of a month, Fort Bend County shall make payment of all undisputed amounts to the Contractor not later than the 15th day of the next month. If an application for payment is received by the County Auditor after the application deadline fixed above, payment shall be made by Fort Bend County not later than 30 days after the County Auditor receives the application for payment.
 - 5.2.3 Application for payment shall indicate the percentage of completion of each portion of the Project as of the end of the period covered by the application for payment.
 - 5.2.4 Subject to the provisions of the contract documents, the amount of each progress payment shall be computed as follows:

- 5.2.4.1 Take that portion of the contract sum properly allocable to completed Project less retainage of ten percent (10%).
- 5.2.4.2 Add that portion of the contract sum properly allocable to materials and equipment delivered and suitably stored at the site for subsequent incorporation in the completed construction (or, if approved by Fort Bend County, suitably stored off the site at a location agreed upon in writing), less retainage of ten percent (10%).
- 5.2.4.3 Subtract the aggregate of previous payments made by Fort Bend County.
- 5.2.4.4 The progress payment amount as determined in above shall be further modified under the following circumstances:

Upon substantial completion of the Project, add a sum sufficient to increase the total payments to one hundred percent (100%) of the contract sum, less such amounts as Fort Bend County shall determine should be deducted for incomplete work and unsettled claims.

- 5.2.4.5 Final payment, constituting the entire unpaid undisputed balance of the contract sum, shall be made by Fort Bend County to the Contractor when Fort Bend County and the Contractor agree that the Contract has been fully performed by the Contractor.
- 5.3 Before the first application for payment, the Contractor shall submit to the Engineering Department a schedule of values allocated to various portions of the work, prepared in such form and supported by such data to substantiate its accuracy as the Engineering Department may require. This schedule, unless objected to by the Engineering Department shall be used as a basis for reviewing the Contractor's application for payment.
- 5.4 Contractor must provide with each application for payment a contractor's affidavit certifying bills against the Contractor for labor, material and expendable equipment employed in the performance of Contractor have been paid in full prior to acceptance of final payment from Fort Bend County.
- 5.5 The Contractor will permit Fort Bend County, or any duly authorized agent of Fort Bend County, to inspect and examine the books and records of the Contractor for the purpose of verifying the amount of work performed under the Contract. Fort Bend County's right to inspect survives the termination of the Contract for a period of five years.

6.0 LIMIT OF APPROPRIATION:

Prior to the execution of this Contract, Contractor has been advised by County, and Contractor clearly understands and agrees, such understanding and agreement being of the absolute essence to this Contract, that County shall have available only those funds specifically allocated in this Contract to fully discharge any and all liabilities which may be incurred by County in bringing this Project to an absolute conclusion, resulting in a complete, fully furnished, fully equipped and fully usable facility, and that the total of any and all basic construction costs, costs of providing the required services and materials, all fees and compensation of any sort to the Contract, and any and all costs for any and all things or purposes coming inuring under or out of this Contract, irrespective of the nature thereof, shall not exceed said specifically allocated sum, notwithstanding any word, statement or thing contained in or inferred from the preceding provision of this Contract which might in any light by any person be interpreted to the contrary.

7.0 **RIGHT TO ASSURANCE:**

Whenever Fort Bend County in good faith has reason to question the Contractor's intent or ability to perform, Fort Bend County may demand that the Contractor give written assurance of its intent to perform and its plan to properly continue performance, including a reasonably detailed timeline. In the event that a demand is made and no assurance is given within five (5) business days, Fort Bend County may treat this failure as an anticipatory repudiation of the Contract.

8.0 PERFORMANCE & PAYMENT BONDS:

Performance and Payment Bonds: In the event the total accepted bid price exceeds \$25,000 the Contractor must provide to the Office of the County Purchasing Agent, a performance bond and a payment bond, each in the amount of 100% of the total contract sum within ten (10) calendar days after receipt of notification of bid award. Such bonds shall be executed by a corporate surety duly authorized and admitted to do business in the State of Texas and licensed in the State of Texas to issue surety bonds with a Best Rating of "A" or better. Fort Bend County reserves the right to accept or reject any surety company proposed by the Contractor. In the event Fort Bend County rejects, the proposed surety company, the Contractor will be afforded five (5) additional days to submit the required bonds issued by a surety company acceptable to Fort Bend County.

9.0 **POWER OF ATTORNEY:**

An attorney-in-fact who signs a bid bond, performance bond or payment bond must file with each bond a certified and effectively dated copy of his or her power of attorney.

10.0 INSURANCE:

10.1 All respondents shall submit, with response, a <u>current</u> certificate of insurance indicating coverage in the amounts stated below. In lieu of submitting a certificate of insurance, respondents may submit, with response, a notarized

statement from an Insurance company, authorized to conduct business in the State of Texas, and acceptable to Fort Bend County, guaranteeing the issuance of an insurance policy, with the coverage stated below, to the firm named therein, if successful, upon award of this Contract.

- 10.2 At contract execution, contractor shall furnish County with properly executed certificates of insurance which shall evidence all insurance required and provide that such insurance shall not be canceled, except on 30 days prior written notice to County. Contractor shall provide certified copies of insurance endorsements and/or policies if requested by County. Contractor shall maintain such insurance coverage from the time Services commence until Services are completed and provide replacement certificates, policies and/or endorsements for any such insurance expiring prior to completion of Services. Contractor shall obtain such insurance written on an Occurrence form (or a Claims Made form for Professional Liability insurance) from such companies having Best's rating of A/VII or better, licensed or approved to transact business in the State of Texas, and shall obtain such insurance of the following types and minimum limits:
 - 10.2.1 Workers' Compensation insurance. Substitutes to genuine Workers' Compensation Insurance will not be allowed.
 - 10.2.2 Employers' Liability insurance with limits of not less than \$1,000,000 per injury by accident, \$1,000,000 per injury by disease, and \$1,000,000 per bodily injury by disease.
 - 10.2.3 Commercial general liability insurance with a limit of not less than \$1,000,000 each occurrence and \$2,000,000 in the annual aggregate. Policy shall cover liability for bodily injury, personal injury, and property damage and products/completed operations arising out of the business operations of the policyholder.
 - 10.2.4 Business Automobile Liability coverage with a combined Bodily Injury/Property Damage limit of not less than \$1,000,000 each accident. The policy shall cover liability arising from the operation of licensed vehicles by policyholder.
- 10.3 County and the members of Commissioners Court shall be named as additional insured to all required coverage except for Workers' Compensation and Professional Liability (if required). All Liability policies including Workers' Compensation written on behalf of contractor, excluding Professional Liability, shall contain a waiver of subrogation in favor of County and members of Commissioners Court.
- 10.4 If required coverage is written on a claims-made basis, contractor warrants that any retroactive date applicable to coverage under the policy precedes the effective date of the contract; and that continuous coverage will be maintained or an

extended discovery period will be exercised for a period of two (2) years beginning from the time that work under the agreement is completed.

- 10.5 Contractor shall not commence any portion of the work under this Contract until it has obtained the insurance required herein and certificates of such insurance have been filed with and approved by Fort Bend County.
- 10.6 No cancellation of or changes to the certificates, or the policies, may be made without sixty (60) days prior, written notification to Fort Bend County.
- 10.7 Approval of the insurance by Fort Bend County shall not relieve or decrease the liability of the Contractor.

11.0 INDEMNIFICATION:

Respondent shall save harmless County from and against all claims, liability, and expenses, including reasonable attorney's fees, arising from activities of respondent, its agents, servants or employees, performed under this agreement that result from the negligent act, error, or omission of respondent or any of respondent's agents, servants or employees.

- 11.1 Respondent shall timely report all such matters to Fort Bend County and shall, upon the receipt of any such claim, demand, suit, action, proceeding, lien or judgment, not later than the fifteenth day of each month; provide Fort Bend County with a written report on each such matter, setting forth the status of each matter, the schedule or planned proceedings with respect to each matter and the cooperation or assistance, if any, of Fort Bend County required by Respondent in the defense of each matter.
- 11.2 Respondent's duty to defend, indemnify and hold Fort Bend County harmless shall be absolute. It shall not abate or end by reason of the expiration or termination of any contract unless otherwise agreed by Fort Bend County in writing. The provisions of this section shall survive the termination of the contract and shall remain in full force and effect with respect to all such matters no matter when they arise.
- 11.3 In the event of any dispute between the parties as to whether a claim, demand, suit, action, proceeding, lien or judgment appears to have been caused by or appears to have arisen out of or in connection with acts or omissions of Respondent, Respondent shall never-the-less fully defend such claim, demand, suit, action, proceeding, lien or judgment until and unless there is a determination by a court of competent jurisdiction that the acts and omissions of Respondent are not at issue in the matter.
- 11.4 Respondent's indemnification shall cover, and Respondent agrees to indemnify Fort Bend County, in the event Fort Bend County is found to have been negligent for having selected Respondent to perform the work described in this request.

- 11.5 The provision by Respondent of insurance shall not limit the liability of Respondent under an agreement.
- 11.6 Respondent shall cause all trade contractors and any other contractor who may have a contract to perform construction or installation work in the area where work will be performed under this request, to agree to indemnify Fort Bend County and to hold it harmless from all claims for bodily injury and property damage that may arise from said Respondent's operations. Such provisions shall be in form satisfactory to Fort Bend County.
- 11.7 Loss Deduction Clause Fort Bend County shall be exempt from, and in no way liable for, any sums of money which may represent a deductible in any insurance policy. The payment of deductibles shall be the sole responsibility of Respondent and/or trade contractor providing such insurance.

12.0 PREVAILING WAGES:

This project is subject to the prevailing wage rate requirements of Chapter 2258 of the Government Code. All persons employed by Contractor shall be compensated at not less than the rates shown below. Contractor shall keep detailed records of each of its workers and said records shall be made available to County for inspection at all reasonable times. The Contractor shall pay Fort Bend County sixty dollars (\$60.00) for each worker employed by the Contractor for the provision of services described herein for each calendar day or part of the day that the worker is paid less than the below stated rates. Contractors may also visit www.wdol.gov/dba.aspx.

General Decision Number: TX20240038 01/05/2024 Superseded General Decision Number: TX20230038

State: Texas Construction Type: Highway

Counties: Austin, Brazoria, Chambers, Fort Bend, Galveston, Hardin, Harris, Jefferson, Liberty, Montgomery, Orange, San Jacinto and Waller Counties in Texas.

HIGHWAY CONSTRUCTION PROJECTS (excluding tunnels, building structures in rest area projects & railroad construction; bascule, suspension & spandrel arch bridges designed for commercial navigation, bridges involving marine construction; and other major bridges).

Note: Contracts subject to the Davis-Bacon Act are generally required to pay at least the applicable minimum wage rate required under Executive Order 14026 or Executive Order 13658. Please note that these Executive Orders apply to covered contracts entered into by the federal government that are subject to the Davis-Bacon Act itself, but do not apply to contracts subject only to the Davis-Bacon Related Acts, including those set forth at 29 CFR 5.1(a)(2)-(60).

If the contract is entered into on or after January 30, 2022, or the contract is renewed or extended (e.g., an option is exercised) on or after January 30, 2022, Executive Order 14026 generally

applies to the contract. The contractor must pay all covered workers at least \$17.20 per hour (or the applicable wage rate listed on this wage determination, if it is higher) for all hours spent performing on that contract in 2024.

If the contract was awarded on or between January 1, 2015 and January 29, 2022, and the contract is not renewed or extended on or after January 30, 2022, Executive Order 13658 generally applies to the contract. The contractor must pay all covered workers at least \$12.90 per hour (or the applicable wage rate listed on this wage determination, if it is higher) for all hours spent performing on that contract in 2024.

The applicable Executive Order minimum wage rate will be adjusted annually. If this contract is covered by one of the Executive Orders and a classification considered necessary for performance of work on the contract does not appear on this wage determination, the contractor must still submit a conformance request.

Additional information on contractor requirements and worker protections under the Executive Orders is available at <u>www.dol.gov/whd/govcontracts</u>.

Modification NumberPublication Date001/05/2024

SUTX2011-013 08/10/2011

	Rates	Fringes
CEMENT MASON/CONCRETE FINISHER (Paving and Structures)	\$ 12.98 **	<
ELECTRICIAN	\$ 27.11	
FORM BUILDER/FORM SETTER	+	
Paving & Curb Structures	\$ 12.34 ** \$ 12.23 **	
	ψ 12.25	
LABORER Asphalt Raker	\$ 12.36 **	
Flagger Laborer, Common	\$ 10.33 ** \$ 11.02 **	
Laborer, Utility Pipelayer	\$ 11.73 ** \$ 12.12 **	
Work Zone Barricade Servicer	\$ 11.67 **	<
PAINTER (Structures)	\$ 18.62	

POWER EQUIPMENT OPERATOR:

Asphalt Distributor	\$ 14.06 **
Asphalt Paving Machine	\$ 14.32 **
Broom or Sweeper	\$ 12.68 **
Concrete Pavement Finishing Machine	\$ 13.07 **
Concrete Paving, Curing, Float, Texturing Machine	\$ 11.71 **
Concrete Saw	\$ 13.99 **
Crane, Hydraulic 80 Tons or less	\$ 13.86 **
Crane, Lattice boom 80 tons or less	\$ 14.97 **
Crane, Lattice boom over 80 Tons	\$ 15.80 **
Crawler Tractor	\$ 13.68 **
Excavator, 50,000 pounds or less	\$ 12.71 **
Excavator, Over 50,000 pounds	\$ 14.53 **
Foundation Drill, Crawler Mounted	\$ 17.43
Foundation Drill, Truck Mounted	\$ 15.89 **
Front End Loader 3 CY or Less	\$ 13.32 **
Front End Loader, Over 3 CY	\$ 13.17 **
Loader/Backhoe	\$ 14.29 **
Mechanic	\$ 16.96 **
Milling Machine	\$ 13.53 **
Motor Grader, Fine Grade	\$ 15.69 **
Motor Grader, Rough	\$ 14.23 **
Off Road Hauler	\$ 14.60 **
Pavement Marking Machine	\$ 11.18 **
Piledriver	\$ 14.95 **
Roller, Asphalt	\$ 11.95 **
Roller, Other	\$ 11.57 **
Scraper	\$ 13.47 **
Spreader Box	\$ 13.58 **
	+
Servicer	\$ 13.97 **
	+
Steel Worker	
Reinforcing Steel	\$ 15.15 **
Structural Steel Welder	\$ 12.85 **
Structural Steel	\$ 14.39 **
	φ 1 neγ
TRUCK DRIVER	
Low Boy Float	\$ 16.03 **
Single Axle	\$ 11.46 **
Single or Tandem Axle Dump	\$ 11.48 **
Tandem Axle Tractor w/Semi Trailer	\$ 12.27 **
	ψ 12.27

WELDERS - Receive rate prescribed for craft performing operation to which welding is incidental.

** Workers in this classification may be entitled to a higher minimum wage under Executive Order 14026 (\$17.20) or 13658 (\$12.90). Please see the Note at the top of the wage determination for more information. Please also note that the minimum wage requirements of Executive Order 14026 are not currently being enforced as to any contract or subcontract to which the states of Texas, Louisiana, or Mississippi, including their agencies, are a party.

Note: Executive Order (EO) 13706, Establishing Paid Sick Leave for Federal Contractors applies to all contracts subject to the Davis-Bacon Act for which the contract is awarded (and any solicitation was issued) on or after January 1, 2017. If this contract is covered by the EO, the contractor must provide employees with 1 hour of paid sick leave for every 30 hours they work, up to 56 hours of paid sick leave each year. Employees must be permitted to use paid sick leave for their own illness, injury or other health-related needs, including preventive care; to assist a family member (or person who is like family to the employee) who is ill, injured, or has other health-related needs, including preventive care; or for reasons resulting from, or to assist a family member (or person who is like family to the employee) who is a victim of, domestic violence, sexual assault, or stalking. Additional information on contractor requirements and worker protections under the EO is available at www.dol.gov/whd/govcontracts.

Unlisted classifications needed for work not included within the scope of the classifications listed may be added after award only as provided in the labor standards contract clauses (29CFR 5.5 (a) (1) (iii)).

The body of each wage determination lists the classification and wage rates that have been found to be prevailing for the cited type(s) of construction in the area covered by the wage determination. The classifications are listed in alphabetical order of "identifiers" that indicate whether the particular rate is a union rate (current union negotiated rate for local), a survey rate (weighted average rate) or a union average rate (weighted union average rate).

Union Rate Identifiers

A four letter classification abbreviation identifier enclosed in dotted lines beginning with characters other than "SU" or "UAVG" denotes that the union classification and rate were prevailing for that classification in the survey. Example: PLUM0198-005 07/01/2014. PLUM is an abbreviation identifier of the union which prevailed in the survey for this classification, which in this example would be Plumbers. 0198 indicates the local union number or district council number where applicable, i.e., Plumbers Local 0198. The next number, 005 in the example, is an internal number used in processing the wage determination. 07/01/2014 is the effective date of the most current negotiated rate, which in this example is July 1, 2014.

Union prevailing wage rates are updated to reflect all rate changes in the collective bargaining agreement (CBA) governing this classification and rate.

Survey Rate Identifiers

Classifications listed under the "SU" identifier indicate that no one rate prevailed for this classification in the survey and the published rate is derived by computing a weighted average

rate based on all the rates reported in the survey for that classification. As this weighted average rate includes all rates reported in the survey, it may include both union and non-union rates. Example: SULA2012-007 5/13/2014. SU indicates the rates are survey rates based on a weighted average calculation of rates and are not majority rates. LA indicates the State of Louisiana. 2012 is the year of survey on which these classifications and rates are based. The next number, 007 in the example, is an internal number used in producing the wage determination. 5/13/2014 indicates the survey completion date for the classifications and rates under that identifier.

Survey wage rates are not updated and remain in effect until a new survey is conducted.

Union Average Rate Identifiers

Classification(s) listed under the UAVG identifier indicate that no single majority rate prevailed for those classifications; however, 100% of the data reported for the classifications was union data. EXAMPLE: UAVG-OH-0010 08/29/2014. UAVG indicates that the rate is a weighted union average rate. OH indicates the state. The next number, 0010 in the example, is an internal number used in producing the wage determination. 08/29/2014 indicates the survey completion date for the classifications and rates under that identifier.

A UAVG rate will be updated once a year, usually in January of each year, to reflect a weighted average of the current negotiated/CBA rate of the union locals from which the rate is based.

WAGE DETERMINATION APPEALS PROCESS

1.) Has there been an initial decision in the matter? This can be:

- * an existing published wage determination
- * a survey underlying a wage determination
- * a Wage and Hour Division letter setting forth a position on a wage determination matter
- * a conformance (additional classification and rate) ruling

On survey related matters, initial contact, including requests for summaries of surveys, should be with the Wage and Hour Regional Office for the area in which the survey was conducted because those Regional Offices have responsibility for the Davis-Bacon survey program. If the response from this initial contact is not satisfactory, then the process described in 2.) and 3.) should be followed.

With regard to any other matter not yet ripe for the formal process described here, initial contact should be with the Branch of Construction Wage Determinations. Write to:

Branch of Construction Wage Determinations Wage and Hour Division U.S. Department of Labor 200 Constitution Avenue, N.W. Washington, DC 20210

2.) If the answer to the question in 1.) is yes, then an interested party (those affected by the action) can request review and reconsideration from the Wage and Hour Administrator (See 29 CFR Part 1.8 and 29 CFR Part 7). Write to:

Wage and Hour Administrator U.S. Department of Labor 200 Constitution Avenue, N.W. Washington, DC 20210

The request should be accompanied by a full statement of the interested party's position and by any information (wage payment data, project description, area practice material, etc.) that the requestor considers relevant to the issue.

3.) If the decision of the Administrator is not favorable, an interested party may appeal directly to the Administrative Review Board (formerly the Wage Appeals Board). Write to:

Administrative Review Board U.S. Department of Labor 200 Constitution Avenue, N.W. Washington, DC 20210

4.) All decisions by the Administrative Review Board are final.

13.0 PERMITS:

It shall be the sole responsibility of the successful bidder to obtain all required permits in the name of Fort Bend County.

14.0 CONTRACTOR'S RESPONSIBILITY FOR WORK:

- 14.1 <u>Preconstruction Work</u>. Contractor shall do (or cause to be done) the following as preconstruction work:
 - 14.1.1 On written demand as requested by Fort Bend County, cause the Contractor's personnel to meet with Fort Bend County and the Engineer to discuss the status of the Project.
 - 14.1.2 On written demand as requested by Fort Bend County, review drawings and specifications with the Engineer to permit the Contractor and the Engineer to determine the compliance of the proposed facility with applicable building codes.
- 14.2 <u>Construction Work</u>. Contractor shall do (or cause to be done) the following as construction work:
 - 14.2.1 Perform (or cause to be performed) all preparatory work at the construction site required herein, including (without limitation) soil and

concrete testing and demolition of improvements existing at the construction site and all actions necessary for compliance with all laws and regulations as to actions to be taken by owners or contractors before construction begins, including without limitation those in regard to archaeological and environmental requirements.

- 14.2.2 Construct and install (or cause to be constructed and installed) the Project on the construction site in accordance with this Contract and the drawings and specifications approved by Fort Bend County.
- 14.2.3 Furnish (or cause to be furnished) all materials, supplies, equipment, tools, labor, supervision, utilities, transportation, and other materials and services necessary to complete the Project described herein.
- 14.2.4 Materials testing necessary for the Project and required by laws and regulations, construction industry standards as approved by Fort Bend County and this Contract; the frequency of testing shall be approved by Fort Bend County. It is the contractor's responsibility to engage a material testing laboratory to perform testing on the structural concrete to be used for foundation work in this project. The cost of testing shall be incidental to bid item for drill shaft foundation. Testing of concrete shall comply with current TXDOT criteria. Contractor has to submit the name of the testing laboratory, intended to be used by the contractor for this project, for County's approval.
- Standards for Review and Approval. Fort Bend County acknowledges that in 14.3 order to meet the deadlines for the completion of the Project, and in order to accomplish the efficient completion of the Project, the Contractor may submit matters to Fort Bend County in stages for approval or consent. Upon receipt of any matter submitted by the Contractor for review and approval, Fort Bend County shall review the same and shall diligently and promptly (but in any event within 14 calendar days for any such matter, other than a proposed change order, and within 28 calendar days for a proposed change order) give the Contractor notice of Fort Bend County's approval or disapproval, setting forth in detail all reasons for any disapproval. Fort Bend County's right to disapprove any such matter submitted (other than a proposed change order) shall be limited to the elements thereof (a) which do not conform substantially to matters previously approved, (b) which are new elements not previously presented and approved and the Contractor is unable to demonstrate that such new element is reasonably necessary for completion of the Project, or (c) which depict matters that are violations of this Contract or applicable laws and regulations.
 - 14.3.1 If Fort Bend County disapproves of a particular matter or Proposed Change Order, the Contractor shall have the right to resubmit such matter or Proposed Change Order to Fort Bend County, altered to satisfy Fort Bend County's basis for disapproval. Any resubmission shall be subject to review and approval by Fort Bend County.

- 14.3.2 Fort Bend County and the Contractor shall attempt in good faith to resolve any disputes concerning the approval of any aspect of the Project expeditiously, so as not to delay the completion of the Project in accordance with this Contract.
- 14.3.3 <u>Expedited Approvals</u>. Fort Bend County recognizes the importance of expeditious action upon all matters submitted to Fort Bend County for review and approval and of expeditious response to those aspects of the Project requiring approval by governmental authorities having jurisdiction there over. Fort Bend County agrees to exercise its rights of review and approval hereunder with due diligence, reasonableness, and good faith. Fort Bend County shall use its reasonable efforts to expedite any required review of the Project or other matters by any governmental authority.

14.4 <u>Changes</u>.

- 14.4.1 <u>General</u>. Fort Bend County may make changes to the Project by altering, adding to, or deducting from the Project. All changes in the Project which (a) require an adjustment in the contract sum or an adjustment in the final completion date or (b) involve a material change in the overall scope or function of the Project shall be requested and authorized before commencing such changes by use of written change order notices, Proposed Change Orders and Change Orders, which change order procedure shall be the exclusive means to effect such changes in the Project.
- 14.4.2 Change Order Procedure. If at any time Fort Bend County desires to make any change in the Project requiring the issuance of a Change Order, Fort Bend County shall so advise the Contractor in writing by delivery to the Contractor of a written notice describing the change. Upon receipt of such notice initiated by Fort Bend County, the Contractor shall within a reasonable period of time advise Fort Bend County of the Contractor's proposal for the adjustments, if any, in the contract sum, the schedule of values, and the final completion date attributable to such change by delivering a written notice thereof (the "Proposed Change Order") to Fort Bend County. Such Proposed Change Order shall contain a description of the proposed change and shall set forth the Contractor's estimate of the increase or decrease, if any, in the contract sum and the change, if any, in the schedule of values and the final completion date attributable to such change. If the Contractor desires to make a change in the Project requiring the issuance of a change order, the Contractor shall deliver to Fort Bend County a Proposed Change Order. Upon execution by Fort Bend County, a Proposed Change Order shall constitute (and be defined herein as) a "Change Order" for purposes of this Contract. The Contractor shall forthwith perform the work as changed in accordance with such Change Order. All work performed pursuant to a Change Order shall be performed in accordance with the terms of this Contract. All Proposed Change Orders

shall be submitted for approval by Fort Bend County. No action, acquiescence or inaction by Fort Bend County or any representative of Fort Bend County shall be construed to be a waiver of requirements set forth in this Contract in regard to Change Orders or ratification of a violation of such requirements, and all acts in violation of this provision shall be considered void.

- 14.4.3 <u>Change Order Authorization</u>. Each Change Order shall be signed by Fort Bend County and an authorized representative of the Contractor.
- 14.4.4 <u>Contract Sum Adjustments</u>. The contract sum and the schedule of values shall be adjusted only as a result of a Change Order requiring such adjustment. Any extra work performed without a proper Change Order shall be considered voluntary and not subject to additional compensation. The Contractor shall not be entitled to an adjustment in the contract sum (or a Change Order permitting such adjustment) or to damages as a result of any delays in the Project caused by the acts or omissions of Fort Bend County, provided that this sentence is not applicable to delays that constitute more than 90 days in any 365-day period or cause the Project to be interrupted for a continuous period of 45 days through no fault of the Contractor.
- 14.4.5 When Fort Bend County and the Contractor agree upon the adjustments in the contract sum, the schedule of values, and the final completion date attributable to such adjustment, such agreement will be documented by preparation and if approved by the Fort Bend County Commissioners Court, execution of an appropriate Change Order.
- 14.5 <u>Site Access</u>. Prior to the transfer date, Fort Bend County and the Contractor shall have uninterrupted access to the construction site. Subsequent to the transfer date, Fort Bend County will permit the Contractor, the Engineer, and their representatives and subcontractors to enter upon the Project at times reasonably necessary to complete the punch list items.
- 14.6 <u>Applicable Laws and Regulations</u>. Contractor shall in its performance of the Project comply with all applicable laws and regulations. Any delays in the prosecution of the Project caused by any changes in the laws and regulations or the application or enforcement of the laws and regulations may entitle the Contractor to an extension of time.
- 14.7 <u>Familiarity with Project</u>. The Contractor represents and accepts that it has: (a) visited the property(ies), (b) taken such other steps as may be necessary to ascertain the nature and location of the Project and the general and local conditions which affect the Project or the cost thereof, (c) investigated the labor situation as regards to the Project, (d) examined the property(ies), the obstacles which may be encountered and all other observable conditions having a bearing upon the performance of the Project, the superintendence of the Project, the time of completion and all other relevant matters, and (e) reported to Fort Bend County

the results of all of the foregoing. The Contractor represents that it is familiar with all phases of the Project and the matters that may affect the Project or its prosecution under this Contract.

- 14.8 <u>Standard of Performance</u>. The Contractor shall prosecute (or cause to be prosecuted) the Project in accordance with the best efforts for the construction and development of projects similar to the Project in the State of Texas, using qualified, careful, and efficient contractors and workers and in conformity with the provisions of this Contract. The Contractor shall perform the work in a good and workmanlike manner.
- 14.9 Warranty of Contractor. The Contractor warrants to Fort Bend County that: (i) the Contractor possesses the skill and knowledge ordinarily possessed by wellinformed members of its trade or profession and the Contractor will use its best efforts to ensure that the services provided under this Contract will be performed, delivered, and conducted in accordance with the best professional standards and in accordance with industry standards, and (ii) the Contractor is fully experienced and properly qualified to perform the class of work provided for herein, and that it is properly equipped, organized and financed to perform such work, and (iii) following the date of acceptance of this Contract, the services provided by the Contractor to Fort Bend County will conform to the representations contained in this Contract, including all attachments, schedules and exhibits. All warranties provided by the Contractor in this Contract shall be cumulative, shall be deemed consistent and not in conflict, are intended to be given full force and effect and to be interpreted expansively to give the broadest warranty protection to Fort Bend County.
- 14.10 Contractor's Personnel. Contractor shall employ only competent, skilled personnel for the Project. Prior to the final completion date, the Contractor shall maintain a superintendent who shall be authorized to act on behalf of the Contractor and with whom Fort Bend County may consult at all reasonable times. The superintendent shall not be transferred from the Project without Fort Bend County's consent (which shall not be unreasonably withheld or delayed); provided, however, the superintendent shall not be assigned solely to the Project and shall be entitled to spend reasonable time working on matters unrelated to the Project so long as such work on other matters does not render the superintendent unavailable to the Project or unavailable to Fort Bend County. However, such obligation to furnish the superintendent and such staff personnel shall not be construed (a) to preclude the promotion within the Contractor's organization of any person assigned to the Project or (b) to give rise to any liability of the Contractor if any person assigned to the Project (including, without limitation, the superintendent) leaves the Contractor's employment. If the superintendent is transferred from the Project, Fort Bend County shall have the right to approve the replacement superintendent (which approval will not be unreasonably withheld or delayed). The Contractor, the Architect, and the other subcontractors shall comply with all applicable health, safety, and loss prevention rules of applicable governmental authorities. The

Contractor shall, at its own expense, remove from the Project any person who fails to comply with such rules and instructions. The Contractor shall at all times enforce strict discipline and good order among its employees and shall not employ on the Project any unfit person or anyone not skilled in the work assigned to him. Fort Bend County may, upon written notice to the Contractor, require the Contractor to remove an individual immediately from providing services for the following reasons: violation of the terms and conditions of this Contract; violation of Fort Bend County's or the Contractor's work rules and regulations; criminal activity; or violation of state, federal, or municipal statutes. Fort Bend County may, upon thirty (30) days written notice to the Contractor, require the removal of any individual from providing services without cause.

- 14.11 <u>Inspection</u>. The Project and all parts thereof shall be subject to inspection from time to time by inspectors designated by Fort Bend County. No such inspections shall relieve The Contractor of any of its obligations hereunder. Neither failure to inspect nor failure to discover or reject any of the work as not in accordance with the drawings and specifications or any provision of this Contract shall be construed to imply an acceptance of such work or to relieve the Contractor of any of its obligations hereunder. Fort Bend County agrees that its right of inspection shall be used reasonably and in a timely manner so as not to delay orderly completion of the Project.
- 14.12 <u>Protection Against Risks</u>. The Contractor shall take all precautions which are necessary and adequate, against conditions created during the progress of the Project which involve a risk of bodily harm to persons or a risk of damage or loss to any property. The Contractor shall regularly inspect all work, materials and equipment to discover and determine any such conditions and shall be responsible for discovery, determination, and correction of any such conditions. The Contractor shall comply with all federal, state, and local occupational hazard and safety standards, codes and regulations applicable in the jurisdiction where the Project is being performed. The Contractor shall include the substance of this clause in its entirety in all subcontracts for any work to be performed at the construction site.
- 14.13 <u>Equipment</u>. Except as expressly provided herein to the contrary, the Contractor shall furnish (or cause to be furnished) all construction, transportation, installation, tools, and other equipment and facilities required for the performance of the Project within the times specified herein. Such equipment and facilities shall be serviceable and kept fit for the uses intended. Defective items shall be removed from the construction site promptly and at the Contractor's cost. The Contractor shall schedule (or cause to be scheduled) its other operations so as to not interfere with its duty to timely furnish the necessary equipment and facilities and personnel to operate the same at the times necessary for the orderly completion of the Project.

- 14.14 <u>Materials</u>. Except as may be specifically provided otherwise in the Contract or approved in advance by Fort Bend County, the Contractor shall provide Fort Bend County with copies of material testing reports and to cause all materials, equipment, and fabricated items incorporated in the Project to be new and of a suitable grade of their respective kinds for their intended use.
- 14.15 <u>Delay, Disruption or Hindrance Damages</u>. Contractor and the County contemplate that Contractor's performance may be delayed, disrupted or interfered with by unanticipated causes including but not limited to the following:
 - a) Severe and unavoidable natural disasters such as fires, floods, epidemics and earthquakes;
 - b) Abnormal weather conditions;
 - c) Acts or failures to act of the County , third party utility owners or other third party entities; and
 - d) Acts of war or terrorism.

Contractor and the County agree and stipulate that an extension of the Contract Time shall be the sole remedy of Contractor for delays in performance of the Work, whether or not such delays are foreseeable, except for delays caused solely by acts of the County that constitute fraud, intentional misrepresentation, gross negligence, intentional arbitrary or capricious acts and/or omissions or intentional interference with Contractor's performance of the Work and then only to the extent such acts continue after Contractor notifies Owner in writing of such conduct. For delays caused by any act(s) other than fraud, intentional misrepresentation, gross negligence, intentional arbitrary or capricious acts and/or omissions or intentional interference with Contractor's performance of the Work Contractor shall not be entitled to any compensation or recovery of any damages including, without limitation, those damages prohibited or limited in Sections 14.15.1 – 14.15.8 below. The County's exercise of any of its rights or remedies under the Contract including, without limitation, ordering changes in the Work or directing suspension, rescheduling, or correction of the Work, in response to any breach or failure by the Contractor to comply with the terms of the Contract Documents or the Contractor's obligations arising therefrom, shall not be construed as intentional interference with Contractor's performance of the Work regardless of the extent or frequency of the County's exercise of such rights or remedies.

Without limiting the foregoing, except as otherwise expressly provided in this Agreement in calculating the amount of any claim recoverable by Contractor, the following limitations on the recovery of damages shall apply:

14.15.1 No indirect or consequential damages will be allowed.

14.15.2 No recovery shall be based on a comparison of planned expenditures to

total actual expenditures, or on estimated losses of labor efficiency, or on a comparison of planned manloading to actual manloading, or any other analysis that is used to show damages indirectly.

- 14.15.3 Damages, to the extent recoverable, are limited to the additional, actual costs specifically shown to have been directly incurred by the Contractor and solely caused by the proven wrong.
- 14.15.4 No damages will be allowed for home office overhead or other home office charges.
- 14.15.5 No exemplary damages or unjust enrichment damages shall be recoverable.
- 14.15.6 No recovery of attorney's fees shall be recoverable except as expressly permitted under the Agreement.
- 14.15.7 No profit will be allowed on any damage claim, except as expressly recoverable under the Agreement as Fee on Cost of the Work incurred.
- 14.15.8 Notwithstanding any other damage limitation herein the County and the Contractor recognize the delays, expense, and difficulties involved in proving in a legal or arbitration proceeding the actual loss suffered by the Contractor if the County is found to have intentionally interfered with Contractor's performance of the Work by fraud, misrepresentation, gross negligence, or intentional arbitrary or capricious acts and/or omissions. Accordingly, instead of requiring any such proof, the County and the Contractor agree that as liquidated damages (in lieu of any other remedy or damages) for delay, disruption or hindrance (but not as a penalty) the County shall pay the Contractor \$1,500.00 for each day that a court of competent jurisdiction finds the County's conduct referenced in Section14.15 (above) is the sole cause of Contractor's delay in completing the Work.

15.0 TERMINATION:

- 15.1 Fort Bend County may terminate the Contract for cause if the Contractor:
 - 15.1.1 Persistently or repeatedly refuses or fails to supply enough properly skilled workers or proper materials.
 - 15.1.2 Fails to make payment to Subcontractors for materials or labor in accordance with the respective agreements between the Contractor and the Subcontractor.

- 15.1.3 Persistently disregards laws, ordinances, or rules, regulations or orders of a public authority having jurisdiction.
- 15.1.4 Otherwise commits substantial breach of a provision of the Contract Documents.
- 15.2 When any of the above reasons exists, Fort Bend County may, without prejudice to any other rights or remedies of Fort Bend County and after giving the Contractor and the Contractor's surety, if any, seven days' written notice, terminate employment of the Contractor and may, subject to any prior rights of the surety:
 - 15.2.1 Take possession of the site and of all materials, equipment, tools, and construction equipment and machinery thereon owned by the Contractor.
 - 15.2.2 Finish the Project by whatever reasonable method Fort Bend County may deem expedient.
 - 15.2.3 When Fort Bend County terminates the Contract for one of the reasons stated in this section, the Contractor shall not be entitled to receive further payment until the Project is finished. Therefore, the Contractor shall be promptly paid for all work actually and satisfactorily completed.
- 15.3 <u>Termination for Convenience of Fort Bend County</u>

Fort Bend county reserves the right, without breach, to terminate the Contract prior to, or during the performance of the Work, for any reason. Upon such an occurrence, the following shall apply.

- 15.3.1 The County will notify Contractor in writing of the county's determination to terminate the contract for convenience and the effective date of the Contract termination. The notice may also contain instructions necessary for the protection, storage or decommissioning of incomplete work or systems, and for safety.
- 15.3.2 Upon receipt of the notice of termination, Contractor shall immediately proceed with the following obligations, regardless of any dispute in determining or adjusting any amounts due at that point in the Contract:
 - 15.3.2.1 Stop all work.
 - 15.3.2.2 Place no further subcontracts or orders for materials or services.
 - 15.3.2.3 Terminate all subcontracts for convenience.
 - 15.3.2.4 Cancel all materials and equipment orders as applicable.

- 15.3.2.5 Take appropriate action that is necessary to protect and preserve all property related to the Contract which is in the possession of Contractor.
- 15.3.2.6 When the Contract is terminated for Owner's convenience, Contractor may recover from Owner payment for all Work executed. Contractor may not claim lost profits or lost business opportunities.
- 15.4 <u>Settlement on Termination.</u> When the Contract is terminated by the County under 15.3, at any time prior to one hundred eighty (180) days after the effective date of termination, Contractor shall submit a final termination settlement proposal to the County based upon recoverable costs as provided under the Contract. If Contractor fails to submit the proposal within the time allowed, the County may unilaterally determine the amount due to Contractor because of the termination and pay the determined amount to Contractor.

16.0 COMPLETION, TRANSFER, & ACCEPTANCE:

- 16.1 <u>Final Completion</u>. Upon the occurrence of the final completion date, the punch list items shall be promptly commenced and thereafter completed within thirty (30) days after final completion.
- 16.2 <u>Transfer and Acceptance</u>. Upon the occurrence of final completion, care, custody and control of the Project shall pass to Fort Bend County. As referenced herein, the "<u>Transfer Date</u>" shall mean the date on which the care, custody and control of the Project passes to Fort Bend County. Subsequent to the Transfer Date all risk of loss with respect to the Project shall be by Fort Bend County and the Contractor shall be thereafter obligated to cover the Project with their Insurance.

17.0 SUSPENSION BY FORT BEND COUNTY FOR CONVENIENCE:

- 17.1 Fort Bend County may, without cause, order the Contractor in writing to suspend, delay or interrupt the Project in whole or in part for such period of time as Fort Bend County may determine.
- 17.2 An adjustment shall be made for increase in the cost of performance, caused by suspension, delay or interruption. No adjustment shall be made to the extent:
 - 17.2.1 That performance is, was or would have been so suspended, delayed or interrupted by another cause for which the Contractor is responsible.
 - 17.2.2 That an equitable adjustment is made or denied under another provision of this Contract.

17.3 Adjustments made in the cost of performance may have a mutually agreed fixed or percentage fee.

18.0 INDEPENDENT CONTRACTOR:

The Contractor shall be an independent contractor and any provisions of this Contract that may appear to give Fort Bend County the right to direct the Contractor as to the details of the manner of doing the Project shall be deemed to mean that the Contractor shall follow the desires of Fort Bend County in the results of the Project only and not in the means whereby the Project is to be accomplished. The Contractor shall be responsible as to the details of completing the Project. Neither the agents, representatives, nor employees of the Contractor, shall be deemed to be the agents, representatives, or employees of Fort Bend County. The Contractor further represents that it accepts a fiduciary role and responsibility with respect to Fort Bend County and will, to its best abilities, act in the best interests of Fort Bend County and the timely completion of the Project. The Contractor agrees and understands that neither it nor any of its agents or employees may act in the name of Fort Bend County except and unless specifically authorized in writing by Fort Bend County to do so. The Contractor shall furnish construction administration and management services and use the Contractor's best efforts to complete the Project in an expeditious and economical manner consistent with the interests of Fort Bend County.

19.0 NOTICE

- 19.1 All written notices, demands, and other papers or documents to be delivered to Fort Bend County under this Contract shall be delivered to the Engineering Department, 301 Jackson, Richmond, Texas 77469, or at such other place or places as Fort Bend County may from time to time designate by written notice delivered to the Contractor. For purposes of notice under this Contract, a copy of any notice or communication hereunder shall also be forwarded to the following address: Fort Bend County, 301 Jackson Street, Richmond, Texas 77469, Attention: County Judge.
- 19.2 All written notices, demands, and other papers or documents to be delivered to the Contractor under this Contract shall be delivered to the Authorized Representative identified in the Contract documents or such other place or places as the Contractor may designate by written notice delivered to Fort Bend County.

20.0 RECORDS:

- 20.1 Fort Bend County shall be the absolute and unqualified owner of all drawings, preliminary layouts, record drawings, sketches and other documents prepared pursuant to the Contract by Contractor.
- 20.2 The Contractor agrees to maintain and preserve for a period of at least five years after the earlier of the expiration of the defects period or termination of this Contract, accurate and complete records relating to the performance of the

Project. The Contractor agrees to, upon request, provide Fort Bend County with such records.

21.0 SUCCESSORS & ASSIGNS:

- 21.1 Fort Bend County and the Contractor bind themselves and their successors, executors, administrators and assigns to the other party of this Contract and to the successors, executors, administrators and assigns of such other party, in respect to all covenants of this Contract.
- 21.2 Neither Fort Bend County nor the Contractor shall assign, sublet or transfer its interest in this Contract without the prior written consent of the other.
- 21.3 Nothing herein shall be construed as creating any personal liability on the part of any officer or agent of any public and/or governmental body that may be a party hereto.

22.0 PUBLIC CONTACT:

Contact with the news media, citizens of Fort Bend County or governmental agencies shall be the sole responsibility of Fort Bend County. Under no circumstances, whatsoever, shall Contractor release any material or information developed in the performance of its services hereunder without the express written permission of Fort Bend County, except where required to do so by law.

23.0 MODIFICATIONS:

This instrument contains the entire Contract between the parties relating to the rights herein granted and obligations herein assumed. Any oral or written representations or modifications concerning this instrument shall be of no force and effect excepting a subsequent written modification signed by both parties hereto.

24.0 SILENCE OF SPECIFICATIONS:

The apparent silence of specifications as to any detail, or the apparent omission from it of a detailed description concerning any point, shall be regarded as meaning that only the best commercial practice is to prevail and that only material and workmanship of the finest quality are to be used. All interpretations of specifications shall be made on the basis of this statement. The items furnished under this contract shall be new, unused of the latest product in production to commercial trade and shall be of the highest quality as to materials used and workmanship. Manufacturer furnishing these items shall be experienced in design and construction of such items and shall be an established supplier of the item bid.

25.0 SEVERABILITY:

In the event one or more of the provisions contained in these requirements or the specifications shall for any reason be held to be invalid, illegal or unenforceable in any respect, such invalidity,

illegality, or unenforceability shall not affect any other provision hereof and these requirements or the specifications shall be construed as if such invalid, illegal, or unenforceable provision had never been contained herein.

26.0 GOVERNING FORMS:

In the event of any conflict between the terms and provisions of these requirements and the specifications, the specifications shall govern. In the event of any conflict of interpretation of any part of this overall document, Fort Bend County's interpretation shall govern.

27.0 TAX EXEMPT:

Fort Bend County is exempt from state and local sales and use taxes under Section 151.309 of the Texas Tax Code. This Contract is deemed to be a separate contract for Texas tax purposes, and as such, Fort Bend County hereby issues its Texas Exemption for the purchase of any items qualifying for exemption under this Contract. Contractor is to issue its Texas Resale Certificate to vendors and subcontractors for such items qualifying for this exemption, and further, contractor should state these items at cost.

28.0 ENTIRE AGREEMENT:

The Parties agree that this Contract contains all of the terms and conditions of the understanding of the parties relating to the subject matter hereof. All prior negotiations, discussions, correspondence and preliminary understandings between the parties and others relating hereto are superseded by this Contract. By entering into this Contract, the parties do not intend to create any obligations, express or implied, other than those specifically set out in this Contract.

29.0 APPLICABLE LAW & VENUE

This Contract shall be construed under and in accord with the laws of the State of Texas, and all obligations of the parties created hereunder are performable in Fort Bend County, Texas, and that venue for any litigation arising out of or related to this Contract shall lie solely in the court of appropriate jurisdiction located in Fort Bend County, Texas.

30.0 ENCLOSURE:

The following being incorporated herein by reference for all purposes as though fully set forth herein word for word.

Enclosure #1 – Specifications and Plans

31.0 PRICING: Complete excel unit pricing form.

32.0 PROJECT DURATION:

Bidder agrees, if awarded the contract, to complete all work required by the contract documents within _____ calendar days (maximum 365 days) after issuance of a purchase order by the

County Purchasing Agent and notice to proceed by the Engineering Department.

33.0 AWARD:

This contract will be awarded to the overall lowest and best bid.

34.0 TEXAS ETHICS COMMISSION FORM 1295:

- 34.1 Effective January 1, 2016 all contracts executed by Commissioners Court, regardless of the dollar amount, will require completion of Form 1295 "Certificate of Interested Parties", per the new Government Code Statute §2252.908. All vendors submitting a response to a formal Bid, RFP, SOQ or any contracts, contract amendments, renewals or change orders are required to complete the Form 1295 online through the State of Texas Ethics Commission website. Please visit: <u>https://www.ethics.state.tx.us/filinginfo/1295/</u>
- 34.2 On-line instructions:
 - 34.2.1 Name of governmental entity is to read: Fort Bend County
 - 34.2.2 Identification number used by the governmental entity is: <u>B24-076</u>
 - 34.2.3 Description is the title of the solicitation: <u>Construction of Stella Road from</u> <u>Cottonwood School Road to W. Fairgrounds Road</u>.
- 34.3 Apparent low bidder(s) will be required to provide the Form 1295 within three (3) calendar days from notification; however, if your company is publicly traded you are not required to complete this form.

35.0 STATE LAW REQUIREMENTS FOR CONTRACTS:

The contents of this section are required by Texas Law and are included by County regardless of content.

- 35.1 Agreement to Not Boycott Israel Chapter 2271 Texas Government Code: Contractor verifies that if Contractor employs ten (10) or more full-time employees and this Agreement has a value of \$100,000 or more, Contractor does not boycott Israel and will not boycott Israel during the term of this Agreement.
- 35.2 Texas Government Code Section 2251.152 Acknowledgment: By signature on vendor form, Contractor represents pursuant to Section 2252.152 of the Texas Government Code, that Contractor is not listed on the website of the Comptroller of the State of Texas concerning the listing of companies that are identified under Section 806.051, Section 807.051 or Section 2253.153.

36.0 HUMAN TRAFFICKING:

By acceptance of this contract, Contractor acknowledges that Fort Bend County is opposed to human trafficking and that no County funds will be used in support of services or activities that violate human trafficking laws

37.0 INDEMNITY FOR BODILY INJURY OR DEATH CLAIMS

Indemnity for certain bodily injury or death claims. To the fullest extent permitted by law, contractor shall indemnify, defend and hold harmless the county from and against all claims, losses, expenses, costs, demands, suits, causes of action, and damages, including without limitation, attorneys' fees and expenses, for bodily injury or death of any employee of contractor, its agents, or its subcontractors of every tier, even if the bodily injury or death is caused by or alleged to have been caused by the sole or partial negligence, fault or strict liability of any indemnitee.

Indemnity for all other claims. For all claims not addressed in the preceding section or section 11.0 above , including, without limitation, claims for damage to or loss of use of property and claims for bodily injury to or death of any person other than that addressed in the immediately preceding section, to the fullest extent permitted by law, contractor shall indemnify, defend and hold harmless the county from and against all claims, losses, expenses, costs, demands, suits, causes of action, and damages, including without limitation, attorneys' fees and expenses, of any nature whatsoever arising out of or related to this contract or the work to be performed under this contract, but only to the extent of the negligence or other fault of the contractor, its agents, representatives, employees or subcontractors of any tier.

38.0 AGREEMENT TO ARBITRATE UNDER THE FEDERAL ARBITRATION ACT

To the maximum extent allowed by law, any controversy or claim arising out of or relating to this contract, or the breach thereof, shall be settled by arbitration under the Federal Arbitration Act, 9 U.S.C. § 1, et seq. administered by the American Arbitration Association under its Construction Industry Arbitration Rules, and judgment on the award rendered by the arbitrator(s) may be entered in any court having jurisdiction thereof. For cases in which the amount in controversy is less than \$250,000, there shall be no discovery other than an expeditious and complete exchange of documents relative to the dispute. For cases in which the amount in controversy is between \$250,000 and \$1,000,000, there shall be no discovery except for an expeditious and complete exchange of such documentary information and up to three (3) depositions per side (including expert depositions, if any). For cases in which the amount in controversy exceeds \$1,000,000, there shall be no discovery except for an expeditious and complete exchange of such documentary information up to five (5) depositions per side (including expert depositions, if any). No formal interrogatories, request for admissions or formal request for production of documents shall be allowed in the arbitration process. The hearing on the merits will be completed no later than ninety (90) days after the initial demand for arbitration is made for disputes involving amounts in controversy of up to \$250,000; no later than no later than one hundred twenty (120) days after the initial demand for arbitration is made for disputes involving amounts in controversy of between \$250,000 and \$1,000,000; and, no

later than three hundred sixty five (365) days after the initial demand for arbitration is made for disputes involving amounts in controversy of over \$1,000,000.

39.0 ADDITIONAL REQUIRED FORMS:

All vendors submitting are required to complete and return with submission

- 39.1 Vendor Form
- 39.2 W9 Form
- 39.3 Tax Form/Debt/Residence Certification
- 39.4 Contractor Acknowledgement of Stormwater Management Program

Contract Sheet Bid 24-076

THE STATE OF TEXAS COUNTY OF FORT BEND

This memorandum of agreement made and entered into on the _____ day of ______, 20____, by and between Fort Bend County in the State of Texas (hereinafter designated County), acting herein by County Judge KP George, by virtue of an order of Fort Bend County Commissioners Court, and ______ (hereinafter designated Contractor).

(company name)

WITNESSETH:

The Contractor and the County agree that the bid and specifications for the **Construction of Stella Road from Cottonwood School Road to W. Fairgrounds Road for Fort Bend County Mobility Bond Project No. 20116** which are hereto attached and made a part hereof, together with this instrument and the bond (when required) shall constitute the full agreement and contract between parties and for furnishing the items set out and described; the County agrees to pay the prices stipulated in the accepted bid.

It is further agreed that this contract shall not become binding or effective until signed by the parties hereto and a purchase order authorizing the items desired has been issued.

Executed at Richmond, Texas this	day of	20
		Fort Bend County, Texas
	By:	County Judge, KP George
	By:	Signature of Contractor
	By:	Printed Name and Title

ge 2.	2 Business name/disregarded entity name, if different from above						
e ns on page	Check appropriate box for federal tax classification; check only one of the following seven boxes: Individual/sole proprietor or C Corporation S Corporation Partnership Trust/estate single-member LLC			4 Exemptions (codes apply only to certain entities, not individuals; see instructions on page 3):			
Print or type Specific Instructions	 Limited liability company. Enter the tax classification (C=C corporation, S=S corporation, P=partners Note. For a single-member LLC that is disregarded, do not check LLC; check the appropriate box in the tax classification of the single-member owner. 		Exemption code (if a	· · · · ·	TCA repo	0	
PI pecific I	Image: Second secon			the U.S.)			
See S	6 City, state, and ZIP code						
D	7 List account number(s) here (optional)						
Par		Canial	security nun	hor			
backu reside entitie	your TIN in the appropriate box. The TIN provided must match the name given on line 1 to ave p withholding. For individuals, this is generally your social security number (SSN). However, for nt alien, sole proprietor, or disregarded entity, see the Part I instructions on page 3. For other s, it is your employer identification number (EIN). If you do not have a number, see <i>How to ge</i>	pra		–			
TIN or	n page 3.	or					
	If the account is in more than one name, see the instructions for line 1 and the chart on page ines on whose number to enter.	4 for Emplo	yer identifica	tion num	ber		

Part II Certification

Under penalties of perjury, I certify that:

- 1. The number shown on this form is my correct taxpayer identification number (or I am waiting for a number to be issued to me); and
- I am not subject to backup withholding because: (a) I am exempt from backup withholding, or (b) I have not been notified by the Internal Revenue Service (IRS) that I am subject to backup withholding as a result of a failure to report all interest or dividends, or (c) the IRS has notified me that I am no longer subject to backup withholding; and
- 3. I am a U.S. citizen or other U.S. person (defined below); and
- 4. The FATCA code(s) entered on this form (if any) indicating that I am exempt from FATCA reporting is correct.

1 Name (as shown on your income tax return). Name is required on this line; do not leave this line blank.

Certification instructions. You must cross out item 2 above if you have been notified by the IRS that you are currently subject to backup withholding because you have failed to report all interest and dividends on your tax return. For real estate transactions, item 2 does not apply. For mortgage interest paid, acquisition or abandonment of secured property, cancellation of debt, contributions to an individual retirement arrangement (IRA), and generally, payments other than interest and dividends, you are not required to sign the certification, but you must provide your correct TIN. See the instructions on page 3.

Sign	Signature of		
Here	U.S. person ►		

0.

General Instructions

Section references are to the Internal Revenue Code unless otherwise noted.

Future developments. Information about developments affecting Form W-9 (such as legislation enacted after we release it) is at *www.irs.gov/fw*9.

Purpose of Form

An individual or entity (Form W-9 requester) who is required to file an information return with the IRS must obtain your correct taxpayer identification number (TIN) which may be your social security number (SSN), individual taxpayer identification number (ITIN), adoption taxpayer identification number (ATIN), or employer identification number (EIN), to report on an information return the amount paid to you, or other amount reportable on an information return. Examples of information returns include, but are not limited to, the following:

- Form 1099-INT (interest earned or paid)
- Form 1099-DIV (dividends, including those from stocks or mutual funds)
- Form 1099-MISC (various types of income, prizes, awards, or gross proceeds)
- Form 1099-B (stock or mutual fund sales and certain other transactions by brokers)
- Form 1099-S (proceeds from real estate transactions)
- Form 1099-K (merchant card and third party network transactions)

Date 🕨

- Form 1098 (home mortgage interest), 1098-E (student loan interest), 1098-T (tuition)
- Form 1099-C (canceled debt)
- · Form 1099-A (acquisition or abandonment of secured property)

Use Form W-9 only if you are a U.S. person (including a resident alien), to provide your correct TIN.

If you do not return Form W-9 to the requester with a TIN, you might be subject to backup withholding. See What is backup withholding? on page 2.

By signing the filled-out form, you:

1. Certify that the TIN you are giving is correct (or you are waiting for a number to be issued),

2. Certify that you are not subject to backup withholding, or

3. Claim exemption from backup withholding if you are a U.S. exempt payee. If applicable, you are also certifying that as a U.S. person, your allocable share of any partnership income from a U.S. trade or business is not subject to the withholding tax on foreign partners' share of effectively connected income, and

4. Certify that FATCA code(s) entered on this form (if any) indicating that you are exempt from the FATCA reporting, is correct. See *What is FATCA reporting?* on page 2 for further information.

Note. If you are a U.S. person and a requester gives you a form other than Form W-9 to request your TIN, you must use the requester's form if it is substantially similar to this Form W-9.

Definition of a U.S. person. For federal tax purposes, you are considered a U.S. person if you are:

• An individual who is a U.S. citizen or U.S. resident alien;

• A partnership, corporation, company, or association created or organized in the United States or under the laws of the United States;

An estate (other than a foreign estate); or

• A domestic trust (as defined in Regulations section 301.7701-7).

Special rules for partnerships. Partnerships that conduct a trade or business in the United States are generally required to pay a withholding tax under section 1446 on any foreign partners' share of effectively connected taxable income from such business. Further, in certain cases where a Form W-9 has not been received, the rules under section 1446 require a partnership to presume that a partner is a foreign person, and pay the section 1446 withholding tax. Therefore, if you are a U.S. person that is a partner in a partnership to enducting a trade or business in the United States, provide Form W-9 to the partnership to establish your U.S. status and avoid section 1446 withholding on your share of partnership income.

In the cases below, the following person must give Form W-9 to the partnership for purposes of establishing its U.S. status and avoiding withholding on its allocable share of net income from the partnership conducting a trade or business in the United States:

• In the case of a disregarded entity with a U.S. owner, the U.S. owner of the disregarded entity and not the entity;

• In the case of a grantor trust with a U.S. grantor or other U.S. owner, generally, the U.S. grantor or other U.S. owner of the grantor trust and not the trust; and

• In the case of a U.S. trust (other than a grantor trust), the U.S. trust (other than a grantor trust) and not the beneficiaries of the trust.

Foreign person. If you are a foreign person or the U.S. branch of a foreign bank that has elected to be treated as a U.S. person, do not use Form W-9. Instead, use the appropriate Form W-8 or Form 8233 (see Publication 515, Withholding of Tax on Nonresident Aliens and Foreign Entities).

Nonresident alien who becomes a resident alien. Generally, only a nonresident alien individual may use the terms of a tax treaty to reduce or eliminate U.S. tax on certain types of income. However, most tax treaties contain a provision known as a "saving clause." Exceptions specified in the saving clause may permit an exemption from tax to continue for certain types of income even after the payee has otherwise become a U.S. resident alien for tax purposes.

If you are a U.S. resident alien who is relying on an exception contained in the saving clause of a tax treaty to claim an exemption from U.S. tax on certain types of income, you must attach a statement to Form W-9 that specifies the following five items:

1. The treaty country. Generally, this must be the same treaty under which you claimed exemption from tax as a nonresident alien.

2. The treaty article addressing the income.

3. The article number (or location) in the tax treaty that contains the saving clause and its exceptions.

4. The type and amount of income that qualifies for the exemption from tax.

5. Sufficient facts to justify the exemption from tax under the terms of the treaty article.

Example. Article 20 of the U.S.-China income tax treaty allows an exemption from tax for scholarship income received by a Chinese student temporarily present in the United States. Under U.S. law, this student will become a resident alien for tax purposes if his or her stay in the United States exceeds 5 calendar years. However, paragraph 2 of the first Protocol to the U.S.-China treaty (dated April 30, 1984) allows the provisions of Article 20 to continue to apply even after the Chinese student becomes a resident alien of the United States. A Chinese student who qualifies for this exception (under paragraph 2 of the first protocol) and is relying on this exception to claim an exemption from tax on his or her scholarship or fellowship income would attach to Form W-9 a statement that includes the information described above to support that exemption.

If you are a nonresident alien or a foreign entity, give the requester the appropriate completed Form W-8 or Form 8233.

Backup Withholding

What is backup withholding? Persons making certain payments to you must under certain conditions withhold and pay to the IRS 28% of such payments. This is called "backup withholding." Payments that may be subject to backup withholding include interest, tax-exempt interest, dividends, broker and barter exchange transactions, rents, royalties, nonemployee pay, payments made in settlement of payment card and third party network transactions, and certain payments from fishing boat operators. Real estate transactions are not subject to backup withholding.

You will not be subject to backup withholding on payments you receive if you give the requester your correct TIN, make the proper certifications, and report all your taxable interest and dividends on your tax return.

Payments you receive will be subject to backup withholding if:

1. You do not furnish your TIN to the requester,

2. You do not certify your TIN when required (see the Part II instructions on page 3 for details),

3. The IRS tells the requester that you furnished an incorrect TIN,

4. The IRS tells you that you are subject to backup withholding because you did not report all your interest and dividends on your tax return (for reportable interest and dividends only), or

5. You do not certify to the requester that you are not subject to backup withholding under 4 above (for reportable interest and dividend accounts opened after 1983 only).

Certain payees and payments are exempt from backup withholding. See *Exempt* payee code on page 3 and the separate Instructions for the Requester of Form W-9 for more information.

Also see Special rules for partnerships above.

What is FATCA reporting?

The Foreign Account Tax Compliance Act (FATCA) requires a participating foreign financial institution to report all United States account holders that are specified United States persons. Certain payees are exempt from FATCA reporting. See *Exemption from FATCA reporting code* on page 3 and the Instructions for the Requester of Form W-9 for more information.

Updating Your Information

You must provide updated information to any person to whom you claimed to be an exempt payee if you are no longer an exempt payee and anticipate receiving reportable payments in the future from this person. For example, you may need to provide updated information if you are a C corporation that elects to be an S corporation, or if you no longer are tax exempt. In addition, you must furnish a new Form W-9 if the name or TIN changes for the account; for example, if the grantor of a grantor trust dies.

Penalties

Failure to furnish TIN. If you fail to furnish your correct TIN to a requester, you are subject to a penalty of \$50 for each such failure unless your failure is due to reasonable cause and not to willful neglect.

Civil penalty for false information with respect to withholding. If you make a false statement with no reasonable basis that results in no backup withholding, you are subject to a \$500 penalty.

Criminal penalty for falsifying information. Willfully falsifying certifications or affirmations may subject you to criminal penalties including fines and/or imprisonment.

Misuse of TINs. If the requester discloses or uses TINs in violation of federal law, the requester may be subject to civil and criminal penalties.

Specific Instructions

Line 1

You must enter one of the following on this line; **do not** leave this line blank. The name should match the name on your tax return.

If this Form W-9 is for a joint account, list first, and then circle, the name of the person or entity whose number you entered in Part I of Form W-9.

a. **Individual.** Generally, enter the name shown on your tax return. If you have changed your last name without informing the Social Security Administration (SSA) of the name change, enter your first name, the last name as shown on your social security card, and your new last name.

Note. ITIN applicant: Enter your individual name as it was entered on your Form W-7 application, line 1a. This should also be the same as the name you entered on the Form 1040/1040A/1040EZ you filed with your application.

b. **Sole proprietor or single-member LLC.** Enter your individual name as shown on your 1040/1040A/1040EZ on line 1. You may enter your business, trade, or "doing business as" (DBA) name on line 2.

c. Partnership, LLC that is not a single-member LLC, C Corporation, or S Corporation. Enter the entity's name as shown on the entity's tax return on line 1 and any business, trade, or DBA name on line 2.

d. **Other entities.** Enter your name as shown on required U.S. federal tax documents on line 1. This name should match the name shown on the charter or other legal document creating the entity. You may enter any business, trade, or DBA name on line 2.

e. **Disregarded entity.** For U.S. federal tax purposes, an entity that is disregarded as an entity separate from its owner is treated as a "disregarded entity." See Regulations section 301.7701-2(c)(2)(ii). Enter the owner's name on line 1. The name of the entity entered on line 1 should never be a disregarded entity. The name on line 1 should be the name shown on the income tax return on which the income should be reported. For example, if a foreign LLC that is treated as a disregarded entity for U.S. federal tax purposes has a single owner that is a U.S. person, the U.S. owner's name is required to be provided on line 1. If the direct owner of the entity is also a disregarded entity, enter the first owner that is not disregarded for federal tax purposes. Enter the disregarded entity's name on line 2, "Business name/disregarded entity complete an appropriate Form W-8 instead of a Form W-9. This is the case even if the foreign person has a U.S. TIN.

Line 2

If you have a business name, trade name, DBA name, or disregarded entity name, you may enter it on line 2.

Line 3

Check the appropriate box in line 3 for the U.S. federal tax classification of the person whose name is entered on line 1. Check only one box in line 3.

Limited Liability Company (LLC). If the name on line 1 is an LLC treated as a partnership for U.S. federal tax purposes, check the "Limited Liability Company" box and enter "P" in the space provided. If the LLC has filed Form 8832 or 2553 to be taxed as a corporation, check the "Limited Liability Company" box and in the space provided enter "C" for C corporation or "S" for S corporation. If it is a single-member LLC that is a disregarded entity, do not check the "Limited Liability Company" box; instead check the first box in line 3 "Individual/sole proprietor or single-member LLC."

Line 4, Exemptions

If you are exempt from backup withholding and/or FATCA reporting, enter in the appropriate space in line 4 any code(s) that may apply to you.

Exempt payee code.

Generally, individuals (including sole proprietors) are not exempt from backup withholding.

• Except as provided below, corporations are exempt from backup withholding for certain payments, including interest and dividends.

• Corporations are not exempt from backup withholding for payments made in settlement of payment card or third party network transactions.

 Corporations are not exempt from backup withholding with respect to attorneys' fees or gross proceeds paid to attorneys, and corporations that provide medical or health care services are not exempt with respect to payments reportable on Form 1099-MISC.

The following codes identify payees that are exempt from backup withholding. Enter the appropriate code in the space in line 4.

1 - An organization exempt from tax under section 501(a), any IRA, or a custodial account under section 403(b)(7) if the account satisfies the requirements of section 401(f)(2)

2-The United States or any of its agencies or instrumentalities

3-A state, the District of Columbia, a U.S. commonwealth or possession, or any of their political subdivisions or instrumentalities

 $4\!-\!\mathrm{A}$ foreign government or any of its political subdivisions, agencies, or instrumentalities

5-A corporation

6-A dealer in securities or commodities required to register in the United States, the District of Columbia, or a U.S. commonwealth or possession

 $7{-}\mathrm{A}$ futures commission merchant registered with the Commodity Futures Trading Commission

8-A real estate investment trust

 $9-\mbox{An entity}$ registered at all times during the tax year under the Investment Company Act of 1940

10-A common trust fund operated by a bank under section 584(a)

11-A financial institution

 $12\mbox{--}A$ middleman known in the investment community as a nominee or custodian

13—A trust exempt from tax under section 664 or described in section 4947 The following chart shows types of payments that may be exempt from backup withholding. The chart applies to the exempt payees listed above, 1 through 13.

IF the payment is for	THEN the payment is exempt for
Interest and dividend payments	All exempt payees except for 7
Broker transactions	Exempt payees 1 through 4 and 6 through 11 and all C corporations. S corporations must not enter an exempt payee code because they are exempt only for sales of noncovered securities acquired prior to 2012.
Barter exchange transactions and patronage dividends	Exempt payees 1 through 4
Payments over \$600 required to be reported and direct sales over \$5,000 ¹	Generally, exempt payees 1 through 5 ²
Payments made in settlement of payment card or third party network transactions	Exempt payees 1 through 4

¹ See Form 1099-MISC, Miscellaneous Income, and its instructions.

² However, the following payments made to a corporation and reportable on Form 1099-MISC are not exempt from backup withholding: medical and health care payments, attorneys' fees, gross proceeds paid to an attorney reportable under section 6045(f), and payments for services paid by a federal executive agency.

Exemption from FATCA reporting code. The following codes identify payees that are exempt from reporting under FATCA. These codes apply to persons submitting this form for accounts maintained outside of the United States by certain foreign financial institutions. Therefore, if you are only submitting this form for an account you hold in the United States, you may leave this field blank. Consult with the person requesting this form if you are uncertain if the financial institution is subject to these requirements. A requester may indicate that a code is not required by providing you with a Form W-9 with "Not Applicable" (or any similar indication) written or printed on the line for a FATCA exemption code.

A—An organization exempt from tax under section 501(a) or any individual retirement plan as defined in section 7701(a)(37)

B-The United States or any of its agencies or instrumentalities

C-A state, the District of Columbia, a U.S. commonwealth or possession, or any of their political subdivisions or instrumentalities

D-A corporation the stock of which is regularly traded on one or more established securities markets, as described in Regulations section 1.1472-1(c)(1)(i)

E-A corporation that is a member of the same expanded affiliated group as a corporation described in Regulations section 1.1472-1(c)(1)(i)

F-A dealer in securities, commodities, or derivative financial instruments (including notional principal contracts, futures, forwards, and options) that is registered as such under the laws of the United States or any state

G-A real estate investment trust

 $\rm H-A$ regulated investment company as defined in section 851 or an entity registered at all times during the tax year under the Investment Company Act of 1940

I-A common trust fund as defined in section 584(a)

J-A bank as defined in section 581

K-A broker

L-A trust exempt from tax under section 664 or described in section 4947(a)(1)

M-A tax exempt trust under a section 403(b) plan or section 457(g) plan

Note. You may wish to consult with the financial institution requesting this form to determine whether the FATCA code and/or exempt payee code should be completed.

Line 5

Enter your address (number, street, and apartment or suite number). This is where the requester of this Form W-9 will mail your information returns.

Line 6

Enter your city, state, and ZIP code.

Part I. Taxpayer Identification Number (TIN)

Enter your TIN in the appropriate box. If you are a resident alien and you do not have and are not eligible to get an SSN, your TIN is your IRS individual taxpayer identification number (ITIN). Enter it in the social security number box. If you do not have an ITIN, see *How to get a TIN* below.

If you are a sole proprietor and you have an EIN, you may enter either your SSN or EIN. However, the IRS prefers that you use your SSN.

If you are a single-member LLC that is disregarded as an entity separate from its owner (see *Limited Liability Company (LLC)* on this page), enter the owner's SSN (or EIN, if the owner has one). Do not enter the disregarded entity's EIN. If the LLC is classified as a corporation or partnership, enter the entity's EIN.

Note. See the chart on page 4 for further clarification of name and TIN combinations.

How to get a TIN. If you do not have a TIN, apply for one immediately. To apply for an SSN, get Form SS-5, Application for a Social Security Card, from your local SSA office or get this form online at *www.ssa.gov*. You may also get this form by calling 1-800-772-1213. Use Form W-7, Application for IRS Individual Taxpayer Identification Number, to apply for an TIN, or Form SS-4, Application for Employer Identification Number, to apply for an EIN. You can apply for an EIN online by accessing the IRS website at *www.irs.gov/businesses* and clicking on Employer Identification Number (EIN) under Starting a Business. You can get Forms W-7 and SS-4 from the IRS by visiting IRS.gov or by calling 1-800-TAX-FORM (1-800-829-3676).

If you are asked to complete Form W-9 but do not have a TIN, apply for a TIN and write "Applied For" in the space for the TIN, sign and date the form, and give it to the requester. For interest and dividend payments, and certain payments made with respect to readily tradable instruments, generally you will have 60 days to get a TIN and give it to the requester before you are subject to backup withholding on payments. The 60-day rule does not apply to other types of payments. You will be subject to backup withholding on all such payments until you provide your TIN to the requester.

Note. Entering "Applied For" means that you have already applied for a TIN or that you intend to apply for one soon.

Caution: A disregarded U.S. entity that has a foreign owner must use the appropriate Form W-8.

Part II. Certification

To establish to the withholding agent that you are a U.S. person, or resident alien, sign Form W-9. You may be requested to sign by the withholding agent even if items 1, 4, or 5 below indicate otherwise.

For a joint account, only the person whose TIN is shown in Part I should sign (when required). In the case of a disregarded entity, the person identified on line 1 must sign. Exempt payees, see Exempt payee code earlier.

Signature requirements. Complete the certification as indicated in items 1 through 5 below

1. Interest, dividend, and barter exchange accounts opened before 1984 and broker accounts considered active during 1983. You must give your correct TIN, but you do not have to sign the certification.

2. Interest, dividend, broker, and barter exchange accounts opened after 1983 and broker accounts considered inactive during 1983. You must sign the certification or backup withholding will apply. If you are subject to backup withholding and you are merely providing your correct TIN to the requester, you must cross out item 2 in the certification before signing the form.

3. Real estate transactions. You must sign the certification. You may cross out item 2 of the certification.

4. Other payments. You must give your correct TIN, but you do not have to sign the certification unless you have been notified that you have previously given an incorrect TIN. "Other payments" include payments made in the course of the requester's trade or business for rents, royalties, goods (other than bills for merchandise), medical and health care services (including payments to corporations), payments to a nonemployee for services, payments made in settlement of payment card and third party network transactions, payments to certain fishing boat crew members and fishermen, and gross proceeds paid to attorneys (including payments to corporations).

5. Mortgage interest paid by you, acquisition or abandonment of secured property, cancellation of debt, qualified tuition program payments (under section 529), IRA, Coverdell ESA, Archer MSA or HSA contributions or distributions, and pension distributions. You must give your correct TIN, but you do not have to sign the certification.

What Name and Number To Give the Requester

For this type of account:	Give name and SSN of:
 Individual Two or more individuals (joint account) 	The individual The actual owner of the account or, if combined funds, the first individual on the account'
3. Custodian account of a minor (Uniform Gift to Minors Act)	The minor ²
 a. The usual revocable savings trust (grantor is also trustee) b. So-called trust account that is not a legal or valid trust under state law 	The grantor-trustee'
 Sole proprietorship or disregarded entity owned by an individual 	The owner ³
6. Grantor trust filing under Optional Form 1099 Filing Method 1 (see Regulations section 1.671-4(b)(2)(i) (A))	The grantor*
For this type of account:	Give name and EIN of:
7. Disregarded entity not owned by an individual	The owner
8. A valid trust, estate, or pension trust	Legal entity⁴
9. Corporation or LLC electing corporate status on Form 8832 or Form 2553	The corporation
10. Association, club, religious, charitable, educational, or other tax- exempt organization	The organization
11. Partnership or multi-member LLC	The partnership
12. A broker or registered nominee	The broker or nominee
13. Account with the Department of Agriculture in the name of a public entity (such as a state or local government, school district, or prison) that receives agricultural program payments	The public entity
14. Grantor trust filing under the Form 1041 Filing Method or the Optional Form 1099 Filing Method 2 (see Regulations section 1.671-4(b)(2)(i) (B))	The trust

List first and circle the name of the person whose number you furnish. If only one person on a joint account has an SSN, that person's number must be furnished.

Circle the minor's name and furnish the minor's SSN.

³ You must show your individual name and you may also enter your business or DBA name on the "Business name/disregarded entity" name line. You may use either your SSN or EIN (if you have one), but the IRS encourages you to use your SSN.

⁴ List first and circle the name of the trust, estate, or pension trust. (Do not furnish the TIN of the personal representative or trustee unless the legal entity itself is not designated in the account title.) Also see Special rules for partnerships on page 2. *Note. Grantor also must provide a Form W-9 to trustee of trust.

Note. If no name is circled when more than one name is listed, the number will be considered to be that of the first name listed.

Secure Your Tax Records from Identity Theft

Identity theft occurs when someone uses your personal information such as your name, SSN, or other identifying information, without your permission, to commit fraud or other crimes. An identity thief may use your SSN to get a job or may file a tax return using your SSN to receive a refund.

To reduce your risk:

- · Protect your SSN,
- Ensure your employer is protecting your SSN, and
- · Be careful when choosing a tax preparer.

If your tax records are affected by identity theft and you receive a notice from the IRS, respond right away to the name and phone number printed on the IRS notice or letter.

If your tax records are not currently affected by identity theft but you think you are at risk due to a lost or stolen purse or wallet, questionable credit card activity or credit report, contact the IRS Identity Theft Hotline at 1-800-908-4490 or submit Form 14039

For more information, see Publication 4535, Identity Theft Prevention and Victim Assistance

Victims of identity theft who are experiencing economic harm or a system problem, or are seeking help in resolving tax problems that have not been resolved through normal channels, may be eligible for Taxpayer Advocate Service (TAS) assistance. You can reach TAS by calling the TAS toll-free case intake line at 1-877-777-4778 or TTY/TDD 1-800-829-4059

Protect yourself from suspicious emails or phishing schemes. Phishing is the creation and use of email and websites designed to mimic legitimate business emails and websites. The most common act is sending an email to a user falsely claiming to be an established legitimate enterprise in an attempt to scam the user into surrendering private information that will be used for identity theft.

The IRS does not initiate contacts with taxpayers via emails. Also, the IRS does not request personal detailed information through email or ask taxpayers for the PIN numbers, passwords, or similar secret access information for their credit card, bank, or other financial accounts.

If you receive an unsolicited email claiming to be from the IRS, forward this message to phishing@irs.gov. You may also report misuse of the IRS name, logo, or other IRS property to the Treasury Inspector General for Tax Administration (TIGTA) at 1-800-366-4484. You can forward suspicious emails to the Federal Trade Commission at: *spam@uce.gov* or contact them at *www.ftc.gov/idtheft* or 1-877-IDTHEFT (1-877-438-4338).

Visit IRS.gov to learn more about identity theft and how to reduce your risk.

Privacy Act Notice

Section 6109 of the Internal Revenue Code requires you to provide your correct TIN to persons (including federal agencies) who are required to file information returns with the IRS to report interest, dividends, or certain other income paid to you; mortgage interest you paid; the acquisition or abandonment of secured property; the cancellation of debt; or contributions you made to an IRA, Archer MSA, or HSA. The person collecting this form uses the information on the form to file information returns with the IRS, reporting the above information. Routine uses of this information include giving it to the Department of Justice for civil and criminal litigation and to cities, states, the District of Columbia, and U.S. commonwealths and possessions for use in administering their laws. The information also may be disclosed to other countries under a treaty, to federal and state agencies to enforce civil and criminal laws, or to federal law enforcement and intelligence agencies to combat terrorism. You must provide your TIN whether or not you are required to file a tax return. Under section 3406, payers must generally withhold a percentage of taxable interest, dividend, and certain other payments to a payee who does not give a TIN to the payer. Certain penalties may also apply for providing false or fraudulent information.

Job No.:

TAX FORM/DEBT/ RESIDENCE CERTIFICATION

(for Advertised Projects)

Taxpa	yer Ide	entification Number (T.I	.N.):
Comp	any Na	ame submitting Bid/Prop	oosal:
Mailir	ng Add	ress:	
Are ye	ou regi	stered to do business in	the State of Texas? 🗌 Yes 🗌 No
		individual, list the name ne(s) under which you o	es and addresses of any partnership of which you are a general partner or any perate your business
I.	nam		roperty in Fort Bend County owned by you or above partnerships as well as any d/b/a sonal property as well as mineral interest accounts. (Use a second sheet of paper if
Fort B	Bend Co	ounty Tax Acct. No.*	Property address or location**
** Fo ada	or real dress w y be st <u>Fort</u>	property, specify the p where the property is lo ored at a warehouse or Bend County Debt - D	to you owe any debts to Fort Bend County (taxes on properties listed in I above,
		ets, fines, tolls, court jud	
		Yes No If y	es, attach a separate page explaining the debt.
III.	requ	ests Residence Certifica	Pursuant to Texas Government Code §2252.001 <i>et seq.</i> , as amended, Fort Bend County tion. §2252.001 <i>et seq.</i> of the Government Code provides some restrictions on the ntracts; pertinent provisions of §2252.001 are stated below:
	(3)	"Nonresident bidder" r	efers to a person who is not a resident.
	(4)		rs to a person whose principal place of business is in this state, including a timate parent company or majority owner has its principal place of business in
		I certify that[Co §2252.001.	is a Resident Bidder of Texas as defined in Government Code [ompany Name]
		Co	is a Nonresident Bidder as defined in Government Code mpany Name] acipal place of business is
Created	05/12	32202.001 und our prin	[City and State]



Contractor Acknowledgement of Storm Water Management Program

I hereby acknowledge that I am aware of the stormwater management program and standard operating procedures developed by Fort Bend County in compliance with the TPDES General Permit No. TXR040000. I agree to comply with all applicable best management practices and standard operating procedures while conducting my services for Fort Bend County. I agree to conduct all services in a manner that does not introduce illicit discharges of pollutants to streets, stormwater inlets, drainage ditches or any portion of the drainage system. The following materials and/or pollutant sources must not be discharged to the drainage system as a result of any services provided:

- 1. Grass clippings, leaves, mulch, rocks, sand, dirt or other waste materials resulting from landscaping activities, (except those materials resulting from ditch mowing or maintenance activities)
- 2. Herbicides, pesticides and/or fertilizers, (except those intended for aquatic use)
- 3. Detergents, fuels, solvents, oils and/or lubricants, other equipment and/or vehicle fluids,
- 4. Other hazardous materials including paints, thinners, chemicals or related waste materials,
- 5. Uncontrolled dewatering discharges, equipment and/or vehicle wash waters,
- 6. Sanitary waste, trash, debris, or other waste products
- 7. Wastewater from wet saw machinery,
- 8. Other pollutants that degrade water quality or pose a threat to human health or the environment.

Furthermore, I agree to notify Fort Bend County immediately of any issue caused by or identified by:

(Company/Contractor)

that is believed to be an immediate threat to human health or the environment.

Contractor Signature

Date

Printed Name

Title

SCOPE OF WORK

STELLA ROAD

The project scope includes demolishing the existing two-lane asphalt roadway and reconstructing it as a two 12-foot lane asphalt roadway with 6-foot shoulders on either side. The existing open ditch storm drainage is to be regraded and detention is to be provided to help mitigate impacts from the roadway improvements to the drainage outfall locations.

Work includes hot mix asphalt concrete surface course, hot mix asphalt concrete base course, lime stabilized subgrade, grading, driveways, culverts, seeding and sodding, placement of signs, pavement markings, etc.

Contractor is responsible for establishing and maintaining a traffic control plan in accordance to the latest version of Texas Manual on Uniform Traffic Control Devices (TMUTCD) and measures shown in the plans.

This description of the scope of work is general in nature and is intended as an overview of the project only. The complete detailed scope of work and bid items are contained in the construction drawings and specifications.

TECHNICAL SPECIFICATIONS

Technical Specifications are to the latest version of specifications from Harris County Engineering Department, Harris County Flood Control District, and City of Houston. These referenced specifications are incorporated herein as if they are copied verbatim including any supplementary specification, or amendments thereto and related specifications herein unless indicated otherwise in the drawings or specifications. Specifications can be found in the following links:

http://www.eng.hctx.net/Consultants/Standards-Specifications/Standard-Engineering-Design-Specifications

https://www.hcfcd.org/Resources/Technical-Manuals/Standard-Specifications-Related-Drawings?folderId=15985&view=gridview&pageSize=10

https://www.houstonpermittingcenter.org/office-city-engineer/design-and-constructionstandards#agency-links-1476

Stella Road: From Cottonwood School Road to W Fairgrounds Road for Fort Bend County Bid 24-076

INDEX OF TECHNICAL SPECIFICATIONS

Reference Harris County Standard Engineering Design Specifications (2023 revision) where applicable.

Harris County Specifications

Item No.	Specification Title				
100	Preparing Right-of-Way				
100	Existing Fence and Gates				
103	Removing Concrete				
104	Removing Base and Asphalt Pavament				
105	Salvage, Hauling, and Stockpiling Reclaimable Asphalt				
106	Pavament				
108	Removing Structures				
110	Excavation				
130	Borrow				
132	Embankment				
140	Eliminating Existing Pavment Markings and Markers				
160	Topsoil				
162	Sodding for Erosion Control				
164	Seeding for Erosion Control				
166	Fertilizer				
216	Subgrade				
247	Flexible Base				
260	Lime Treatment (Road-Mixed)				
265	Fly Ash or Lime-Fly Ash Treatment (Road-Mixed)				
275	Cement Treatment (Road-Mixed)				
276	Cement Treatment (Plant-Mixed)				
292	Dense-Graded Hot-Mix Asphalt Base Course				
295	Full-Depth Reclamation Using Cement (Road-Mixed)				
206	Full-Depth Reclamation Using Asphalt Emulsion (Road-				
296	Mixed)				
312	Tack Coat				
314	Emulsified Asphalt (Prime Coat)				
340	Dense-Graded Hot-Mix Asphalt				
341	Dense-Graded Hot-Mix Asphalt Surface Course				
358	Hot In-Place Recycling of Asphalt Concrete Surfaces				
360	Concrete Pavement				
361	Repair of Concrete Pavement				
400	Excavation and Backfill for Strcutures				
401	Flowable Backfill				
402	Trench Excavation Protection				
404	Driving Piling				
407	Temporary Steel Sheet Piling				
409	Prestressed Concrete Piling				
416	Drilled Shaft Foundations				
420	Concrete Structures				
421	Hydraulic Cement Structures				
424	Prestressed Concrete Members (Fabrication)				
425	Precast Prestressed Concrete Structural Members				
425	Bridge Plaque				
432	RipRap				
434	Elastomeric Materials				
438	Joint Sealants				
440	Reinforcement for Concrete				
441	Steel Structures				
442	Metal for Structures				
446	Painting and Protective Coating				
447	Structural Bolting				
448	Structural Field Welding				
450	Railing				
451	Retrofit Railing				
462	Reinforced Concrete Box Culverts				
464	Reinforced Concrete Pipe				
465	Concrete Manholes and Junction Boxes				
466	Inlets				
467	Safety End Treatment				
476	Jacking, Boring, or Tunneling Pipe or Box				
479	Adjusting Monholes, Junction Boxes, and Inlets				
-	, , , , , , , , , , , , , , , , , , , ,				

Item No.	Specification Title		
481	Pipe for Drains		
482	Thermoplastic Pipe Culverts and Drains		
501	Tree protection and Trimming		
502	Traffic Signs, Roadside Signs, and Mailboxes		
520 Weighing and Measurement Equipment			
528	Colored Concrete for Median Noses		
530	Concrete Curb, Concrete Curb and Gutter, Sidewalks and Driveways		
536	Concrete Medians and Directional Islands		
540	Metal Beam Guard Fence		
550	Chain Link Fencing		
552	Wire Fence		
554	Wood Fence		
556	Construction Safety Fence		
560	Project Site Cleanup and Maintenance		
561	Project Site Documentation		
580	Precast Concrete Wheel Stop		
590	Notice of Intent		
550	Temporary Erosion, Sedimentation, and		
591	Enironmental Controls		
610	Roadway Illumination Assemblies		
618	Conduit		
618			
	Zinc-Coated Steel Wire Strand		
627	Treated Timber Poles		
636	Signs		
644	Roadside Sign Supports		
658	Delineators and Object Markers		
662	Work Zone Pavement Markings		
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668	Prefabricated Pavement Markers		
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699	Temporary Polyethylene Water-Filled Barrier		
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800	Hydraulic Cement		
801	Fly Ash		
802	Membrane Curing		
820	Lime and Lime Slurry		
821	Fly Ash for Soil Treatment		
860	Sign Face Materials		
861	Traffic Paint (Solvent Based)		
862	Glass Reflective Spheres for Traffic Paint		
863	Twelve Inch LED Traffic Signal Lamp Unit		
864	Pedestrian LED Traffic Signal Lamp Unit		
865	Flasher Assemblies		
866	LED Sign Lights for Traffic Signals		

Stella Road: From Cottonwood School Road to W Fairgrounds Road for Fort Bend County Bid 24-076

INDEX OF TECHNICAL SPECIFICATIONS (CONTINUED)

City of Houston Standard Specifications

For water line construction of this project, the contractor shall reference the City of Houston, Department of Public Works and Engineering, <u>STANDARD CONSTRUCTION SPECIFICATIONS</u> (2021 revision), when identified in the Bid Form specification reference as "COH"

Item No.	Specification Title
02511	Water Lines
02512	Water Tap and Service Line Installation
02513	Wet Connections
02514	Disinfection of Water Lines
02515	Hydrostatic Testing of Pipelines
02516	Cut, Plug, and Abandonment of Mains
02517	Water Line in Tunnels
02520	Fire Hydrant
02521	Gate Valves
02525	Tapping Sleeves and Valves
02526	Water Meters
02527	Polyurethane Coatings on Steel or Ductile Iron Pipe
02528	Polyethylene Wrap
02529	Tape Coatings on Steel Pipe

Harris County Flood Control District Specifications

Reference 2020 Standard Construction Specifications and Details for Harris County Flood Control District where applicable when identified in the Bid Form specification reference as "HCFCD"

Item No.	Specification Title		
02120	Material Disposal		
02316	Structural Excavating and Backfilling		
02321	Cement Stabilized Sand		
02922	Sod		
03310	Concrete		

Other specifications to be listed as applicable.

Geotechnical Investigation (included herein)





GEOTECHNICAL INVESTIGATION

STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD FORT BEND COUNTY, TEXAS

Reported to: McDonough Engineering Corporation Houston, Texas

by

Aviles Engineering Corporation 5790 Windfern Houston, Texas 77041 713-895-7645

REPORT NO. G126-21 (Revision 1)

August 2022



August 29, 2022

Austin McLean, P.E. Project Manager McDonough Engineering Corporation 5625 Schumacher Lane Houston, Texas 77057

Reference: Geotechnical Investigation Stella Road Improvements From Cottonwood School Road to Band Road Fort Bend County Mobility Bond Project #21060 Fort Bend County, Texas AEC Report No. G126-21 (Revision 1)

Dear Mr. McLean,

Aviles Engineering Corporation (AEC) is pleased to present this report of the results of our geotechnical investigation for the above referenced project. Project terms and conditions were in accordance with the Professional Services Consultant Agreement between McDonough Engineering Corporation (MEC) and AEC, dated May 13, 2021. The project scope of services is in accordance with AEC Proposal No. G2021-03-02R2, dated March 31, 2021, and AEC Proposal G2021-03-02SR, dated January 14, 2022.

AEC appreciates the opportunity to be of service to you. Please call us if you have any questions or comments concerning this report or when we can be of further assistance.

Respectfully submitted, *Aviles Engineering Corporation* (TBPELS Firm Registration No. F-42)

1

1

Wilber L. Wang, P.E. Senior Engineer

Senior Engineer



08/29/2022

Reports Submitted:

McDonough Engineering Corporation (electronic) File (electronic)

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EXECUTIVE SUMMARY

The report submitted herein presents the results of Aviles Engineering Corporation's (AEC) geotechnical investigation for Fort Bend County's (FBC) proposed Stella Road Improvements from Cottonwood School Road to Band Road project in Precinct 1, Fort Bend County, Texas (Fort Bend County Key Map Nos.: 604X, and 644 B & C). A project vicinity map is presented on Plate A-1, in Appendix A. According to the information provided by McDonough Engineering Corporation (MEC), Stella Road along the project alignment is an existing 2 lane (one lane in each direction) asphalt roadway, which will be reconstructed to a new 2 lane (one lane in each direction) asphalt roadway with the main traffic lanes widened to 12 feet plus new 6 foot wide shoulders. The roadway will have roadside drainage swales. There will be a detention basin with a depth of 3.4 to 4.3 feet and slope inclination of H:V = 4:1 at the approximate mid-point of the alignment.

AEC notes that the project alignment has been updated after AEC completed its original borings (Borings B-1 through B-17). At its west limit, Stella Road will connect to Cottonwood School Road approximately 400 feet to the south from its current location. The east limit of the project has also changed from Band Road to W. Fairgrounds Road. The S-curve portion of the alignment from approximately Station 25+00 to 48+00 no longer follows the existing Stella Road alignment. After the alignment was updated, additional borings (Borings B-18 and B-19) were drilled along the new roadway alignment in March 2022, and detention basin borings (Borings B-20 through B-23) were drilled in May 2022.

- 1. <u>Existing Pavement Conditions:</u> In general, approximately 3.25 to 7 inches of asphalt surface with 5 to 15 inches of sand and gravel base was encountered along Stella Road. A summary of existing pavement sections encountered in AEC's borings is presented on Table 4 in Section 4.0 of this report.
- 2. Subsurface Soil Conditions: Details of the soils encountered during drilling are presented in the boring logs (see Plates A-3 through A-25, in Appendix A). Generalized soil profiles along the roadway alignment are presented on Plates B-1a through B-1c, in Appendix B. Based on Borings B-1 through B-19, the subsurface conditions along the project alignment generally consist of soft to hard lean/fat clay (CL/CH), including fill. Approximately 1 to 3 feet of clayey sand (SC) fill material was encountered at the ground surface in Borings B-2, B-5, B-10, B-13, and B-21. Granular soils such as loose to very dense silty sands (SM) and medium dense to very dense silts (ML) were encountered beginning from a depth of 14 to 16 feet down to the boring termination depths in Borings B-1, B-2, B-9, B-11 through B-13, B-15 through B-17, B-19, and B-22.
- 3. <u>Subsurface Soil Properties:</u> The subsurface clayey soils (including fill but excluding clayey sand) encountered in the borings have medium to very high plasticity (see "Degree of Plasticity of Cohesive Soils" on Plate A-27, in Appendix A), with liquid limits (LL) ranging from 27 to 98, and plasticity indices (PI) ranging from 11 to 69. The cohesive soils encountered are classified as "CL", "CL-ML", and "CH" type soils, and granular soils encountered are classified as "ML", "SC", "SC-SM", and "SM" type soils in accordance with ASTM D 2487.
- 4. <u>Groundwater Conditions:</u> Groundwater was not encountered in any of the boreholes during or upon completion of drilling.
- 5. <u>Hazardous Materials</u>: No signs of visual staining or odors were encountered during field drilling or during processing of the soil samples in the laboratory.



EXECUTIVE SUMMARY (Cont.)

- 6. <u>Geologic Conditions:</u> AEC performed a desk top fault study which included a review of public maps, available literature, and aerial photographs. AEC reviewed a University of Houston Master's Thesis entitled A Geophysical Study of Active Faulting in Fort Bend County, Texas concerning faulting in Fort Bend County, Texas (Schmidt, May 2013). According to the thesis, the closest fault to the project alignment is the northeast-southwest oriented Pleak Fault located approximately 3.9 miles south of the southeastern end of the project alignment. This fault is too distant to have an impact on the project alignment. Additional fault study is not recommended.
- 7. <u>Roadway Reconstruction</u>: Recommendations for reconstructing the existing asphalt pavement roadway are presented in Section 5.1 of this report. As directed, AEC first considered an asphalt pavement section that meets Fort Bend County's minimum pavement thickness requirements, which includes a 3 inch asphalt surface, 8 inch asphalt stabilized base, and 8 inch thick stabilized subgrade. Based on available traffic data, AEC estimates that FBC's minimum pavement section would provide a service life of approximately 8 years. AEC estimates an asphalt pavement section consisting of 3.5 inch asphalt surface, 10 inch asphalt-stabilized base, and 8 inch thick stabilized subgrade will be needed to provide a 20 year service life. Based on our borings, AEC estimates that most of the new roadway subgrade will be within highly expansive fat clay (CH) soil, which will require stabilization with a minimum of 7 percent lime. However, clayey sand (SC) fill material (likely placed when the original roadway was constructed) was encountered at the ground surface in Borings B-2 and B-5. Where sandy soils are exposed at the ground surface, the subgrade should be stabilized with a minimum of 3 percent lime and 7 percent fly ash.
- 8. <u>Detention Basin</u>: Recommendations for the detention basin is presented in Section 5.2 of this report. Based on Borings B-22 and B-23, the detention basin excavation will encounter very stiff to hard fat clay (CH). Groundwater was not encountered in the borings during drilling. The calculated minimum factor of safety (FS) for slope stability of the southeast bank of the detention basin ranges from 17.66 to 19.43 for short term condition, ranges from 1.85 to 2.22 for long term condition, and ranges from 1.31 to 1.52 for rapid drawdown condition. None of the soil excavated from the detention basin can be reused as select clay fill.

This Executive Summary is intended as a summary of the investigation and should not be used without the full text of this report.



GEOTECHNICAL INVESTIGATION

STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD FORT BEND COUNTY, TEXAS

1.0 INTRODUCTION

The contents of this revised report (G126-21 Revision 1, dated August 29. 2022) prepared by Aviles Engineering Corporation (AEC) supersede AEC's geotechnical investigation report (G126-21, dated May 9, 2022) previously issued for this project.

1.1 General

The report submitted herein presents the results of AEC's geotechnical investigation for Fort Bend County's (FBC) proposed Stella Road Improvements from Cottonwood School Road to Band Road project in Precinct 1, Fort Bend County, Texas (Fort Bend County Key Map Nos.: 604X, and 644 B & C). A project vicinity map is presented on Plate A-1, in Appendix A. According to the information provided by McDonough Engineering Corporation (MEC), Stella Road along the project alignment is an existing 2 lane (one lane in each direction) asphalt roadway, which will be reconstructed to a new 2 lane (one lane in each direction) asphalt roadway, which will be a detention basin with a depth of 3.4 to 4.3 feet and slope inclination of H:V = 4:1 at the approximate mid-point of the alignment.

AEC notes that the project alignment has been updated after AEC completed its original borings (Borings B-1 through B-17). At its west limit, Stella Road will connect to Cottonwood School Road approximately 400 feet to the south from its current location. The east limit of the project has also changed from Band Road to W. Fairgrounds Road. The S-curve portion of the alignment from approximately Station 25+00 to 48+00 no longer follows the existing Stella Road alignment. After the alignment was updated, additional borings (Borings B-18 and B-19) were drilled along the new roadway alignment in March 2022, and detention basin borings (Borings B-20 through B-23) were drilled in May 2022.



1.2 Purpose and Scope

The purpose of this geotechnical investigation is to evaluate the subsurface soil and groundwater conditions along the project alignment and develop geotechnical engineering recommendations for design and construction of asphalt pavement and the detention basin. The scope of this geotechnical investigation is summarized below:

- 1. Drilling and sampling twenty-three geotechnical borings ranging from 15 to 20 feet below existing grade.
- 2. Soil laboratory testing on selected soil samples.
- 3. Engineering analyses and recommendations for reconstruction of roadways with asphalt pavement, including pavement thickness design and subgrade preparation.
- 4. Engineering analyses and recommendations for the detention basin, including slope stability analysis, erosion protection requirements (if required), and evaluation of excavated soil for use as select fill.
- 5. Construction recommendations and groundwater control guidelines for the proposed roadway and detention basin.

2.0 <u>SUBSURFACE EXPLORATION</u>

2.1 Soil Borings

Boring spacing and depths were selected in general accordance with Chapter 8 of the August 2020 FBC Engineering Design Manual (Draft). AEC drilled a total of twenty-three soil borings (Borings B-1 through B-23) for the project improvements to depths ranging from 15 to 20 feet below existing grade. Boring locations were marked by AEC personnel in the field using a handheld GPS unit. The total drilling footage is 440 feet. After completion of drilling, the locations of Borings B-1 through B-19 were surveyed by Landtech, Inc. The locations of Borings B-20 through B-23 were not surveyed; AEC estimated the coordinates of the borings from the handheld GPS unit, and estimated the boring elevations from available topographic data in the drawings provided by MEC. The boring locations are shown on the Boring Location Plan on Plate A-2, in Appendix A. Boring survey data (in State Plane Grid Coordinates, Texas South Central Zone 4204, US Survey Feet) is summarized on Table 1 below and is also included on the representative boring logs.



Boring No.	Boring Depth (ft)	Northing ⁽¹⁾ (Grid, ft)	Easting ⁽¹⁾ (Grid, ft)	Boring Surface Elevation (ft)	Station (Stella Rd Baseline)	Offset (ft)
B- 1	20	13,752,613.63	2,977,451.48	99.65	3+13.50	-29.76
B-2	20	13,752,122.69	2,977,409.05	97.70	7+91.95	-4.35
B-3	20	13,751,769.18	2,977,721.50	97.86	12+85.68	-5.67
B-4	20	13,751,439.93	2,978,079.39	97.58	17+72.04	-6.05
B-5	20	13,751,103.01	2,978,427.64	97.30	22+56.53	5.76
B-6	20	13,750,779.58	2,978,795.46	96.28	27+46.25	-5.63
B-7	20	13,750,466,84	2,979,177.94	96.38	32+44.94	-6.86
B-8	20	13,750,519.24	2,979,671.65	96.64	37+71.16	-6.59
B-9	20	13,750,858.76	2,980,018.92	96.11	42+58.29	7.52
B-10	20	13,751,072.08	2,980,400.76	96.26	47+14.00	6.22
B-11	20	13,750,890.73	2,980,828.65	96.32	52+00.53	7.47
B-12	20	13,750,586.11	2,981,181.36	96.07	56+66.37	-8.33
B-13	20	13,750,262.50	2,981,532.35	95.97	61+43.85	-9.00
B-14	20	13,749,931.30	2,981,888.21	95.67	66+30.04	-7.41
B-15	20	13,749,608.87	2,982,234.96	95.35	71+03.59	-6.08
B-16	20	13,749,294.44	2,982,576.06	95.16	75+67.57	-6.79
B-17	20	13,748,987.65	2,982,888.19	94.52	80+05.09	6.57
B-18	20	13,750,595.24	2,979,443.158	93.80	31+30.51	-6.82
B-19	20	13,750,882.72	2,980,368.432	94.19	41+31.40	73.75
B-20 ⁽²⁾	15	13,751,108.27	2,980,993.076	98	-	-
B-21 ⁽²⁾	15	13,750,814.12	2,981,100.064	97	-	-
B-22 ⁽²⁾	16	13,750,851.89	2,979,703.639	95	-	-
B-23 ⁽²⁾	15	13,750,724.23	2,979,319.16	95	_	-

Table 1. Summary of Boring Survey Data

Note: (1) Northing and easting coordinates referenced to Texas Coordinate System, South Central Zone 4204. Coordinates are provided in grid format.

(2) Boring location not surveyed. Coordinates and elevations are estimated.

2.2 Drilling and Sampling Methods

Prior to drilling, existing pavement at Borings B-1, B-4, B-8, B-11, and B-14 through B-17 were first cut with a core barrel. Borings were drilled using either a truck-mounted (Borings B-1 through B-17) or buggy-



mounted (Borings B-18 through B-23) drilling rig and advanced using dry auger method alone. Undisturbed samples of cohesive soils were obtained from the borings by pushing 3-inch diameter thin-wall, seamless steel Shelby tube samplers in general accordance with ASTM D 1587. Granular soils were sampled with a 2-inch split-barrel sampler in accordance with ASTM D 1586. Standard Penetration Test resistance (N) values were recorded for the granular soils as "Blows per Foot" and are shown on the boring logs. Strength of the cohesive soils was estimated in the field using a hand penetrometer. The undisturbed samples of cohesive soils were sealed in plastic bags to reduce moisture loss and disturbance. The samples were then placed in core boxes and transported to the AEC laboratory for testing and further study. Groundwater was not encountered in any of the borings during or after completion of drilling. After completion of drilling, boreholes located on existing pavement were grouted with cement-bentonite grout and existing pavement was patched with cold-placed asphalt patch. Boreholes located on grass were backfilled with bentonite chips.

3.0 LABORATORY TESTING PROGRAM

Soil laboratory testing was performed by AEC personnel. Samples from the borings were examined and classified in the laboratory by a technician under the supervision of a geotechnical engineer. Laboratory tests were performed on selected soil samples to evaluate the engineering properties of the foundation soils in accordance with applicable ASTM Standards. Atterberg limits, moisture contents, percent passing a No. 200 sieve, and dry unit weight tests were performed on selected samples to establish the index properties and confirm field classification of the subsurface soils. Strength properties of cohesive soils were determined by means of torvane (TV), unconfined compression (UC), unconsolidated-undrained (UU), and consolidated-undrained (CU) triaxial tests performed on relatively undisturbed samples. The laboratory test results are presented on the representative boring logs (see Plates A-3 through A-25, in Appendix A). A key to the boring logs, classification of soils for engineering purposes, terms used on boring logs, and reference ASTM Standards for laboratory testing are presented on Plates A-26 through A-29, in Appendix A.

<u>Crumb Dispersion Tests</u>: To evaluate the dispersive characteristics of clayey soils in the detention basin, crumb tests were performed on selected soil samples in accordance with ASTM D 6572, Method A. The results of the crumb tests are summarized on Table 2 and are presented on Plate A-30, in Appendix A.



Sample ID and Description	Dispersive Grade	Dispersive Classification
B-20, 0'-2', Fill: Fat Clay (CH)	1	Non-dispersive
B-20, 6'-8', Fat Clay (CH)	1	Non-dispersive
B-21, 0'-2', Fill: Fat Clay (CH)	1	Non-dispersive
B-21, 8'-10', Fat Clay (CH)	1	Non-dispersive
B-22, 2'-4', Fat Clay (CH)	1	Non-dispersive
B-22, 6'-8', Fat Clay (CH)	1	Non-dispersive
B-23, 0'-2', Fat Clay (CH)	1	Non-dispersive
B-23, 6'-8', Fat Clay (CH)	1	Non-dispersive

 Table 2.
 Summary of Crumb Test Results

<u>Consolidated-Undrained Triaxial Tests</u>: CU triaxial tests were performed in accordance with ASTM D 4767 to determine shear strength parameters of the soils in the detention basin. Using the CU data, AEC plotted the stress paths and determined the k_f (critical state) line from the stress paths in accordance with the US Army Corps of Engineers Engineering Manual, Appendix D, Section D-4. Based on the k_f line, AEC determined the strength parameters (cohesion and friction angle) of the soil. Mohr's circles were developed based on the failure criteria (either maximum effective stress obliquity or maximum deviator stress) presented in ASTM D 4767. The Mohr-Coulomb diagrams (with Mohr's Circles at failure) generated from the CU triaxial tests are included on Plates A-31 and A-32, in Appendix A. The shear strength parameters obtained from the CU triaxial tests are summarized below in Table 3.

Sample ID and Description	Effectiv	ve Stress	Total Stress	
Sample ID and Description	c' (psf)	φ' (deg)	c _{cu} (psf)	φ _{cu} (deg)
B-21, 6'-8', Fat Clay (CH)	480	12.5	450	9.7
B-23, 4'-6', Fat Clay (CH)	250	17.7	230	13.4

Table 3. Summary of Shear Strength Parameters from CU Triaxial Tests

Notes: (1) c' = effective cohesion, φ' =effective friction angle, obtained from CU tests with pore pressure measurements. (2) c_{cu} = cohesion in total stress, φ_{cu} = friction angle in total stress, obtained from CU tests.

4.0 <u>SITE CONDITIONS</u>

The existing roadway along the project alignment is a two-lane (one lane in each direction) asphalt roadway (no shoulder) with roadside drainage swales. During our site visit, AEC observed that the existing roadway



between W. Fairground Road and Band Road appears to be in average to good condition, while the roadway between W. Fairground Road and Cottonwood School Road is generally in poor condition. For the segment between W. Fairground Road and Cottonwood School Road, AEC observed numerous roadway longitudinal cracks, occasional transverse cracks, alligator cracking, and rutting in wheel paths AEC also observed asphalt patches at several areas along the roadway. A summary of existing pavement sections encountered in AEC's borings is presented on Table 4 below.

Boring No.	Street	Pavement Section
B-1	Stella Road	5.5" asphalt, 6" gravel base, and 7.75" stabilized sand subbase
B-2	Stella Road	3.5" asphalt, 8.5" stabilized sand and gravel base
B-3	Stella Road	4.5" asphalt, 14" stabilized sand and gravel base
B-4	Stella Road	3.5" asphalt, 6" sand and gravel base, 2.5" asphalt, and 7.5" sand and gravel subbase
B-5	Stella Road	4.5" asphalt, 15" stabilized sand and gravel base
B-6	Stella Road	7" asphalt, 5" stabilized sand and gravel base
B-7	Stella Road	5" asphalt, 12" stabilized sand and gravel base
B-8	8 Stella Road 4" asphalt, 14.5" stabilized sand base	
B-9	B-9 Stella Road 4" asphalt, 10" stabilized sand and gravel base	
B-10	Stella Road 4" asphalt, 8" stabilized sand and gravel base	
B-11	Stella Road	3.5" asphalt, 5.5" stabilized sand and gravel base, and 10" sand and gravel subbase
B-12	Stella Road	4" asphalt, 8" stabilized sand and gravel base
B-13	Stella Road	4" asphalt, 8" stabilized sand and gravel base
B-14	Stella Road	3.5" asphalt, 13.5" sand and gravel base
B-15	Stella Road	4" asphalt, 14" sand and gravel base
B-16	Stella Road	3.5" asphalt, 12.5" sand and gravel base
B-17 Stella Road 3.25" asphalt, 14.25" sand and gravel base		3.25" asphalt, 14.25" sand and gravel base

Table 4.	Summary o	of Existing	Pavement	Thickness
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4.1 Subsurface Conditions

Details of the soils encountered during drilling are presented in the boring logs on Plates A-3 through A-25, in Appendix A. Soil strata encountered in the borings are summarized below. Boring log profiles along the project alignment are presented on Plates B-1a through B-1c, in Appendix B.



Boring B-1	<u>Depth (ft)</u> 0 - 1.6 1.6 - 12 12 - 16 16 - 20	<u>Description of Stratum</u> Pavement and base: see Table 4 in Section 4.0 of this report. Soft to very stiff, Fat Clay (CH), with slickensides Soft to very stiff, Lean Clay (CL), with calcareous nodules Loose to medium dense, Silty Sand (SM), wet
B-2	0 - 1 1 - 2 2 - 8 8 - 10 10 - 12	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Stiff to very stiff, Fat Clay (CH), with ferrous nodules Very stiff, Lean Clay (CL), with fat clay and calcareous powder pockets, and calcareous and ferrous nodules Very stiff, Silty Clay (CL-ML), with lean clay pockets, calcareous nodules, and ferrous stains
	12 - 14 14 - 16	Very stiff, Sandy Lean Clay (CL), with sandy silt partings, silty clay pockets, calcareous nodules, and ferrous stains Sandy Silt (ML), with lean clay pockets, calcareous nodules, and ferrous
	16 - 18	stains Firm to very stiff, Lean Clay (CL), with silt and calcareous powder pockets, and calcareous nodules
	18 - 20	Stiff to very stiff, Fat Clay (CH), with slickensides, silt pockets and calcareous nodules
B-3	0 - 1.5 1.5 - 2 2 - 6 6 - 10 10 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Fill: stiff to very stiff, Fat Clay (CH), with gravel Very stiff, Fat Clay (CH), with slickensides Very stiff to hard, Lean Clay (CL), with calcareous nodules, pockets, and seams Stiff to hard, Fat Clay (CH), with slickensides
B-4	0 - 1.6 1.6 - 12 12 - 14 14 - 16 16 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Very stiff to hard, Fat Clay (CH) Very stiff, Silty Clay (CL-ML), with ferrous and calcareous nodules, and fat clay pockets Very stiff, Lean Clay (CL), with calcareous nodules and powder pockets Very stiff to hard, Fat Clay (CH), with slickensides
B-5	0 - 1.6 1.6 - 4 4 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Silty Clayey Sand (SC-SM), with gravel Stiff to hard, Fat Clay (CH) Hard, Lean Clay (CL), with calcareous nodules and powder pockets Very stiff to hard, Fat Clay (CH), with slickensides
B-6	0 - 1 1 - 4 4 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: stiff to very stiff, Lean Clay (CL), with gravel Stiff to hard, Fat Clay (CH), with slickensides.



<u>Boring</u> B-7	<u>Depth (ft)</u> 0 - 1.4 1.4 - 2 2 - 4 4 - 20	<u>Description of Stratum</u> Pavement and base: see Table 4 in Section 4.0 of this report. Fill: hard, Sandy Lean Clay (CL), with gravel Fill: very stiff, Fat Clay (CH), with gravel and lean clay seams Stiff to hard, Fat Clay (CH), with slickensides
B-8	0 - 1.5 1.5 - 2 2 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: stiff to very stiff, Fat Clay (CH), with gravel and sand pockets Stiff to hard, Fat Clay (CH), with slickensides
B-9	0 - 1.2 1.2 - 2 2 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: stiff, Fat Clay (CH), with gravel and lean clay pockets Stiff to very stiff, Fat Clay (CH), with slickensides Very stiff, Lean Clay (CL), with sandy silt partings Medium dense to dense, Sandy Silt (ML)
B-10	0 - 1 1 - 4 4 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Stiff to hard, Fat Clay (CH), with slickensides
B-11	0 - 1.6 1.6 - 2 2 - 18 18 - 20	Pavement: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Stiff to hard, Fat Clay (CH), with slickensides Dense, Sandy Silt (ML), with lean clay pockets and siltstone nodules
B-12	0 - 1 1 - 2 2 - 16 16 - 20	Pavement: see Table 4 in Section 4.0 of this report. Fill: stiff to very stiff, Fat Clay (CH), with sandy lean clay seams and gravel Stiff to hard, Fat Clay (CH), with slickensides Dense to very dense, Sandy Silt (ML)
B-13	0 - 1 1 - 4 4 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: Clayey Sand (SC), with gravel Firm to hard, Fat Clay (CH), with slickensides Very dense, Sandy Silt (ML), with siltstone nodules
B-14	0 - 1.4 1.4 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Stiff to hard, Fat Clay (CH), with slickensides
B-15	0 - 1.5 1.5 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Stiff to very stiff, Fat Clay (CH), with slickensides Stiff to hard, Lean Clay (CL), with sandy silt seams Dense to very dense, Silt with Sand (ML)
B-16	0 - 1.3 1.3 - 2 2 - 12 12 - 14 14 - 20	Pavement and base: see Table 4 in Section 4.0 of this report. Fill: very stiff, Fat Clay (CH), with gravel, sandy lean clay seams, and asphalt pieces Stiff to very stiff, Fat Clay (CH), with slickensides Stiff to very stiff, Lean Clay (CL), with calcareous nodules and silt partings Medium dense, Silt with Sand (ML)
		8



Boring B-17	Depth (ft) 0 - 1.5 1.5 - 2 2 - 10 10 - 12 12 - 14 14 - 20	 <u>Description of Stratum</u> Pavement and base: see Table 4 in Section 4.0 of this report. Fill: very stiff, Sandy Fat Clay (CH), with gravel seams and shell fragments Stiff to very stiff, Fat Clay (CH), with slickensides Silt with Sand (ML) Stiff to very stiff, Lean Clay (CL), with silt seams Medium dense, Silt with Sand (ML)
B-18	0 - 20	Firm to hard, Fat Clay (CH), with slickensides
B-19	0 - 14 14 - 16 16 - 20	Stiff to hard, Fat Clay (CH), with slickensides Stiff, Lean Clay (CL), with silt partings and siltstone nodules Very dense, Silty Sand (SM), with cement sand nodules
B-20	0 - 4 4 - 14 14 - 15	Fill: hard, Fat Clay (CH), with silty sand pockets and gravel Stiff to hard, Fat Clay (CH), with slickensides Hard, Lean Clay (CL), with slickensides and ferrous nodules
B-21	0 - 2 2 - 4 4 - 15	Fill: hard, Fat Clay (CH), with calcareous nodules and roots Fill: Clayey Sand (SC), with gravel and asphalt pieces Stiff to hard, Fat Clay (CH), with slickensides and ferrous nodules
B-22	0 - 14 14 - 16	Very stiff to hard, Fat Clay (CH), with slickensides and ferrous nodules Very dense, Silt (ML), with sandy lean clay pockets
B-23	0 - 15	Stiff to hard, Fat Clay (CH), with slickensides and ferrous nodules

<u>Subsurface Soil Properties:</u> The subsurface clayey soils (including fill but excluding clayey sand) encountered in the borings have medium to very high plasticity (see "Degree of Plasticity of Cohesive Soils" on Plate A-27, in Appendix A), with liquid limits (LL) ranging from 27 to 98, and plasticity indices (PI) ranging from 11 to 69. The cohesive soils encountered are classified as "CL", "CL-ML", and "CH" type soils, and granular soils encountered are classified as "ML", "SC", "SC-SM", and "SM" type soils in accordance with ASTM D 2487. "CH" soils undergo significant volume changes due to seasonal changes in soil moisture contents. "CL" type soils with lower LL (less than 40) and PI (less than 20) generally do not undergo significant volume changes with changes in moisture content. However, "CL" soils with LL approaching 50 and PI greater than 20 essentially behave as "CH" soils and could undergo significant volume changes. Slickensides were encountered in most fat clay (CH) and some lean clay (CL) soils.

<u>Groundwater Conditions:</u> Groundwater was not encountered in any of the boreholes during or upon completion of drilling.



The information in this report summarizes conditions found on the dates the borings were drilled. However, it should be noted that our groundwater observations are short-term; groundwater depths and subsurface soil moisture contents will vary with environmental variations such as frequency and magnitude of rainfall and the time of year when construction is in progress.

4.2 Hazardous Materials

No signs of visual staining or odors were encountered during field drilling or during processing of the soil samples in the laboratory. However, AEC notes that the presence of potential hazardous material at other locations along the project alignments cannot be discounted based upon the very small and limited number of samples taken.

4.3 Geologic Hazards

AEC performed a desk top fault study which included a review of public maps, available literature, and aerial photographs. The project alignment is not covered by the maps entitled "*Principal Active Faults of the Houston Area (after O'Neill and Van Siclen, May 1984)*", and "*Principal Surface Faults in the Central Houston Metropolitan Area (after O' Neill, Van Siclen, with additions by C. Norman, May 13, 2004)*". AEC reviewed a University of Houston Master's Thesis entitled *A Geophysical Study of Active Faulting in Fort Bend County, Texas* concerning faulting in Fort Bend County, Texas (Schmidt, May 2013). According to the thesis, the following fault systems are located in Fort Bend County: the Addicks, the Longpoint, the Needville, the Pleak, the Thompsons, and the Arcola, along with some other short unnamed faults in the eastern portion of the county. The closest fault to the project alignment is the northeast-southwest oriented Pleak Fault located approximately 3.9 miles south of the southeastern end of the project alignment. This fault is too distant to have an impact on the project alignment.

Twenty-three Google Earth aerial photographs from 1985 to 2021 were reviewed. No evidence of faulting was observed from the review of the aerial photographs in or near the project alignment.

AEC does not recommend any further fault studies.



<u>Limitations</u>: The preliminary fault study investigation provided in this report is limited to a review of available literature, aerial photographs and maps, and limited field observations. Distances are scaled from maps. Faults may exist in, cross, or adjoin the project area which were not identified in this report due to the following reasons: limitations of the scope of work and cost; lack of documentation in the literature; the scale of the maps available; lack of visible displacement in the field; and not observed during the reconnaissance due to the presence of obscuring vegetation, man-made structures and environmental features, and modification of the land surface by human activities. Faults may also be present below ground but do not currently have surface expressions. Identification of these faults is beyond the scope of work for this study.

4.4 Subsurface Variations

It should be emphasized that: (i) at any given time, groundwater depths can vary from location to location, and (ii) at any given location, groundwater depths can change with time. Groundwater depths will vary with seasonal rainfall and other climatic/environmental events. Subsurface conditions may vary away from and between the boring locations.

Clay soils in the Greater Houston area typically have secondary features such as slickensides, calcareous and ferrous nodules, and contain sand/silt seams/lenses/layers/pockets/partings. It should be noted that the information in the boring logs is based on 3-inch diameter soil samples which were obtained continuously at intervals of 2 feet from the ground surface to the boring termination depths of 15 to 20 feet below grade. A detailed description of the soil secondary features may not have been obtained due to the small sample size and sampling interval between the samples. Therefore, while a boring log shows some soil secondary features, it should not be assumed that the features are absent where not indicated on the boring logs.

5.0 <u>GEOTECHNICAL ENGINEERING RECOMMENDATIONS</u>

According to the information provided by MEC, Stella Road along the project alignment is an existing 2 lane (one lane in each direction) asphalt roadway, which will be reconstructed to a new 2 lane (one lane in each direction) asphalt roadway with the main traffic lanes widened to 12 feet plus new 6 foot wide shoulders. The roadway will have roadside drainage swales. There will be a detention basin with a depth of 3.4 to 4.3 feet and slope inclination of H:V = 4:1 at the approximate mid-point of the alignment.



<u>Design and Construction Standards</u>: AEC performed the roadway design for Stella Road based on the May 2021 (Draft) FBC Engineering Design Manual (EDM). AEC has also referenced applicable Fort Bend County Construction Details (FBCCD) (dated March 1, 2022) wherever applicable for this project. For technical specifications, the FBCCD makes referrals to Harris County Standard Engineering Design Specifications (HCSEDS). AEC should be notified if different construction specifications should be used, so that our recommendations can be updated if necessary.

5.1 Roadway Reconstruction

The existing roadway along the project alignment is a two-lane (one lane in each direction) asphalt roadway (no shoulder) with roadside drainage swales. Based on drawings (dated April 21, 2022) prepared by MEC, the existing roadway will be reconstructed with two lanes (one lane in each direction), with the main lanes widened to 12 feet, plus 6 foot wide shoulders on each side of the roadway. The reconstructed roadway will have roadside drainage swales on both sides. Based on preliminary plan and profile drawings, AEC portions of the centerline of the new pavement will be placed at or near existing grade, although additional fill will need to be placed in some areas, such as within existing roadside swales to accommodate the roadway widening.

Fort Bend County's Engineering Design Manual (Draft) Requirements: Section 3.12 of the May 2021 FBC EDM requires that the minimum pavement structure for permanent asphalt roadways shall consist of a 3 inch asphalt surface, 8 inch asphalt stabilized base, and 8 inch thick stabilized subgrade.

<u>Traffic Volume</u>: AEC checked the FBC Engineering GIS, Texas A&M Transportation Institute (TTI) "Houston Regional Traffic Count Map", and the Texas Department of Transportation (TxDOT) "District Traffic Web Map" websites to determine the classification of the existing roadways within the project area and to check if any traffic count data was available. The FBC GIS website does not list Stella Road as either a 'collector' or 'major thoroughfare' roadway and traffic data for the Stella Road was not available at the time this report was prepared. The TTI and TxDOT websites also do not have traffic data for Stella Road, however it has 24 hour traffic volume counts for Cottonwood School Road with which Stella Road intersects. Traffic data for Cottonwood School Road are shown on Table 5 below.



Traffic Count Location	Year	24 Hour Traffic Volume (vpd)	Source of Traffic Data
1100 Cottonwood School Road	2006 2012	2,940 2,250	TTI
	2021	1,177	TxDOT

Table 5. Summary of Available Traffic Data

Form the traffic volume count from the TTI and TxDOT websites, it appears that the traffic volume on Cottonwood School Drive decreased from 2006 to 2021. However, traffic volume changes hour to hour, from weekdays to weekend, and season to season. AEC cannot validate the traffic volume decrease based on 24 hour volume counts from 2006, 2012, and 2021 unless further data becomes available. In the absence of other data, AEC conservatively used the 2006 traffic count data obtained from the TTI website for analyses. An annual traffic growth rate of 3 percent was assumed. A design life of 20 years was considered, which is typical for asphalt pavement roadways in the area. Based on the 2006 TTI traffic data and the assumed traffic volume growth rate, AEC estimated a 2022 ADT of 4,611 vpd. AEC should be notified if different traffic loads, design parameters, or pavement design life are required for pavement design at the site so that our recommendations can be updated accordingly.

5.1.1 Roadway Design Traffic Load

A traffic analysis was not available at the time this report was prepared and AEC understands that FBC is not planning to perform one for the Stella Road reconstruction project. As noted in Section 5.1 of this report, AEC has estimated roadway design traffic loading based on traffic count data from Cottonwood School Road in 2006, 2012, and 2021. AEC notes that traffic data that is from an adjoining roadway should be considered *unreliable* for estimating the traffic loading on Stella Road. AEC recommends that a current traffic count be performed along the Stella Road alignment, if possible, so that the roadway design traffic load estimate can be updated.

<u>Estimated Traffic Loads</u>: Pavement design is based on the anticipated design number of 18-kip Equivalent Single Axle Loads (ESAL) the pavement is subjected to during its design life. The equation to calculate the number of 18-kip ESAL repetitions to use for pavement design is presented in Equation (1). Assumptions made by AEC to estimate 18-kip ESAL repetitions are presented on Table 6.

18-kip ESAL = (ADT)(T)(T_f)(D)(L)(G)(Y)(365)Equation (1)



where: ESAL = 18-kip Equivalent Single-Axle Load repetitions.

- ADT = Average Daily Traffic, vehicles per day.
- T = Percent of heavy trucks.
- $T_{\rm f}$ = Truck factor.
- D = Directional factor.
- L = Lane factor.
- G = Growth factor.
- Y = Design life, in years.

Table 6. Traffic Volume Parameters for Asphalt Pavement Design

Traffic Parameter	Stella Road from Band Road to Cottonwood School Road	
Average Daily Traffic (ADT) projected for 2022	4,611 vpd (conservatively estimated from 2006 data)	
ADT projected for 2042	8,401 vpd	
ADT Percent Heavy Duty Truck (T), FHWA Class 5 and higher	5% (assumed)	
Truck Factor (T _f)	0.60 (assumed)	
Directional Factor (D)	0.5 (two way road)	
Lane Factor (L)	1.0 (one lane in each direction)	
Total Growth Rate Factor (G)	1.34 (3% annual growth rate from 2022 to 2042, assumed)	
Design Life (Y)	20 years (assumed)	
Estimated 18-kip ESAL Loading over Design Life	678,526	

AEC notes that the calculated number of 18-kip ESAL repetitions is highly sensitive to parameters such as percent heavy trucks, truck factor, and traffic volume growth rate in pavement design. Differences between assumed and actual traffic parameters can have significant effects on overall pavement thickness design and ultimate pavement performance. AEC should be notified if different traffic loads or design parameters are required for pavement design at the site so that our analysis can be updated accordingly.

5.1.2 Asphalt Pavement Design

Flexible pavement design procedure includes determination of the structural number (SN) for the proposed pavement, as well as the thickness of individual components of the surface course, base course, and subgrade. The basic equation developed by the AASHTO Road Test is:



 $SN = a_1(D_1) + a_2(D_2) + a_3(D_3)$ Equation (2)

where: SN = Structural Number for the total flexible pavement structure. a_1, a_2, a_3 = layer coefficients for surface, base, and subgrade course, respectively. D_1, D_2, D_3 = thickness of surface, base, and subgrade course, respectively, in inches.

Layer coefficients used for design are presented on Table 7.

Pavement Layer	Layer Coefficient	
Hot Mix Asphaltic Concrete (HMAC)	a1 = 0.44	
Black Base	a2 = 0.34	
Stabilized Subgrade*	a3 = 0.11	

Table 7. Layer Coefficients for Asphalt Pavements

Note: (*) Subgrade stabilization recommendations are presented in Section 5.1.4 of this report.

The parameters that were used in computing the flexible pavement for the Stella Road are as follows:

Roadbed Soil Resilient Modulus (M _R)	1,500 psi
Drainage Coefficient (C _d)	1.0
Overall Standard Deviation (S_0)	0.45
Reliability Level (R)	90%
Initial Serviceability (P ₀)	4.2
Terminal Serviceability (Pt)	2.5

AEC should be notified if different parameters are required for asphalt pavement design. As directed by MEC, AEC performed designs for two asphalt pavement sections. AEC first considered a pavement section that meets the FBC EDM minimum requirements and then estimated a service life based on the design roadway traffic load presented on Table 6 in Section 5.1.1 of this report. AEC then considered a pavement section that will provide a load capacity that meets the estimated 20 year design life roadway traffic load that was presented in Table 6. The pavement section that meets FBC EDM minimum thickness requirements is presented on Table 8.

Table 8. Asphalt Pavement Section based on FBC Minimum Thickness Requirement

Pavement Layer	Stella Road from Band Road to Cottonwood School Road	
Hot Mix Asphaltic Concrete	3"	



Pavement Layer	Stella Road from Band Road to Cottonwood School Road
Black Base (BB)	8"
Stabilized Subgrade*	8"
Structural Number (SN)	4.92
Pavement 18-kip ESAL Load Capacity	226,828
Estimated 18-kip ESAL Loading over Design Life (See Table 6)	678,526
Estimated Service Life	8 years

Note: (*) Subgrade stabilization recommendations are presented in Section 5.1.4 of this report.

Based on the DARWin v3.0 computer program, the pavement section presented in Table 8 should sustain 226,828 repetitions of 18-kip ESALs. As presented on Table 6 in Section 5.1.1 of this report, AEC estimated a traffic loading of 678,526 18-kip ESALs over a 20 year design life. Based on AEC's current assumptions, AEC estimates that the service life of the FBC EDM minimum pavement section would be reached in about 8 years. As noted in Section 5.1.1 of this report, AEC notes that our estimate of service life is based on traffic data from an adjacent roadway, as well as assumptions made without supporting data being available. The service life presented by AEC should therefore be considered a very rough estimate.

As noted above, AEC performed a second pavement design which provides a load capacity that meets the estimated traffic loading of 678,526 18-kip ESALs over a 20 year design life. The pavement section that meets AEC's estimated design loading is provided on Table 9 below.

Pavement Layer	Stella Road from Band Road to Cottonwood School Road
Hot Mix Asphaltic Concrete	3.5"
Black Base (BB)	10"
Stabilized Subgrade*	8"
Structural Number (SN)	5.82

Table 9. Recommended Asphalt Pavement Section based on 20 year Design Life



Pavement Layer	Stella Road from Band Road to Cottonwood School Road	
Pavement 18-kip ESAL Load Capacity	760,267	
Estimated 18-kip ESAL Loading over Design Life (See Table 6)	678,526	

Note: (*) Subgrade stabilization recommendations are presented in Section 5.1.4 of this report.

Based on the DARWin v3.0 computer program, the pavement section presented on Table 9, should provide an estimated load capacity of 760,267 18-kip ESALs, which exceeds the estimated 20 year design traffic load. AEC notes that MEC's drawings (dated April 21, 2022) show a proposed pavement section that matches the section presented in Table 9 above.

The DARWin outputs for asphalt pavement are presented on Plate C-1 through C-3, in Appendix C, for reference. The design engineer should verify whether the proposed pavement section will provide enough ESALs for the anticipated amount of site traffic. AEC should be notified if different standards or constants are required for pavement design at the site, so that our recommendations can be updated accordingly.

<u>Asphalt Pavement:</u> HMAC pavement should be constructed in general accordance with Item 340 of the 2017 HCSEDS. The HMAC shall be placed and compacted in accordance with 'Compaction Criteria' requirements of Section 340.14 of HCSEDS Item 340, which is 3 to 8 percent air voids as determined by TxDOT test method Tex-207-F, Part III.

<u>Black Base:</u> Asphalt-stabilized (black) base shall be in accordance with Item 250 of the 2017 HCSEDS. The black base shall be placed and compacted in accordance with 'Compaction Criteria' requirements of Section 250.12 of HCSEDS Item 250, which is 3 to 8 percent air voids as determined by TxDOT test method Tex-207-F, Part III.

<u>Prime Coat:</u> The surface of the completed subgrade and base course should be primed in accordance with Item 310 of the 2017 HCSEDS.

5.1.3 <u>Roadway Fill</u>

For roadway areas that require fill to achieve final grade, existing pavement and base (if any), vegetation, trees, roots, organic soils, and other deleterious materials should first be removed and wasted. The exposed



soils should then be proof-rolled in accordance with Item 216 of the 2014 TxDOT Standard Specifications for Construction and Maintenance of Highways, Streets, and Bridges to identify and remove any weak, compressible, or other unsuitable materials; such materials should be replaced with compacted clay fill. After proof rolling, compacted general clay fill should be used to raise existing grade to final grade.

For areas where new pavement will be placed, the top 8 inches of the exposed subgrade (after general clay fill has been placed) should then be stabilized with a minimum of 7 percent hydrated lime (by dry soil weight). Recommendations for lime subgrade stabilization and general clay fill are included in Sections 5.1.4 and 5.3.3 of this report, respectively.

5.1.4 Pavement Subgrade Preparation

As noted in Section 5.1 of this report, based on preliminary plan and profile drawings, AEC portions of the centerline of the new pavement will be placed at or near existing grade, although additional fill will need to be placed in some areas, such as within existing roadside swales to accommodate the roadway widening.

Based on Borings, B-2 through B-6, B-9, B-11, B-12, B-18, and B-19, the surficial soils along the existing Stella Road alignment primarily consist of lean/fat clay (CL/CH). However, more than 6 inches of clayey sand (SC) fill material was encountered at the ground surface in Borings B-2 and B-5, which AEC assumes is fill material placed for the original roadway construction, and AEC also assumes that this material will be left in place. In general, AEC anticipates that most of the pavement subgrade will require stabilization with hydrated lime. However, lime and fly ash stabilization will be required whenever clayey sand (SC) fill material is exposed along the roadway alignment.

<u>Subgrade Preparation</u>: Subgrade preparation should extend a minimum of 2 feet beyond the paved area perimeters. Existing pavement and base should be removed in accordance with Item 540 of the 2017 HCSEDS. Reclaimed asphalt pavement (RAP) can be reused for HMAC and/or asphalt-stabilized (black) base mixes in accordance with Items 340 and 250 of the 2017 HCSEDS, respectively. For new widening areas to be paved (that did not previously have pavement and base on top), a minimum of 3 inches of surface soils, existing vegetation, trees, roots, and other deleterious materials should be removed and deleterious materials to greater depths. The exposed soils should then be proof-rolled in accordance with Item 216 of the 2014 TxDOT Standard Specifications for Construction and Maintenance of Highways,



Streets, and Bridges to identify and remove any weak, compressible, or other unsuitable materials; such materials should be replaced with compacted clay fill. General clay fill recommendations are presented in Section 5.3.3 of this report.

After proof rolling, scarify the exposed subgrade to a depth of 8 inches and stabilize with either hydrated lime or lime fly ash, depending on type of exposed subgrade soil. Exposed clay should be stabilized with a minimum of 7 percent lime (by dry soil weight). Exposed sand or silt should be stabilized with a minimum of 3 percent hydrated lime and 7 percent fly ash (by dry soil weight). Lime and lime fly ash stabilization shall be performed in accordance with Items 220 and 223 of the 2017 HCSEDS, respectively. The percentage of lime and lime fly ash required for stabilization is a preliminary estimate for planning purposes only; laboratory testing (such as optimum lime content versus pH, in accordance with ASTM D6276) should be performed to determine optimum contents for stabilization prior to construction. The stabilized soils should be compacted to 95 percent of their ASTM D698 (Standard Proctor) dry density at a moisture content ranging from optimum to 3 percent above optimum.

5.1.5 Roadside Drainage Swales

Based on the drawings prepared by MEC, roadside drainage swales will be added on both sides of the new roadway. According to FBC EDM, a maximum slope inclination of H:V = 4:1 should be considered for the slope adjacent to the roadway, and a slope inclination of H:V = 3:1 for the slope on the opposite side can be considered. In general, AEC recommends that the flattest side slopes that are possible along the project alignments be used for swale design. Roadside drainage swale construction shall be performed in accordance with applicable 2018 FBCCD Construction Details.

5.2 Detention Basin

AEC notes that in April 2022, there were two detention basins shown on MEC's drawings that were available at the time Borings B-20 through B-23 were drilled. However, based on updated drawings provided by MEC in July 2022, the eastern detention basin (near the intersection of Stella Road and W. Fairground Road) was deleted from the project area. As a result, only Borings B-22 and B-23 are in the vicinity of the remaining west detention basin (near the S-curve at the mid-point of the project alignment).



Detention basin plan and cross section drawings (prepared by r.g. Miller Engineers) are presented on Plates D-1 and D-2, in Appendix D, for reference. The basin detention storage volume is 9.6 acre-feet (considering 1 foot of freeboard); the 100 year water surface elevation (WSE) is +94.20 feet Mean Sea Level (MSL). Top of bank around the basin has an elevation of +94.85 feet MSL. The toe of slope around the basin has an elevation range of +91.44 to +91.45 feet MSL. The bottom of basin flowline (i.e. pilot channels) is at an elevation of approximately +89.7 feet MSL. The resulting basin depths range from 3.4 to 4.3 feet, and the basin slopes will have an inclination of H:V = 4:1.

<u>Soil and Groundwater Conditions in Basin Area:</u> Based on Borings B-22 and B-23, the soil conditions in the detention basin area generally consist of stiff to hard fat clay (CH), considering a basin depth of 3.4 to 4.3 feet. Groundwater was not encountered in the borings during drilling.

5.2.1 Slope Stability Analysis

Based on AEC's borings and the basin cross section drawings (see Plate D-2, in Appendix D), AEC selected the southeast bank (closest to the Stella Road S-curve) of the detention basin (using Cross Section A-A) as the 'most critical' section to perform basin slope stability analysis on. AEC performed the slope stability analyses based on three different conditions: the short-term condition, long-term condition, and rapid drawdown condition. Based on AEC's previous discussions with the Fort Bend County Drainage District (FBCDD), there are no minimum factor of safety (FS) requirements for slope stability analyses for detention basins in Fort Bend County. AEC should be notified if FBCDD has different requirements so that our analyses can be revised if necessary.

<u>Clay Desiccation Zone</u>: AEC notes that the clay soils present in the top 10 feet of Borings B-22 and B-23 predominantly consist of fat clay (CH). These clays have plasticity indices that range from 41 to 57, indicating that the clay soils have high to very high expansive potential (see "Degree of Plasticity of Cohesive Soils" on Plate A-27, in Appendix A). Exposing these clays to the atmosphere and cycles of wetting-drying from seasonal moisture changes will result in desiccation, cracking, and a reduction in their shear strengths, which in turn will result in progressive slope movement and eventual slope failure. For fat clay soils, we considered a desiccation zone of approximately 8 feet below the ground/slope surface. For fat clay within the desiccation zone, we estimated effective stress residual shear strengths (c'_r and ϕ'_r) to evaluate slope stability for both the long-term condition and rapid drawdown condition based on A. Saleh and S. Wright (1977). We also reduced the c' and c_{cu} of lean clay soils (with a PI greater than 20) within the



non-desiccated (i.e. weathered) zone based on a combination of methods by G. Mesri (1999) and S. Wright (2005).

<u>Soil Parameters:</u> Soil parameters used in the analyses include moist unit weights, unconsolidated-undrained (UU) shear strengths, effective stress shear strength (developed using total stress parameters and pore water pressure measurements), and total stress shear strength obtained from consolidated-undrained (CU) triaxial tests. Design soil parameters used for AEC's slope stability analyses on the southeast bank of the detention basin are presented on Plate D-3, in Appendix D.

We used the Simplified Bishop Method of Slices option in the 2021 GeoStudio (SLOPE/W) computer program to analyze slope stability for 2-dimensional limiting equilibrium. The program has the capability to compute pore water pressures based on a defined piezometric surface.

<u>Groundwater Level</u>: For the analyses, we considered different groundwater conditions for short-term, longterm, and rapid drawdown conditions. Since groundwater was not encountered in Borings B-22 or B-23 during drilling, a groundwater table was not considered for either the short term or long term condition. For rapid drawdown condition, AEC considered the groundwater level to be at the ground surface, from top of the slope to the proposed basin bottom; this models a post-flood condition, where the basin fills with stormwater and then drains quickly, before pore-water pressures in the slope are allowed to dissipate.

<u>Required Safety Factor:</u> Stability analyses for the basin slopes were conducted for the short-term (end-ofconstruction), long-term, and rapid drawdown conditions. A brief description of these conditions is presented below:

- 1. <u>Short Term/End-of-Construction Condition</u> This condition models rapid construction loading taking place, so that there is no time for the induced excess pore water pressure to dissipate or for consolidation to occur during the loading period. UU shear strength parameters were used for this analysis.
- 2. <u>Long-Term Condition</u> This condition models long-term steady seepage through embankments and the long-term stability of slopes in stiff clays. Effective stress shear strength parameters (obtained from CU triaxial tests with pore water pressure measurements) were used for this analysis.
- 3. <u>Rapid Drawdown Condition</u> Most slope failures in the Gulf Coast area occur under rapid drawdown conditions. This condition models when the slope becomes fully saturated and



consolidated and is at equilibrium with the existing stress system, then encounters rapid drawdown and simultaneously allows no drainage to occur. Total stress shear strength parameters (obtained from CU triaxial tests) were used for this analysis.

<u>Basin Slope Stability</u>: Using the soil information encountered in Boring B-23, AEC performed slope stability analyses on the southeast bank of the basin based on the "A-A" cross-section presented on r.g. Miller Engineers drawings (see Plate D-2, in Appendix D). Design soil parameters used for the slope stability analyses are presented on Plate D-3, in Appendix D. A 300 psf construction surcharge was added to the top of bank for the short-term condition while a 250 psf surcharge was added to the top of the bank for the long-term and rapid drawdown conditions.

The results of the basin slope stability analyses under short-term, long-term, and rapid drawdown conditions are presented on Plates D-4 through D-9, in Appendix D. A summary of the FS for the proposed basin slopes under short-term, long-term, and rapid drawdown conditions is presented on Table 10.

Section Analyzed	Estimated Minimum Factor of Safety (FS)			
Section Analyzeu	Short-Term	Long-Term	Rapid Drawdown	
Southeast Bank of Basin, H:V = 4:1, Global Slide	17.66 (Plate D-4)	2.22 (Plate D-6)	1.52 (Plate D-8)	
Southeast Bank of Basin, H:V = 4:1, Local Slide	19.43 (Plate D-5)	1.85 (Plate D-7)	1.31 (Plate D-9)	

Table 10. Detention Basin Slope Stability Analysis Results (Based on Boring B-23)

5.2.2 Protection of Basin Slopes and Bottom

<u>Erosion Protection</u>: Based on AEC's borings, most of the soils that will be encountered in the basin slopes and bottom areas will consist of stiff to hard fat clay (CH) soils with low erosion potential. AEC does not anticipate that erosion protection (such as riprap, liners, or articulating blocks) will be required for the basin slopes or bottom.

<u>Dispersive Soils</u>: Based on the results of the crumb dispersion tests (see Table 2 in Section 3.0 of this report), the soils within the proposed basin consist of non-dispersive clays. Additional remediation of the basin slopes or bottoms for potentially dispersive soils is not required.



5.2.3 <u>Basin Excavation</u>

<u>Basin Soil and Groundwater Conditions:</u> Considering a basin depth ranging from 3.4 to 4.3 feet below grade, AEC anticipates that the basin excavation will generally encounter stiff to hard fat clay (CH) soils. Since groundwater was not encountered in Borings B-22 and B-23 (or any of AEC's borings), AEC does not anticipate that groundwater will be encountered during basin excavation. However, there could still be some groundwater seepage into the excavation; open drainage method (such as sump and pump) will likely be sufficient for groundwater control, if necessary. However, if the seepage rate into the excavation is insufficient for open drainage method to control, then pre-drainage method (such as ejector/eductor type systems) may be necessary. Generalized groundwater control recommendations are presented in Section 6.2 of this report.

<u>Excavations</u>: The contractor should be responsible for designing, constructing, and maintaining safe excavations and protecting existing structures in the vicinity of the proposed detention basin. Excavations should be in accordance with OSHA, Safety and Health Regulations, 29 CFR, Part 1926, Subpart P (Excavation and Trenches).

We recommend that the surcharge on the basin banks be limited to 300 psf or less during construction. AEC recommends that general clay fill be used for basin berm construction. If fill will be placed on slopes steeper than H:V = 4:1, then the slopes should be cut back into benches to provide a good construction joint between the existing soil and new fill. Where possible, each bench should be a minimum of 8 feet wide and a maximum of 3 feet high. Voids in the excavated slopes (if any) should be backfilled with general clay fill in accordance with Section 5.3.3 of this report.

<u>Re-use of Basin Borrow Soil as Select Clay Fill:</u> Based on Borings B-22 and B-23, none of the clay soils that will be excavated in the basin area (considering a basin depth of 3.4 to 4.3 feet) meet the requirements of select clay fill, as presented in Section 5.3.2 of this report. AEC recommends that the excavated basin soils be limited to use as general clay fill soil only, as presented in Section 5.3.3 of this report. Alternatively, the excavated soil can be treated with lime and then used as lime-stabilized clay, in accordance with Section 5.3.1 of this report.



5.3 Fill Requirements

5.3.1 Lime Stabilized Clay

<u>Soils Stabilized with Hydrated Lime:</u> AEC prefers that lime-stabilized clay be used as structural fill. Either: (i) imported lime-stabilized clay soils (stabilized offsite before delivery to the project site); or (ii) clay soils excavated onsite and treated with hydrated lime can be used. Clay soils excavated onsite should first be stabilized with a minimum of 7 percent hydrated lime (by dry soil weight). The amount of hydrated lime provided in this report is for estimation purposes only. The actual amount of lime required for stabilization should be determined by lime-series curve or pH method in a laboratory prior to construction. Lime stabilized soil as structural fill since compacted stabilized soil generally has high strength, low compressibility, and relatively low permeability.

<u>Lifts and Compaction</u>: Lime-stabilized clay fill should be placed in loose lifts not exceeding 8 inches in thickness. Backfill within 3 feet of walls or columns should be placed in loose lifts no more than 4-inches thick and compacted using hand tampers, or small self-propelled compactors.

Lime-stabilized clay should be compacted to a minimum of 95 percent of the ASTM D 698 (Standard Proctor) maximum dry unit weight at a moisture content ranging between optimum and 3 percent above optimum.

5.3.2 Select Clay Fill

<u>'Select' Clay Fill:</u> It is AEC's experience that 'select' clay fill material imported from sand and clay pits in the Greater Houston area is generally non-homogenous (i.e., composed of a mixture of sands, silts, and clays, instead of a homogenous sandy clay material) and of poor quality, and either contains too much sand or has large clay clods with high expansive potential. Use of this non-homogenous soil can result in poor long term performance of structures and pavements placed on top of the fill.

<u>Precautions:</u> Prior to construction, the Contractor should determine if they can obtain qualified select clay fill meeting the below select fill criteria. The closest sand and clay pit to the project site may not be able to deliver fill material that meets the requirements below. The Contractor should also be aware



that testing of select clay fill (see below) typically takes a minimum of 1.5 days to complete and they should accommodate testing in their fill placement in their project schedule. In addition, imported fill that is delivered to the project site may vary from day to day; material delivered to the site may pass one day but fail the next.

AEC notes that although lime-stabilized clay (see Section 5.3.1 of this report) may have a higher material and/or labor cost compared to select clay fill; the delays associated with locating, testing, and approving qualified select clay fill may exceed the costs of using lime stabilized clay instead. Potential risks associated with importing poor quality fill material to the site include: (i) accepting delivery of fill material that does not meet specifications, which could end up as wasted material if there is no use for it in other applications; (ii) removal of already placed lifts of compacted soil prior to laboratory testing results becoming available, resulting in schedule delays; and (iii) bringing additional equipment onsite to further manipulate the fill, such as a pulvimixer.

<u>Requirements:</u> Select clay fill (whether imported from offsite or if it is already onsite) should consist of *uniform*, non-active inorganic lean clays with a PI between 10 and 20 percent, and more than 50 percent passing a No. 200 sieve. Material intended for use as select fill shall not have clay clods with PI greater than 20, clay clods greater than 2 inches in diameter, or contain sands/silts with PI less than 10. Sand and clay mixtures/blends are unacceptable for use as select fill. Sand/silt with clay clods is unacceptable for use as select fill. Mixing sand into clay or mixing clay into sand/silt is also unacceptable for use as select fill. **The testing lab shall** <u>reject</u> any material intended for use as select fill that does not meet the PI, sieve, and clay clod requirements above, without exceptions.

<u>Lifts and Compaction</u>: All material intended for use as select fill should be tested prior to use to confirm that it meets select fill criteria. The fill should be placed in loose lifts not exceeding 8 inches in thickness. Backfill within 3 feet of walls or columns should be placed in loose lifts no more than 4-inches thick and compacted using hand tampers, or small self-propelled compactors.

Select fill should be compacted to a minimum of 95 percent of the ASTM D 698 (Standard Proctor) maximum dry unit weight at a moisture content ranging between optimum and 3 percent above optimum.



5.3.3 <u>General Clay Fill</u>

<u>General Clay Fill:</u> AEC recommends that general clay fill consist of a clean, cohesive soil (USCS Classification "CL" or "CH"). Granular soils (i.e., sands, silts, and gravel; not more than 50 percent retained on No. 200 sieve) should not be used as general clay fill.

General clay fill should be placed in loose lifts not exceeding 8 inches in thickness. General clay fill should be compacted to 95 percent of its ASTM D 698 (Standard Proctor) maximum dry unit weight at a moisture content ranging between optimum and 3 percent above optimum.

6.0 <u>CONSTRUCTION CONSIDERATIONS</u>

6.1 Site Preparation

To mitigate site problems that may develop following prolonged periods of rainfall, it is essential to have adequate drainage to maintain a relatively dry and firm surface prior to starting any work at the site. Adequate drainage should be maintained throughout the construction period. Methods for controlling surface runoff and basining include proper site grading, berm construction around exposed areas, and installation of sump pits with pumps.

6.2 Groundwater Control

The need for groundwater control will depend on the depth of excavation relative to the groundwater depth at the time of construction. If there is heavy rain prior to or during construction, the groundwater table may be higher than indicated in this report; higher seepage is also likely and may require a more extensive groundwater control program. In addition, groundwater may be pressurized in certain areas of the alignment, requiring further evaluation and consideration of the excess hydrostatic pressures. Groundwater control should be in general accordance with Item 436 of the 2017 HCSEDS.

The Contractor should be responsible for selecting, designing, constructing, maintaining, and monitoring a groundwater control system and adapt his operations to ensure the stability of the excavations. Groundwater information presented in Section 4.1 of this report and elsewhere, along with consideration for potential environmental and site variation between the time of our field exploration and construction, should



be incorporated in evaluating groundwater depths. The following recommendations are intended to guide the Contractor during design and construction of the dewatering system.

Groundwater control methods typically can be classified into three categories: (i) open pumping, where water is allowed to flow into an excavation and is collected in ditches or sumps and pumped away; (ii) predrainage, where the water table is lowered before excavation using wellpoints, ejector/eductor systems, deep wells, etc.; and (iii) cut off or exclusion, where the groundwater is prevented from entering the excavation by an impermeable barrier, such as by sheet piling, grouting, deep soil mixing, ground freezing, slurry shields, etc.

<u>Cohesive Soils:</u> Groundwater control in cohesive soils can typically be performed using open pumping methods. Seepage rates are lower than in granular soils and groundwater is usually collected in sumps and/or channeled by gravity flow to storm sewers. If cohesive soils contain significant secondary features, seepage rates will be higher. This may require larger sumps and drainage channels, or if significant granular layers are interbedded within the cohesive soils, methods used for granular soils may be required. Where it is present, pressurized groundwater will also yield higher seepage rates.

<u>Granular Soils:</u> Groundwater control in granular soils will typically require predrainage methods or cutoff/exclusion methods. For excavations that are less than 15 feet deep that will occur within saturated sands, a predrainage method such as wellpoints can be considered. For excavations that are greater than 15 feet deep, other predrainage methods that can be considered include multiple staged wellpoints, ejectors/eductors (primarily for use when silty soils are present), or deep wells with submersible pumps. Generally, with predrainage methods, the groundwater depth should be lowered at least 3 feet below the excavation bottom to be able to work on a firm surface when water-bearing granular soils are encountered.

If predrainage methods cannot be used, then a cutoff/exclusion method such as interlocking water-tight sheet piles, drilled shaft/secant pile wall (with grout between the shafts/piles), or jet grouting of the granular strata may be necessary.

<u>Extended Dewatering</u>: Extended and/or excessive dewatering can result in settlement of existing structures in the vicinity of the dewatering operations; the Contractor should take the necessary precautions to minimize the effect on existing structures in the vicinity of the dewatering operation. We recommend that the Contractor verify the groundwater depths and seepage rates prior to and during construction and retain



the services of a dewatering expert (if necessary) to assist them in identifying, implementing, and monitoring the most suitable and cost-effective method of controlling groundwater.

<u>Bottom Heave or Boiling:</u> For excavation in cohesive soils, the possibility of bottom heave must be considered due to the removal of the weight of excavated soil. In lean and fat clays, heave normally does not occur unless the ratio of Critical Height to Depth of Cut approaches one. In silty clays, heave does not typically occur unless an artificially large head of water is created using impervious sheeting in bracing the cut. If the excavation extends below groundwater and the soils at or near the bottom of the excavation are mainly sands or silts, the bottom can fail by blow-out (boiling) when a sufficient hydraulic head exists. The potential for boiling or in-flow of granular soils increases where the groundwater is pressurized. To reduce the potential for boiling of excavations terminating in granular soils below pressurized groundwater, the groundwater table should be lowered at least 3 feet below the excavation.

<u>Perched Water (If Present)</u>: Although it may be present at a shallower depth than the normal groundwater level, perched water should still be considered a form of groundwater. If perched water is encountered during the construction phase, the groundwater control methods mentioned above would still be the same. Depending on the size of the perched reservoir and recharge rates, the contractor should not assume that perched water can be completely dewatered during a normal construction period.

6.3 Construction Monitoring

Pavement construction and subgrade preparation, as well as excavation of detention basin should be monitored by qualified geotechnical professionals to check for compliance with project documents and changed conditions, if encountered. AEC should be allowed to review the design and construction plans and specifications prior to release to check that the geotechnical recommendations and design criteria presented herein are properly interpreted.

7.0 <u>LIMITATIONS</u>

The information contained in this report summarizes conditions found on the dates the borings were drilled. The attached boring logs are true representations of the soils encountered at the specific boring locations on the dates of drilling. Reasonable variations from the subsurface information presented in this report should be anticipated. If conditions encountered during construction are significantly different from those



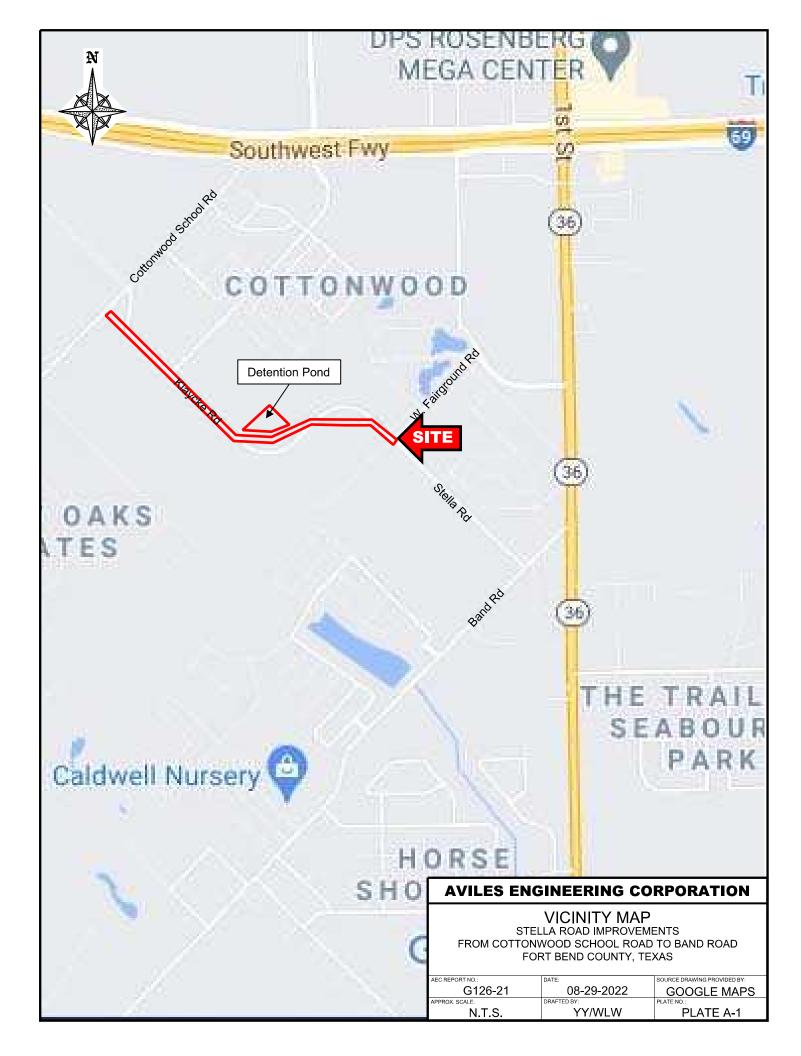
presented in this report; AEC should be notified immediately.

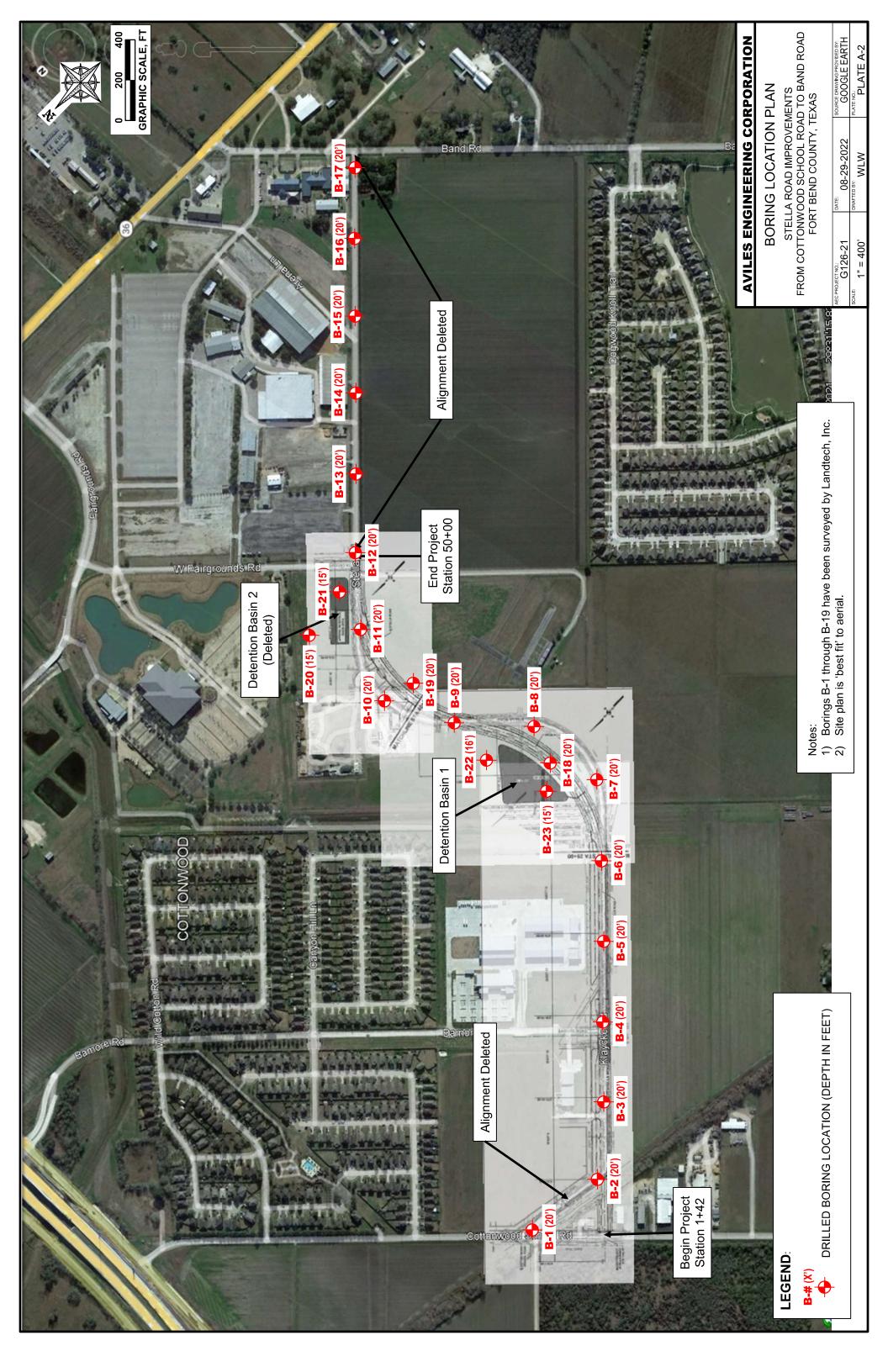
This investigation was performed using the standard level of care and diligence normally practiced by recognized geotechnical engineering firms in this area, presently performing similar services under similar circumstances. This report is intended to be used in its entirety. The report has been prepared exclusively for the project and location described in this report. If pertinent project details change or otherwise differ from those described herein, AEC should be notified immediately and retained to evaluate the effect of the changes on the recommendations presented in this report and revise the recommendations if necessary. The recommendations presented in this report should not be used for other structures located along the alignments or similar structures located elsewhere, without additional evaluation and/or investigation.



APPENDIX A

Plate A-1	Vicinity Map
Plate A-2	Boring Location Plan
Plates A-3 to A-25	Boring Logs
Plate A-26	Key to Symbols
Plate A-27	Classification of Soils for Engineering Purposes
Plate A-28	Terms Used on Boring Logs
Plate A-29	ASTM & TXDOT Designation for Soil Laboratory Tests
Plate A-30	Crumb Test Results
Plates A-31 and A-32	Mohr-Coulomb Diagrams (from CU Triaxial Tests)







GEOTECHNICAL ENGINEERS

B-1

PROJECT: Stella Road Improvements

DATE 6/10/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. **INDEX** GRID Coordinates (US Survey ft): **Confined Compression** Δ DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT Unconfined Compression PLASTICITY 200 MESH Eastina: 2977451.48 \bigcirc **Pocket Penetrometer** SYMBOL P.H Northing: 13752613.63 ЯΥ Torvane Elevation: 99.65 'n 05 0 Pavement: 5.5" asphalt Base: 6" gravel Subbase: 7.75" stabilized sand 96 67 25 42 35 Soft to very stiff, dark gray Fat Clay (CH), with slickensides 41 79.6 -with ferrous nodules 2'-12' 4 -tannish gray 4'-6', with calcareous nodules 4'-12' 32 -dark brown and gray 6'-8' 96 60 19 41 25 8 -tannish gray 8'-10' 30 94.1 -tan, with calcareous powder pockets 10'-12' 24 12 Soft to very stiff, tan and gray Lean Clay (CL), 40 15 25 with calcareous nodules 17 -with calcareous powder pockets 12'-14' -with silty clay pockets and and silt seams 14'-16' 19 16 Loose to medium dense, tan Silty Sand (SM), wet 13 22 -tan and light gray, with fat clay seams 18'- 20' 45 10 28 20 Termination Depth = 20 feet 24 28 BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING 😤 WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 DRAFTED BY LOGGED BY DRILLED BY Van and Sons YY YY/DN



B-2

PLASTICITY INDEX

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11

PROJECT: Stella Road Improvements

DATE <u>6/10/2021</u> TYPE <u>4" Dry Auger</u>			LOCATIO	ON See Boring Locat	ion P	lan	1	
	DESCRIPTION		% '	SH	HEAR STRENGTH, TSF			
DEPTH IN FEET SYMBOL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2977409.05 Northing: 13752122.69 Elevation: 97.70	S.P.T. BLOWS / FT.	MOISTURE CONTENT,	DRY DENSITY, PCF	Confined Compression Unconfined Compression Pocket Penetrometer Torvane 0.5 1 1.5 2	-200 MESH	LIQUID LIMIT	PLASTIC LIMIT
	Pavement: 3.5" asphalt	-	8					
	Base: 8.5" stabilized sand and gravel Fill: brown Clayey Sand (SC), with gravel -with fat clay seams 1.5'-2'	-	13		0	26	36	22
4	Stiff to very stiff, gray Fat Clay (CH), with ferrous nodules		26	98.6		-		
	top and grow with colorrous podulos CL 2		25		·····↔			
	-tan and gray, with calcareous nodules 6'-8'		21		+++++ &		63	19
8	Very stiff, tan and gray Lean Clay (CL), with fat clay and calcareous powder pockets, and calcareous and ferrous nodules		17	112.9				
	Very stiff, tan Silty Clay (CL-ML), with lean clay pockets, calcareous nodules, and ferrous stains		17			-		
12	Very stiff, tan Sandy Lean Clay (CL), with sandy silt partings, silty clay pockets, calcareous nodules, and ferrous stains		16			70	27	16
	Tan Sandy Silt (ML), with lean clay pockets, calcareous nodules, and ferrous stains		21					
16	Firm to very stiff, tan Lean Clay (CL), with silt and calcareous powder pockets, and		24			97		
	 calcareous nodules Stiff to very stiff, reddish tan Fat Clay (CH), with 							
20	slickensides, silt pockets, and calcareous nodules		26	97.3				
	Termination Depth = 20 feet							
- 24 -								
28 -								
						<u> </u>		
	ATER ENCOUNTERED AT <u>N/A</u> FEET WHILI ATER LEVEL AT N/A FEET AFTER COMPL			IG ≇				
	ILLED BY <u>Van and Sons</u> DRAFTED BY		Y	,		Y/DN		
PRO	JECT NO. G126-21					PLAT	Έ	A-4



ENGINEERING CORP. GEOTECHNICAL ENGINEERS BORING B-3

-	

PROJECT: Stella Road Improvements

DATE 6/10/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan

				% '		S	HEAR STRENGTH,	TSF				
	, AL	DESCRIPTION		MOISTURE CONTENT,	Ц							ХШ
EET	TERV	GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204	P.T. BLOWS / FT	CON.	DENSITY, PCF		Confined Compres			μ	МΙΤ	PLASTICITY INDEX
DEPTH IN FEET	E N.		BLOV	URE	ENSI		Unconfined Compr Pocket Penetrome		ESH	רומחום רושוב	PLASTIC LIMIT	LCIT)
EPTH	SYMBOL SAMPLE	Northing: 13751769.18	P.T.	OIST	ΟΚΥ Ο		Torvane	.01	200 MESH	auid	-AST	-AST
10 0	S, S		Ś	ž	Ō		0.5 1 1.5	2	Ρ̈́		Ы	Ы
	\bigotimes	Pavement: 4.5" asphalt Base: 14" stabilized sand with gravel		10								
		Fill: dark gray Clayey Sand (SC), with gravel		10					36	63	20	43
		Fill: stiff to very stiff, dark gray Fat Clay (CH),		25	97.1							
- 1 -	>>>	with gravel		20	57.1							
4	\bigotimes	-with sand seams 4'-6'		04								
				24								
	\square	Very stiff, gray Fat Clay (CH), with slickensides								57	16	41
		-with ferrous nodules 6'-10'		22								
- 8 -		-grayish tan, with calcareous nodules 8'-10'										
	\square			25	99.3							
		Very stiff to hard, reddish brown Lean Clay										
		(CL), with calcareous nodules, pockets, and seams		17				ĦΫŦ				
- 12 -		-with silt pockets and ferrous stains 12'-14'								39	16	23
				18				Ŕ\\ \				
		Stiff to hard, reddish tan Fat Clay (CH), with						\pm	96			
	\square	slickensides		16	114.9			++				
- 16 -		-with silt seams 14'-16' -with calcareous nodules 16'-20'										
				25				+++++++++++++++++++++++++++++++++++++++				
	\square									C1	~~~	20
				25				+++++++++++++++++++++++++++++++++++++++		61	22	39
- 20 -	//	Termination Depth = 20 feet										
							+++++++++++++++++++++++++++++++++++++++	+++++				
							+++++++++++++++++++++++++++++++++++++++	+++++				
								$\frac{1}{1}$				
- 24 -												
- 28 -												
		G DRILLED TO <u>20</u> FEET WITHOUT DR										
		R ENCOUNTERED AT <u>N/A</u> FEET WHILE R LEVEL AT N/A FEET AFTER COMPLE			NG =	÷						
		ED BY Van and Sons DRAFTED BY		_ ۲	Y		LOGGED BY	Y	Y/DI	١		
		T NO. G126-21							PLA		A-5	



GEOTECHNICAL ENGINEERS

PROJECT: Stella Road Improvements

DATE 6/10/2021 TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH. TSF **MOISTURE CONTENT, %** DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** Λ DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT Unconfined Compression 200 MESH 2978079.39 Eastina: \bigcirc Pocket Penetrometer SYMBOL P.T. 13751439.93 Northing: ЯΥ Torvane Elevation: 97.58 (n 0.5 0 Pavement: 3.5" asphalt Base: 6" sand and gravel Pavement: 2.5" asphalt 26 Subbase: 7.5" sand and gravel 96 67 17 50 24 Very stiff to hard, dark gray Fat Clay (CH) -with sandy lean clay partings 1.6'-2' and 4 ferrous nodules 1.6'-10' -gray, with calcareous nodules 2'-4' 23 101.6 -tan and gravish tan 6'-8', with calcareous nodules 6'-12' 24 8 -brown and gray 8'-10', with calcareous powder 89 56 17 39 pockets 8'-12' 20 -tan 10'-12' 16 113.7 12 Very stiff, reddish tan Silty Clay (CL-ML), with ferrous and calcareous nodules, and fat clay 19 pockets Very stiff, tan and light gray Lean Clay (CL), 47 17 30 with calcareous nodules and powder pockets 16 16 Very stiff to hard, tan and light gray Fat Clay (CH), with slickensides 23 102.9 -with calcareous nodules 16'-18' -with calcareous powder pockets 18'-20' 24 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING 😤 WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 DRAFTED BY LOGGED BY DRILLED BY Van and Sons YY YY/DN

BORING **B-4**

PLASTICITY INDEX



ENGINEERING CORP. BORING

ORING **B-5**

DATE <u>6/10/2021</u> TYPE <u>4" Dry Auger</u>				LOCATION See Boring Location Plan							
						SHEAR STRENGTH, TSF					
DEPTH IN FEET	SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2978427.64 Northing: 13751103.01 Elevation: 97.30	S.P.T. BLOWS / FT.	MOISTURE CONTENT,	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 					
0		Pavement: 4.5" asphalt Base: 15" stabilized sand with gravel		12							
				7		15 24 20 4					
	XXX g	Fill: gray Silty Clayey Sand (SC-SM), with gravel -with fat clay seams and lime pockets 2'-4'		8							
- 4 -		Stiff to hard, gray and dark gray Fat Clay (CH) -with ferrous nodules 4'-6'		25	96.0						
		-gray, with calcareous nodules 6'-8'		24		95 62 18 44					
- 8 -		light grayish tan 8'-10'		27							
		-tan and grayish tan, with calcareous and ferrous nodules, and lean clay pockets 10'-12'		19	109.0						
- 12 -		Hard, tan Lean Clay (CL), with calcareous nodules and powder pockets		15		39 16 23					
		Very stiff to hard, tan Fat Clay (CH), with slickensides -with calcareous nodules and ferrous stains		26							
- 16 -		14'-18' -tan and gray 16'-20'		21	108.4						
				25		65 25 40					
- 20 -		Termination Depth = 20 feet									
		-									
- 24 -											
- 28 -											
╞──┍					 - () F						
	BORING DRILLED TO _20_ FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING ₩										
		LEVEL AT <u>N/A</u> FEET AFTER COMPLE	ETE								
	DRILLED	BY <u>Van and Sons</u> DRAFTED BY		Y	Y	LOGGED BY YY/DN					
PF	ROJECTI	NO. G126-21				PLATE A-7					

PROJECT: Stella Road Improvements



ENGINEER

ENGINEERING CORP. BORING B-6

DATE	6/9/2021

PROJECT: Stella Road Improvements

TYPE 4" Dry Auger

LOCATION See Boring Location Plan

					%		5	SHEAR STRENGTH,	TSF				
			DESCRIPTION		NT,								
<u> </u>		<u> SVAL</u>	GRID Coordinates (US Survey ft):	P T BLOWS / FT	MOISTURE CONTENT,	DRY DENSITY, PCF		Confined Comments	vion				PLASTICITY INDEX
DEPTH IN FEET		LTER	Texas State Plane Zone: 4204	WS /	CO	üTΥ,		Confined Compres Unconfined Compr			١١	IMIT	ΥIN
Z T	Ы	<u>∠</u> Ш	Easting: 2978795.46	вго	'URE	ENS				ESH) LIN	-IC L	TICIT
ΕΡΤΗ	SYMBOL	SAMPLE	Northing: 13750779.58		OIST	סאצ		-		-200 MESH	ΓΙΩUID LIMIT	PLASTIC LIMIT	-AST
0 0	ŝ	/S	Elevation: 96.28	Ś	ž	Ō		0.5 1 1.5	2	Ϋ́	Ē	Ы	Ы
Ů	$\times\!\!\times\!\!\times$		Pavement: 7" asphalt Base: 5" stabilized sand and gravel		8								
	>>>		Fill: stiff to very stiff, dark brown Lean Clay		22					93	46	15	31
	\bigotimes		(CL), with gravel							1			
			-with ferrous nodules 1'-2' -with fat clay seams 2'- 4'		24			++++++ ?+++++		-			
- 4 -			Stiff to hard, gray and dark gray Fat Clay (CH),										
			with slickensides		28	93.5	$\left \right $	┝┼┼╋╬╬┼┼┼┼┼	++++				
			-with ferrous nodules 4'-14'						++++	-			
			-tannish gray 6'-8', with calcareous nodules 6'- 16'		26						68	19	49
- 8 -			-tan 8'-14'										
					26								
			-with calcareous powder pockets 10'-12'										
					17	111.5		++++++++	+174+				
- 12 -									+/++				
					19				ЬШ	92	55	17	38
			-reddish tan 14'-18', with silt partings 14'-16'										
			reducin tan in ito, with one parange in ito		22			++++++++++++++++++++++++++++++++++++++					
- 16 -			-with ferrous stains 16'-18'						\mathbb{N}				
					27	100.4			+Pd+	-			
			-tan and light gray, with silt partings 18'-20'						+/ $+$ / $+$	-			
			tan and light gray, with site partings to 20		22				8444	-	56	19	37
- 20 -													
20			Termination Depth = 20 feet										
- 24 -													
24													
- 28 -										-			
			G DRILLED TO <u>20</u> FEET WITHOUT DR						<u></u>	1			
	WATER ENCOUNTERED AT <u>N/A</u> FEET WHILE DRILLING												
			LEVEL AT <u>N/A</u> FEET AFTER <u>COMPLE</u>	TE					-	~~			
			D BY <u>Van and Sons</u> DRAFTED BY		Y	ſ		LOGGED BY		Y/D			
PF	ROJE	СТ	NO. G126-21							PLA'	TE	A-8	



TYPE 4" Dry Auger

DATE 6/9/2021

SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** Δ DEPTH IN FEET Texas State Plane Zone: 4204 Unconfined Compression AMPLE IN 200 MESH Eastina: 2979177.94 \bigcirc Pocket Penetrometer SYMBOL Ъ.Ч Northing: 13750466.84 ЛЯΥ Torvane Elevation: 96.38 (n 0.5 0 Pavement: 5" asphalt 6 Base: 12" stabilized sand and gravel 52 14 Fill: hard, brown and dark gray Sandy Lean Clay (CL), with gravel 23 Fill: very stiff, dark brown Fat Clay (CH), with gravel and lean clay seams 4 Stiff to hard, dark gray Fat Clay (CH), with 30 91.9 slickensides -with ferrous nodules 4'-6' 29 8 -gray 8'-10', with ferrous nodules 8'-12' 96 30 -tan 10'-12', with calcareous nodules 10'-14' 24 99.7 12 -reddish tan 12'-16' 20 24 16 -tan and gray 16'-20', with calcareous nodules 16'-18' 25 102.8 -with silt partings 18'-20' 22 20 Termination Depth = 20 feet 24 28 BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING 😤 WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺

DRAFTED BY

PROJECT NO. G126-21

Van and Sons

DRILLED BY

YY/DN PLATE A-9

LOGGED BY

YΥ

ENGINEERING CORP. BORING

LOCATION See Boring Location Plan

PLASTICITY INDEX

PLASTIC LIMIT

20 41

71 21 50

62

23 39

LIQUID LIMIT



ENGINEERING CORP. BORING

B-8

Ж

PROJECT: Stella Road Improvements DATE 6/9/2021 LOCATION See Boring Location Plan TYPE 4" Dry Auger ITENT, % DESCRIPTION Ŀ. Ч Ā dinatas /LIS Su ----DEPTH IN FEET 0 4

SHEAR STRENGTH, TSF

	SYMBOL	GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2979671.65 Northing: 13750519.24 Elevation: 96.64	S.P.T. BLOWS / I	MOISTURE CON	DRY DENSITY, F	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 	PLASTICITY IND		
	\otimes	Pavement: 4" asphalt Base: 14.5" stabilized sand Fill: stiff to very stiff, dark gray Fat Clay (CH), with gravel and sand pockets Stiff to hard, dark gray Fat Clay (CH), with		28 36	86.2	97 76 24	52		
		slickensides -with ferrous nodules 2'-4'		31 32		97 85 24	61		
		-gray 8'-10', with calcareous nodules 8'-12' -reddish tan and light brown 10'-12'		32	90.7				
		-tan 12'-14'		30 24		61 20	41		
		-tan and light gray, with calcareous powder pockets 14'-16'		22	103.6				
		 -reddish tan and gray, with silt seams 16'-18' -tan and gray, with sandy silt seams 18'-20' 		24		97			
		Termination Depth = 20 feet	26	20					
-									
W, W,	BORING DRILLED TO <u>20</u> FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT <u>N/A</u> FEET WHILE DRILLING \rightleftharpoons WATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPLETE</u>								
	XILL	LED BY <u>Van and Sons</u> DRAFTED BY		Y	T	LOGGED BY YY/DN			

8

12

16

20

24



B-9

PLASTICITY INDEX PLASTIC LIMIT LIQUID LIMIT

63 26 37

25 65

PI	PROJECT: Stella Road Improvements ENGINEERING CORP. BORING B-9									
D	ATE <u>6/8/</u> 2	2021 TYPE <u>4" Dry Auger</u>				CATION See Boring Location Pla	n			
DEPTH IN FEET	YMBOL AMPLE INTE	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2980018.92 Northing: 13750858.76 Elevation: 96.11	S.P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF Confined Compression Unconfined Compression Pocket Penetrometer Torvane 0.5 1 1.5 2				
0 4 -	P B F all S	avement: 4" asphalt ase: 10" stabilized sand and gravel ill: stiff, dark brown Fat Clay (CH), with gravel nd lean clay pockets tiff to very stiff, dark gray Fat Clay (CH), with lickensides		6 32 34	85.9	96 63				
8 -		vith ferrous nodules 6'-12' an and gray, with calcareous nodules 8'-10'		33 32		90				
12 -		an 10'-12' ery stiff, tan Lean Clay (CL), with sandy silt		31 22	91.1 103.5	89 31				
16 -		artings ledium dense to dense, tan Sandy Silt (ML)	35	15 7						
		vith cemented sand nodules 16'-18' vith lean clay seams 18'-20'	31 28	9 10		60				
20 -	ΤΠΠΥ - -	ermination Depth = 20 feet								
24										
28 -										
١	BORING DRILLED TO <u>20</u> FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT <u>N/A</u> FEET WHILE DRILLING \neq WATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPLETE</u> \neq DRILLED BY Van and Sons DRAFTED BY YY LOGGED BY YY									



ENGINEERING CORP. BORING

ING **B-10**

	ATE <u>6/8/2021</u> TYPE <u>4" Dry Auger</u>			LOC	CATION See Boring Location Plan			
	DESODIDITION		% '		SHEAR STRENGTH, TSF			
DEPTH IN FEET	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2980400.76 Northing: 13751072.08 Elevation: 96.26	S.P.T. BLOWS / FT	MOISTURE CONTENT,	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 			
0	Pavement: 4" asphalt Base: 8" stabilized sand and gravel Fill: dark brown Clayey Sand (SC), with gravel -gray 2'-3'		5 7		<u> </u>			
- 4 -	Stiff to hard, dark gray Fat Clay (CH), with slickensides -with ferrous nodules 4'-12'		39 39	84.6				
			36		96 83 26 57			
- 8 -	-tan and gray, with calcareous nodules 8'-10' -tan 10'-14'		36	85.9				
- 12 -			28		65 27 38			
	-tan and light gray 14'-20'		26 29	98.4				
- 16 -			28					
- 20 -	Termination Depth = 20 feet		24		60 26 34			
- 24 -								
- 28 -								
	BORING DRILLED TO <u>20</u> FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT <u>N/A</u> FEET WHILE DRILLING ₩ WATER LEVEL AT N/A FEET AFTER COMPLETE ₩							
	DRILLED BY <u>Van and Sons</u> DRAFTED BY		Y	Y	LOGGED BY YY			
PF	ROJECT NO. G126-21				PLATE A-12			



ENGINEERING CORP. BORING

B-11

DA	DATE <u>6/8/2021</u> TYPE <u>4" Dry Auger</u>				CATION See Boring Location Plan
O DEPTH IN FEET	DESCRIPT GRID Coordinates (US Sul Texas State Plane Zone: Easting: 2980828.65 Northing: 13750890.73 Elevation: 96.32		S.P.T. BLOWS / FT. MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2
- 4 -	Pavement: 3.5" asphalt Base: 5.5" stabilized sand Subbase: 10" sand and gra Fill: dark brown Clayey Sa Stiff to hard, dark brownish with slickensides -with ferrous nodules 2'-6' -dark gray 4'-8'	avel nd (SC), with gravel	4 26 35 34	85.4	46 61 23 3 • • • • • • • • • • • • • • • • • • •
- 8 -	-dark gray and tannish gra stains 8'-12' -reddish tan 10'-14', with c		35 34	90.9	
- 12 -	-tan and reddish brown, wi silt partings 14'-16'		30 26 24		
- 16 -	-tan and light gray 16'-18' Dense, tan and light gray S lean clay pockets and silts	tono nodulos	43 15		
- 20 -	Termination Depth = 20 fe		-0 10		
- 28 -					
	BORING DRILLED TO <u>20</u> FI WATER ENCOUNTERED AT WATER LEVEL AT <u>N/A</u> FEE ⁻ DRILLED BY <u>Van and Sons</u>	N/A FEET WHILE	DRILL TE		
PF	ROJECT NO. G126-21				



ENGINEERING CORP. BORING

6 <u>B-12</u>

DATE <u>6/8/2021</u> TYPE <u>4" Dry Auger</u>			LOC	CATION See Boring Location Plan		
		DESCRIPTION		NT, %		SHEAR STRENGTH, TSF
DEPTH IN FEET	SYMBOL	GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2981181.36 Northing: 13750586.11 Elevation: 96.07	S.P.T. BLOWS / FT.	MOISTURE CONTENT,	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 × 300 1.5 1.5 2
0	$\times\!\!\times\!\!\times$	 Pavement: 4" asphalt Base: 8" stabilized sand and gravel 		10		
	\bigotimes	_ Fill: stiff to very stiff, dark gray Fat Clay (CH),	-	33		96 67 23 44
- 4 -		\with sandy lean clay seams and gravel Stiff to hard, dark gray Fat Clay (CH), with slickensides	-	36	83.7	
				37		
		-dark gray and browinsh gray 6'-8', with ferrous nodules 6'-16'		33		81 24 57
- 8 -		-reddish tan 8'-10'		31	91.7	
		-tan and light gray 10'-12'		27		
- 12 -		-reddish tan and light gray 12'-14'		26		68 22 46
		-tan and light gray, with silt seams 14'-16'		24	102.2	
- 16 -		Dense to very dense, tan and light gray Sandy	_	24	102.2	
		Silt (ML) -with calcareous nodules 16'-18'	62	11		52
- 20 -		-tan, with silty clay pockets and siltstone nodules 18'-20'	37	9		
- 20 -		Termination Depth = 20 feet]			
- 24 -						
- 28 -						
		NG DRILLED TO <u>20</u> FEET WITHOUT DF				
		ER ENCOUNTERED AT <u>N/A</u> FEET WHIL ER LEVEL AT N/A FEET AFTER COMPL			NG -	
		ED BY Van and Sons DRAFTED BY		Y	Y	LOGGED BY YY
PF	ROJE	CT NO. G126-21				PLATE A-14



ENGINEERING CORP. BORING

B <u>B-13</u>

DATE <u>6/</u>	8/2021 TYPE <u>4" Dry Auger</u>	LOCATION See Boring Location Plan							
DEPTH IN FEET SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2981532.35 Northing: 13750262.50	P.T. BLOWS / FT.	MOISTURE CONTENT, %	Y DENSITY, PCF	SHEAR STRENGTH, TSF A Xadata △ Confined Compression H ● Unconfined Compression HSalu ○ Pocket Penetrometer HSalu □ Torvane O 5 1				
SY DE	Elevation: 95.97	S.F	ω	DRY					
	Pavement: 4" asphalt Base: 8" stabilized sand and gravel Fill: dark brown Clayey Sand (SC), with gravel -with fat clay pockets 1'-2' -brown, with fat clay seams 2'-4'		11 10 16	104.4	17 31 13 18				
	Firm to hard, dark gray Fat Clay (CH), with slickensides -with calcareous nodules 6'-10'		33 32	90.4	78 27 51				
8	-tan and gray 8'-10', with ferrous stains 8'- 12'		29						
12	-tan 10'-12'		28		98 69 24 45				
	-tan and light gray, with calcareous nodules, and lean clay and calcareous powder pockets 12'-14'	-	18	108.5					
- 16 -	Very dense, light tan Sandy Silt (ML), with siltstone nodules	77	5						
	-tan 16'-20'	61	7		51				
20	-with sandy lean clay pockets 18'-20' Termination Depth = 20 feet	50/5"	8						
- 24 -									
- 28 -									
	G DRILLED TO <u>20</u> FEET WITHOUT DF R ENCOUNTERED AT N/A FEET WHILE								
WATER	WATER ENCOUNTERED AT <u>N/A</u> FEET WHILE DRILLING # WATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPLETE</u> DRILLED BY Van and Sons DRAFTED BY YY LOGGED BY YY								

B-14 G

-200 MESH

96

PLASTICITY INDEX

PLASTIC LIMIT LIQUID LIMIT

78 28 50

87 24 63

69 24 45

100 56 21 35

					AVILES
PF	ROJECT: Stella Road Improvements			E	AND A CORP. BORING
D	ATE 6/7/2021 TYPE 4" Dry Auger				ATION See Boring Locat
			, %		SHEAR STRENGTH, TSF
DEPTH IN FEET	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2981888.21 Northing: 13749931.30 GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Elevation: 95.67	S.P.T. BLOWS / FT	MOISTURE CONTENT, %	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane
0	Pavement: 3.5" asphalt		2		
	Base: 13.5" sand and gravel Stiff to hard, dark gray Fat Clay (CH), with		33		·····
- 4 -	slickensides		33	86.6	••••••
-			34		
	-tannish gray and dark gray 6'-8', with calcareous nodules 6'-10'		32		
8 -	-reddish tan 8'-14'		30	91.7	
					− −−−−
- 12 -			27		
	ten and light area 441.461		27		
	-tan and light gray 14'-16'		25	101.6	
16 -	-reddish tan and light gray, with silt seams 16'- 18'	•	20		
	-tan and light gray, with silt pockets 18'-20'		25		C-
- 20 -	Termination Depth = 20 feet	-			
- 24 -					
- 28 -					
	BORING DRILLED TO 20 FEET WITHOUT D				
	WATER ENCOUNTERED AT N/A FEET WHIL	.E DR	RILLII	NG 🗄	¥-

YΥ PLATE A-16

LOGGED BY

YΥ

PROJECT NO. G126-21

DRILLED BY Van and Sons

WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺

DRAFTED BY

ation Plan



PROJECT: Stella Road Improvements	E	BOR BOR BOR	ING	B-15	·		
DATE <u>6/7/2021</u> TYPE <u>4" Dry Auger</u>				CATION See Boring L	ocation	Plar	<u> </u>
DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2982234.96 Northing: 13749608.87 Elevation: 95.35	S.P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	 SHEAR STRENGTH, TS △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 	on sion _{II}	LIQUID LIMIT	PLASTIC LIMIT PLASTICITY INDEX
 Pavement: 4" asphalt Base: 14" sand and gravel Stiff to very stiff, dark gray Fat Clay (CH), with slickensides -with ferrous nodules 2'-6' 		32 36 36	83.3		96	80	27 53
 -brown and gray 6'-8' -tan and gray 8'-10', with calcareous and ferrous nodules 8'-12' 		32 30	91.9			86	23 63
 -reddish tan 10'-12' Stiff to hard, light reddish tan and gray Lean Clay (CL), with sandy silt seams 	_	25 28	90.9		91	31	15 16
Dense to very dense, tan Silt with Sand (ML)	37	15					
18' 	70 50/5"	19 8		<u></u>	77		
Termination Depth = 20 feet							
²⁸ BORING DRILLED TO <u>20</u> FEET WITHOUT DR WATER ENCOUNTERED AT N/A FEET WHIL							
WATER LEVEL AT <u>N/A</u> FEET AFTER <u>COMPL</u> DRILLED BY <u>Van and Sons</u> DRAFTED BY _				LOGGED BY	YY		
PROJECT NO. G126-21					PLA	TE	A-17



DATE <u>6/7/2021</u> TYPE <u>4" Dry Auger</u>

LOCATION See Boring Location Plan

B-16

		550051571011		%		S	SHEAR STRENGTH, 1	SF				
		DESCRIPTION		MOISTURE CONTENT,	共							×
Ш	SYMBOL SAMPLE INTERVA	GRID Coordinates (US Survey ft):	P.T. BLOWS / FT	ONT	DENSITY, PCF	Δ	Confined Compress	ion			E	PLASTICITY INDEX
DEPTH IN FEET		Texas State Plane Zone: 4204	SWO.	R C	(TISI	\bullet	Unconfined Compre	ssion	н	LIQUID LIMIT	PLASTIC LIMIT	Σ
HT	BOL DIF	Easting: 2982576.06 Northing: 13749294.44	L. BL	STUF	DEN	0	Pocket Penetromete	er	200 MESH	ID L	STIC	STIC
DEP	SYMBOL	Northing: 13749294.44 Elevation: 95.16	S.P.	MOIS	DRY		Torvane 0.5 1 1.5 2	>	-200	ΓIGΓ	PLA	PLA
0		Pavement: 3.5" asphalt										
		Base: 12.5" sand and gravel							0.5	70	05	47
	\bigotimes	Fill: very stiff, dark gray Fat Clay (CH), with		27			<u> </u>		95	72	25	47
		gravel, sandy lean clay seams, and asphalt pieces		30	90.8							
		Stiff to very stiff, dark gray Fat Clay (CH), with										
4		slickensides										
		-with ferrous nodules 2'-6' -dark brownish gray 4'-6'		29								
		-tan and gray 6'-8'								73	23	50
				29	94.7	+ +	┼┼┼╋┼┼┼╠╣┼┼┼┼┼	+ + + + +			20	50
- 8 -		-tan and dark gray, with ferrous stains 8'-10',										
		and calcareous nodules 8'-12'		27								
		-red and tan, with silt pockets and seams 10'- 12'										
		12		22								
- 12 -		Stiff to very stiff, tan and gray Lean Clay (CL),							98	29	18	11
		with calcareous nodules and silt partings		21			+++		50	20	10	
		Madium dance ten Ciltwith Sand (ML)										
		Medium dense, tan Silt with Sand (ML) -with siltstone seams 14'-16'										
- 16 -			20	20								
10									79			
			20	18					19			
		-with silty clay pockets 18'-20'	21	15								
- 20 -	<i> </i> /	Termnation Depth = 20 feet										
		remnation Depth – 20 leet										
- 24 -												
						┝┼┼┼		++++				
							+++++++++++++++++++++++++++++++++++++++	+ + + +				
- 28 -												
	BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID											
		R ENCOUNTERED AT <u>N/A</u> FEET WHILE			lG ≟	<u>_</u>						
		R LEVEL AT <u>N/A</u> FEET AFTER <u>COMPLE</u>	ETE		_							
		ED BY <u>Van and Sons</u> DRAFTED BY		Y	Y		_ LOGGED BY _		ΥY			
PF	ROJEC	T NO. G126-21							PLA [.]	TE	A-18	3



TYPE 4" Dry Auger

DATE 6/7/2021

SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. GRID Coordinates (US Survey ft): **Confined Compression** Λ DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT Unconfined Compression 200 MESH Eastina: 2982888.19 \bigcirc Pocket Penetrometer SYMBOL P.T. Northing: 13748987.65 JRY Torvane Elevation: 94.52 'n 0.5 0 Pavement: 3.25" asphalt Base: 14.25" sand and gravel 22 63 50 17 Fill: very stiff, dark gray Sandy Fat Clay (CH), with gravel seams and shell fragments 23 Stiff to very stiff, gray and dark gray Fat Clay (CH), with slickensides 4 -with ferrous nodules 2'-6' 100.9 -tan and gray, with calcareous nodules 4'-6' 25 -reddish tan, with lean clay pockets 6'-8' and 20 47 67 calcareous powder pockets 6'-10' 21 8 -reddish brown, with silt pockets 8'-10' 109.6 19 Reddish tan Silt with Sand (ML) 16 12 Stiff to very stiff, tan and light gray Lean Clay 93 32 15 (CL), with silt seams 109.0 18 Medium dense, tan Silt with Sand (ML) -with silty clay pockets 14'-16' 16 21 16 -boring cave-in at 16' after completion of drilling 81 19 24 17 20 Termination Depth = 20 feet 24 28 FEET WITHOUT DRILLING FLUID BORING DRILLED TO 20 WATER ENCOUNTERED AT N/A FEET WHILE DRILLING 😤 WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 DRAFTED BY LOGGED BY DRILLED BY Van and Sons YY YY

LOCATION See Boring Location Plan

ENGINEERING CORP. **GEOTECHNICAL ENGINEERS**

BORING **B-17**

PLASTICITY INDEX

33



GEOTECHNICAL ENGINEERS

B-18

PROJECT: Stella Road Improvements

DATE 3/25/2022 TYPE 4" Dry Auger LOCATION See Boring Location Plan SHEAR STRENGTH, TSF MOISTURE CONTENT, % DESCRIPTION DENSITY, PCF BLOWS / FT. INDEX GRID Coordinates (US Survey ft): **Confined Compression** Δ DEPTH IN FEET PLASTIC LIMIT Texas State Plane Zone: 4204 LIQUID LIMIT Unconfined Compression PLASTICITY • SAMPLE IN 200 MESH Easting: 2979443.158 Ο Pocket Penetrometer SYMBOL Ъ.Ч Northing: 13750595.24 DRY [Torvane Elevation: 93.80 (n 0.5 0 Firm to hard, gray Fat Clay (CH), with 67 20 47 96 slickensides 29 -with roots 0'-2' -dark gray 2'-6', with ferrous nodules 2'-18' 36 85.5 4 32 -tan 6'-12' 20 48 97 68 22 8 -with calcareous nodules 8'-10' 106.3 22 21 12 -tan and gray 12'-18' 23 103.0 24 16 -with silty sand seams and partings 16'-20' 18 33 99 51 18 -tan 18'-20' 20 20 Termination Depth = 20 feet 24 28 BORING DRILLED TO 20 FEET WITHOUT DRILLING FLUID WATER ENCOUNTERED AT N/A FEET WHILE DRILLING 🐺 WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 DRAFTED BY LOGGED BY DRILLED BY Van and Son SA AZ



NG **B-19**

D	AIE :	3/25/2022 TYPE <u>4" Dry Auger</u>	LOCATION See Boring Location Plan						<u> </u>	
		DESCRIPTION		Γ, %		SHEAR STRENGTH, TSF				
DEPTH IN FEET	SYMBOL SAMPI F INTERVAL	GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2980368.432 Northing: 13750882.72 Elevation: 94.19	S.P.T. BLOWS / FT.	MOISTURE CONTENT,	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 	J -200 MESH	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX
0		Stiff to hard, dark gray Fat Clay (CH), with slickensides		42			96	79	25	54
		-with roots 0'-4'								
				37			+			
- 4 -		-gray and tan 4'-6', with ferrous nodules 4'-8'		35	87.9					
		-reddish tan 6'-8', with calcareous nodules 6'-		30	07.9					
		10'		23						
- 8 -		-tan and tannish gray 8'-10'					98	68	22	46
				28	96.4					
		-gray and reddish tan 10'-12'		29						
- 12 -		-tan and light gray, with calcareous nodules					+			
		12'-14'		22	105.7		+			
		Stiff, tan and light gray Lean Clay (CL), with silt partings and siltstone nodules		12		C+		32	15	17
- 16 -		Very dense, tan Silty Sand (SM), with					37			
	X	cemented sand nodules	54	5						
	X	-with clayey sand pockets 18'-20'	61	7			+			
- 20 -	X	Termination Depth = 20 feet					+			
- 24 -							+			
							+			
- 28 -							+			
		I NG DRILLED TO <u>20</u> FEET WITHOUT DR					<u>+1</u>			L
		R ENCOUNTERED AT <u>N/A</u> FEET WHILE R LEVEL AT N/A FEET AFTER COMPLE			lG ≟					
		ED BY Van and Sons DRAFTED BY		S	۹	LOGGED BY	AZ			
PF	ROJEC	CT NO. G126-21					PLA	TE	A-2	1

B-20 ì

PLASTICITY INDEX

PLASTIC LIMIT LIQUID LIMIT -200 MESH

87 77 24 53

79 25 54

49 18 31

						AVIL	ES				
PI	ROJE	CT: Stella Road Improvements			E	ENGINEERING GEOTECHNICAL E	G CORP.	BORING	E	3-20	
D	ATE 🛓	5/12/2022 TYPE <u>4" Dry Auger</u>				CATION S			ion l	Plan	1
DEPTH IN FEET	SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2980993.076 Northing: 13751108.265 Elevation: 98*	S.P.T. BLOWS / FT	MOISTURE CONTENT, %	DRY DENSITY, PCF	🕒 Unco	ned Com nfined Co et Penetr	pression ompression ometer	-200 MESH	LIQUID LIMIT	
- 4 -		Fill: hard, black and gray Fat Clay (CH), with silty sand pockets and gravel -with roots and calcareous nodules 0'-2' -with ferrous nodules 2'-4' Stiff to hard, dark gray Fat Clay (CH), with slickensides -with ferrous nodules 4'-6'		23 32 33 28	87.8		•		87	77	
- 8 -		-tan and tannish gray, with calcareous nodules 8'-10', and ferrous nodules 8'-14' -tan 10'-12'		23 23	104.5			О		79	
- 12 -		-tan and gray, with calcareous nodules 12'-14'		24						49	
- 16 -		Hard, tan and gray Lean Clay (CL), with slickensides and ferrous nodules Termination Depth = 15 feet *: boring not surveyed; coordinates and elevation are estimated.		17						49	
- 20 -											
- 24 -											
- 28 -											
		G DRILLED TO <u>15</u> FEET WITHOUT DR R ENCOUNTERED AT N/A FEET WHILE									-

WATER LEVEL AT N/A FEET AFTER COMPLETE 🐺 DRILLED BY Van and Sons DRAFTED BY

PROJECT NO. G126-21 BA

LOGGED BY

LW



ENGINEERING CORP. GEOTECHNICAL ENGINEERS BORING B-21

DATE	5/12/2022	

PROJECT: Stella Road Improvements

DATE 5/12/2022 TYPE 4" Dry Auger LOCATION See Boring Location Plan

				%		S	HEAF	R ST	RE	NG	TH,	TSF	-				
		DESCRIPTION		, L N I	Ļ.												×
H	ERVA	GRID Coordinates (US Survey ft):	%/ FT	ILNO	, PC	Δ	Cor	nfine	d C	omr	ores	sion	1			_	NDE)
DEPTH IN FEET	NT	Texas State Plane Zone: 4204	S.P.T. BLOWS / FT	MOISTURE CONTENT,	DENSITY, PCF	•	Und	confi	ned	d Co	mpr	essi		н	MIT	PLASTIC LIMIT	PLASTICITY INDEX
́ Н ⊨	SYMBOL	Easting: 2981100.064 Northing: 13750814.117	T. BL	STUF	DEN	0		cket l		netro	ome	ter		200 MESH	LIQUID LIMIT	STIC	STICI
DEP	SYM SAM	Elevation: 97*	SP	MOI	DRY		Tor 0.5	vane 1		1.5		2		-200	LIQL	PLA	PLA
0		Fill: hard, black and reddish tan Fat Clay (CH),					ÎÎ	ΠŤ	\prod	ŤŤ							
		with calcareous nodules and roots		22				\dagger	$\parallel \parallel$		\parallel		\mathbb{P}^+				
		Fill: dark brown Clayey Sand (SC), with gravel				┞┼┼┼		\ddagger	\ddagger	+++	++			17	47	16	31
		and asphalt pieces		9		┞┼┼┼		\ddagger	\ddagger	+++	+		β+				
4 -		Stiff to hard, gray Fat Clay (CH), with				┞┼┼┼		\ddagger	\ddagger	+++	+	/					
		slickensides and ferrous nodules		31	90.9	┞┽┼┼	┟┼┼┼	\ddagger	\ddagger	+++	•	ĦΫ					
		-olive gray and brown, with calcareous nodules				┞┼┼┼	$\left \right \right $	+++	\ddagger	+++	++	$\left \right $		95	98	29	69
		6'-8'		29	95.5	┞┼┼┼	╎┤┤┤	\ddagger	\ddagger	+++	++	ΗŔ					
8 -		-tan 8'-10'				┞┼┼┼		+++	\ddagger	+++	+						
				26		┞┼┼┼		\ddagger	\ddagger	+++	+		р +				
		-tan and gray 10'-12'				┞┽┼┼	┞┼┼┼	\ddagger	\ddagger	+++	+						
				24	102	┞┼┼┼		怜	\ddagger	+++	+	·	р+				
12 -		-tan 12'-14'				┞┼┼┼		\ddagger	\ddagger	+++	++	$\left \right \right $			66	24	42
				24				\ddagger	\ddagger	+++	++	·	р +				
		-tan and gray 14'-15'		24				+++	$\left \right \right $	+++	++		₿Н				
		Termination Depth = 15 feet				┞┼┼┼		\ddagger	\ddagger	+++	++						
16 -		*: boring not surveyed; coordinates and elevation are estimated.				┞┼┼┼		\ddagger	\ddagger	+++	+						
						┞┼┼┼		+++	\ddagger	+++	++						
						┞┼┼┼		\ddagger	\ddagger	+++	+						
						┞┼┼┼	$\left \right \right $	\ddagger	$\left \right \right $	+++	++						
20 -						┞┼┼┼	╎┤┤┤	+++	+++	+++	++	$\left \right \right $					
							╞┼┼┼	+++	+++	+++	++	$\left \right \right $					
						┞┼┼┼		+++	\ddagger	+++	++	$\left \right \right $					
						┞┼┼┼		+++	$\parallel \mid$	+++	+	$\left \right \right $					
24 -						┞┼┼┼	╞┼┼┼	+++	$\left \right \right $	+++	++						
						┞┽┼┼	┟┼┼┼	+++	+++	+++	++	$\left \right \right $					
						┞┼┼┼	╞┼┼┼	+++	+++	+++	++	$\left \right \right $					
						┞┼┼┼	╞┼┼┼	+++	+++	+++	++						
28 -							$\left \right \left \right $	+++	$\parallel \mid$	+++	++						
E	BORIN	G DRILLED TO 15 FEET WITHOUT DR		VG F	LUIC	<u> </u>)											
		RENCOUNTERED AT <u>N/A</u> FEET WHILE			1G -	Ţ											
		R LEVEL AT <u>N/A</u> FEET AFTER <u>COMPLE</u>	ETE		v			201	~	~ ~	~			P -			
		D BY Van and Sons DRAFTED BY		LV	V		(COC	ובר 	В	Ϋ́			BA			
PF	ROJEC.	Г NO. G126-21												PLA [.]	ΓF	Δ_2	3



ENGINEERING CORP. GEOTECHNICAL ENGINEERS BORING B-22

DATE <u>5/1</u>	2/2022 TYPE <u>4" Dry Auger</u>	LOCATION See Boring Location Plan					
	DECODIDITION		%		SHEAR STRENGTH, TSF		
DEPTH IN FEET SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2979703.639 Northing: 13750851.892 Elevation: 95*	S.P.T. BLOWS / FT	MOISTURE CONTENT,	DRY DENSITY, PCF	 △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2 		
	Very stiff to hard, dark gray Fat Clay (CH), with slickensides and ferrous nodules -with roots 0'-4' -tan and gray 4'-6' -tan 6'-8' -reddish tan and gray 8'-10', with calcareous nodules and pockets 8'-14' -tan and gray 10'-14', with silt partings 10'-12' Very dense, tan Silt (ML), with sandy lean clay pockets Termination Depth = 16 feet *: boring not surveyed; coordinates and elevation are estimated.	59	27 26 32 22 24 22 18 6	96.4	96 82 25 57		
- 28 -							
BORING	DRILLED TO 15 FEET WITHOUT DE		ng f	L LUIE	L+++++		
	ENCOUNTERED AT N/A FEET WHIL			NG -	$\overline{\Sigma}_{\overline{2}}$		
	LEVEL AT <u>N/A</u> FEET AFTER <u>COMPL</u> D BY Van and Sons DRAFTED BY	ETE		N	LOGGED BY BA		
PROJECT					<u></u>		



-tan and gray 14'-15'

16

20

24

28

Termination Depth = 15 feet

elevation are estimated.

*: boring not surveyed; coordinates and

11	(OJLO				G	EOTECHNICAL ENGINEERS
D	ATE <u>5</u> /	12/2022 TYPE 4" Dry Auger			LOC	CATION See Boring Location Plan
DEPTH IN FEET	I SYMBOL SAMPLE INTERVAL	DESCRIPTION GRID Coordinates (US Survey ft): Texas State Plane Zone: 4204 Easting: 2979319.157 Northing: 13750724.229 Elevation: 95*	S.P.T. BLOWS / FT.	MOISTURE CONTENT, %	DRY DENSITY, PCF	SHEAR STRENGTH, TSF △ Confined Compression ● Unconfined Compression ○ Pocket Penetrometer □ Torvane 0.5 1 1.5 2
0		Stiff to hard, dark gray Fat Clay (CH), with slickensides and ferrous nodules		25		
		-with roots 2'-4'		25	99.7	
- 4 -		-gray and reddish brown 4'-6'		29	92.1	97 76 23 53
		-tan 6'-8'		24		
- 8 -		-tan and gray 8'-12', with calcareous nodules 8'-10'		25	102.3	
				24		100 63 22 41
- 12 -		-reddish tan 12'-14'				

FEET WITHOUT DRILLING FLUID

LW

WATER ENCOUNTERED AT N/A FEET WHILE DRILLING 🐺

DRAFTED BY

WATER LEVEL AT N/A FEET AFTER COMPLETE

Van and Sons

24

25

99.0

DRILLED BY

BORING DRILLED TO 15

BA

LOGGED BY

B-23

KEY TO SYMBOLS

Symbol	Description	Symbol	Description
Strata	symbols		Auger
	Paving		
	Fill		
	High plasticity clay		
	Low plasticity clay		
	Silty sand		
	Silty low plasticity clay		
	Silt		
Misc. S	Symbols		
	Torvane		
0	Pocket Penetrometer		
•	Unconfined Compression		
\bigtriangleup	Confined Compression		
<u>Soil Sa</u>	amplers		
	Rock core		
	Undisturbed thin wall Shelby tube		
\square	Standard penetration test		



CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES

ASTM Designation D-2487

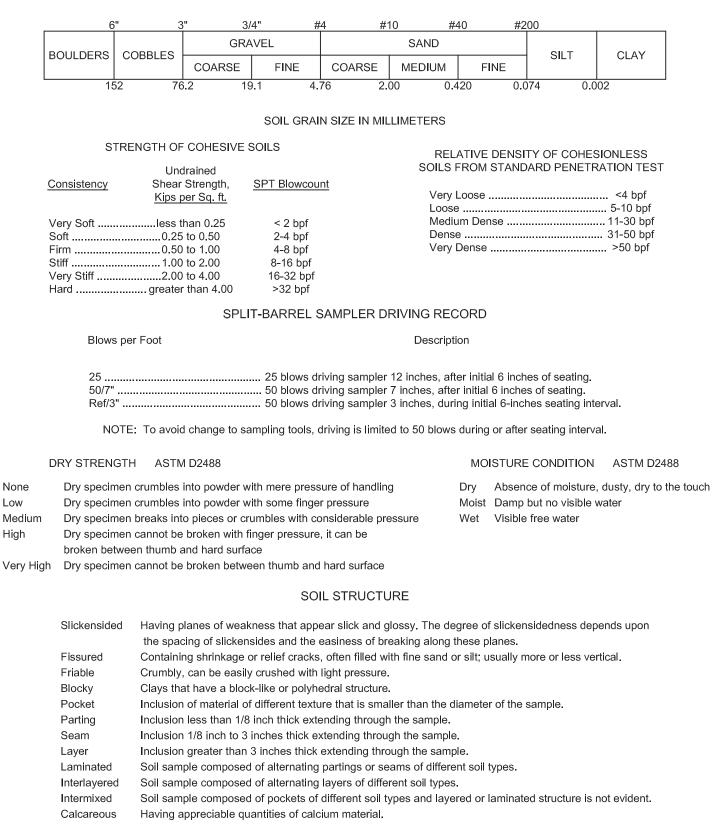
MAJOR DIVISIONS					TYPICAL NAMES		
COARSE-GRAINED SOILS (Less than 50% passes No. 200 sieve)	GRAVELS (Less than 50% of coarse fraction passes No. 4 sieve)	CLEAN GRAVELS (Less than 5% passes No. 200 sieve)		GW	Well-graded gravel, well-graded gravel with sand		
				GP	Poorly-graded gravel, poorly-graded gravel with sand		
		GRAVELS WITH FINES (More than 12% passes	Limits plot below "A" line & hatched zone on plasticity chart	GM	Silty gravel, silty gravel with sand		
		No. 200 sieve)	Limits plot above "A" line & hatched zone on plasticity chart	GC	Clayey gravel, clayey gravel with sand		
	SANDS (50% or more of coarse fraction passes No. 4 sieve)	CLEAN SANDS (Less than 5% passes No. 200 sieve)		SW	Well-graded sand, well-graded sand with gravel		
				SP	Poorly-graded sand, poorly-graded sand with gravel		
		SANDS WITH FINES (More than 12% passes	Limits plot below "A" line & hatched zone on plasticity chart	SM	Silty sand, silty sand with gravel		
		No. 200 sieve)	Limits plot above "A" line & hatched zone on plasticity chart	SC	Clayey sand, clayey sand with gravel		
	ve)			ML	Silt, silt with sand, silt with gravel, sandy silt, gravelly silt		
FINE-GRAINED SOILS (50% or more passes No. 200 sieve)			AND CLAYS t Less Than 50%)	CL	Lean clay, lean clay with sand, lean clay with gravel, sandy lean clay, gravelly lean clay		
				OL	Organic clay, organic clay with sand, sandy organic clay, organic silt, sandy organic silt		
				МН	Elastic silt, elastic silt with sand, sandy elastic silt, gravelly elastic silt		
			AND CLAYS nit 50% or More)	СН	Fat clay, fat clay with sand, fat clay with gravel, sandy fat clay, gravelly fat clay		
	(50			ОН	Organic clay, organic clay with sand, sandy organic clay, organic silt, sandy organic silt		
		ween 5% and 12% passing th hart are to have dual symbols.	e No. 200 sieve and fine-grained so	oils with limit	s plotting in the hatched zone		
PLASTICITY CHART					DEGREE OF PLASTICITY OF COHESIVE SOILS Degree of Plasticity Plasticity Index None		
Equ	ation of U-Lin	e: Vertical at LL=16 to PI=7,		Clay (CL) PLATE A-27			



TERMS USED ON BORING LOGS

SOIL GRAIN SIZE

U.S. STANDARD SIEVE





ASTM & TXDOT DESIGNATION FOR SOIL LABORATORY TESTS

SOIL TEST	ASTM TEST DESIGNATION	TXDOT TEST DESIGNATION		
Unified Soil Classification System	D 2487	Tex-142-E		
Moisture Content	D 2216	Tex-103-E		
Specific Gravity	D 854	Tex-108-E		
Sieve Analysis	D 6913	Tex-110-E (Part 1)		
Hydrometer Analysis	D 7928	Tex-110-E (Part 2)		
Minus No. 200 Sieve	D 1140	Tex-111-E		
Liquid Limit	D 4318	Tex-104-E		
Plastic Limit	D 4318	Tex-105-E		
Standard Proctor Compaction	D 698	Tex-114-E		
Modified Proctor Compaction	D 1557	Tex-113-E		
California Bearing Ratio	D 1883	-		
Swell	D 4546	-		
Consolidation	D 2435	-		
Unconfined Compression	D 2166	-		
Unconsolidated-Undrained Triaxial	D 2850	Tex-118-E		
Consolidated-Undrained Triaxial	D 4767	Tex-131-E		
Permeability (constant head)	D 5084	-		
Pinhole	D 4647	-		
Crumb	D 6572			
Double Hydrometer	D 4221	-		
pH of Soil	D 4972	Tex-128-E		
Soil Suction	D 5298	-		
Soil Sulfate	C 1580	Tex-145-E		
Organics	D 2974	Tex-148-E		

AVILES ENGINEERING CORPORATION

Consulting Engineers - Geotechnical, Construction Materials Testing, Environmental

RESULTS OF CRUMB TESTS (ASTM D 6572)

Project Name: Stella Road Reconstruction from Cottonwood School to Band Road, Fort Bend County, TexasProject No.: G126-21Test Date: 5/26/22

Boring Number	Depth, feet	2 Minutes		1 Hour		6 Hours	
Number	ieet	Grade	C (deg)	Grade	C (deg)	Grade	C (deg)
B-20	0-2	1	22.2	1	22.2	1	22.3
B-20	6-8	1	22.2	1	22.2	1	22.3
B-21	0-2	1	22.2	1	22.2	1	22.3
B-21	8-10	1	22.2	1	22.2	1	22.3
B-22	2-4	1	22.2	1	22.2	1	22.3
B-22	6-8	1	22.2	1	22.2	1	22.3
B-23	0-2	1	22.2	1	22.2	1	22.3
B-23	6-8	1	22.2	1	22.2	1	22.3
		<u> </u>					

Grade Classification:

Grade 1 Non-dispersive; No reaction

Grade 2 Intermediate; Slight reaction

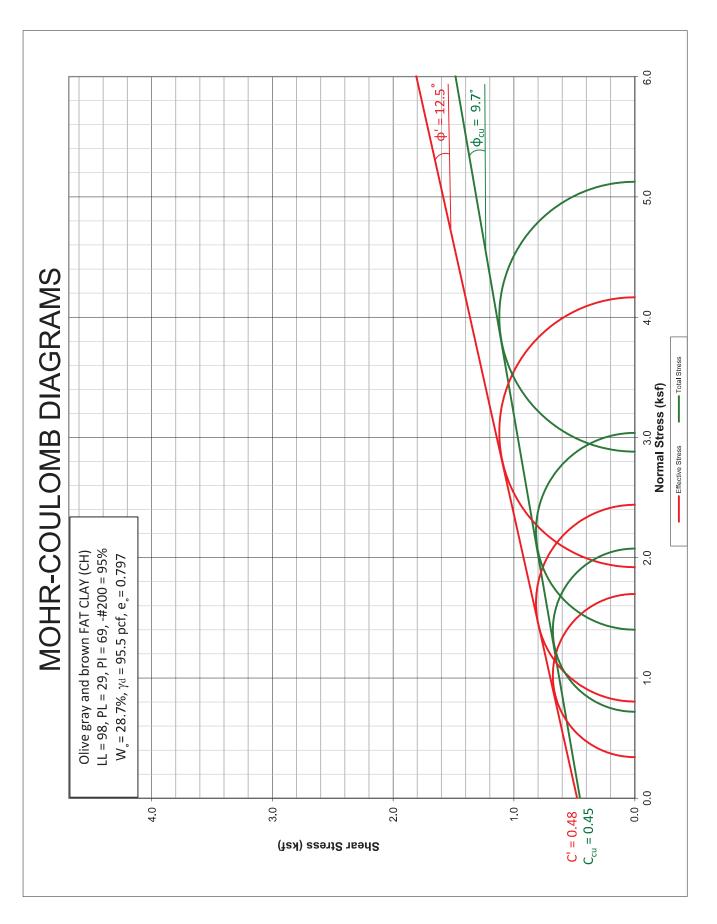
Grade 3 Dispersive; Moderate reaction

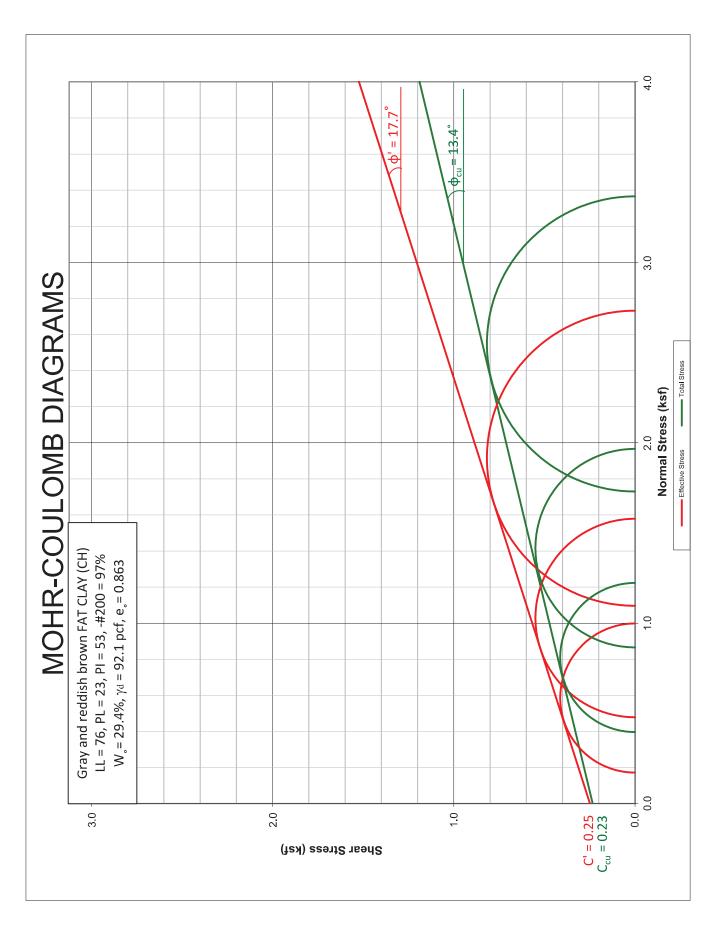
Grade 4 Highly Dispersive; Strong reaction

Interpretation:

Under normal conditions, use the 1 hour reading to determine dispersive grade.

However, if the dispersive grade changes from 2 to 3 or from 3 to 4 between the 1 and 6 hour readings, use the 6 hour reading instead.

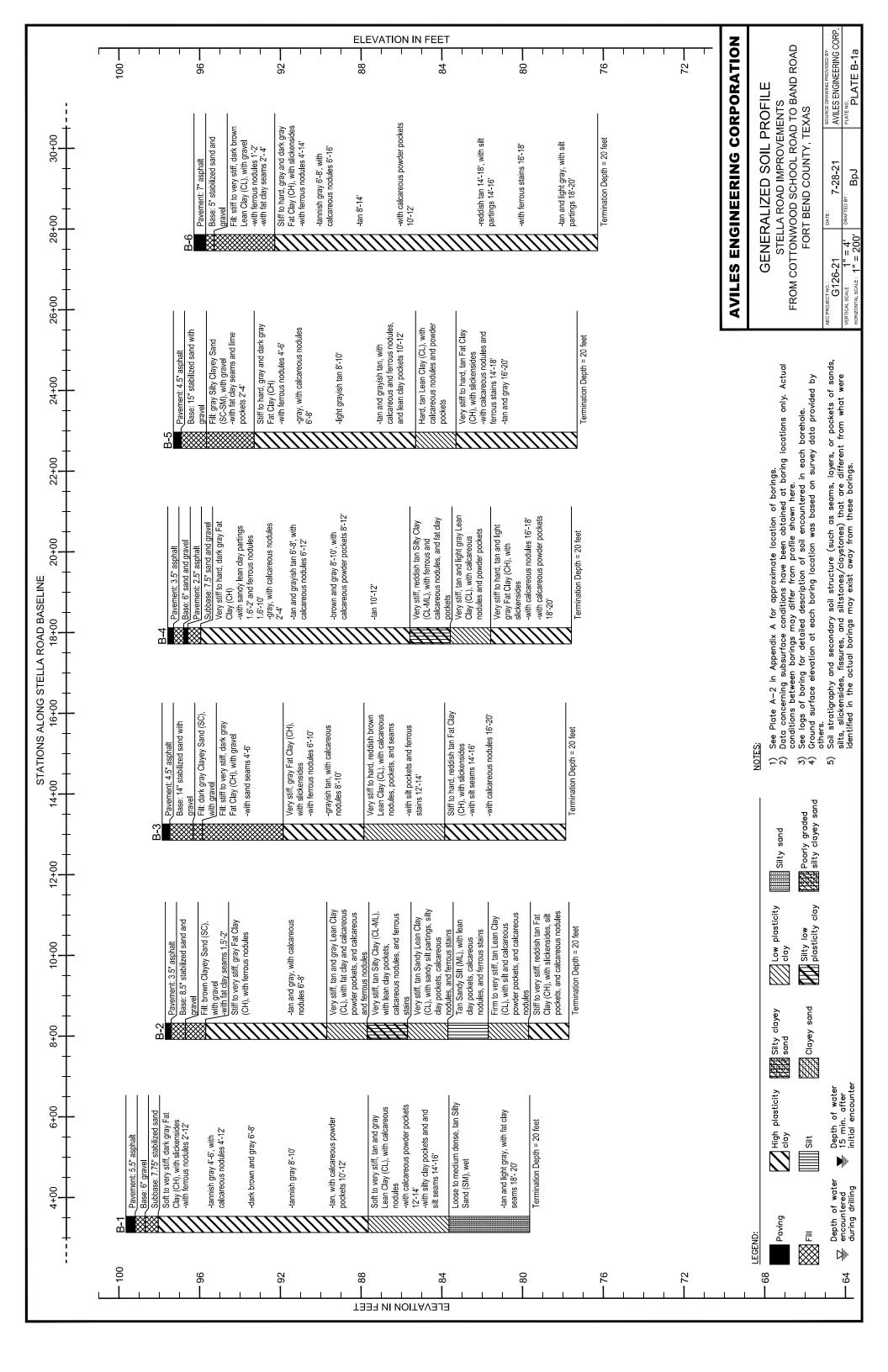


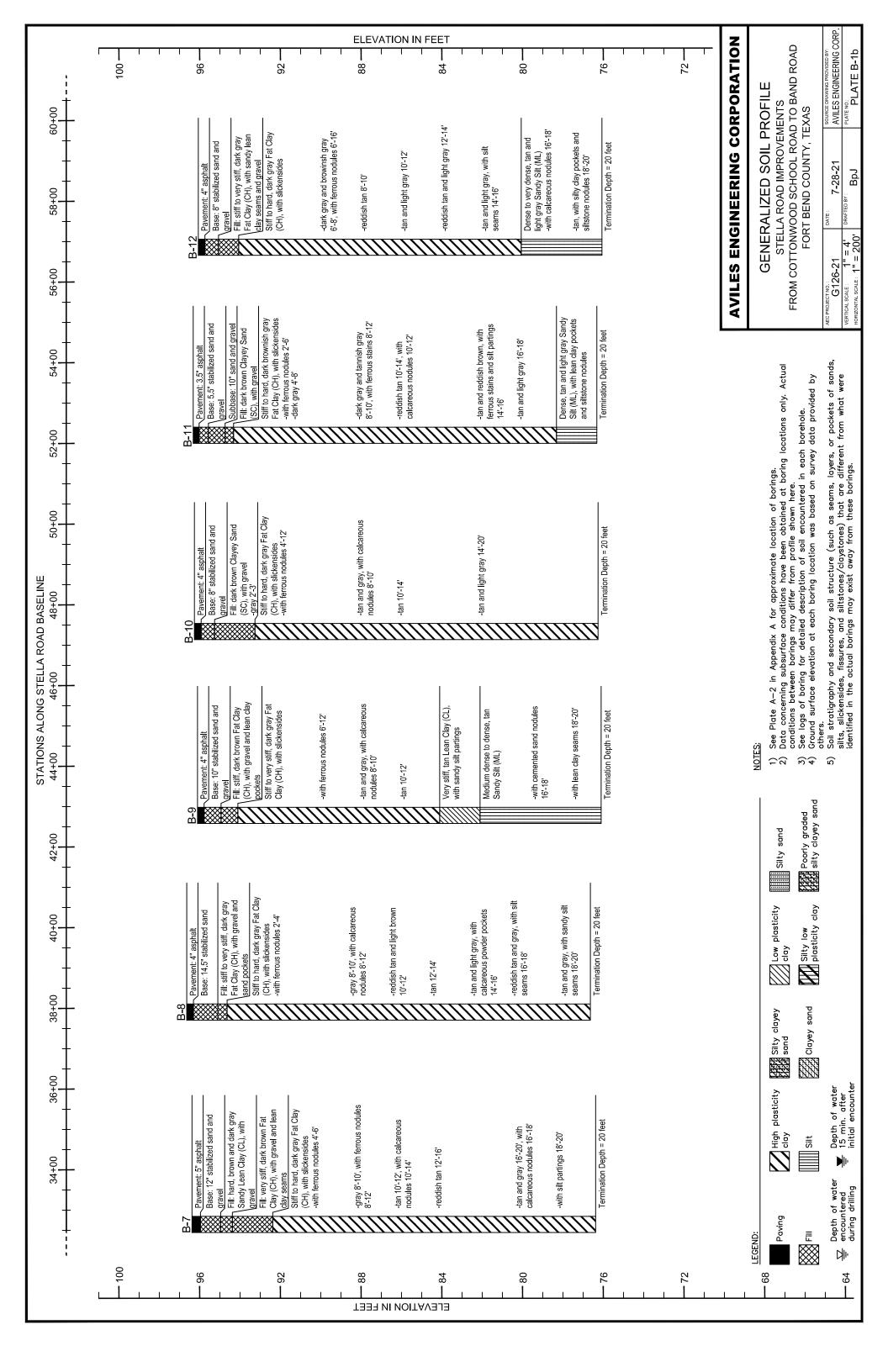


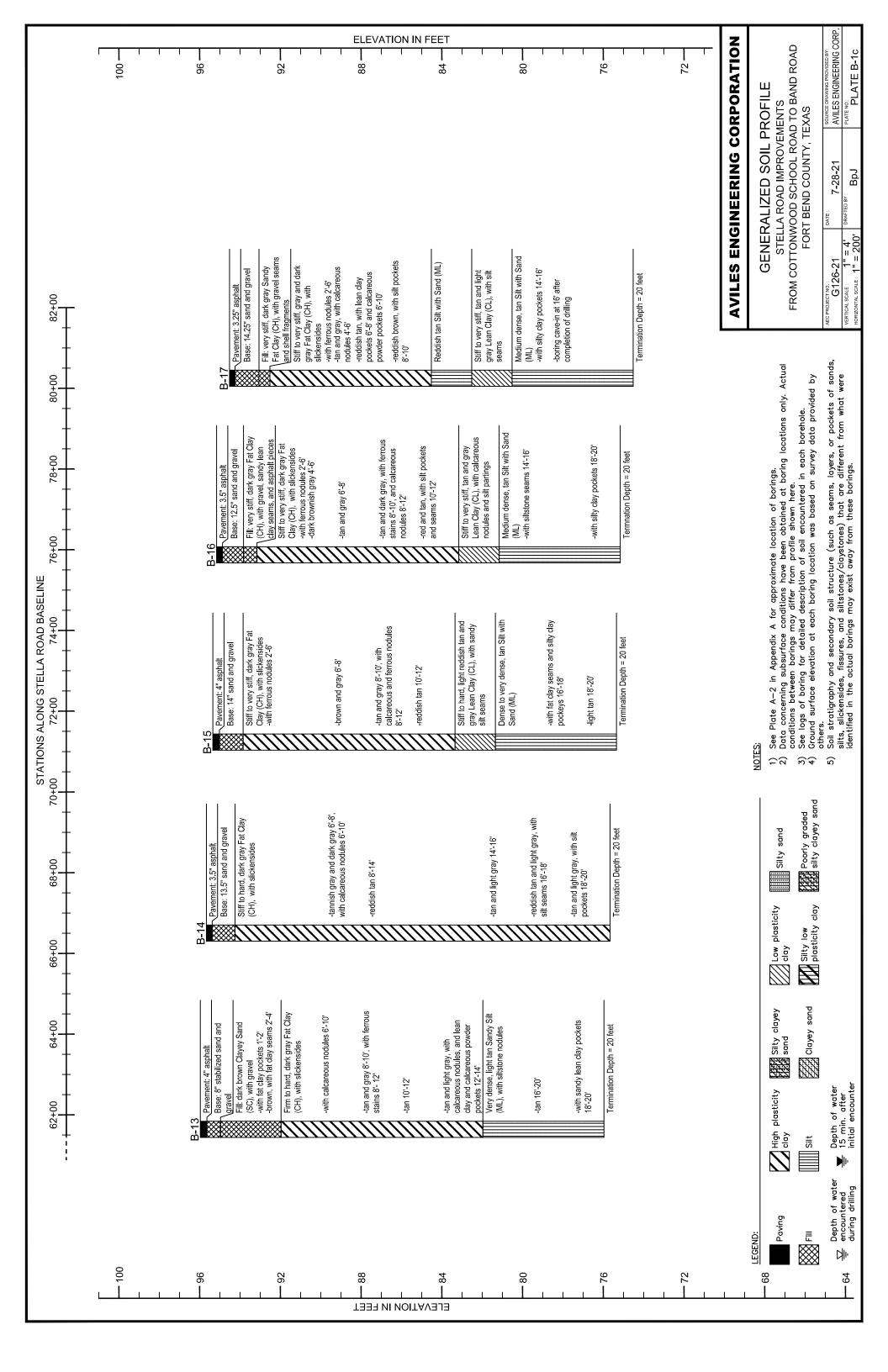


APPENDIX B

Plates B-1a to B-1c Generalized Soil Profiles









APPENDIX C

Plates C-1 to C-3 DARWin v3.0 Outputs for Asphalt Pavement Design

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product AVILES

Flexible Structural Design Module

Stella Road Reconstruction ESALs Calculation with FBC's Minumun Thickness Requirements

Flexible Structural Design

Structural Number	4.92 in
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	90 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	1,500 psi
Stage Construction	1
18-kip ESALs Over Initial Performance Period	226,282

Effective Roadbed Soil Resilient Modulus

Period 1

Description 1

Calculated Effective Modulus

Specified Layer Design

1,500 psi

		Struct	Drain			
		Coef.	Coef.	Thickness	Width	Calculated
Layer	Material Description	<u>(Ai)</u>	<u>(Mi)</u>	<u>(Di)(in)</u>	<u>(ft)</u>	<u>SN (in)</u>
1	Asphalt	0.44	1	3	-	1.32
2	Black Base	0.34	1	8	-	2.72
3	Stabilized Subgrade	0.11	1	8	-	0.88
Total	-	-	-	19.00	-	4.92

Roadbed Resilient

Modulus (psi)

1,500

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product AVILES

Flexible Structural Design Module

Stella Road Reconstruction Structural Number Calculation to Meet Estimated Design Life ESALs

Flexible Structural Design

18-kip ESALs Over Initial Performance Period	678,526
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	90 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	1,500 psi
Stage Construction	1
Calculated Design Structural Number	5.73 in

Effective Roadbed Soil Resilient Modulus

Period 1 Description 1 Roadbed Resilient <u>Modulus (psi)</u> 1,500

Calculated Effective Modulus

1,500 psi

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product AVILES

Flexible Structural Design Module

Stella Road Reconstruction ESALs Calculation with Recommended Pavement Design

Flexible Structural Design

Structural Number	5.82 in
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	90 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	1,500 psi
Stage Construction	1
18-kip ESALs Over Initial Performance Period	760,267

Effective Roadbed Soil Resilient Modulus

Period 1

Description 1

Calculated Effective Modulus

Specified Layer Design

1,500 psi

		Struct	Drain			
		Coef.	Coef.	Thickness	Width	Calculated
Layer	Material Description	<u>(Ai)</u>	<u>(Mi)</u>	<u>(Di)(in)</u>	<u>(ft)</u>	<u>SN (in)</u>
1	Asphalt	0.44	1	3.5	-	1.54
2	Black Base	0.34	1	10	-	3.40
3	Stabilized Subgrade	0.11	1	8	-	0.88
Total	-	-	-	21.50	-	5.82

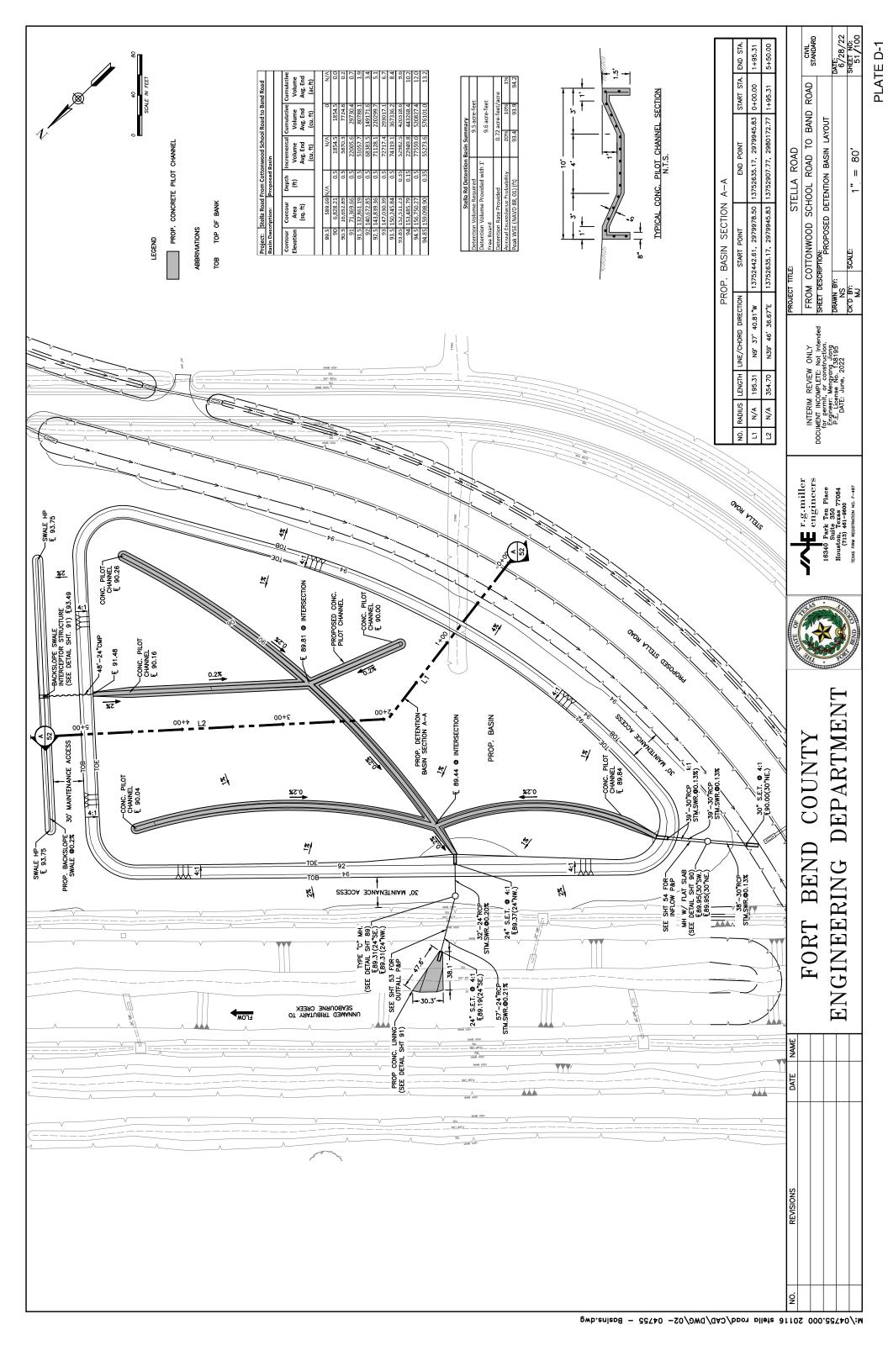
Roadbed Resilient 1,500

Modulus (psi)



APPENDIX D

Plates D-1 and D-2r.g. Miller Engineers drawings, Detention Basin Layout and Cross Sections, dated
June 28, 2022Plate D-3Design Soil Parameters for Slope Stability AnalysesPlates D-4 to D-9Detention Basin Slope Stability Analyses





1" = 40'

SCALE:

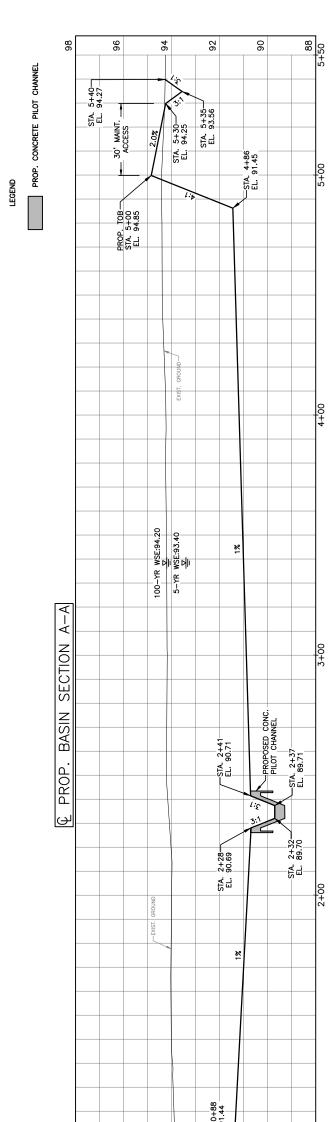
DATE: 6/28/22 SHEET NO: 52 /100

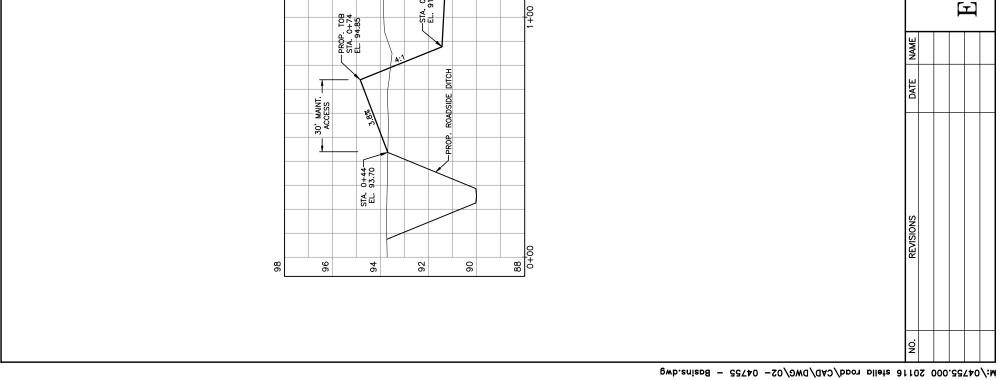
CIVIL STANDARD

0 20 40 SCALE IN FEET

STELLA ROAD

FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD SHEET DESCRIPTION: PROPOSED DETENTION BASIN SECTION PROJECT TITLE: DRAWN BY: NS CK'D BY: MJ INTERIM REVIEW ONLY DOCUMENT INCOMPLETE: Not Intended for permit, or construction. Fingmeer: Mengyong Jung P.E. License No., 1233 95 DATE: June, 2023 r.g.miller engineers 18340 Park Ten Place Bute 350 77084 (713) 461-9600 TENS FRM REGISTRATION NO. F-467 ENGINEERING DEPARTMENT FORT BEND COUNTY







Elevation (ft)	Soil Type	γm	Short- Term/Undrained Parameters		Effective Stress Parameters		Total Stress Parameters	
		(pcf)	C _u (psf)	φ _u (deg)	C' (psf)	φ' (deg)	C _{cu} (psf)	ф _{cu} (deg)
94 to 90	Hard CH	125	2500	0	$160 (C_r = 55)$	$16 \ (\phi_r = 15)$	$150 (C_r = 55)$	$15 (\phi_r = 15)$
90 to 88	Stiff CH	119	1750	0	$120 (C_r = 55)$	$16 \\ (\phi_r = 15)$	$110 (C_r = 55)$	$15 (\phi_r = 15)$
88 to 79	Very stiff to hard CH	124	2,100	0	$140 (C_r = 55)$	$\frac{16}{(\phi_r = 15)}$	$120 (C_r = 55)$	$\frac{15}{(\phi_r = 15)}$

Design Soil Parameters for Slope Stability Analyses Detention Basin (Based on Boring B-23)

Notes: (1) γ_m = moist unit weight of soil.

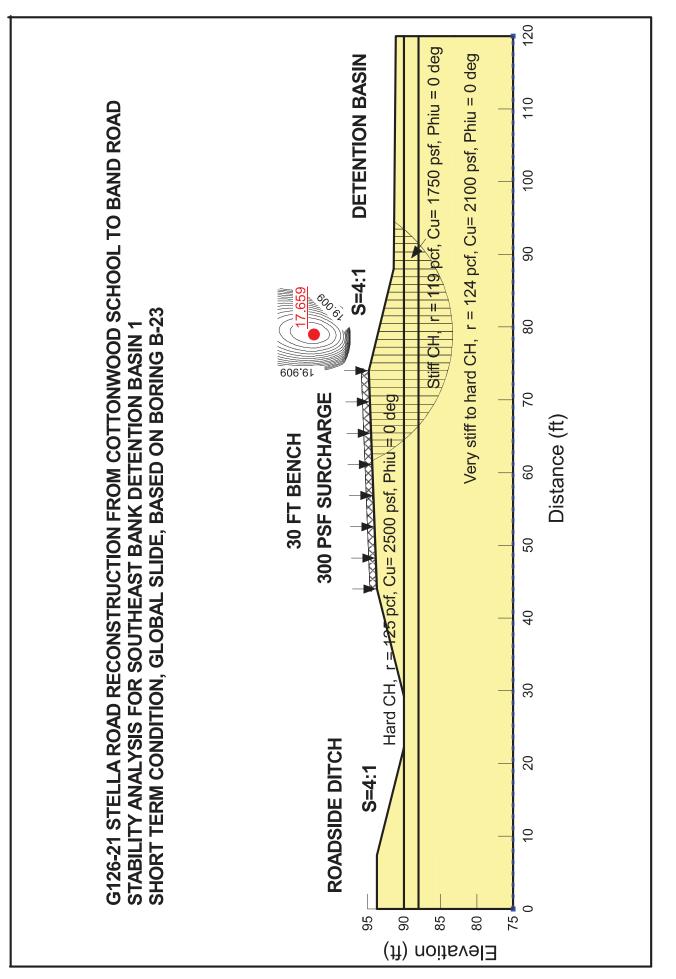
(2) C_u =undrained cohesion, ϕ_u = angle of internal friction, under short term conditions. UU = strength parameters that were determined from Unconsolidated-Undrained (UU)triaxial tests.

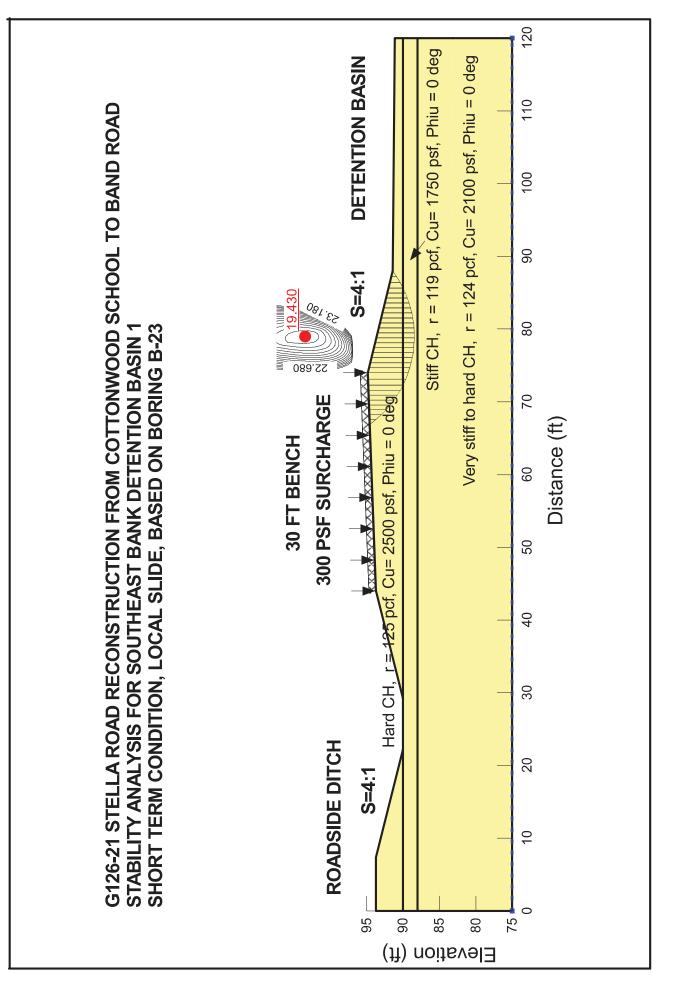
(3) C' =effective cohesion, φ' =effective friction angle, effective stress parameters that were determined from Consolidated-Undrained (CU) triaxial tests with pore water pressure measurements.

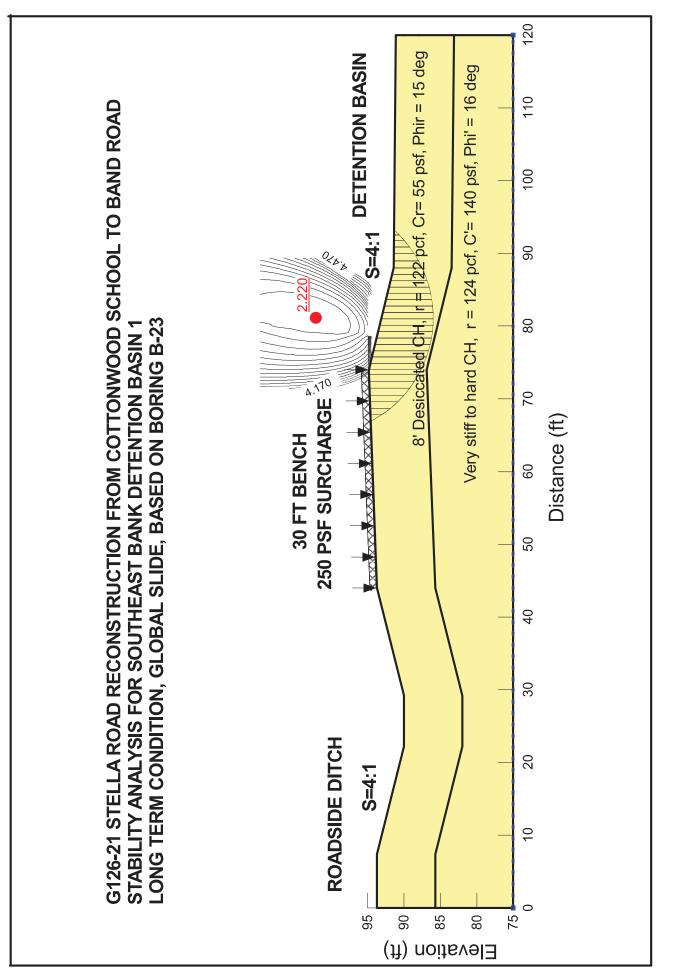
(4) C_{cu} = cohesion, ϕ_{cu} = friction angle, total stress parameters that were developed from CU triaxial tests.

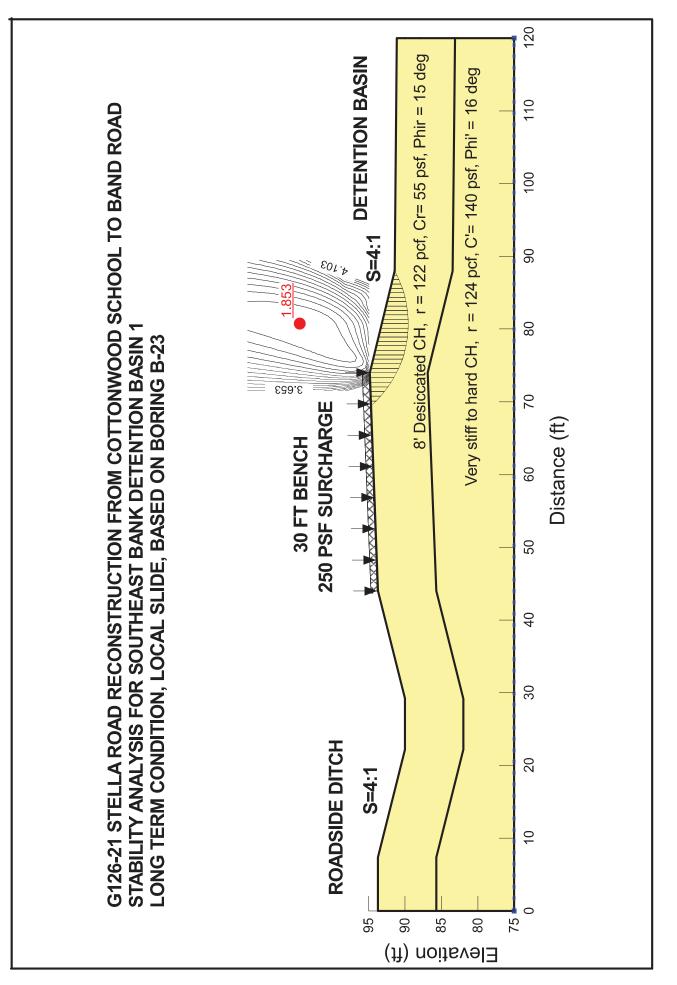
(5) C_r = cohesion for desiccated fat clay, ϕ_r = friction angle for desiccated fat clay.

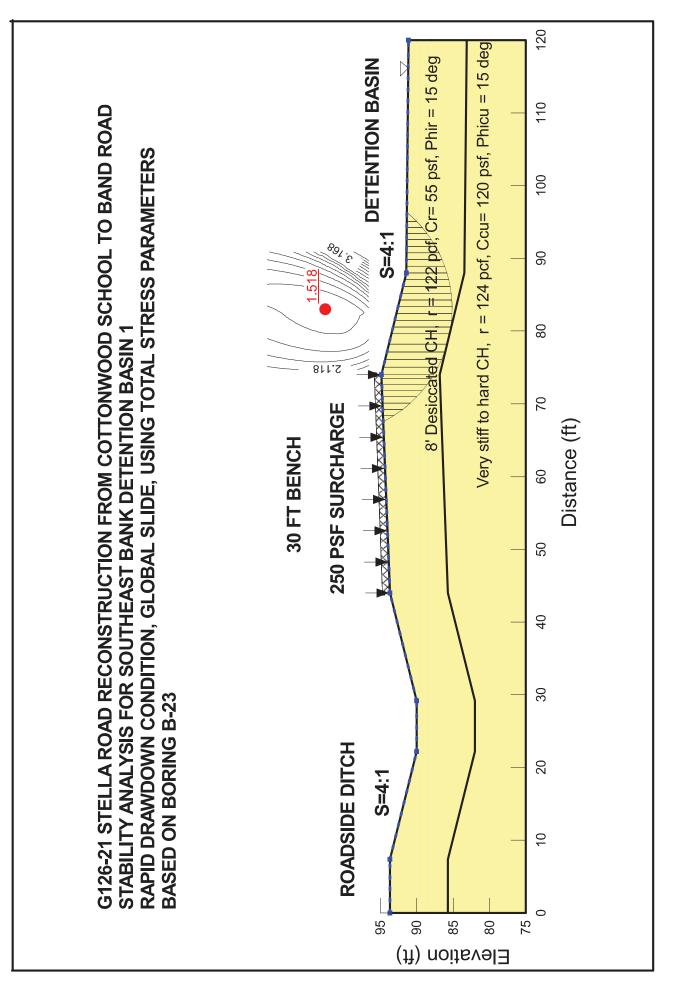
(6) CH = Fat Clay

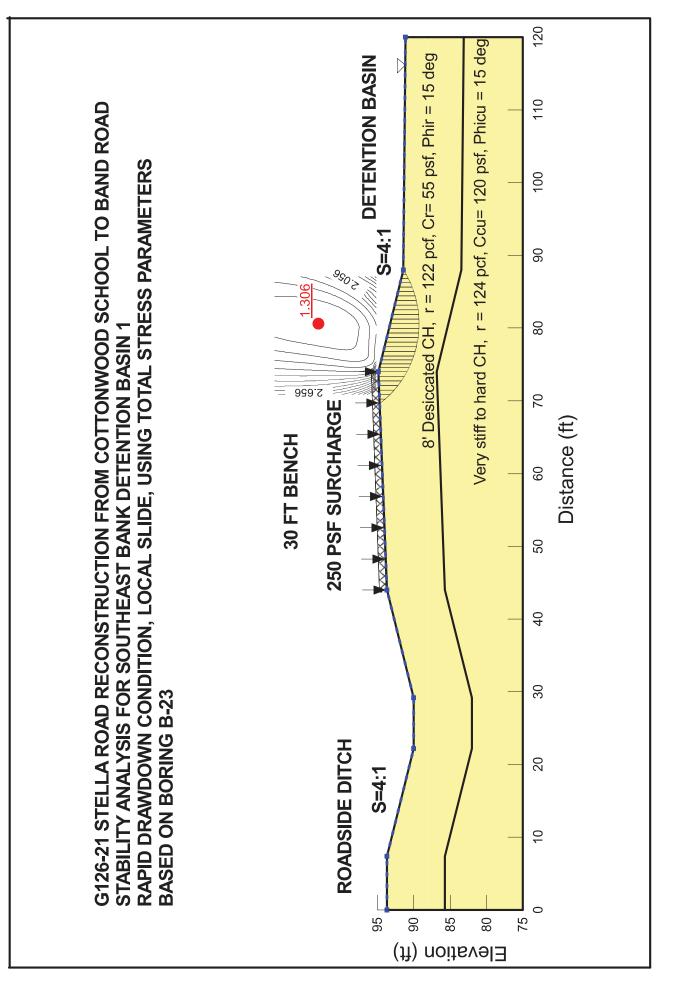






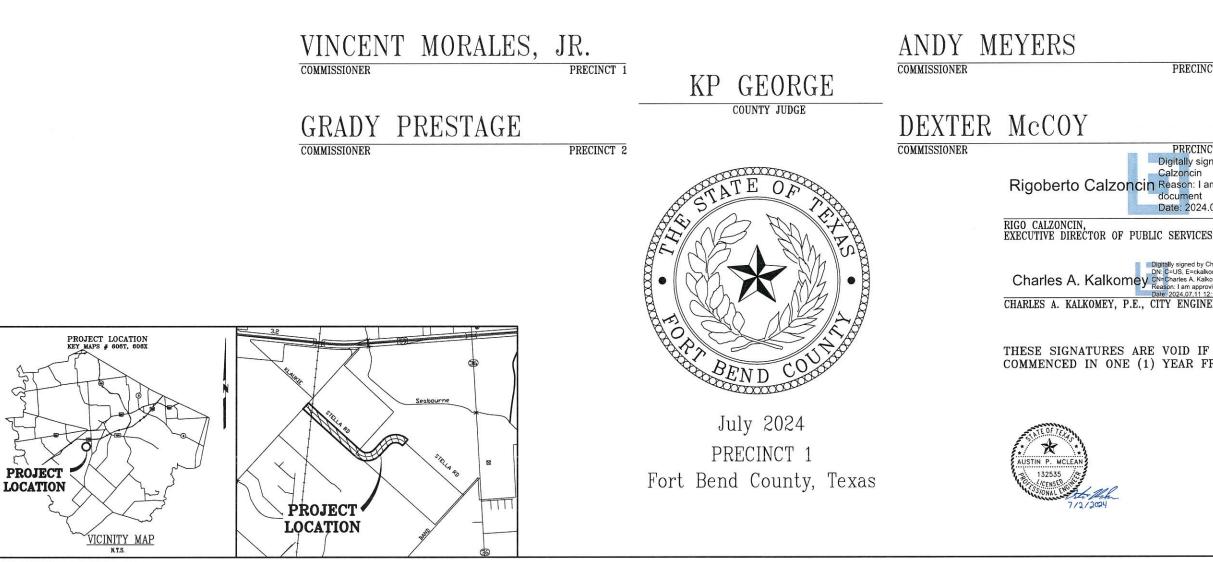






FORT BEND COUNTY ENGINEERING DEPAR' STELLA ROAD FROM COTTONWOOD SCHOOL RD TO W FAIRGROUNDS RD

FORT BEND COUNTY MOBILITY BOND PROJECT #20116 PRECINCT 1



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PRECINCT 3

PRECINCT 4 Digitally signed by Rigoberto Calzoncin Rigoberto Calzoncin Reason: I am approving this document Date: 2024.07.12 08:02:36-05'00'

DATE

Charles A. Kalkomey DN: C=US, E=ckalkomey@rosenbergb.gov CN=Charles A. Kalkomey Reason: I am approving this document Date: 2024.07.11112:14:18-05'00' CHARLES A. KALKOMEY, P.E., CITY ENGINEER

DATE

THESE SIGNATURES ARE VOID IF CONSTRUCTION HAS NOT COMMENCED IN ONE (1) YEAR FROM DATE OF APPROVALS.

SLAWINSKI, P.E.

GENERAL

PROJECT LAYOUT	97-100 SWPPP SHEETS
TYPICAL PAVEMENT SECTIONS	
CONSTRUCTION NOTES	SWPPP
GENERAL NOTES	
LEGEND	
INDEX SHEET	
COVER SHEET	92-96 SIGNING AND PAVEMENT MARKING PLA
	INDEX SHEET LEGEND GENERAL NOTES CONSTRUCTION NOTES TYPICAL PAVEMENT SECTIONS

RIGHT-OF-WAY LAYOUT

		105	FBCED STORM SEWER CONSTRUCTION
		104	FBCED DRIVEWAY DETAILS FOR MAJ
37	HORIZONTAL ALIGNMENT DATA SHEET	103	FBCED ASPHALT DRIVEWAY DETAILS
9-36	TOPOGRAPHIC AND ROW SURVEY	102	FBCED TYPICAL PAVEMENT SECTION

REMOVAL PLAN

38-44	REMOVAL PLAN

PLAN AND PROFILE

45 - 56	PLAN AND PROFILE
57	DRIVEWAY SUMMARY TABLE
58	PROPOSED LATERAL CROSS SECTIONS
DRAINAGE	
50	
59	PROPOSED OVERALL DRAINAGE LAYOUT
60	PROPOSED DETENTION BASIN LAYOUT
61	PROPOSED DETENTION BASIN SECTION
62	PROPOSED DETENTION BASIN OUTFALL P&P
63	PROPOSED DETENTION BASIN INFLOW P&P
64	PROPOSED DETENTION BASIN GEOMETRIC LAYOUT
65	GEOMETRIC LAYOUT POINT DATA
66-67	EXISTING DRAINAGE MAP
68-69	PROPOSED DRAINAGE MAP
70	EXISTING DRAINAGE CALCULATIONS
71	PROPOSED DRAINAGE CALCULATIONS
72	PROPOSED CULVERT INPUT DATA

- 73 EXISTING CULVERT INPUT DATA 74 PROPOSED CULVERT NODE RESULTS
- PROPOSED CULVERT LINK RESULTS 75

TRAFFIC CONTROL PLAN

76-77	TCP TYPICAL SECTIONS
78-79	OVERALL TCP PHASING
80-84	TRAFFIC CONTROL PLAN PHASE 1
85-90	TRAFFIC CONTROL PLAN PHASE 2
91	STELLA ROAD DETOUR PLAN

NO.	REVISIONS	DATE	NAM
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SIGNING AND PAVEMENT MARKING

97-100	SWPPP SHEETS
101	PROPOSED DETENTION BASIN SWPPP

CONSTRUCTION DETAILS

- ONS FOR DEVELOPMENT PROJ.
- LS
- AJOR ROADWAY CONSTRUCTION
- TION DETAILS
- 106 FBCED PRECAST CONCRETE STORM SEWER MANHOLE DETAILS
- 107 FBCED JUNCTION BOX AND MANHOLE DETAILS
- 108 TXDOT PRECAST S.E.T.-TYII-PARALLEL DRAINAGE (PSET-SP)
- TXDOT PRECAST S.E.T.-TYII-CROSS DRAINAGE (PSET-SC) 109
- 110 INTERCEPTOR STRUCTURE DETAILS
- 111 URBAN INTERCEPTOR STRUCTURE DETAILS
- 112 STORM SEWER AND RIPRAP DETAILS 113 CONCRETE HEADWALL DETAIL
- 114 CONCRETE CHANNEL LINING DETAIL
- 115 SWPPP DETAILS
- 116 SINGLE BOX CULVERT DETAIL
- 117-118 FBCED PAVEMENT MARKING DETAILS
- 119 FBCED TYPE III BARRICADE DETAILS

FORT BEND COUNTY TEXAS

- 120 FBCED TYPICAL GROUND SIGN INSTALLATION
- 121 FBCED STREET SIGN NAME DETAILS
- 122 FBCED SWPPP DETAILS
- 123-124 FBCED PROJECT SIGN DETAILS

CROSS SECTIONS

125-133 CROSS SECTIONS

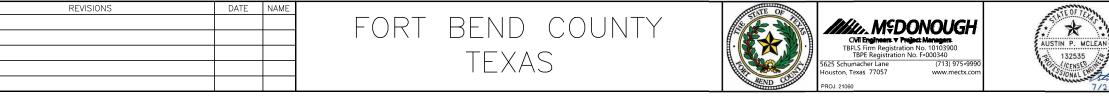
5625 Schumacher Lane iston, Texas 77057

201 21060

24	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CLEAN	CK'D BY: AM	SHEET DESCRIPTION: INDEX SHEET	
NCH Com	SCALE:		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	2 / 133

	SURVEY
C	- GUY ANCHOR
-•-	- POWER POLE
	- TELE. PEDESTAL
×	- WATER SPIGOT
Ø	- LIGHT POLE
	- MAILBOX
q	- STREET SIGN
	- FLUSH VALVE
×	- WATER VALVE
×	- BLOW OFF VALVE
	- WATER METER
——EI—	- OVERHEAD UTILITY LINE
	- BARBWIRE FENCE
	- WOOD FENCE
	- HOGWIRE FENCE
"S"	- SET 5/8" IR W/CAP
P.K.F.B.C. I	- PLAT RECORDS,
	FORT BEND CO.

D.R.F.B.C.T. - DEED RECORDS, FORT BEND CO. O.R.F.B.C.T. - OFFICIAL RECORDS, FORT BEND CO. O.P.R.F.B.C.T. - OFFICIAL PUBLIC RECORDS, FORT BEND CO.



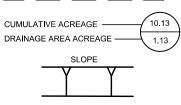
NO.

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LEGEND

PROPOSED

SOUTH/WEST ROW NORTH/EAST ROW SWALE ____ STORM SEWER SANITARY SEWER WATERLINE (WL) CURB PHONE DUCTS GAS LINE SAWCUT TRAFFIC SIGN BARRICADE GATE VALVE **م** GUY WIRE TYPE "BB" INLET TYPE "C" INLET TYPE "A" INLET STORM MANHOLE 0 TOP OF CURB TC FLOW LINE FL CENTER LINE CL PAVEMENT PVMT. STREET LIGHT PROFILE GRADE LINE PROFILE GRADE LINE PGL HYDRAULIC GRADE LINE HGL VERTICAL POINT OF INTERSECTION VPI POINT OF TANGENCY PT POINT OF COMMENCEMENT PC LINEAR FEET LF LINEAR FEET LF WATER SURFACE ELEVATION WSEL INVERT INV DIRECTION OF FLOW DRAINAGE BOUNDARY



EXISTING

EXIGNING
SOUTH/WEST ROW
SWALE
STORM SEWER
SANITARY SEWER
WATERLINE (WL)
CURB
GAS LINE
ESMT. LINE
BARRICADE
F.H. W/ VALVE
GATEVALVE
GUY WIRE
TYPE "H-2" INLET
TYPE "A" INLET
STORM MANHOLE
SANITARY MANHOLE
TOP OF CURB
TC FLOW LINE
FL
CENTER LINE CL
PAVEMENT
PVMT. STREET LIGHT
0
BUSINESS SINGN
POWER POLE
PHONE MH
EBOX
WATER SURFACE ELEVATION WSEL
GRAVEL
REMOVE CONCRETE
SLOPE
Y Y
STELLA ROAD

24	E: STELLA ROAD		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CLEAN	CK'D BY: AM	SHEET DESCRIPTION: LEGEND	
Charles I and	SCALE:		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	3 / 133

GENERAL NOTES

- 1. THE CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS BEFORE BEGINNING CONSTRUCTION.
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING SECURITY TO PROTECT THE PROJECT SITE, CONTRACTOR PROPERTY, EQUIPMENT, AND WORK.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR CLEANING STREETS OF CONSTRUCTION DIRT AND DEBRIS AT CLOSE OF EACH WORK DAY.
- 4. THE CONDITION OF THE ROAD AND/OR RIGHT-OF-WAY, UPON COMPLETION OF THE JOB SHALL BE AS GOOD AS OR BETTER THAN PRIOR TO STARTING WORK.
- PRIOR TO CONSTRUCTION, THE CONTRACTOR, ALONG WITH CONCURRENCE FROM THE FIELD ENGINEER, SHALL DETERMINE HIS/HER LAY-DOWN AND/OR STAGING AREA LOCATIONS.
- 6. THE CONTRACTOR SHALL NOTIFY ALL PROPERTY OWNERS A MINIMUM OF 24 HOURS PRIOR TO BLOCKING DRIVEWAYS OR ENTERING UTILITY EASEMENTS.
- 7. TRAFFIC INGRESS AND EGRESS FOR DRIVEWAYS AND PEDESTRIAN ACCESS FACILITIES SHALL BE
- 8. THE CONTRACTOR SHALL REMOVE ANY FENCES, POSTS, MAILBOXES, PLANTERS, PERMANENT TRASH CONTAINERS, CULVERTS, ETC. OR SECTIONS THEREOF, THAT ENCROACH WITHIN THE COUNTY'S RIGHT-OF-WAY. NOTE: PRIOR TO CONSTRUCTION, THE PROPERTY OWNER WAS PAID TO RELOCATE OR REPLACE THESE ITEMS OUTSIDE OF THE COUNTY'S RIGHT-OF-WAY. IF THE OWNER HAS FAILED TO DO SO, THE CONTRACTOR WILL REPLACE THEM WITH THE MINIMUM LEVEL OF QUALITY NEEDED TO SECURE THE PROPERTY AND/OR MAINTAIN MAIL DELIVERY. IN THAT CASE, PAYMENT FOR THESE INSTALLATIONS WILL BE INCLUDED AS EXTRA WORK ITEMS OR AS OVERRUNS TO EXISTING PAY ITEMS.

ANY DAMAGE CAUSED BY THE CONTRACTOR TO SUCH ITEMS LOCATED OUTSIDE OF THE COUNTY'S RIGHT-OF-WAY, SHALL BE REPLACED WITH LIKE-KIND OR BETTER AT THE CONTRACTOR'S EXPENSE.

ALSO, IF THESE ITEMS ARE LOCATED WITHIN THE PROJECT RIGHT-OF-WAY AND ARE DESIGNATED TO REMAIN, ANY DAMAGE CAUSED BY THE CONTRACTOR TO SUCH ITEMS. SHALL BE REPLACED WITH LIKE-KIND OR BETTER AT THE CONTRACTOR'S EXPENSE.

TREES, BUSHES, SHRUBBERY AND OTHER DAMAGED PLANTINGS DESIGNATED TO REMAIN SHALL BE REPLACED WITHIN 72 HOURS OF REMOVAL AND ARE TO BE THOROUGHLY WATERED-IN. NO SEPARATE PAY.

- 9. PAVED SURFACES. PAVEMENT MARKERS AND MARKINGS SHALL BE PROTECTED FROM DAMAGE
- 10. IRON RODS DISTURBED DURING CONSTRUCTION ARE TO BE REPLACED BY A REGISTERED PROFESSIONAL LAND SURVEYOR FOR THE ORIGINAL PROPERTY OWNER AT NO SEPARATE PAY.
- 11. CONSTRUCTION STAKING WILL BE PROVIDED BY THE CONTRACTOR. TWO COPIES OF STAKING NOTES TO BE PROVIDED TO THE ENGINEER PRIOR TO CONSTRUCTION.
- 12. THE COUNTY OR THE COUNTY'S SURVEYOR SHALL PROVIDE A BENCHMARK OR TEMPORARY BENCHMARK AND SURVEY CONTROLS.
- 13. THE CONTRACTOR SHALL MAINTAIN UPDATED RED-LINED RECORD DRAWINGS ON SITE FOR INSPECTION BY THE ENGINEER.
- 14. MOWING, MAINTENANCE, AND CLEAN-UP OF THE PROJECT SHALL MEET THE REQUIREMENT OF SPECIFICATION ITEM 560 (NO SEPARATE PAY). MOWING, MAINTENANCE, AND CLEAN-UP IS REQUIRED FOR THE PROJECT LIMITS AND DURATION, REGARDLESS OF THE CONTRACTOR'S SCOPE OF ACTIVITIES WITHIN THE PROJECT LIMITS.
- 15. THE REMOVAL OF ANY ABANDONED UTILITIES REQUIRED TO COMPLETE THE WORK SHALL BE INCIDENTAL AND NO SEPARATE PAYMENT SHALL BE MADE.
- 16. IT IS THE CONTRACTOR'S RESPONSIBILITY TO STOCKPILE NECESSARY MATERIAL ON-SITE OR AT A SECURED OFF-SITE LOCATION AT NO ADDITIONAL EXPENSE TO FORT BEND COUNTY. ANY SUITABLE EXCAVATED MATERIAL ON THE PROJECT WHICH IS AVAILABLE AT THE TIME OF NEED; WHETHER FROM STORM SEWER, ROADWAY, AND/OR CHANNEL EXCAVATION, SHALL BE USED BEFORE BORROW IS BROUGHT ON-SITE.
- 17. MANHOLES, JUNCTION BOXES, INLETS, AND RISERS ARE TO BE PRE-CAST OR CAST IN PLACE.
- 18. THE FOLLOWING DETAILS ARE MINIMUM REQUIREMENTS AND MAY BE SUPERSEDED BY GEOTECHNICAL ENGINEER RECOMMENDATIONS OR MORE STRINGENT REQUIREMENTS FROM THE CITY'S ETJ PROJECT IS
- 19. POP UP DRAINS ARE NOT ALLOWED IN FORT BEND COUNTY RIGHT OF WAY.
- 20. CONTRACTOR IS RESPONSIBLE FOR HIS OWN HORIZONTAL AND VERTICAL CONTROL, REFERENCE POINTS AND CONSTRUCTION STAKING AS INCIDENTAL TO THE PROJECT.
- 21. CONTRACTOR TO FIELD VERIFY ALL BOUNDARY AND TOPOGRAPHIC INFORMATION PRIOR TO BEGINNING
- 22. EXISTING UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY BASED ON THE BEST AVAILABLE INFORMATION. CONTRACTOR TO FIELD VERIFY LOCATION OF ANY EXISTING UTILITIES AND OTHER FACILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO LOCATE AND PRESERVE ANY AND ALL EXISTING FACILITIES.
- 23 THE LENGTH OF PROPOSED LINDERGROUND LITULTY LINES SHOWN ARE APPROXIMATE ONLY LENGTHS. OF LINES MAY VARY DUE TO FIELD CONDITIONS ENCOUNTERED AT THE TIME OF CONSTRUCTION. CONTRACTOR SHALL PROVIDE ALL LABOR AND MATERIALS NECESSARY FOR UTILITY LINES TO SERVE THEIR INTENDED PURPOSE AND SHALL BE RESPONSIBLE FOR THE REROUTING OF LINES OCCASIONED BY CONFLICTS WITH OTHER UTILITIES AND SITE FEATURES.
- 24. WATER METERS, UTILITY LINES AND APPURTENANCES, DRIVEWAYS, AND ALL OTHER ITEMS TO BE LOCATED WITHIN THE STREET RIGHT-OF-WAY OR A PUBLIC EASEMENT, ARE TO BE CONSTRUCTED IN STRICT ACCORDANCE WITH CURRENT GOVERNING CITY, COUNTY AND STATE STANDARDS
- 25. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY BUILDING PERMITS AND FOR NOTIFICATION OF ALL AUTHORIZED INSPECTORS, SUPERINTENDENTS OR PERSONS IN CHARGE OF PRIVATE OR PUBLIC UTILITIES AFFECTED BY HIS OPERATIONS PRIOR TO COMMENCEMENT OF WORK.
- 26. CONTRACTOR SHALL NOTIFY UTILITY COORDINATING COMMITTEE BY TELEPHONE AT LEAST TWO FULL WORKING DAYS BEFORE STARTING WORK IN ANY STREET RIGHT-OF-WAY OR PUBLIC EASEMENT.

DATE NAME

RJS

3-1-22

GENERAL NOTES CONT.

- 27. CONTRACTOR TO BE RESPONSIBLE FOR PROVIDING AND MAINTAINING ALL CONFORMACIÓN DEL LESFONSIDEL FORMUNICA AND WAINING ALLE SAND TRAFFIC CONTROL DEVICES IN CONFORMANCE WITH PART VI OF THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", (TEXAS M.U.T.C.D. MOST RECENT EDITION AS REVISED) DURING CONSTRUCTION.
- 28. IF CONTRACTOR OPTS TO USE OPEN CUT METHOD OF CONSTRUCTION, TRENCH BEDDING AND BACKFILL SHALL MEET CITY OF STAFFORD REQUIREMENTS AND ALL OPEN EXCAVATIONS IN VEHICULAR TRAFFIC AREAS SHALL BE COVERED WITH ANCHORED STEEL PLATES CAPABLE OF SUPPORTING HS 20 LOADING AT END OF EACH DAYS WORK OR WHEN NOT IN USE

TRAFFIC CONTROL

- THE CONTRACTOR SHALL PROVIDE AND INSTALL TRAFFIC CONTROL DEVICES IN CONFORMANCE WITH PART VI OF THE MOST RECENT EDITION OF THE <u>MANUAL</u> <u>UNIFORM TRAFFIC CONTROL DEVICES</u> AND THE APPROVED TRAFFIC CONTROL F
- THE CONTRACTOR SHALL MAINTAIN AT LEAST ONE LANE OF TRAFFIC IN EACH DIRECTION DURING WORKING HOURS EXCEPT DURING FLAGGING OPERATION OR PROVIDE DETOURS AROUND THE CONSTRUCTION SITE AND PROVIDE PUBLIC NOTIFICATION.
- 3. LANE CLOSURES SHALL BE DURING OFF-PEAK HOURS ONLY (MONDAY THROUGH FRIDAY 9 A.M. TO 4 P.M.) UNIFORMED PEACE OFFICERS OR FLAGGERS IN RADIO CONTACT ARE REQUIRED TO DIRECT TRAFFIC DURING LANE CLOSURES.
- 4. DETOURS REQUIRE PRIOR APPROVAL OF THE FIELD ENGINEER AND PRECINCT. DETOUR PLANS, IF ALLOWED, MUST INCLUDE APPROPRIATE DETOUR SIGNAGE, PUBLIC NOTICE VIA SIGNAGE TWO WEEKS IN ADVANCE STATING THE DATES OF THE AGREED UPON DATE OF CLOSURE AND DATE THE ROAD WILL RE-OPEN TO TRAFFIC. CONTRACTOR TO USE (WITH PRIOR APPROVAL OF THE FIELD ENGINEER) HIGH EARLY STRENGTH CONCRETE AND OTHER RELATED CONSTRUCTION METHODS TO MINIMIZE THE DURATION OF THE DETOUR AND TO ENSURE THAT THE ROADWAY IS OPEN ON, OR PRIOR TO, THE AGREED UPON DATE.
- 5. ONE DAY PRIOR TO THE IMPLEMENTATION OF A TRAFFIC CONTROL PLAN PHASE OR STEP, OR THE IMPLEMENTATION OF AN ADDITIONAL, REVISED, OR NEW TRAFFIC CONTROL ELEMENT, THE CONTRACTOR SHALL MEET WITH THE ENGINEER TO GIVE A DETAILED DESCRIPTION OF THE CONTRACTOR'S PLAN AND PREPARATIONS. THE CONTRACTOR SHALL OBTAIN WRITTEN CONCURRENCE FROM THE ENGINEER THAT ADEQUATE PROJECT SHALL OBTAIN WRITTEN CONCORRENCE FROM THE ENGINEER THAT ADEQUATE PROJECT PROCRESS HAS BEEN ACHIEVED AND THAT ADEQUATE PREPARATIONS ARE IN PLACE PRIOR TO SWITCHING TRAFFIC. IF, IN THE OPINION OF THE ENGINEER, REQUIRED PROCRESS AND ADEQUATE PREPARATIONS ARE NOT COMPLETE, THE CONTRACTOR SHALL NOT IMPLEMENT THE NEXT PHASE, STEP, OR ELEMENT OF TRAFFIC CONTROL UNTIL INCOMPLETE CONSTRUCTION ITEMS OR PREPARATIONS ARE COMPLETED. TIME EXTENSIONS WILL NOT BE GRANTED FOR DELAYS CAUSED BY THE INCOMPLETE CONSTRUCTION ITEMS OR INADEQUATE CONTRACTOR PREPARATIONS REQUIRED TO IMPLEMENT TRAFFIC CONTROL. TIME EXTENSIONS
- 6. TRAFFIC CONTROL PER THE CONTRACT IS REQUIRED FOR THE ENTIRE DURATION OF THE PROJECT, INCLUDING THE PUNCHLIST PERIOD. PAYMENT FOR TRAFFIC CONTROL THAT IS PROPERLY INSTALLED FOR LESS THAN A FULL MONTH SHALL BE BASED ON A PERCENTAGE BASIS OF THE TIME INSTALLED. TRAFFIC CONTROL PAYMENTS TO THE CONTRACTOR SHALL END 10 DAYS AFTER SUBSTANTIAL COMPLETION, ALTHOUGH PROPER TRAFFIC CONTROL MUST BE MAINTAINED UNTIL PUNCHLIST COMPLETION.
- 7 THE PURPOSE OF THE CONSTRUCTION SEQUENCE AND TRAFFIC HANDLING OUTLINED HEREIN THE BASIS OF ESTIMATION FOR THE TRAFFIC CONTROL BID ITEMS, AND IS TO BE UTILIZED AND IMPLEMENTED, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

IF THE CONTRACTOR CHOOSES TO USE A DIFFERENT TCP, HE/SHE SHALL PREPARE AND SUBMIT THE ALTERNATIVE TCP TO THE COUNTY FOR APPROVAL NO LESS THAN 10 WORKING DAYS PRIOR TO THE PROPOSED IMPLEMENTATION DATE. THE TCP SHALL BE DRAWN TO SCALE AND SIGNED & SEALED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF TEXAS. UPON APPROVAL BY FORT BEND COUNTY, THE ALTERNATIVE PLAN SHALL BECOME THE BASIS FOR A "CHANGE IN CONTRACT" TO REVISE THE TRAFFIC CONTROL BID ITEMS ACCORDINGLY AND BECOME PART OF THE CONTRACT DOCUMENTS.

- 8. ALL TEMPORARY PAVEMENT MARKINGS ON PERMANENT PAVEMENT SHOULD BE RPMS OR TABS.
- TRAFFIC PATTERN CHANGES REQUIRE CHANGEABLE MESSAGE BOARDS PLACED AT LEAST 2 WEEKS IN ADVANCE OF PROPOSED CHANGE. QUANTITY, PLACEMENT AND WORDING TBD BY FBC.

STORM SEWER CONSTRUCTION NOTES

- STORM SEWER PIPE TO BE POLYVINYL CHLORIDE PIPE (PVC) CONFORMING TO ASTM D3034, SDR 35 (PIPE SIZE 12"), AND REINFORCED CONCRETE PIPE (RCP) CONFORMING TO ASTM C-76, CLASS III, EXCEPT AS OTHERWISE NOTED ON THE PLANS.
- 2. ALL STORM SEWERS TO RECEIVE BEDDING AND BACKFILL IN ACCORDANCE WITH THE DETAILS CONTAINED IN THE PLANS. STORM SEWERS WITHIN PUBLIC STREET RIGHTS-OF-WAY OR EASEMENTS TO RECEIVE BEDDING AND BACKFILL IN ACCORDANCE WITH FORT BEND COUNTY W.C.I.D. NO.2 SPECIFICATIONS FOR SEWER CONSTRUCTION, LATEST PRINTING AND AMENDMENTS
- WHERE MANHOLES, GRATE INLETS, OR JUNCTION BOXES ARE LOCATED WITHIN PAVED AREAS, CONTRACTOR SHALL SET RIM ELEVATIONS TO MATCH TOP OF PAVEMENT ELEVATIONS.
- 4. CONTRACTOR TO ALLOW A MINIMUM OF 6-INCH VERTICAL CLEARANCE BETWEEN STORM SEWER AND OTHER EXISTING OR PROPOSED UTILITIES.

- TEXAS/SWBT_FACILITIES.

CAUTION: UNDERGROUND GAS FACILITIES

LOCATED.

EXCAVATION BEGINS.

PROCEDURES

WARNING: OVERHEAD ELECTRICAL LINES

AT (713) 207-2222

STORM WATER QUALITY



TEOFTE	PROJECT TITL	E: STELLA ROAD	
<i>{</i> ; ∕ ★ ``.	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AUSTIN P. MCLEAN	CK'D BY: AM	SHEET DESCRIPTION: GENERAL NOTES	
132535 (CENSED)	SCALE:		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	4 / 133

REVISIONS

ORIGINAL STANDARD ISSUED

AT&T TEXAS/SWBT FACILITIES

1. THE LOCATIONS OF AT&T TEXAS/SWBT FACILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THIS FAILURE TO EXACTLY LOCATE AND PRESERVE THESE UNDERGROUND UTILITIES

2. THE CONTRACTOR SHALL CALL 1-800-344-8377 (TEXAS 811) A MINIMUM OF 48 HOURS PRIOR O CONSTRUCTION TO HAVE UNDERGROUND LINES FIELD LOCATED.

3. WHEN EXCAVATING WITHIN EIGHTEEN INCHES (18") OF THE INDICATED LOCATION OF AT&T TEXAS/SWBT FACILITIES, ALL EXCAVATIONS MUST BE ACCOMPLISHED USING NON-MECHANIZED EXCAVATION PROCEDURES. WHEN BORING, THE CONTRACTOR SHALL EXPOSE THE AT&T

4. WHEN AT&T TEXAS/SWBT FACILITIES ARE EXPOSED, THE CONTRACTOR WILL PROVIDE SUPPORT TO PREVENT DAMAGE TO THE CONDUIT DUCTS OR CABLES. WHEN EXCAVATING NEAR TELEPHONE POLES THE CONTRACTOR SHALL BRACE THE POLE FOR SUPPORT.

5. THE PRESENCE OR ABSENCE OF AT&T TEXAS/SWBT UNDERGROUND CONDUIT FACILITIES OR BURIED CABLE FACILITIES SHOWN ON THESE PLANS DOES NOT MEAN THAT THERE ARE NO DIRECT BURIED CABLES OR OTHER CABLES IN CONDUIT IN THE AREA.

 PLEASE CONTACT THE AT&T TEXAS DAMAGE PREVENTION MANAGER MR. ROOSEVELT LEE JR. AT (713) 567-4552 OR EMAIL HIM AT RL7259@ATT.COM, IF THERE ARE QUESTIONS ABOUT BORING OR EXCAVATING NEAR OUR AT&T TEXAS/SWBT FACILITIES.

THE CONTRACTOR SHALL CONTACT THE UTILITY COORDINATING COMMITEE AT 1-800-545-6005 OR 811 A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION TO HAVE MAIN AND SERVICE LINES FIELD

*WHEN CENTERPOINT ENERGY PIPE LINE MARKINGS ARE NOT VISIBLE, CALL (713) 945-8036 OR (713) 945-8037 (7:00 A.M. TO 4:30P.M.) FOR STATUS OF LINE LOCATION REQUEST BEFORE

*WHEN EXCAVATING WITHIN EIGHTEEN INCHES (18") OF THE INDICATED LOCATION OF CENTERPOINT ENERGY FACILITIES, ALL EXCAVATION MUST BE ACCOMPLISHED USING NON-MECHANIZED EXCAVATION

*WHEN CENTERPOINT ENERGY GAS FACILITIES ARE EXPOSED, SUFFICIENT SUPPORT MUST BE PROVIDED TO THE FACILITIES TO PREVENT EXCESSIVE STRESS ON THE PIPING.

* FOR EMERGENCIES REGARDING GAS LINES CALL (713) 659-3552 OR (713) 207-4200

THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY DAMAGES CAUSED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE THESE UNDERGROUND FACILITIES.

OVERHEAD LINES MAY EXIST ON THE PROPERTY. THE LOCATION OF OVERHEAD LINES HAS NOT BEEN SHOWN ON THESE DRAWINGS AS THE LINES ARE CLEARLY VISIBLE, BUT YOU SHOULD LOCATE THEM PRIOR TO BEGINNING ANY CONSTRUCTION. TEXAS LAW, SECTION 752, HEALTH & SAFETY CODE FORBIDS ACTIVITIES THAT OCCUR IN CLOSE PROXIMITY TO HIGH VOLTAGE LINES, SPECIFICALLY:

· ANY ACTIVITY WHERE PERSON OR THINGS MAY COME WITHIN SIX(6) FEET OF LIVE OVERHEAD HIGH VOLTAGE LINES; AND • OPERATING A CRANE, DERRICK, POWER SHOVEL, DRILLING RIG, PILE DRIVER, HOISTING EQUIPMENT, OR SIMILAR APPARATUS WITHIN 10 FEET OF LIVE OVERHEAD HIGH VOLTAGE LINES.

PARTIES RESPONSIBLE FOR THE WORK, INCLUDING CONTRACTORS ARE LEGALLY RESPONSIBLE FOR THE SAFETY OF CONSTRUCTION WORKERS UNDER THIS LAW. THIS LAW CARRIES BOTH CRIMINAL AND CIVIL LIABILITY. TO ARRANGE FOR LINES TO BE TURNED OFF OR REMOVED CALL CENTERPOINT ENERGY

ACTIVITIES ON OR ACROSS CENTERPOINT ENERGY FEE OR EASEMENT PROPERTY NO APPROVAL TO USE, CROSS OR OCCUPY CENTERPOINT FEE OR EASEMENT PROPERTY IS GIVEN. IF YOU NEED TO USE CENTERPOINT PROPERTY, PLEASE CONTACT OUR SURVEYING & RIGHT OF WAY DIVISION AT (713) 207-6348 OR (713) 207-5769.

1. SWQMP: THIS PROJECT DOES NOT REQUIRE A STORM WATER QUALITY MANAGEMENT PLAN (SWOMP) BECAUSE IT DOES NOT MEET THE DEFINITION OF NEW DEVELOPMENT OR SIGNIFICANT REDEVELOPMENT UNDER THE FORT BEND COUNTY REGULATIONS.

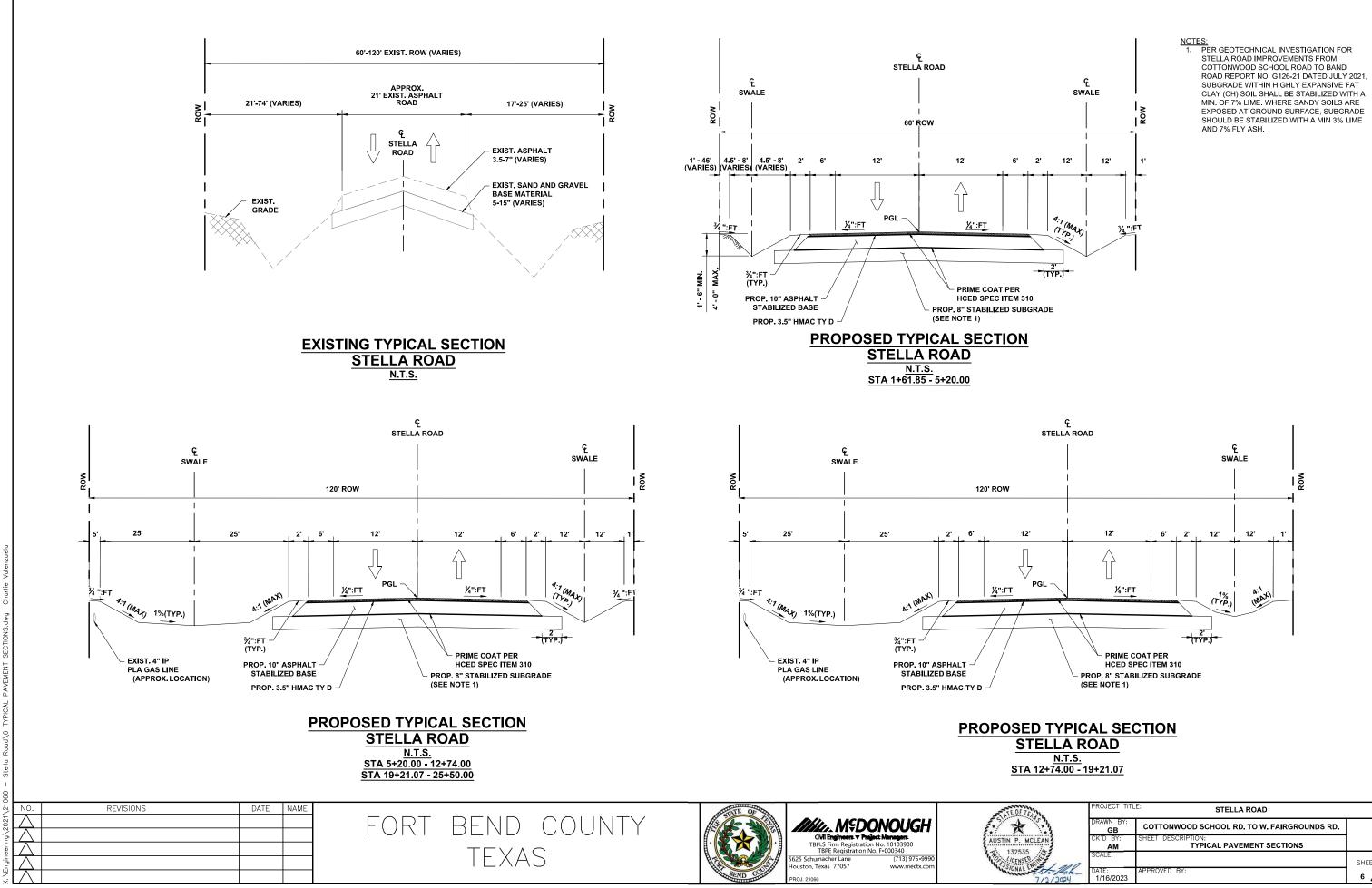
CONSTRUCTION

- 1. FORT BEND COUNTY MUST BE INVITED TO THE PRE-CONSTRUCTION MEETING.
- 2. CONTRACTOR SHALL NOTIFY FORT BEND COUNTY ENGINEERING DEPARTMENT 48 HOURS PRIOR TO COMMENCING CONSTRUCTION AND 48 HOUR NOTICE TO ANY CONSTRUCTION ACTIVITY WITHIN THE LIMITS OF THE PAVING AT CONSTRUCTION@FBCTX.GOV.
- 3. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED FROM FORT BEND COUNTY PRIOR TO COMMENCING CONSTRUCTION OF ANY IMPROVEMENTS WITHIN COUNTY ROAD RIGHT OF WAYS.
- ALL PAVING IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH FORT 4 BEND COUNTY "RULES, REGULATIONS AND REQUIREMENTS" RELATING TO THE APPROVAL AND ACCEPTANCE OF IMPROVEMENTS IN SUBDIVISIONS AS CURRENTLY AMENDED.
- 5. ALL ROAD WIDTHS, CURB RADII AND CURB ALIGNMENT SHOWN INDICATES BACK OF CURB.
- 6. A CONTINUOUS LONGITUDINAL REINFORCING BAR SHALL BE USED IN THE CURBS.
- 7. ALL CONCRETE PAVEMENT SHALL BE 51/2 SACK CEMENT WITH A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS. TRANSVERSE EXPANSION JOINTS SHALL BE INSTALLED AT EACH CURB RETURN AND AT A MAXIMUM SPACING OF 60 FEET.
- 8. ALL WEATHER ACCESS TO ALL EXISTING STREETS AND DRIVEWAYS SHALL BE MAINTAINED AT ALL TIMES.
- 9. 4" X 12" REINFORCED CONCRETE CURB SHALL BE PLACED IN FRONT OF SINGLE FAMILY LOTS ONLY. ALL OTHER AREAS SHALL BE 6" REINFORCED CONCRETE CURB.
- 10. CURB HEADERS ARE REQUIRED AT CURB CONNECTIONS TO HANDICAP RAMPS, WITH NO CONSTRUCTION JOINT WITHIN 5' OF RAMPS.
- 11. GUIDELINES ARE SET FORTH IN THE TEXAS "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", AS CURRENTLY AMENDED, SHALL BE OBSERVED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE FLAGMEN, SIGNING, STRIPING AND WARNING DEVICES, ETC., DURING CONSTRUCTION -BOTH DAY AND NIGHT.
- 12. ALL R1-1 STOP SIGNS SHALL BE A MINIMUM OF 36"X36" WITH DIAMOND GRADE SHEETING PER TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES.
- 13. STREET NAME SIGNAGE SHALL BE ON A 9" HIGH SIGN FLAT BLADE W/REFLECTIVE GREEN BACKGROUND. STREET NAMES SHALL BE UPPER AND LOWERCASE LETTERING WITH UPPERCASE LETTERS OF 6" MINIMUM AND LOWERCASE LETTERS OF 4.5" MINIMUM. THE LETTERS SHALL BE REFLECTIVE WHITE. STREET NAME SIGNS SHALL BE MOUNTED ON STOP SIGN POST.
- 14. A BLUE DOUBLE REFLECTORIZED BUTTON SHALL BE PLACED AT ALL FIRE HYDRANT LOCATIONS. THE BUTTON SHALL BE PLACED 12 INCHES OFF OF THE CENTERLINE OF THE STREET ON THE SAME SIDE AS THE HYDRANT.
- 15. THE PROJECT AND ALL PARTS THEREOF SHALL BE SUBJECT TO INSPECTION FROM TIME TO TIME BY INSPECTORS DESIGNATED BY FORT BEND COUNTY. NO SUCH INSPECTIONS SHALL RELIEVE THE CONTRACTOR OF ANY OF ITS OBLIGATIONS HEREUNDER. NEITHER FAILURE TO INSPECT NOR FAILURE TO DISCOVER OR REJECT ANY OF THE WORK AS NOT IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS, REQUIREMENTS AND SPECIFICATIONS OF FORT BEND COUNTY OR ANY PROVISION OF THIS PROJECT SHALL BE CONSTRUED TO IMPLY AN ACCEPTANCE OF SUCH WORK OR TO RELIEVE THE CONTRACTOR OF ANY OF ITS OBLIGATIONS HEREUNDER
- 16. STABILIZED SUBGRADE: DETERMINE THE THICKNESS OF THE STABILIZED SUBGRADE AFTER CURING AND COMPACTION. IF THE SUBGRADE DEPTH IS GREATER THAN THE PROPOSED THICKNESS BY 20% OR MORE, THE CMT LAB MUST PROVIDE VERIFICATION THE PERCENTAGE OF MATERIAL BEING USED TO STABILIZE THE SUBGRADE MEETS OR EXCEEDS PROJECT REQUIREMENTS. TEST RESULTS REQUIRED.
- 17. CONTRACTOR TO PROVIDE MONTHLY SCHEDULE UPDATES AND WEEKLY LOOK AHEAD

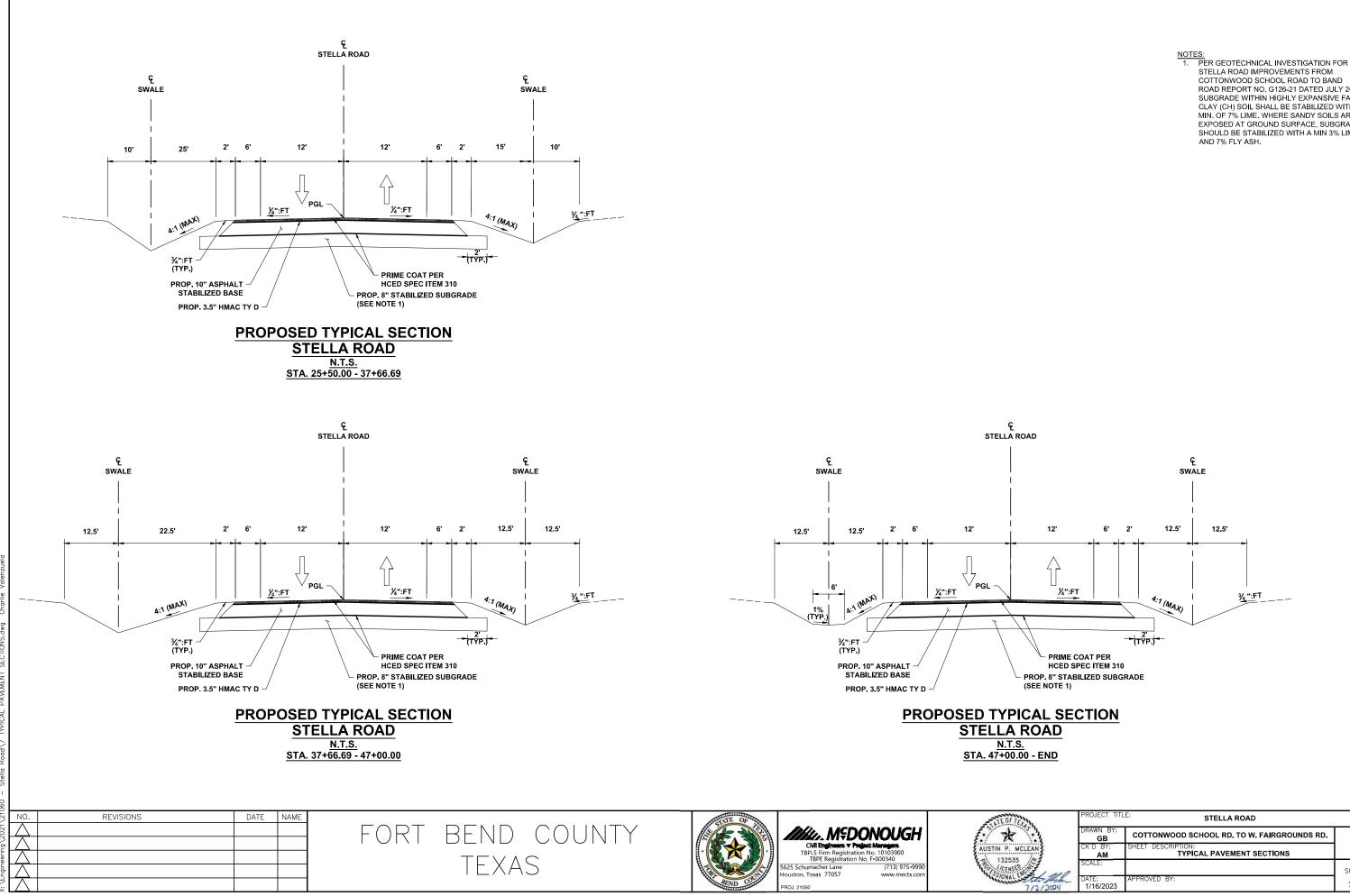
NOTE: FORT BEND COUNTY NOTES SUPERSEDE ANY CONFLICTING NOTES.

		REVISIONS ORIGINAL STANDARD ISSUED ADDED NOTE 17	DATE 3-1-22 3-1-23	NAME RJS RJS	FORT BEND COUNTY TEXAS	TE OF CIVIE Engineers + Project Managers TBPLS Firm Registration No. 10103900 TBPE Registration No. 1000340 5625 Schumacher Lane (713) 975-9990	N P. MC
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	PROJECT TITI	STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: CONSTRUCTION NOTES	
1	SCALE:		SHEET NO
- 12h	DATE: 1/16/2023	APPROVED BY:	5 / 13

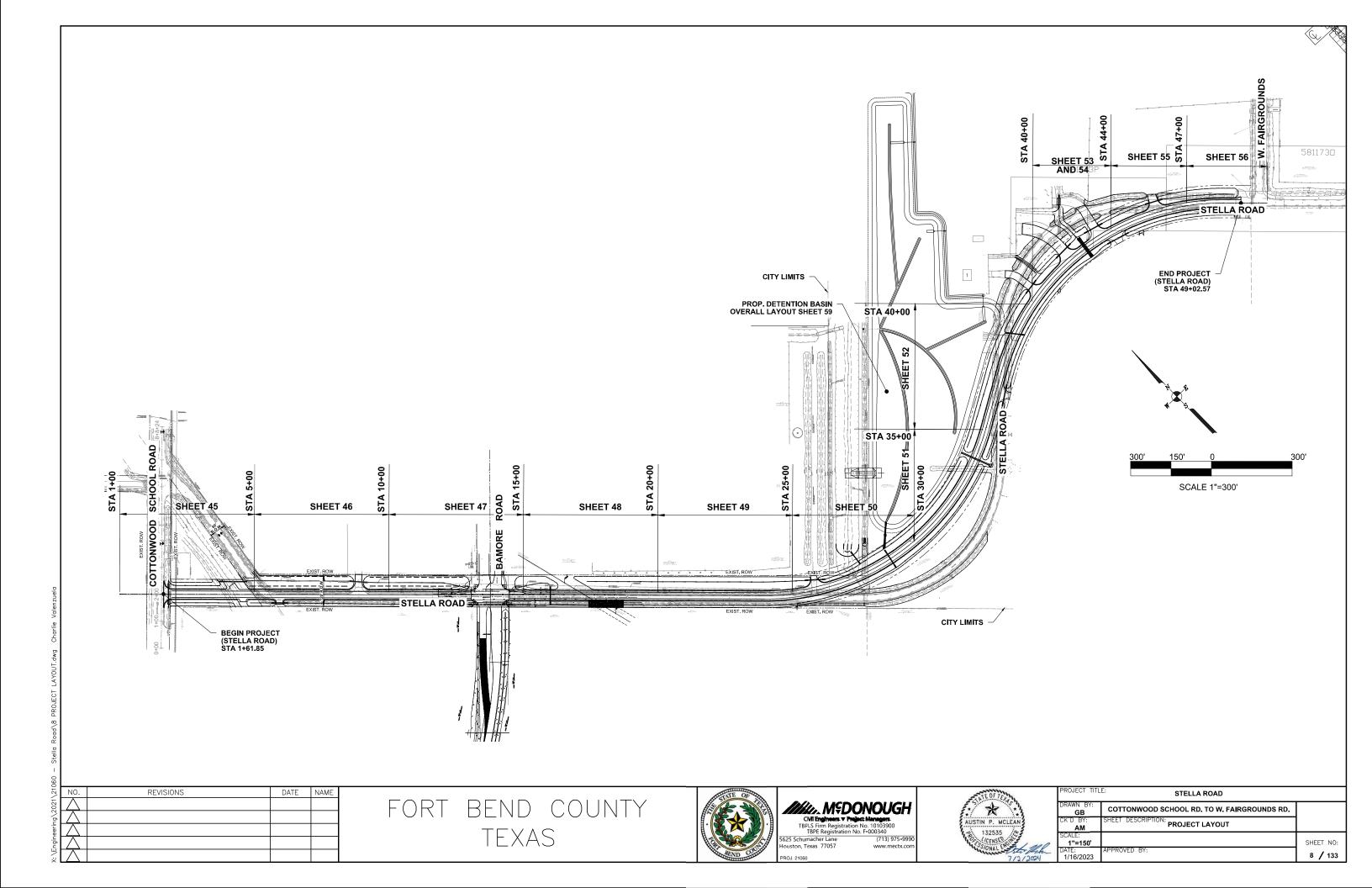


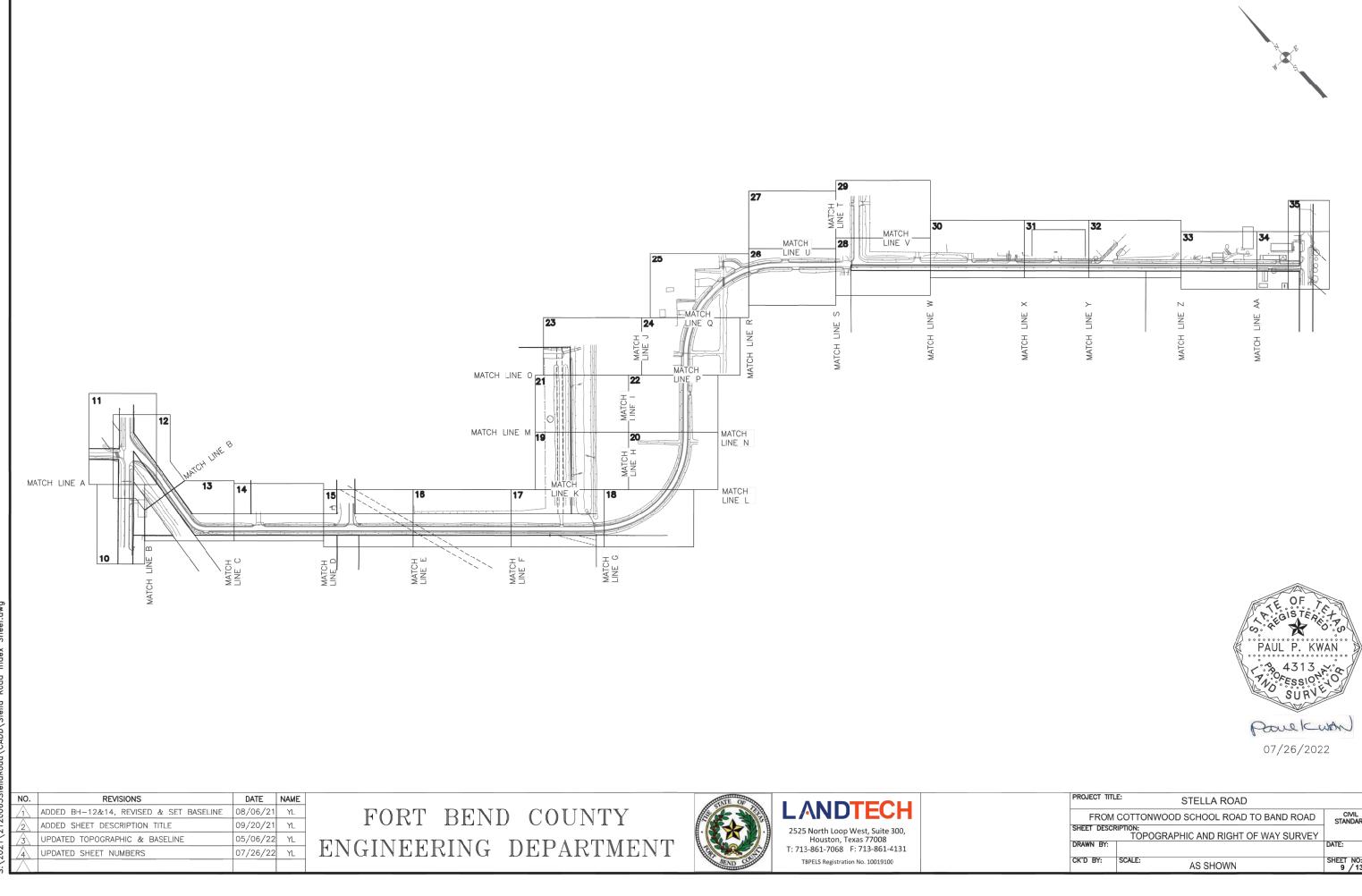
		STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CLEAN	CK'D BY: AM	SHEET DESCRIPTION: TYPICAL PAVEMENT SECTIONS	
NG HUNGAL MAR	SCALE:		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	6 / 133



STELLA ROAD IMPROVEMENTS FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD REPORT NO. G126-21 DATED JULY 2021, SUBGRADE WITHIN HIGHLY EXPANSIVE FAT CLAY (CH) SOIL SHALL BE STABILIZED WITH A MIN. OF 7% LIME. WHERE SANDY SOILS ARE EXPOSED AT GROUND SURFACE, SUBGRADE SHOULD BE STABILIZED WITH A MIN 3% LIME

	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: TYPICAL PAVEMENT SECTIONS	
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7/2/2024	DATE: 1/16/2023	APPROVED BY:	7 / 133

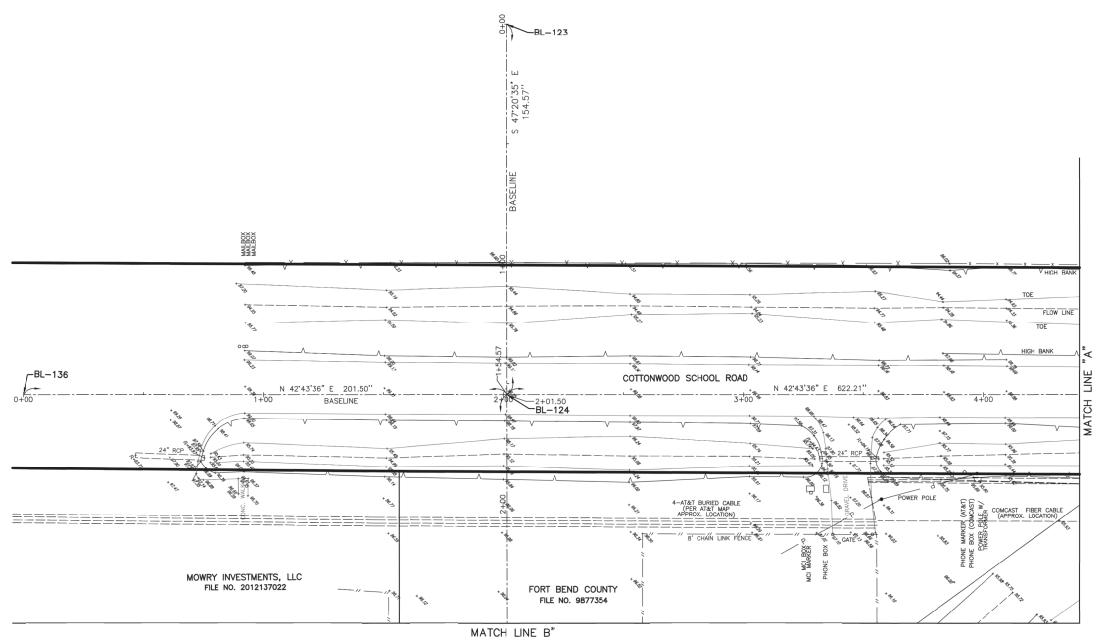


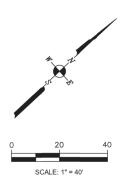


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PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 9 / 133

			ESTMENTS, LLC	- // **********************************	FORT BEND COUNTY		WC	***
				MATCH	LINE B"			
	BASELINE	DATA						
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	ADDED BH-12&14, REVISED & SET BASELINE	DATE NAME 08/06/21 YL		DEND CO		A STATE OF	LANDTECH	
	ADDED SHEET DESCRIPTION TITLE	09/20/21 YL	FORT	BEND CO	JUNIX	E CARACTER C		
3	UPDATED TOPOGRAPHIC & BASELINE	05/06/22 YL	ENCINEE.	DINC DEI			2525 North Loop West, Suite 300, Houston, Texas 77008	
	UPDATED SHEET NUMBERS	07/26/22 YL	ENGINEE.	KING DEI	PARTMENT	BEND CON	T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100	
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NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NABB3 (2011 AUJUSTMENT, EPOCH 2010.00). COORDINATES 3HOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986825336. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEET

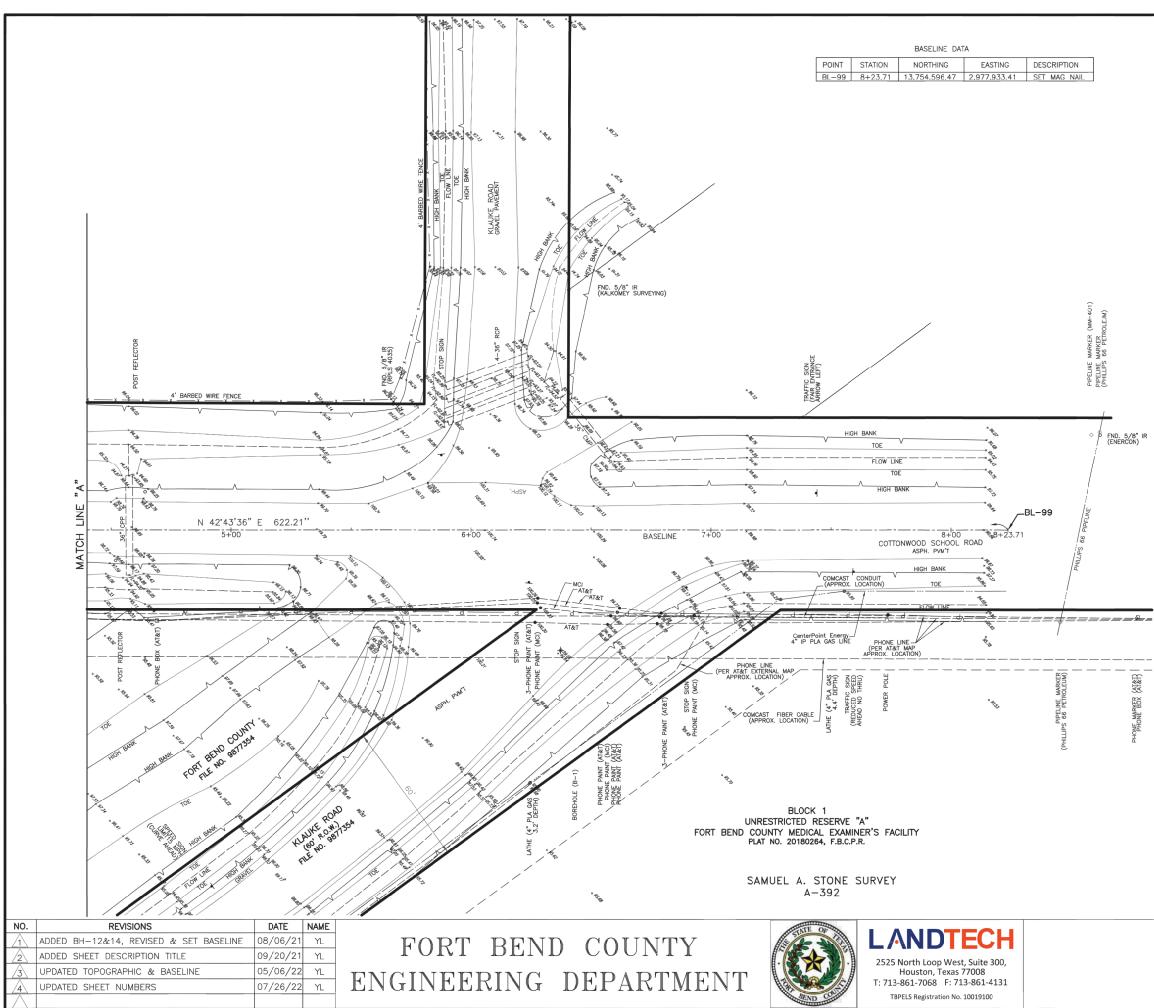
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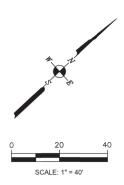
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07/26/2022

 PROJECT TITL	E: STELLA ROAD	
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 10 / 133





NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 AUJUSTMENT, EPOCH 2010.00). COORDINATES 3HOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968626336, UNIT OF MEASURE IS U.S. SURVEY FOOT.
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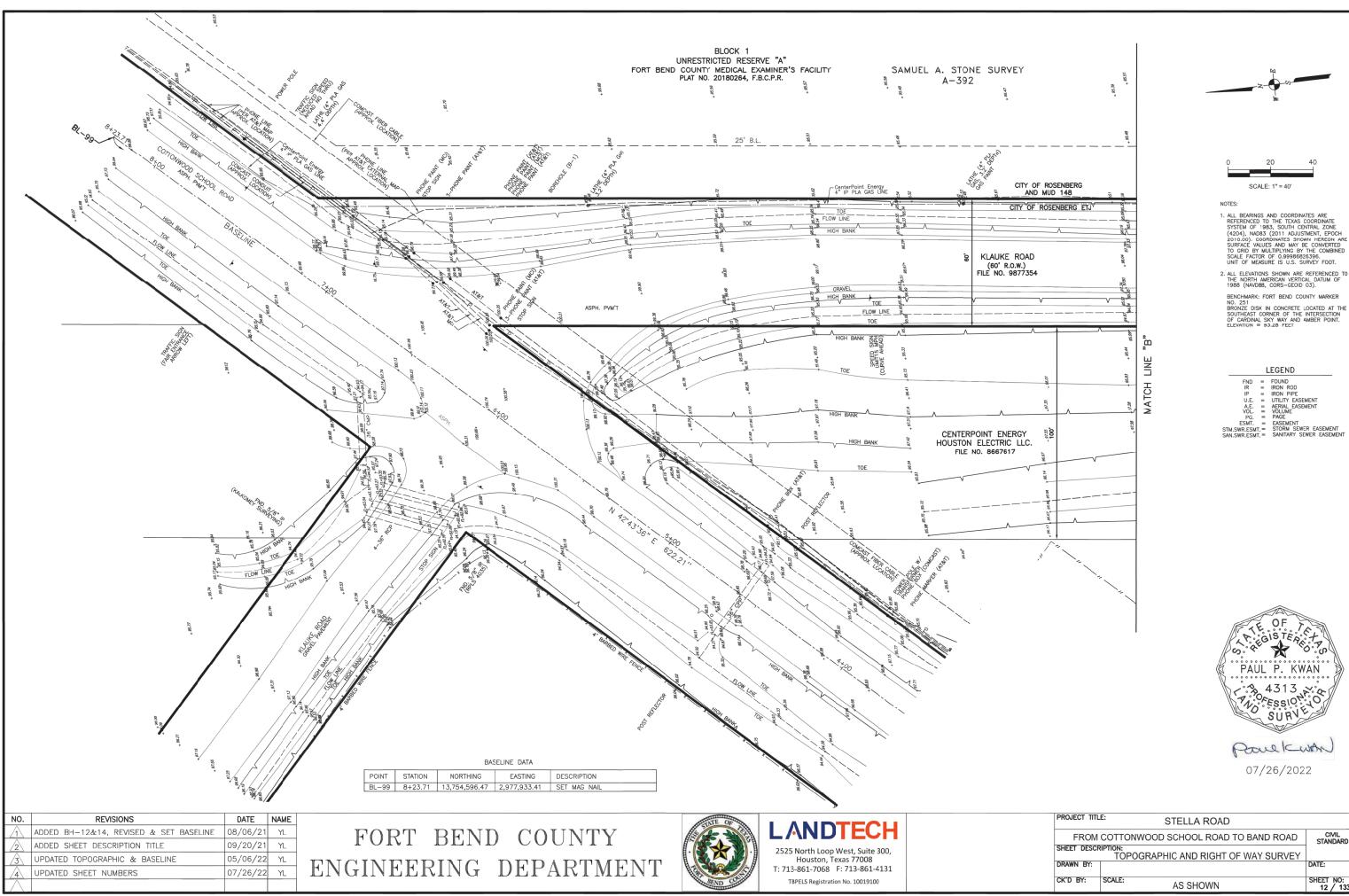
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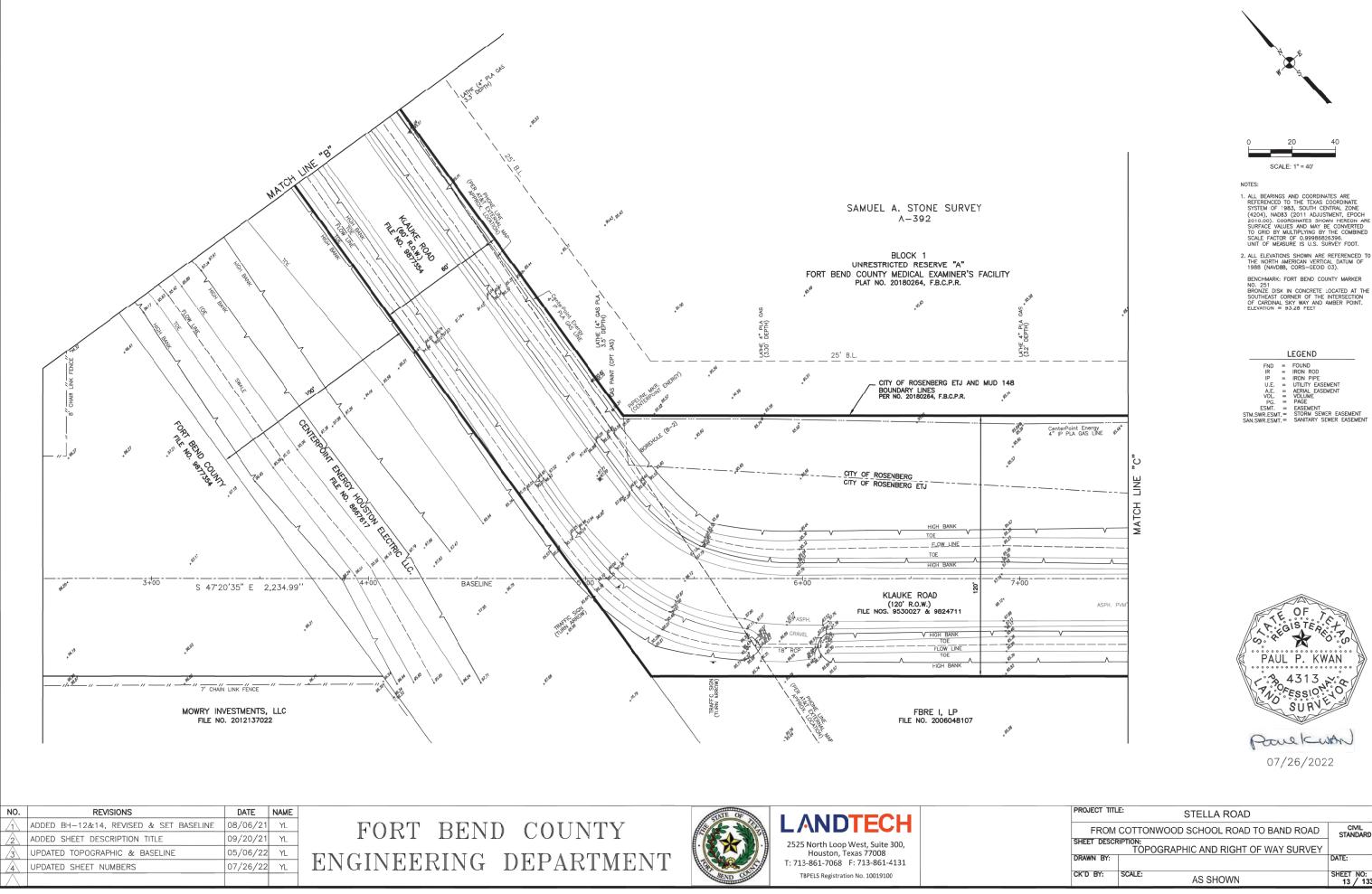
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PROJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
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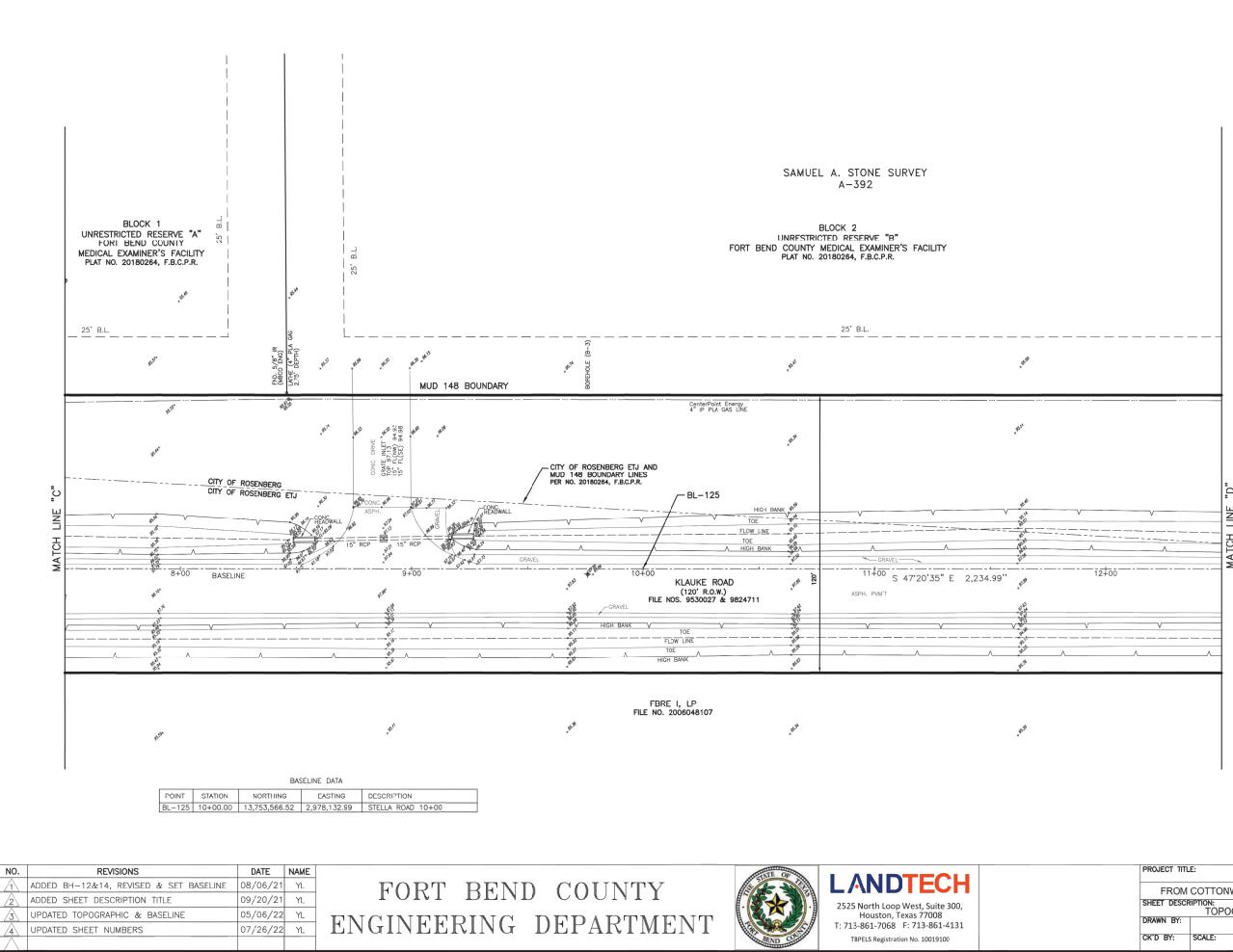


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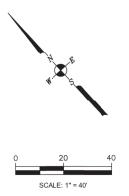
PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 12 / 133



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FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL
SHEET DESCR		STANDARD
	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
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NOTES:

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BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND ANBER POINT. ELEVATION = 9-3.26 FEET

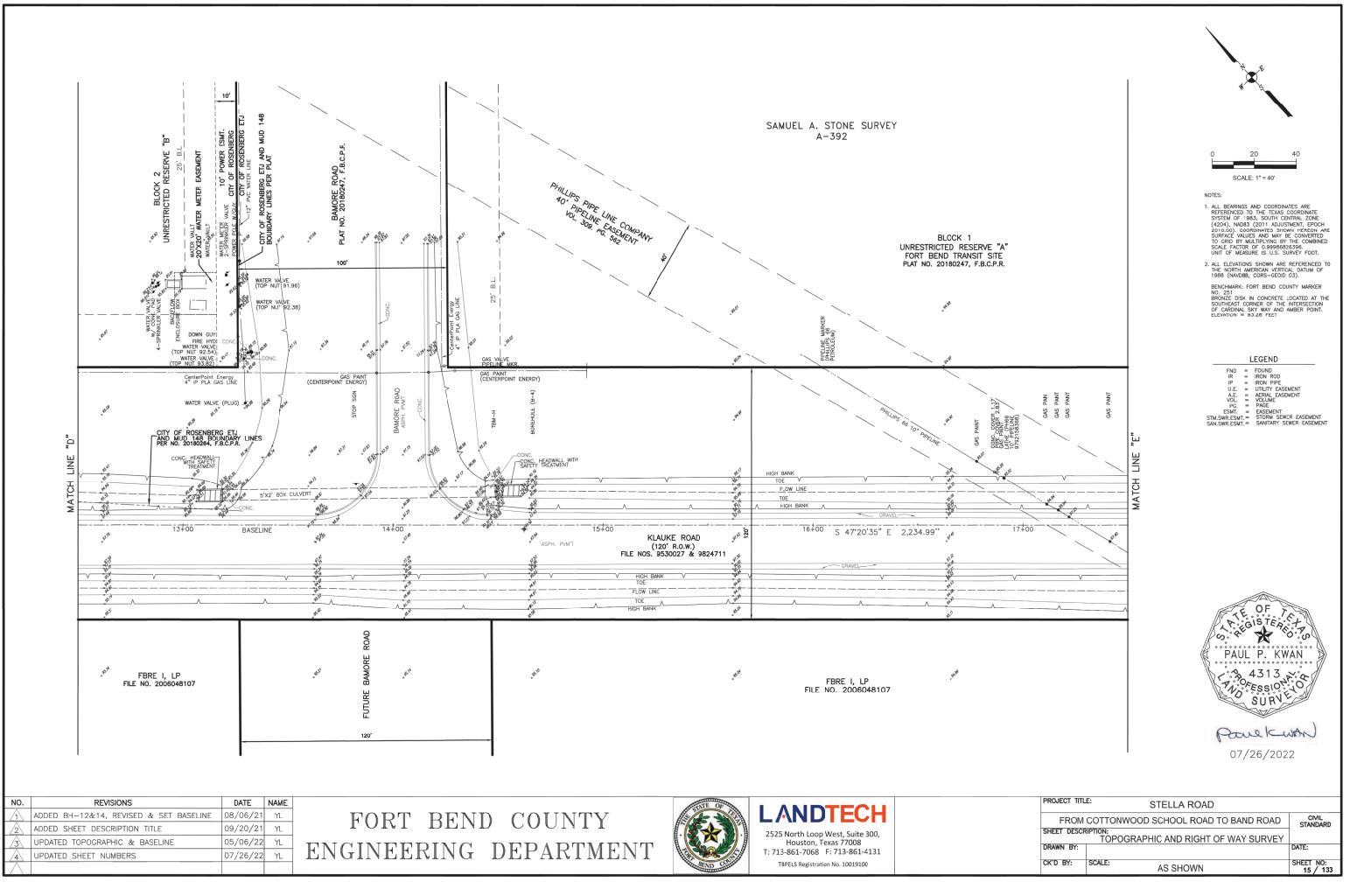


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SA	N.SWR.ESM	Г. =	SANITARY SEWER EASEMEN

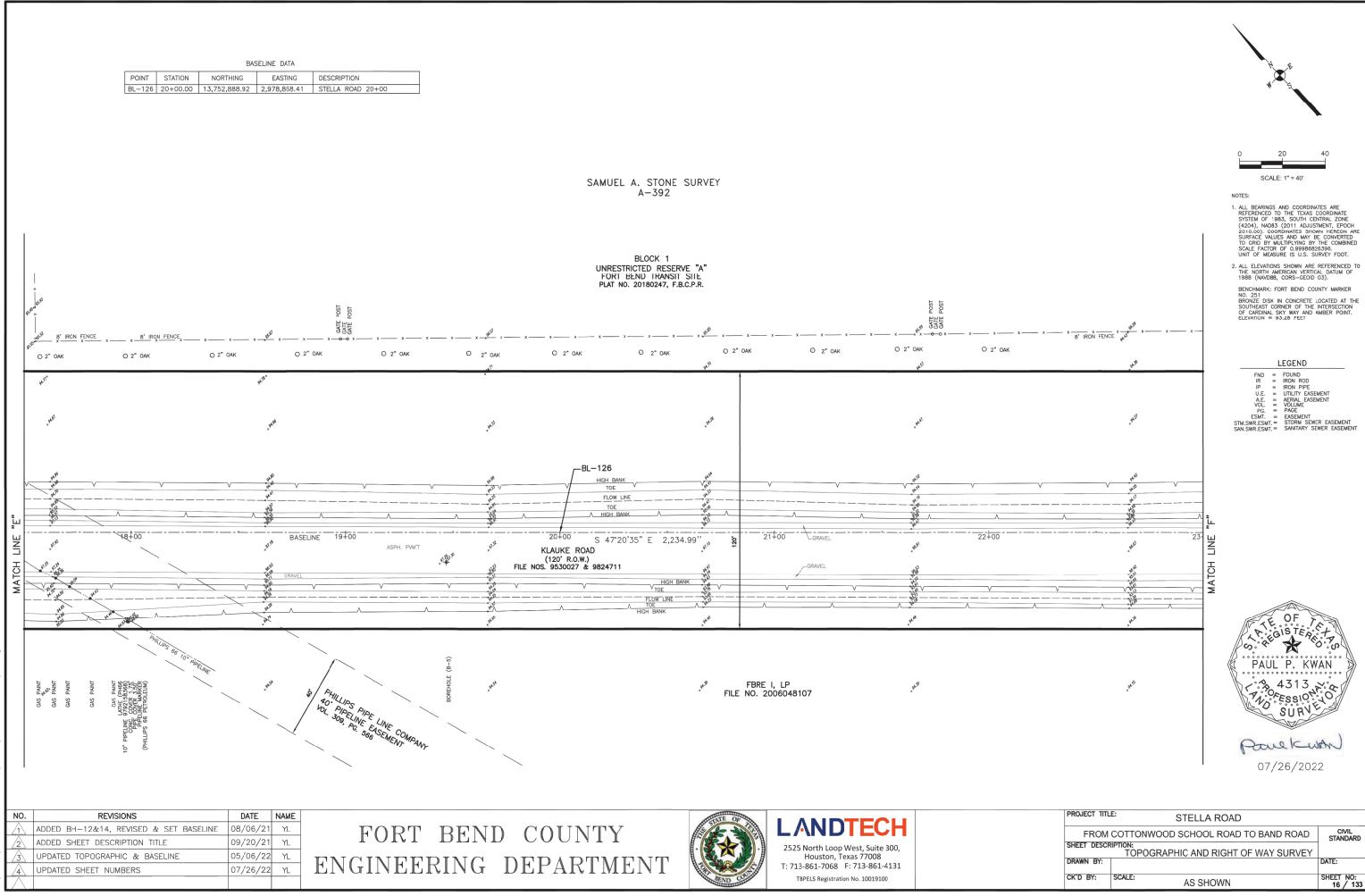


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PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
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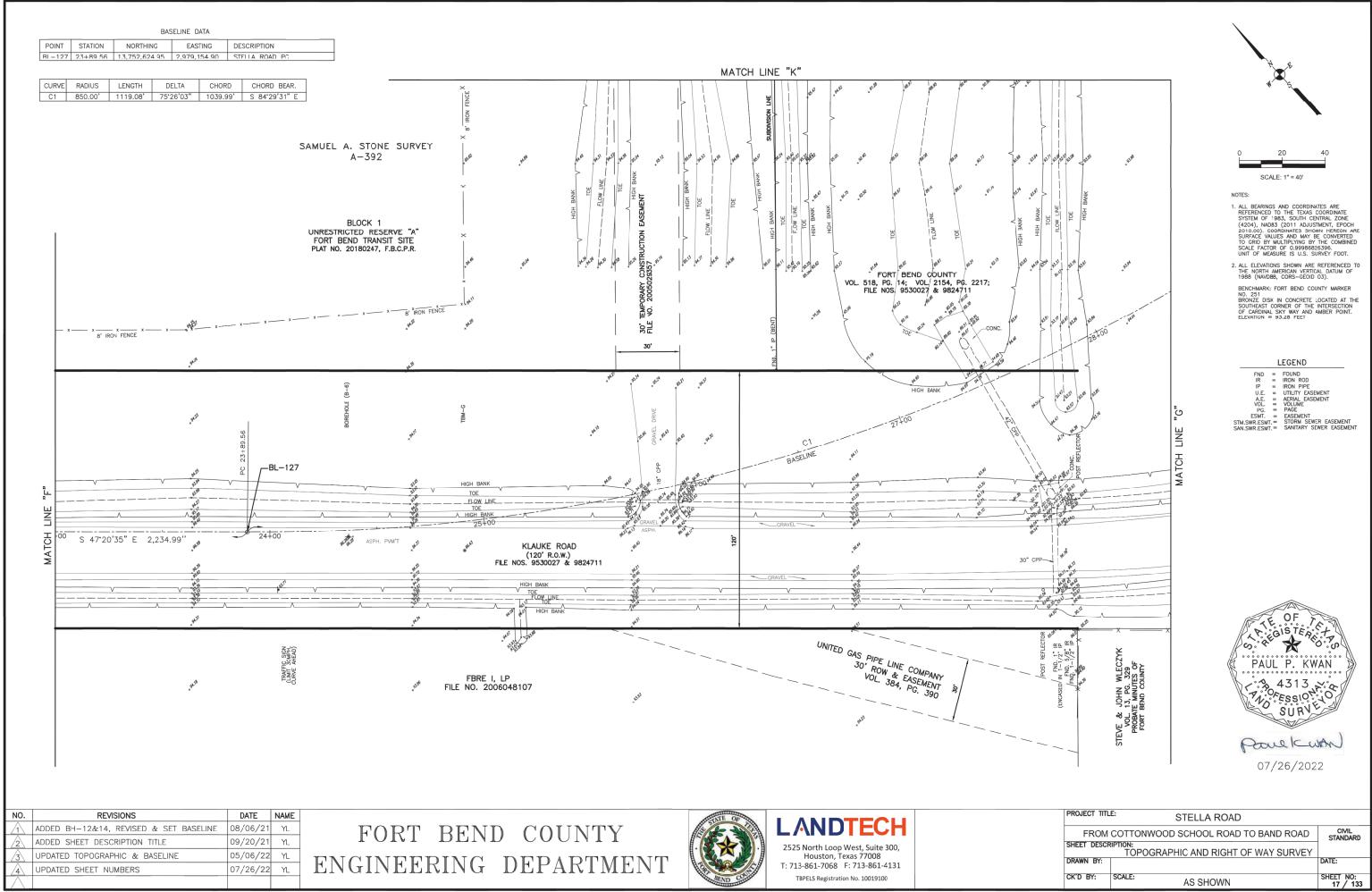


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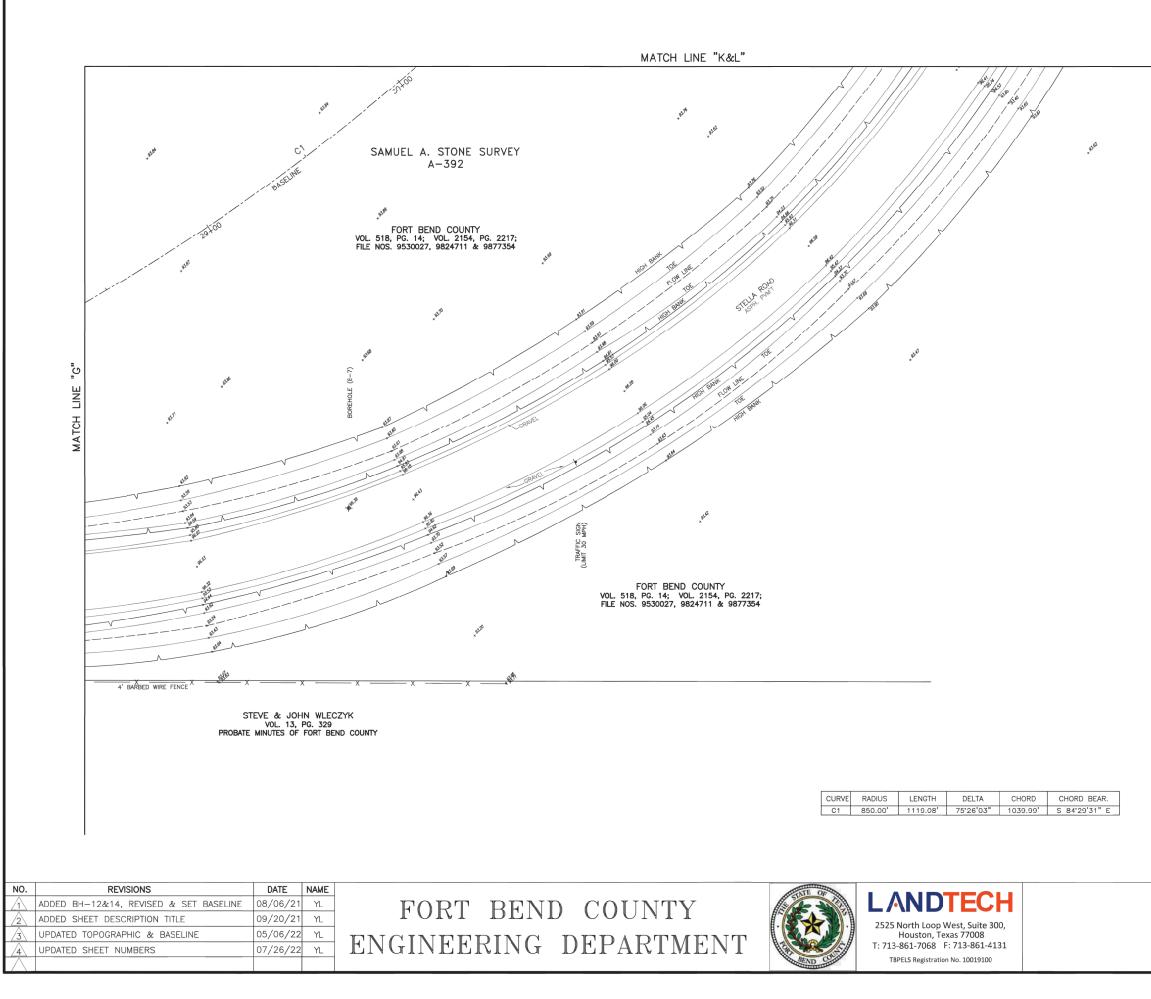


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	STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESCR		
DRAWN BY:	DATE:	
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FROM	COTION	WOOD SCHOOL ROAD TO BAND ROAD	STANDARD
SHEET DESCR			1
	TOPC	OGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:			DATE:
CK'D BY:	SCALE:	AS SHOWN	SHEET NO: 17 / 133



:\2021\2120065StellaRoad\CADD\Stella Road Topo.dwg



NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 ADUJISTNENT, EPOCH 2010.00). COORDINATES 3HOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826336. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

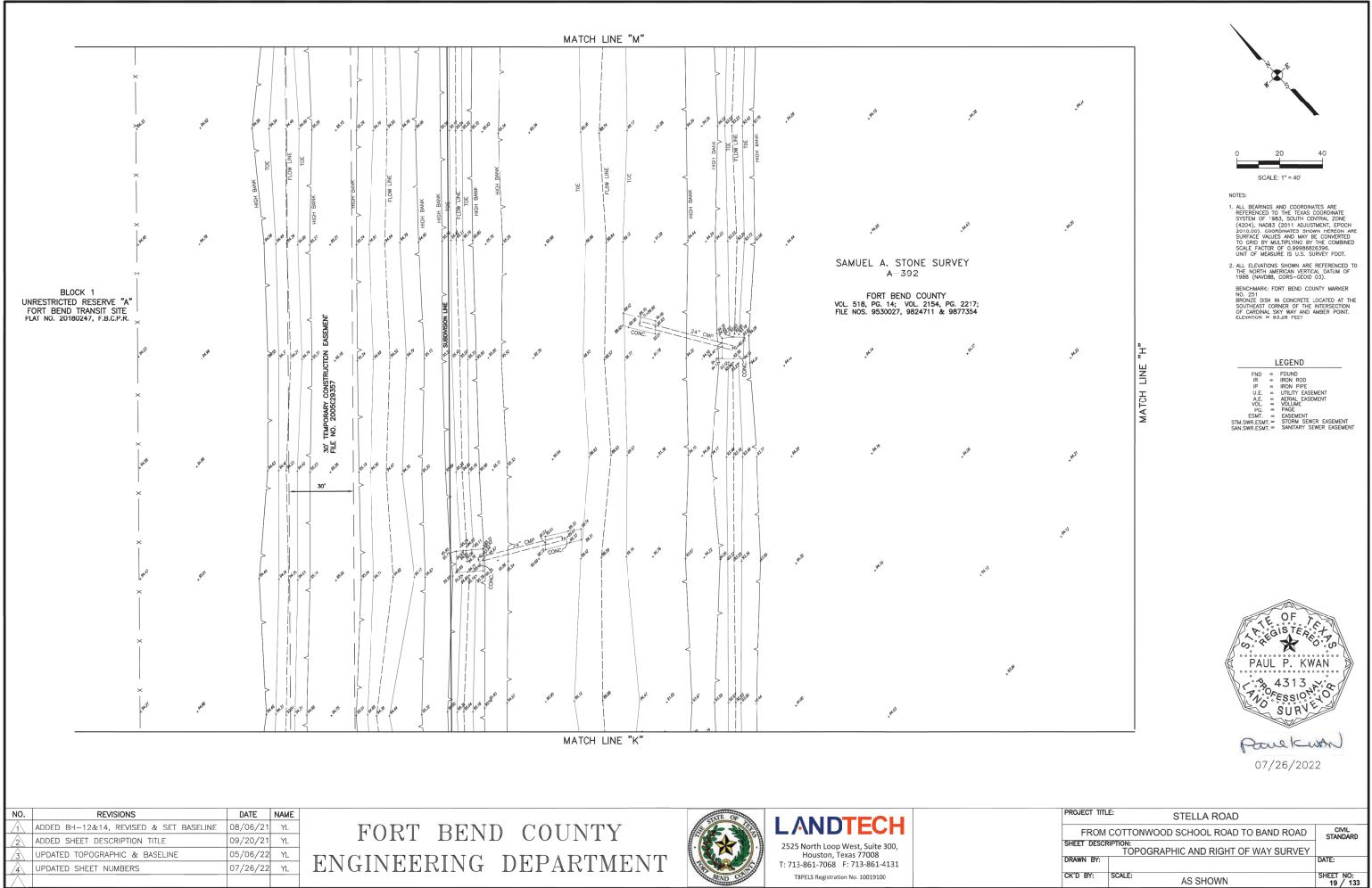
BENCHMAR; FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEET

LEGEND

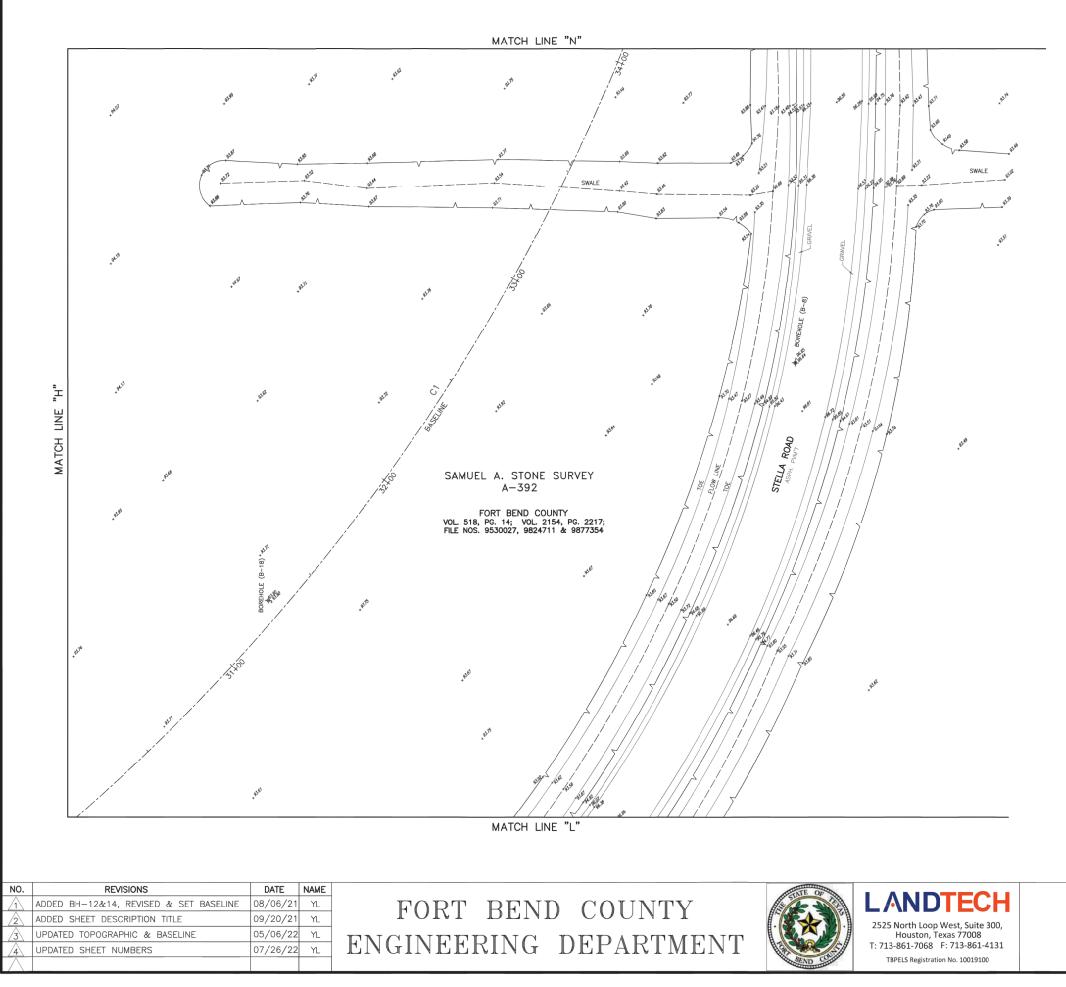
	FND	=	FOUND
	IR	=	IRON ROD
	IP	=	IRON PIPE
	U.E.	=	UTILITY EASEMENT
	A.E.	=	AERIAL EASEMENT
	VOL.	=	VOLUME
	PG.	=	PAGE
	ESMT.	=	EASEMENT
STN	I.SWR.ESM	Г. =	STORM SEWER EASEMENT
SAN	SWR.ESM	Г. =	SANITARY SEWER EASEMENT



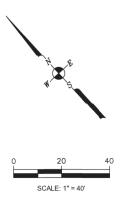
PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 18 / 133



DRAWN BY: DATE: CK'D BY: SCALE: AS SHOWN SHEET NO: 19 / 133	SHEET DESCR		OGRAPHIC AND RIGHT OF WAY SURVEY		
	DRAWN BY:			DATE:	
	CK'D BY:	SCALE:	AS SHOWN		



2021\2120065StellaRoad\CADD\Stella Road Topo.



NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADUISTMENT, EPOCH 2010.00). COORDINATES 3HOWN HEREON ARE SURFACE VALUES AND MAY BE CONVENTED TO GRID BY MULTPLYING BY THE COMBINED SCALE FACTOR OF 0.9986826336. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

BENCHMAR; FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEET

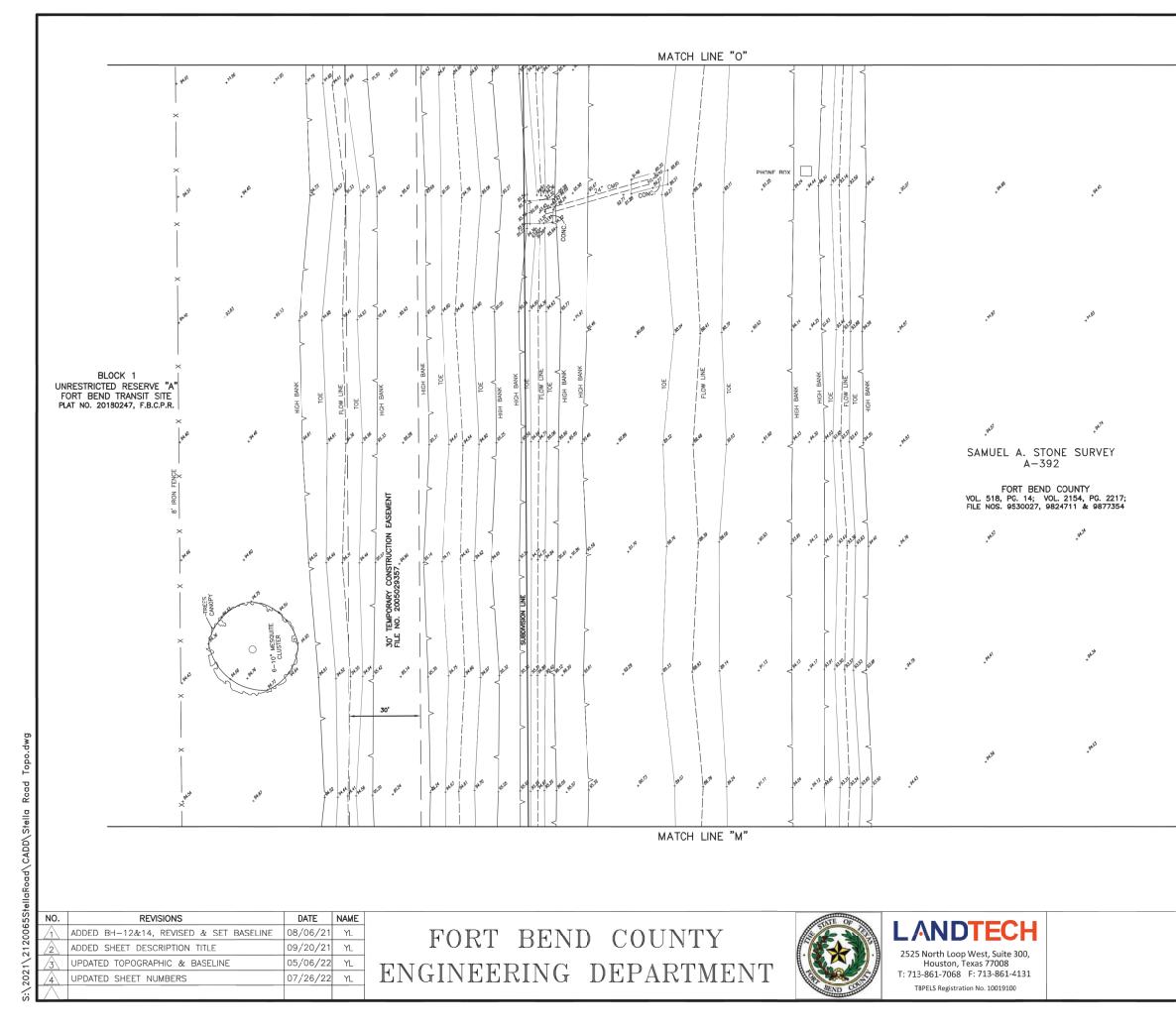
LEGEND

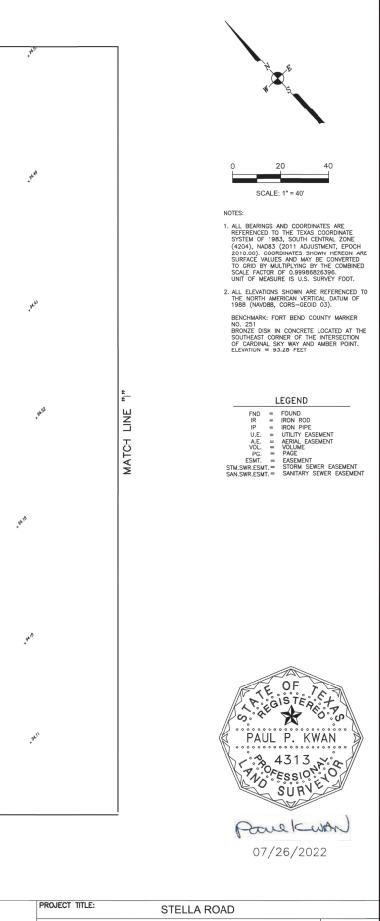
FND IR IP U.E. A.E. VOL. PG. ESMT.		FOUND IRON ROD IRON PIPE UTILITY EASEMENT AERIAL EASEMENT YOLUME PAGE EASEMENT STORM SEWER EASEMENT
ESMT.	=	
STM.SWR.ESMT		
SAN.SWR.ESMT	=	SANITARY SEWER EASEMENT

Γ	CURVE	RADIUS	LENGTH	DELTA	CHORD	CHORD BEAR.
Γ	C1	850.00'	1119.08'	75*26'03"	1039.99'	S 84°29'31" E

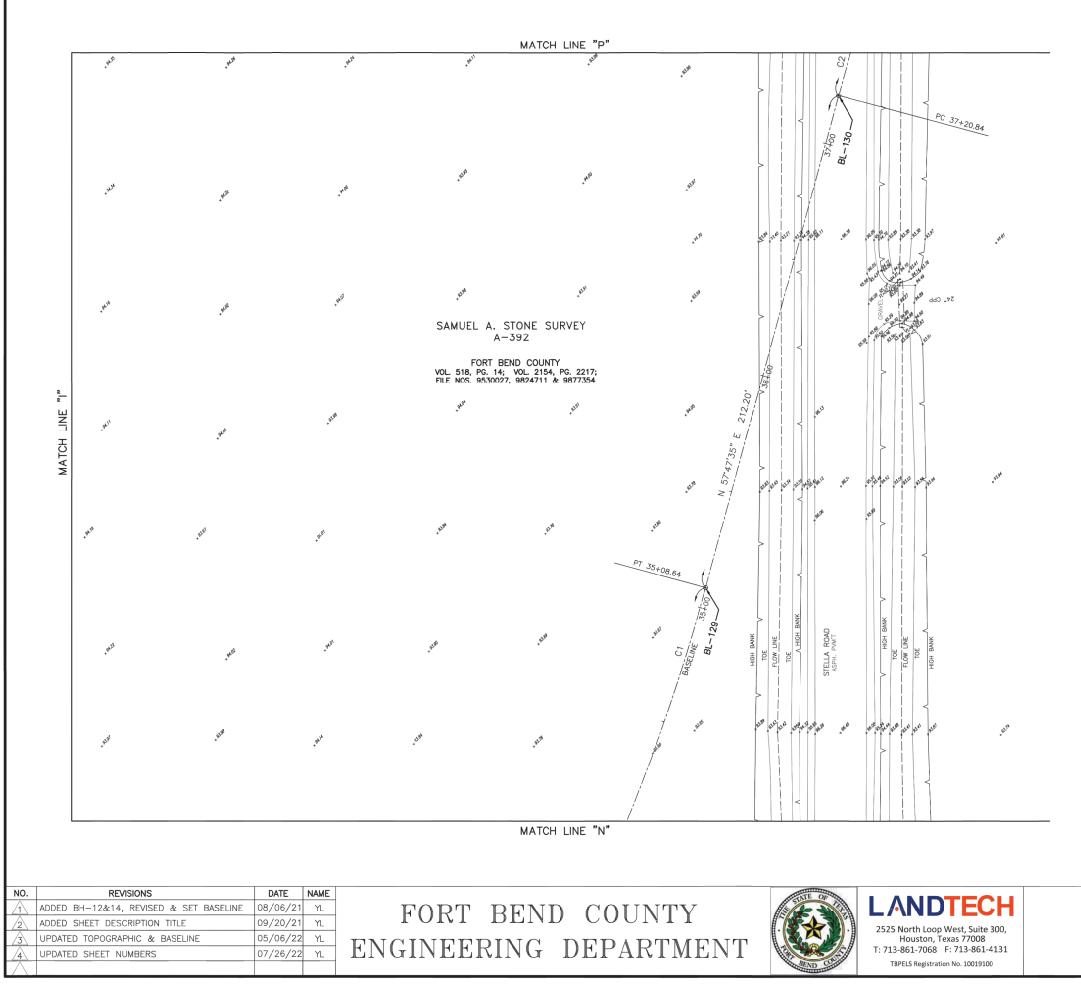


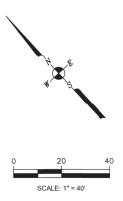
PROJECT TITL	E: STELLA ROAD	
FROM	CIVIL STANDARD	
SHEET DESCR		
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 20 / 133
	FROM SHEET DESCR DRAWN BY:	FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY DRAWN BY: CK'D BY:





	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD					
SHEET DESCR	SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY						
DRAWN BY:		DATE:					
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 21 / 133					





NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 AUJUSTNENT, EPOCH 2010.00). COORDINATES 3HOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEAT

LEGEND

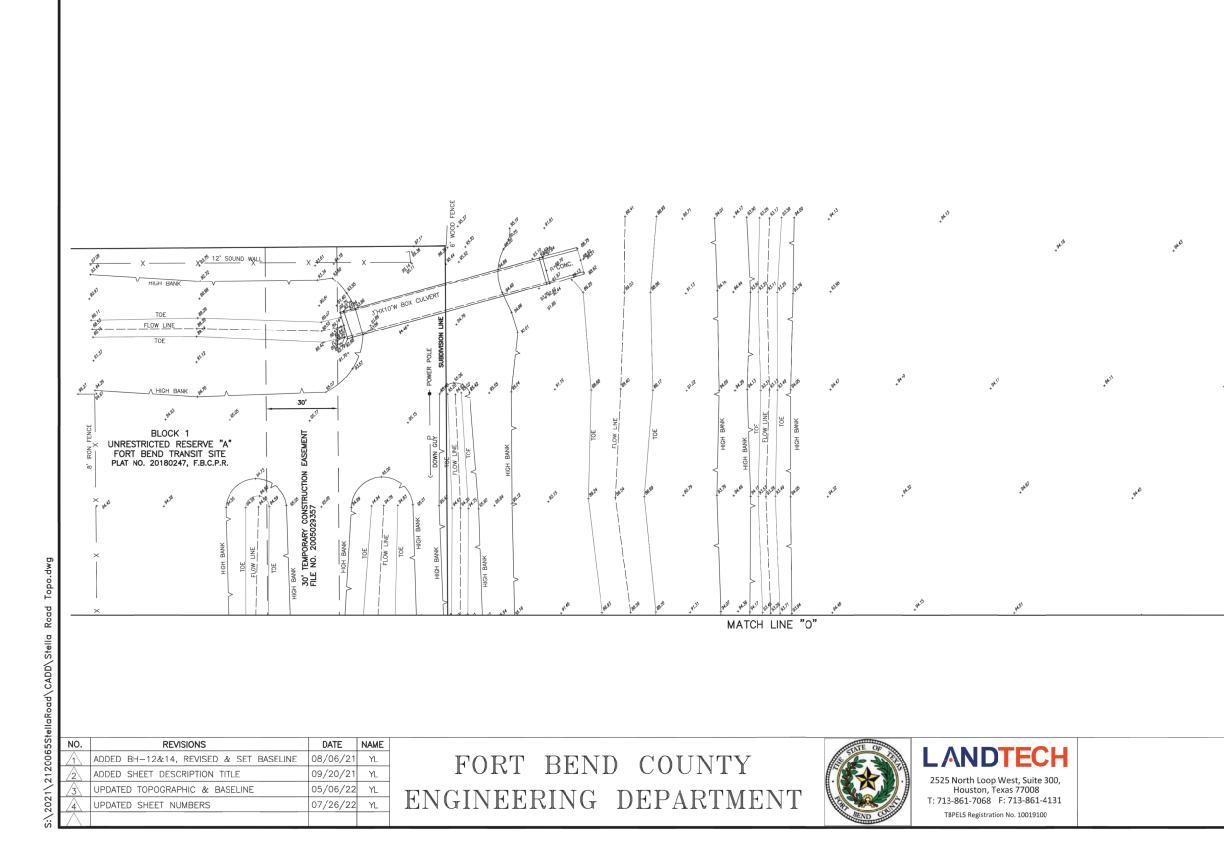
IR IP U.E. A.E. VOL. PG. ESMT.		FOUND IRON ROD IRON PIPE UTILITY EASEMENT AERIAL EASEMENT VOLUME EASEMENT STORM SEWER EASEMENT
	=	

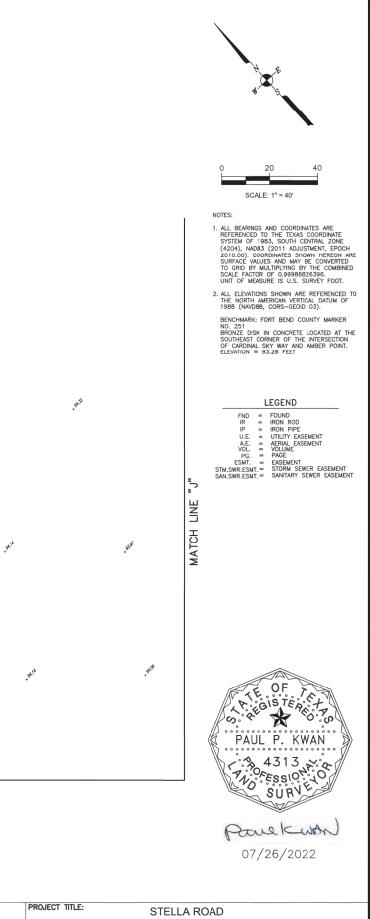
CURVE	RADIUS	LENGTH	DELTA	CHORD	CHORD BEAR.
C1	850.00'	1119.08'	75*26'03"	1039.99'	S 84°29'31" E
C2	850.00'	1115.61	75'12'00"	1037.25'	S 84'50'34" E

BASELINE DATA										
POINT STATION NORTHING EASTING DESCRIPTION										
BL-129 35+	08.64 13,752,5	25.13 2,980,190	0.09 STELLA I	ROAD PT						
BL-130 37+20.84 13,752,638.23 2,980,369.64 STELLA ROAD PC										

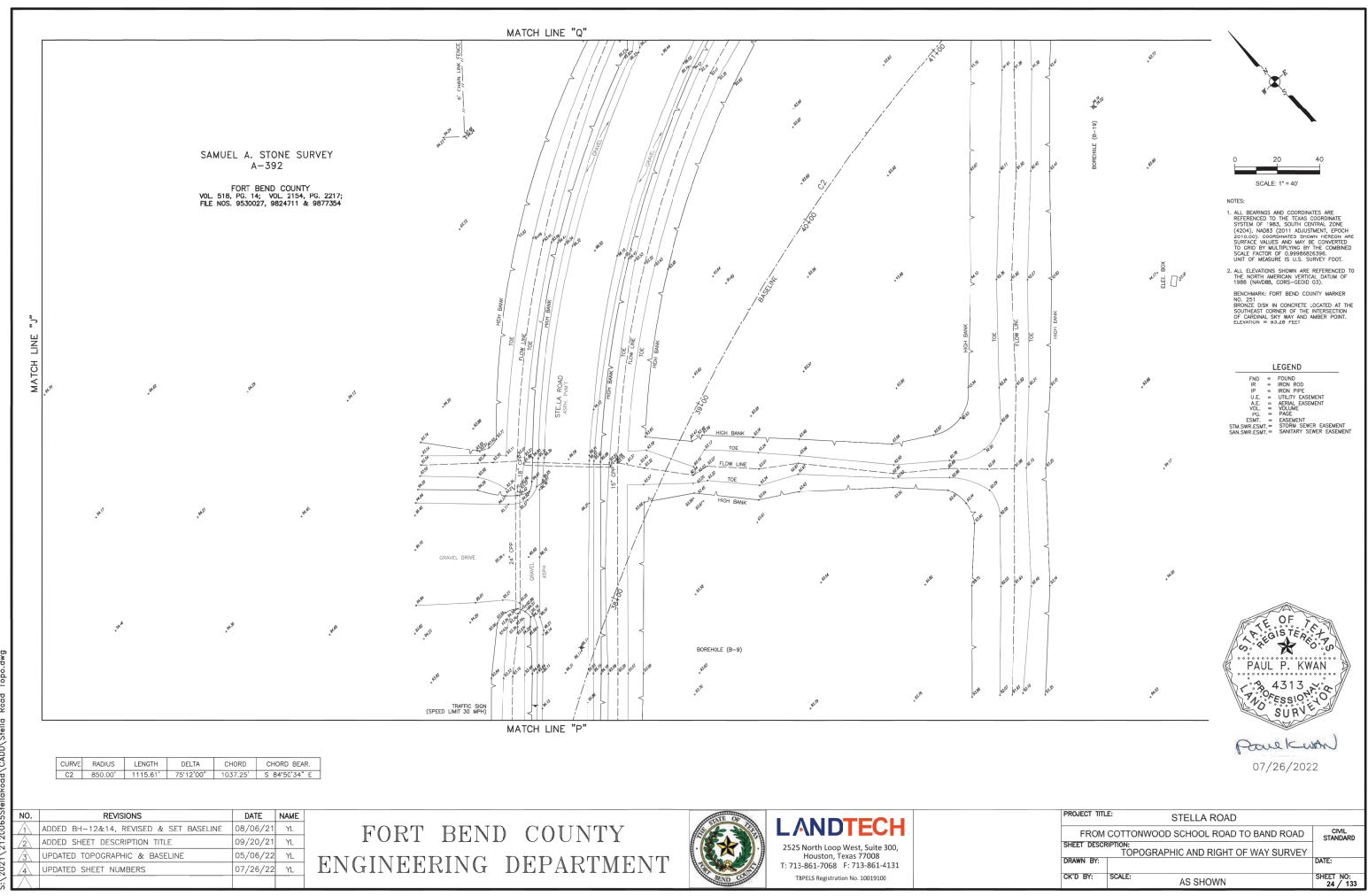


PROJECT TITL		
FROM	CIVIL STANDARD	
SHEET DESCR		
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 22 / 133

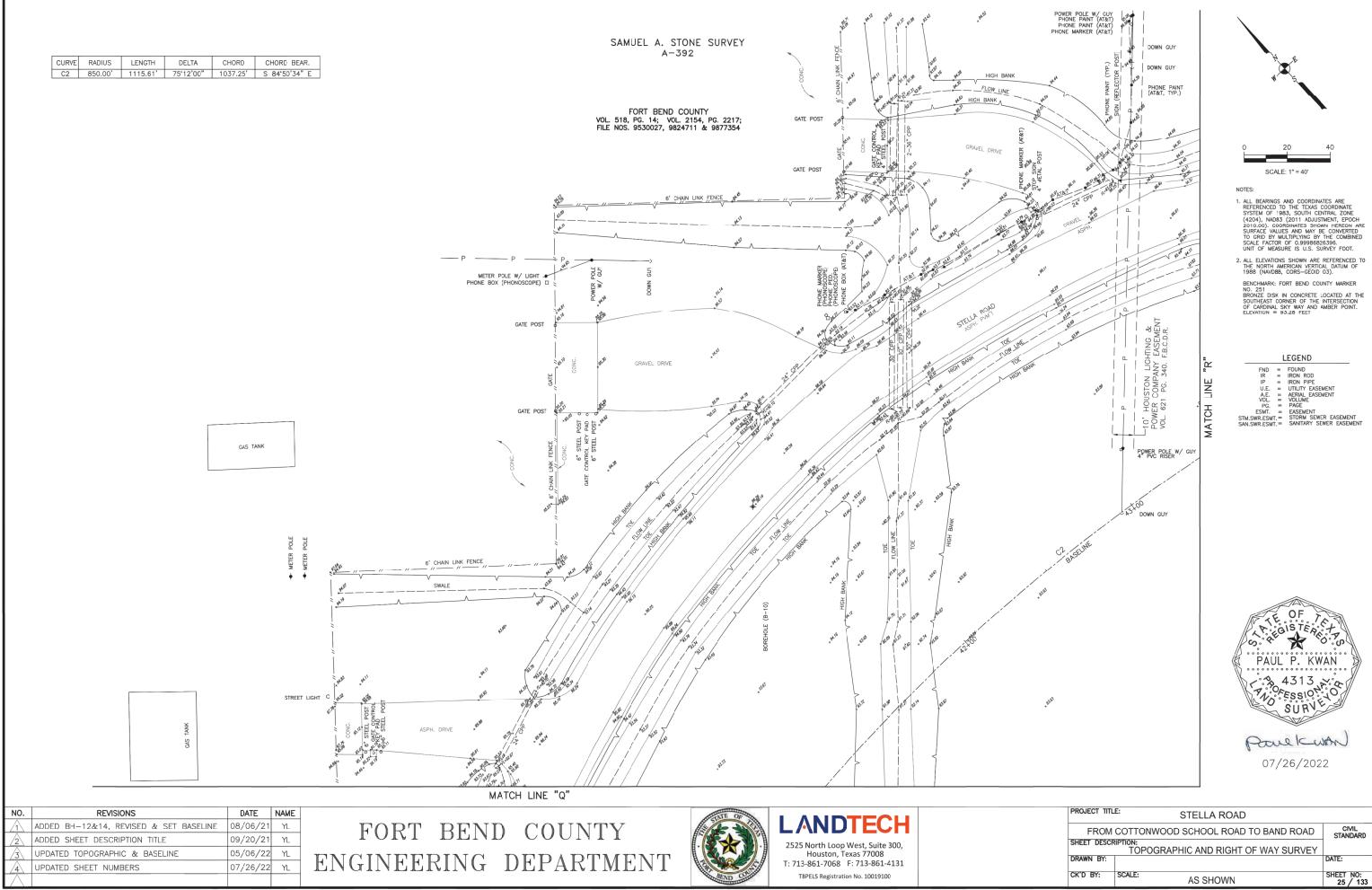




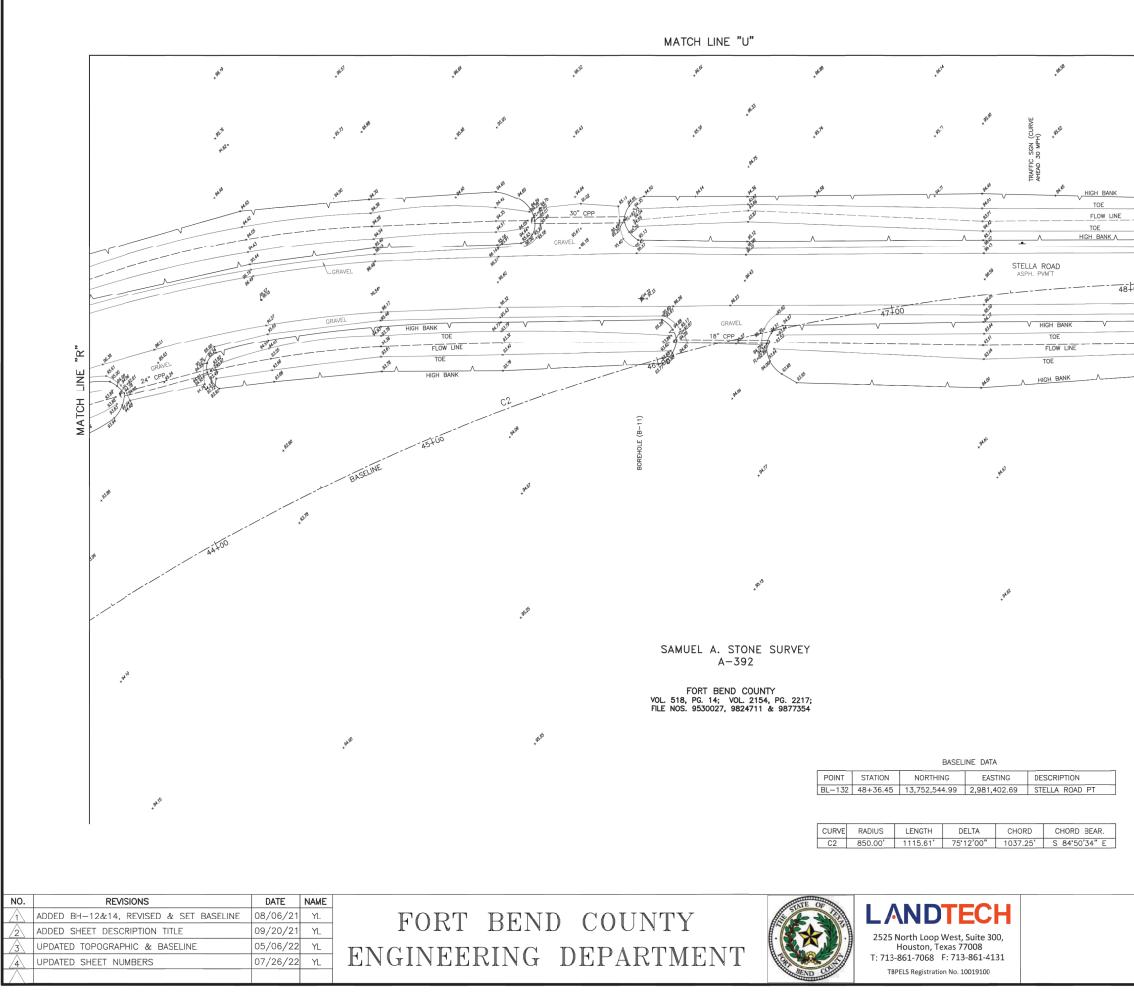
GTELEATROAD								
	FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD							
	DATE:							
	DRAWN BY:							
	CK'D BY:	SCALE: AS SHOWN	SHEET NO: 23 / 133					



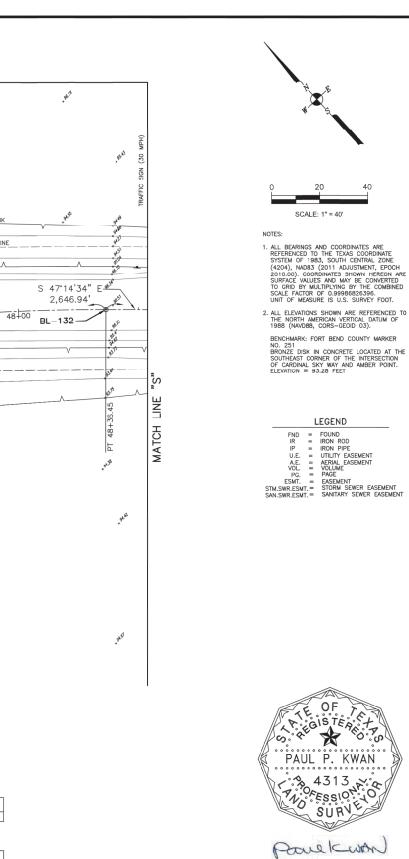
DRAWN BY:			DATE:			
CK'D BY:	SCALE:	AS SHOWN	SHEET NO: 24 / 133			



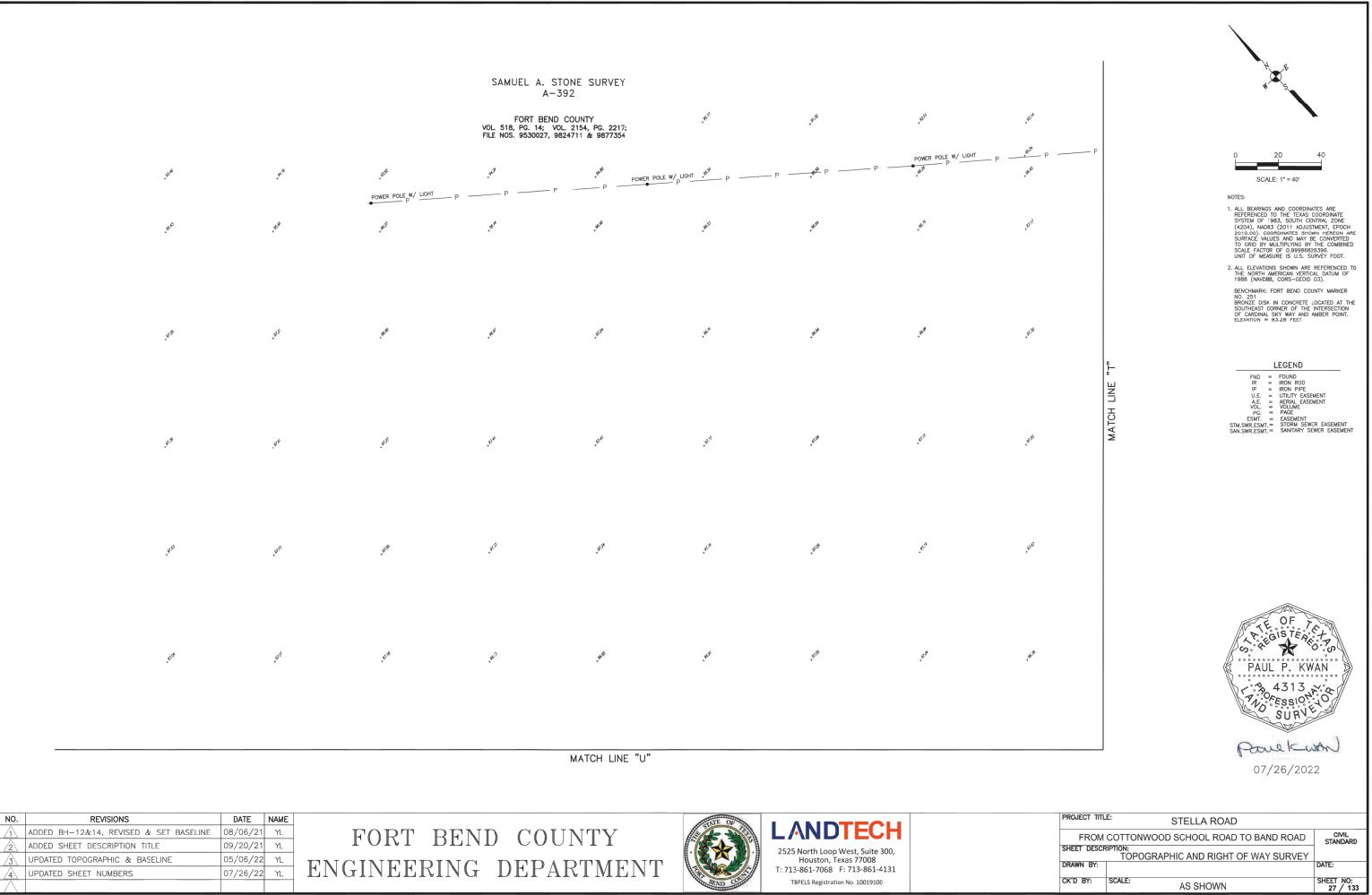
:021\2120065StellaRoad\CADD\Stella Road To



2021\2120065StellaRoad\CADD\Stella Road Topo.c

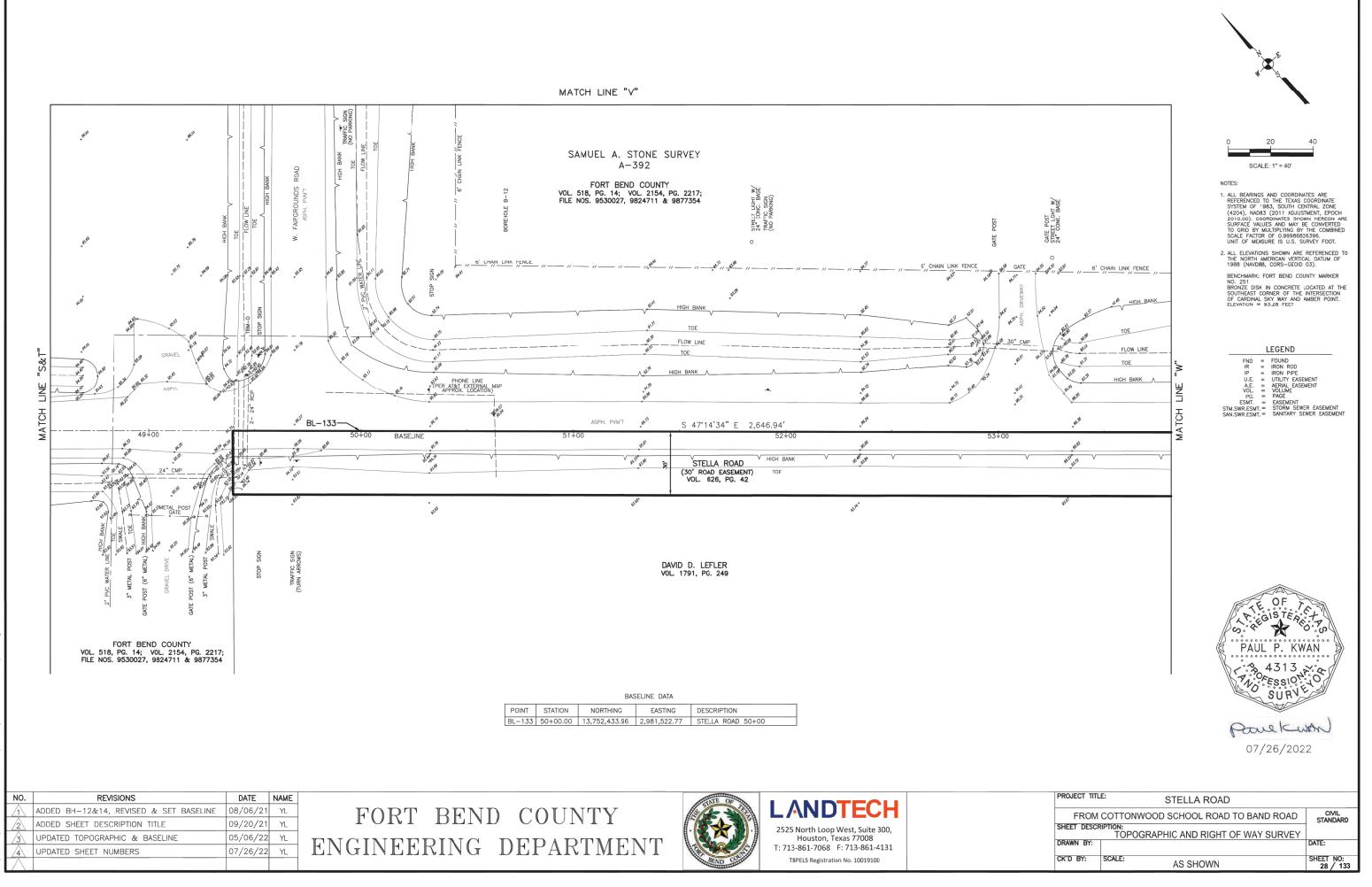


	PROJECT TITL						
FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD							
	SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY						
	DRAWN BY:		DATE:				
	CK'D BY:	SCALE: AS SHOWN	SHEET NO: 26 / 133				



2021/21

SCALE:	AS SHOWN	SHEET NO



:\2021\2120065StellaRoad\CADD\Stella Road Topo.dv

s:\2021\2120065StellaRoad\CADD\Stella Road Topo.dwg		MATCH LINE "T"	**** **** **** **** **** ****	Image: state stat	SAMUEL A. STONE SURVEY A-302 FORT BEND COUNTY FUL 518, PG 14: VOL 2154, PG. 2217; FILE NOS. 9530027, 9824711 & 9877354
S:\2021\2120065	NO. REVISIONS ADDED BH-12&14, REVISED & SET BASELINE ADDED SHEET DESCRIPTION TITLE Image: Strength of the strengt of the strength of the strength of the strength of the str	DATE 08/06/ 09/20/ 05/06/ 07/26/	(21 YL) (21 YL) (22 YL)	FORT BEND COUNTY ENGINEERING DEPARTMENT	LANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPELS Registration No. 10019100



NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAB3 (2011 AUJUSTMENT, EPOCH 2010.00). COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99968625396, UNIT OF MEASURE IS U.S. SURVEY FOOT.
- 2. ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD8B, CORS-GEOID 03).

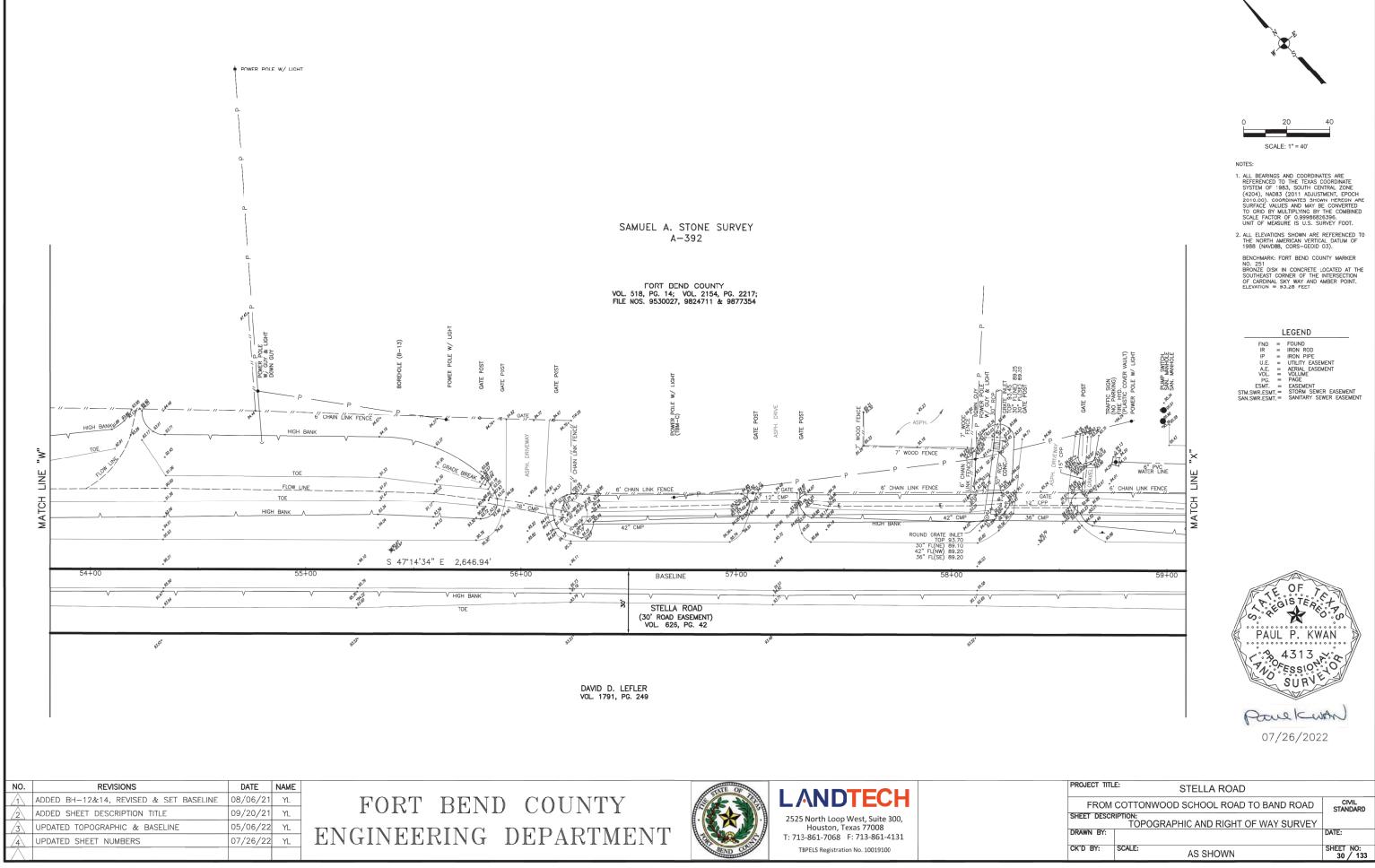
BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.20 FEET

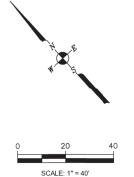
LEGEND

IR = IRON ROD IP IRON PIPE U.E. U.E. = UTILTY EASEMENT A.E. AERAL EASEMENT YOL = VOLUME PG = PAGE ESMT. = EASEMENT STM.SWR.ESMT.= STORM SEWER EASEMENT SAN.SWR.ESMT.= SANLERATE

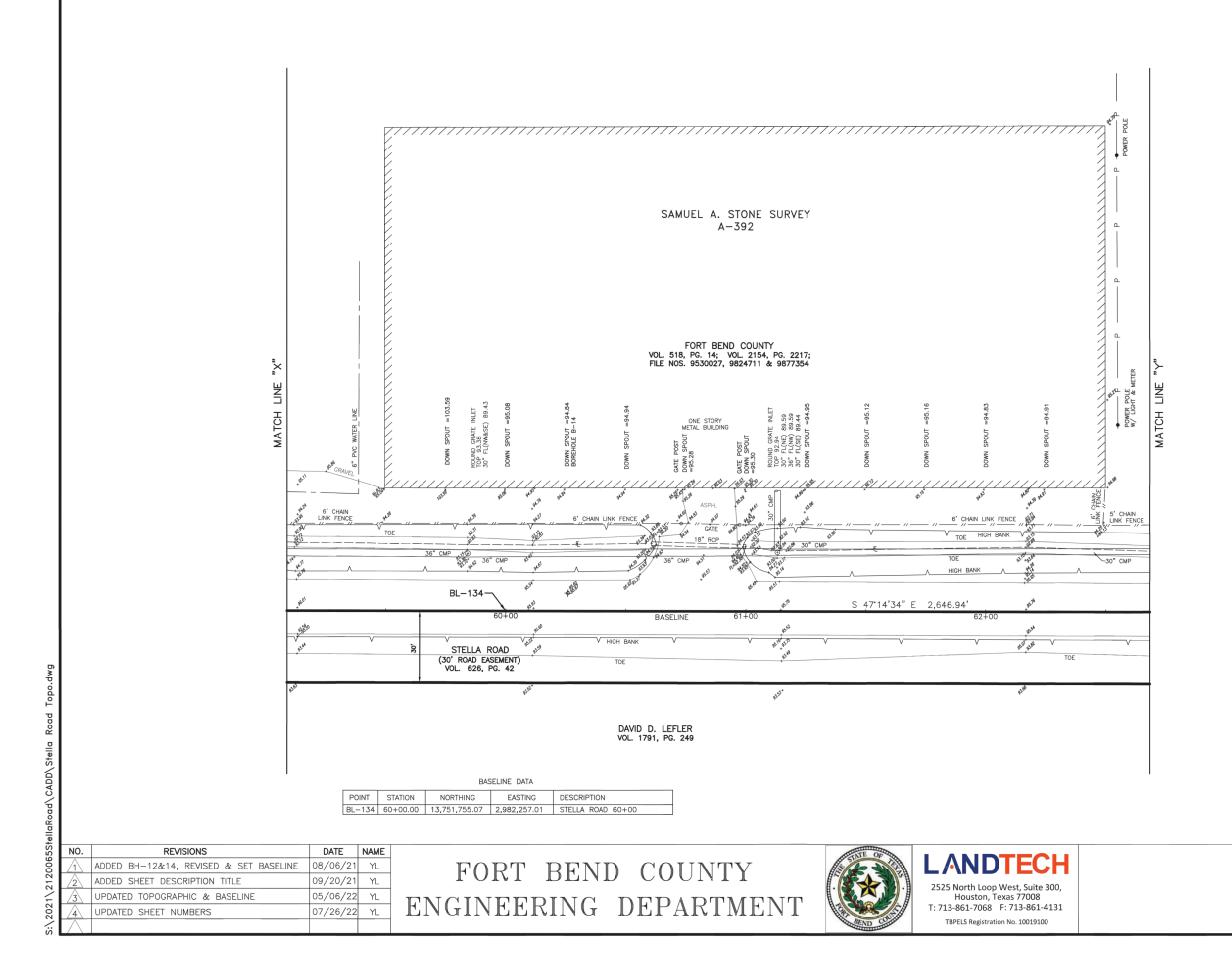


PROJECT TITL	E: STELLA ROAD					
FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD						
SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY						
DRAWN BY:		DATE:				
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 29 / 133				





PROJECT TITL	E: STELLA ROAD					
FROM	FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD					
SHEET DESCR	SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY					
DRAWN BY:		DATE:				
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 30 / 133				





NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 ADUISTNENT, EPOCH 2010.00). COORDINATES 3HOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

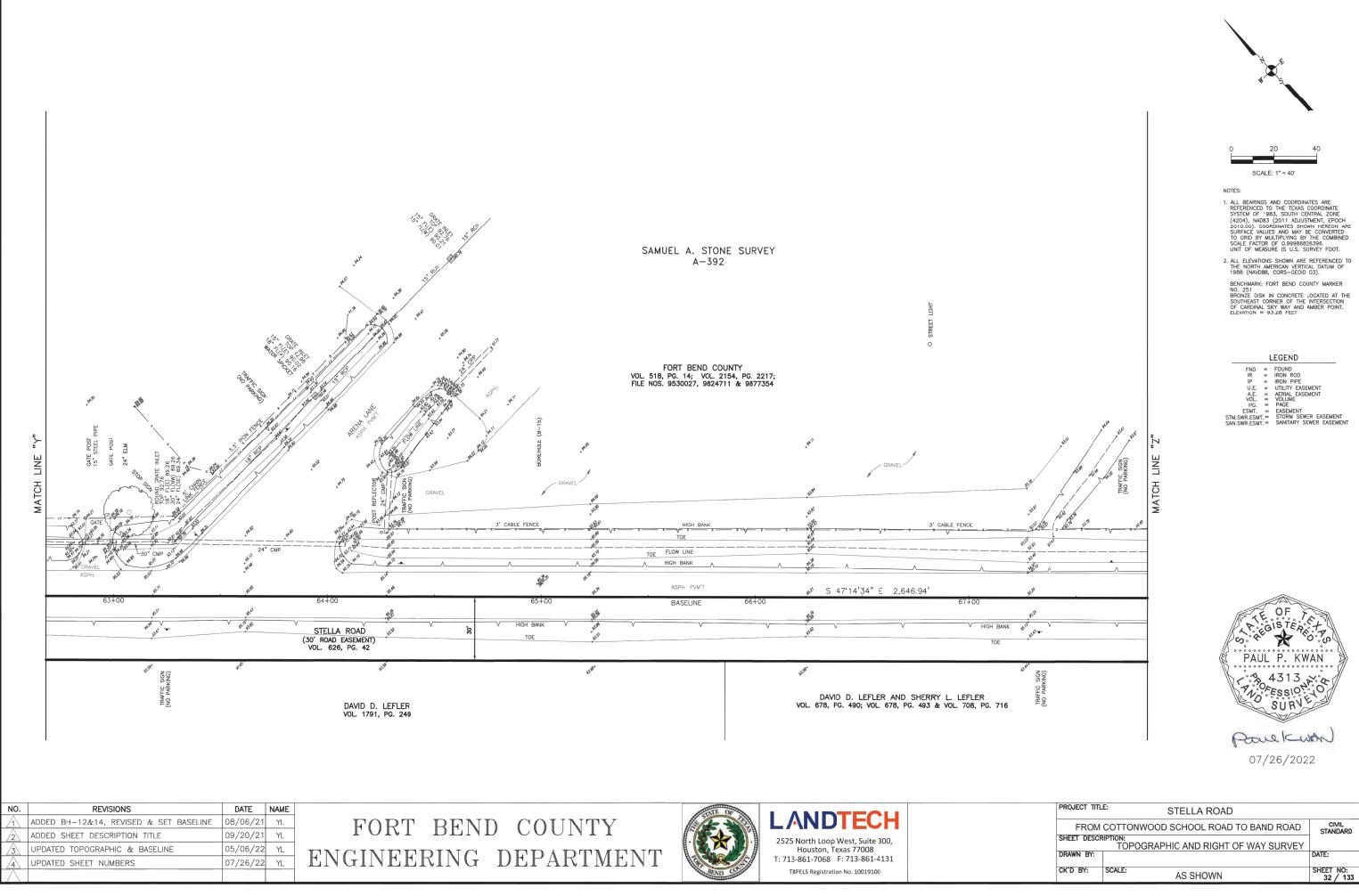
BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 9-3.26 FEAT

LEGEND

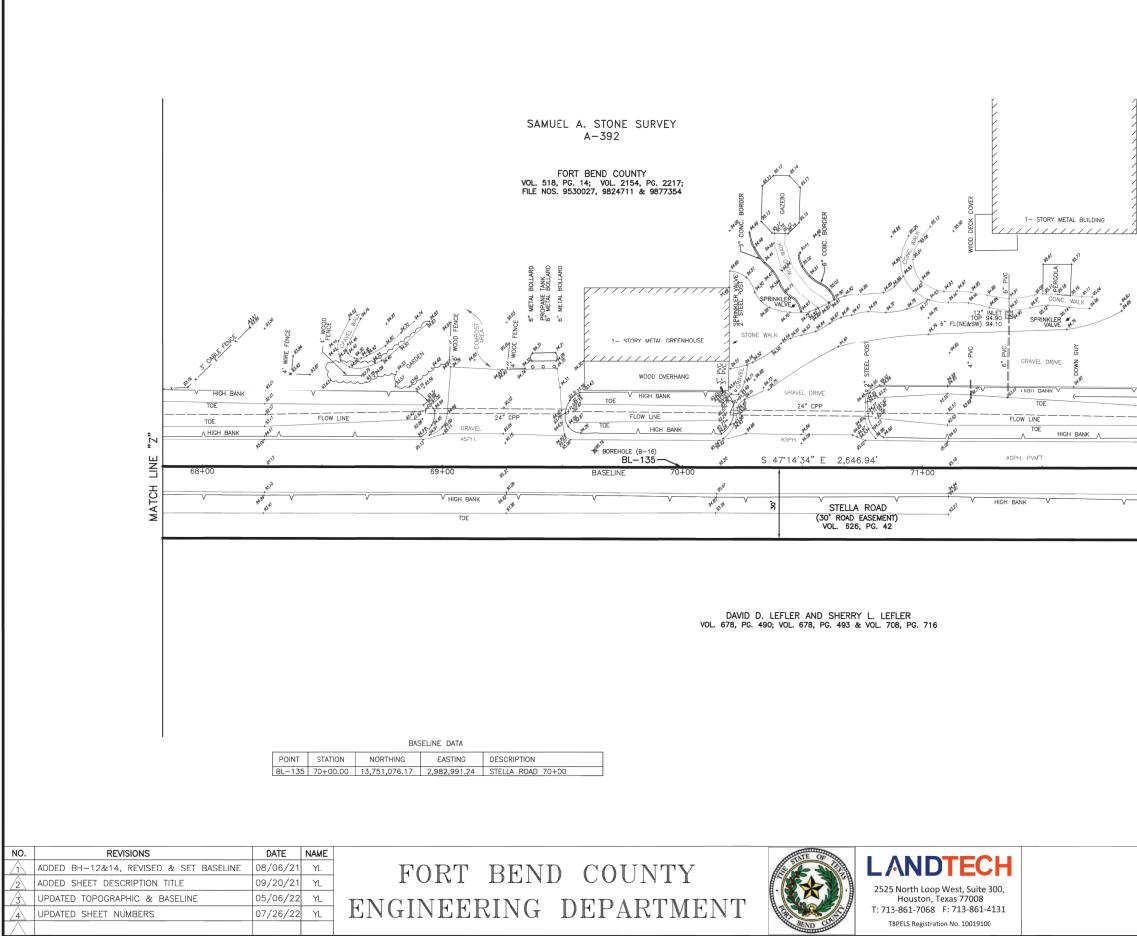
	FND	=	FOUND
	IR	=	IRON ROD
	IP	=	IRON PIPE
	U.E.	=	UTILITY EASEMENT
	A.E.	=	AERIAL EASEMENT
	VOL.	=	VOLUME
	PG.	=	PAGE
	ESMT.	=	EASEMENT
ST	M.SWR.ESM	Г. =	STORM SEWER EASEMENT
SA	N.SWR.ESM	Г. =	SANITARY SEWER EASEMENT



PROJECT TITL	E: STELLA ROAD					
FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD						
SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY						
DRAWN BY:		DATE:				
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 31 / 133				



FROM	CIVIL STANDARD				
SHEET DESCRIPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY					
DRAWN BY:		DATE:			
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 32 / 133			



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NOTES:

3" METAL RESER POWER POLE W/ 2-TRANSFORMERS

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MATCH

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 ADJUSTNENT, FPOCH 2010.00), COORDINATES SHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.99986826396. UNIT OF MESURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

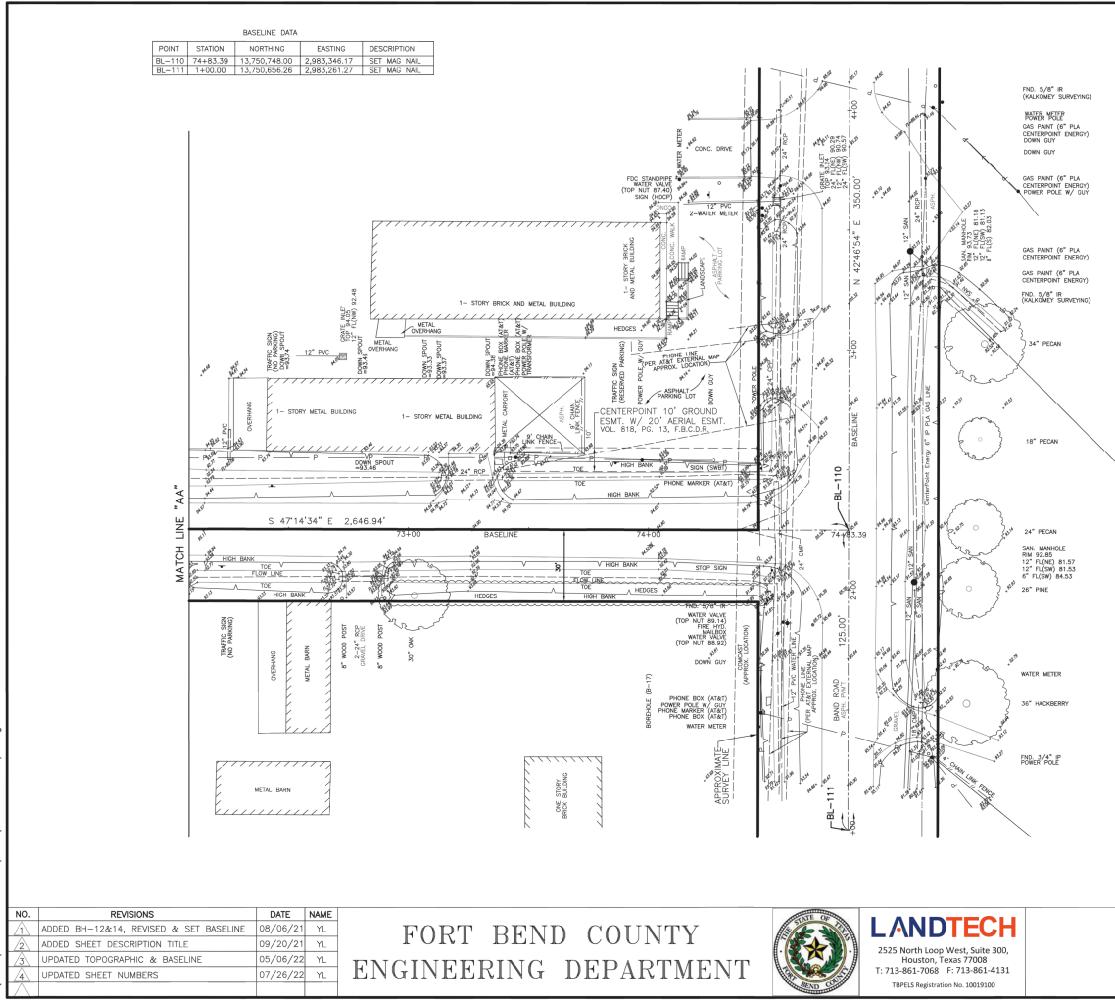
BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 95.26 FEE

LEGEND

	FND	=	FOUND
	IR	=	IRON ROD
	IP	=	IRON PIPE
	U.E.	=	UTILITY EASEMENT
	A.E.	=	AERIAL EASEMENT
	VOL.	=	VOLUME
	PG.	=	PAGE
	ESMT.	=	EASEMENT
ST	M.SWR.ESM	r. =	STORM SEWER EASEMENT
SA	N.SWR.ESM	r. =	SANITARY SEWER EASEMENT



_			
	PROJECT TITL	E: STELLA ROAD	
	FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
	SHEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
	DRAWN BY:		DATE:
	CK'D BY:	SCALE: AS SHOWN	SHEET NO: 33 / 133



2021\2120065StellaRoad\CADD\Stella Road Top



NOTES:

- ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NADB3 (2011 ADJUSTMENT, EPOCH 2010.00). COORDINATES JHOWN HEREON ARE SURFACE VALUES AND MAY BE CONVERTED TO ORID BY MULTIPL'NING BY THE COMBINED SCALE FACTOR OF 0.99986826396. UNIT OF MESURE IS U.S. SURVEY FOOT.
- ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

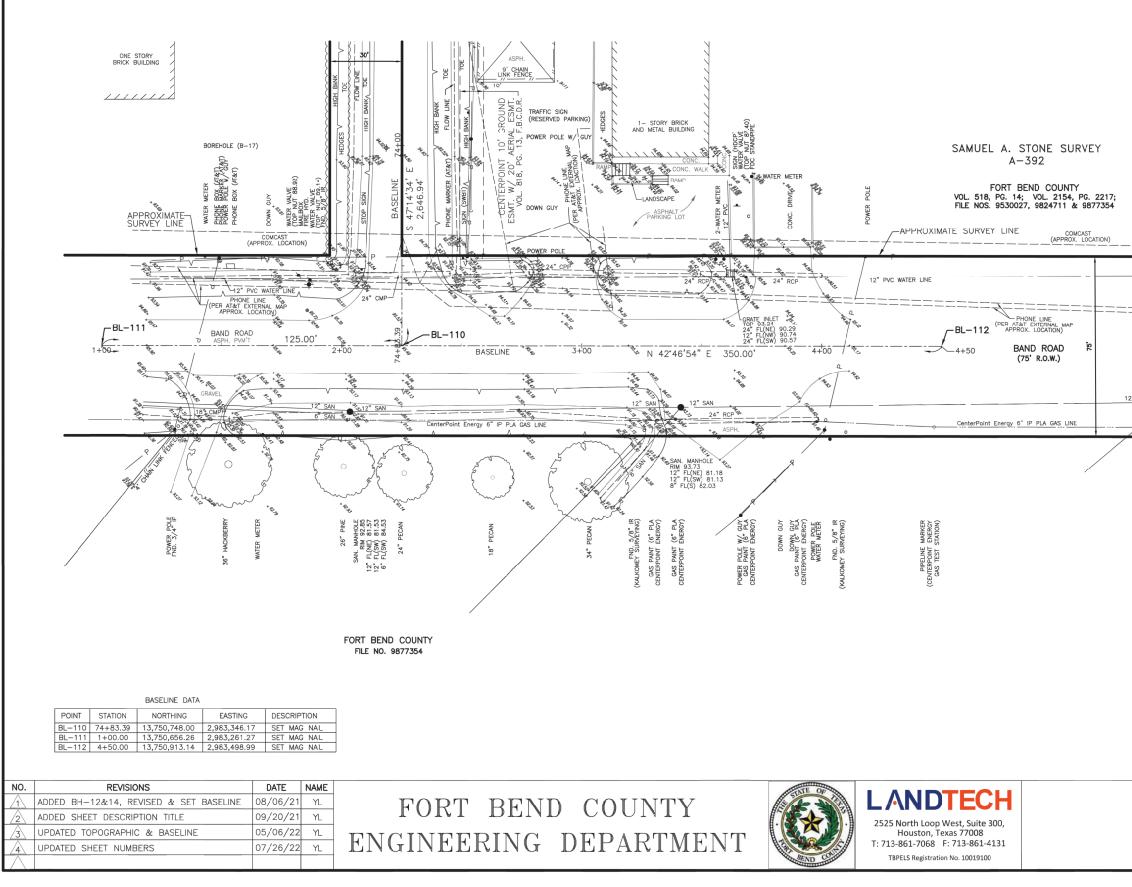
BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = SY.26 FEET

LEGEND

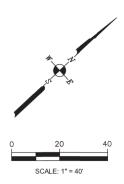
	FND	=	FOUND
	IR	=	IRON ROD
	IP	=	IRON PIPE
	U.E.	=	UTILITY EASEMENT
	A.E.	=	AERIAL EASEMENT
	VOL.	=	VOLUME
	PG.	=	PAGE
	ESMT.	=	EASEMENT
ST	M.SWR.ESM	Г. =	STORM SEWER EASEMENT
SA	N.SWR.ESM	Г. =	SANITARY SEWER EASEMENT



PROJECT TITL	E: STELLA ROAD	
FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 34 / 133



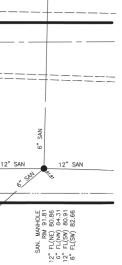
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NOTES:

- 1. ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAD83 (2011 ADUJSTNENT, EPOCH 2010.00). COORDINATES 3HOWN HERCON ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY MULTPLYING BY THE COMBINED SCALE FACTOR OF 0.99868623306. UNIT OF MEASURE IS U.S. SURVEY FOOT.
- 2. ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).

BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WAY AND AMBER POINT. ELEVATION = 93-26 FEET

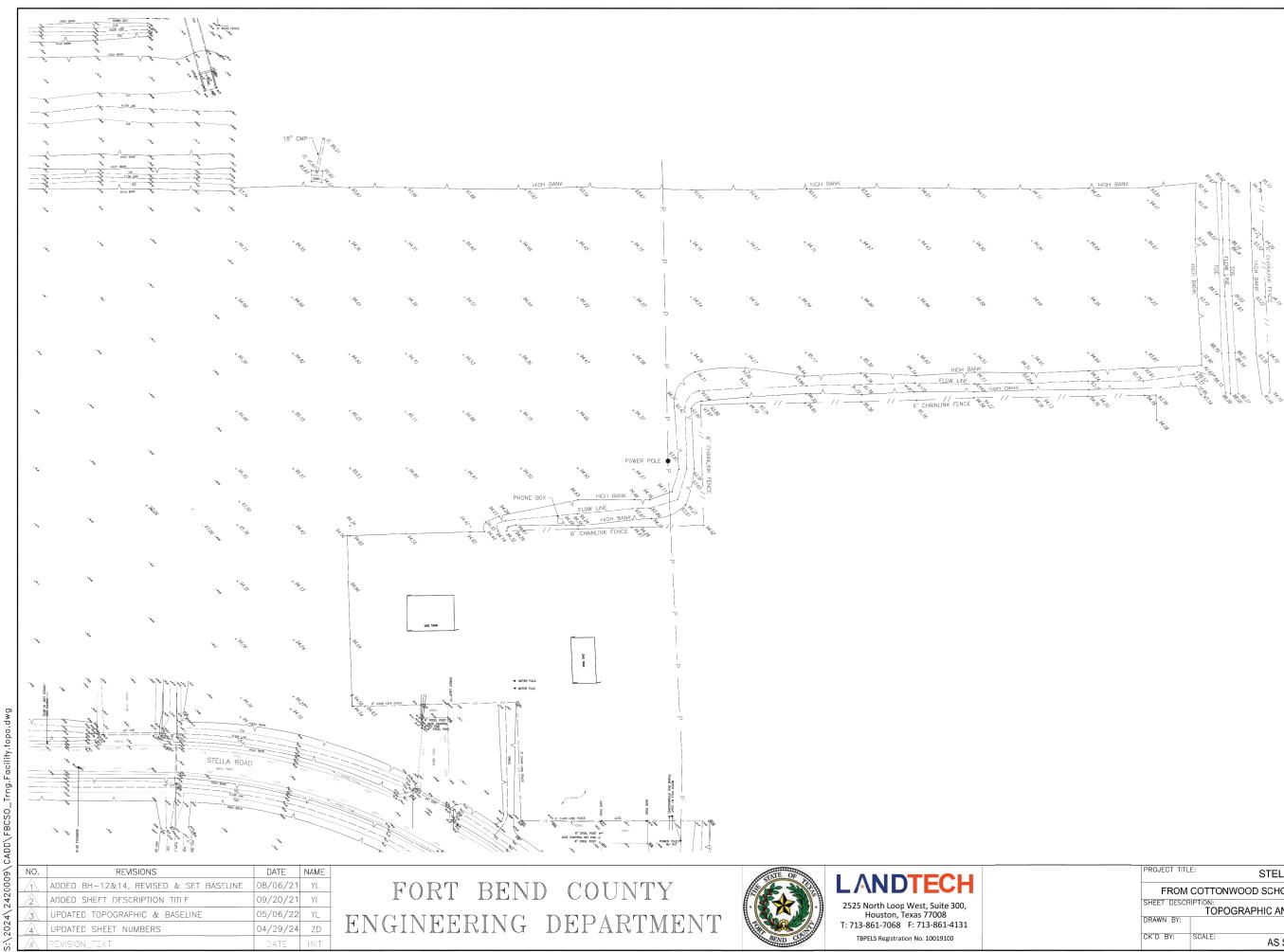


LEGEND

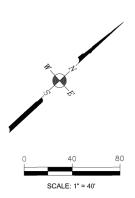
FND = IR = IP = U.E. = VOL. = PG. = ESMT. = SAN.SWR.ESMT. =	FOUND IRON ROD IRON PIPE UTILITY EASEMENT AERRAL EASEMENT VOLUME FAGE EASEMENT STORM SEWER EASEMENT SANTRARY SEWER EASEMENT
SAN.SWR.ESMI.=	SANITART SEWER EASEMENT



PROJECT TITL	E: STELLA ROAD	
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 35 / 133



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NOTES:

- ALL BEARINGS AND COORDINATES ARE REFERENCED TO THE TEXAS COORDINATE SYSTEM OF 1983, SOUTH CENTRAL ZONE (4204), NAB3 (2011 ADJUSTMENT, EPOCH 2010 A0), COORDINATES JUIOWI LIERCOM / SURFACE VALUES AND MAY BE COVVERTED TO CPDIP BY MILT THE VINGE OF THE COMBINE OF COMBINING OF THE COMBINE OF COMBINE OF COMBINING OF THE COMBINE OF COM GRID BY MULTIPLYING BY THE COMBINED ACTOR OF 0.99986826396. MEASURE IS U.S. SURVEY FOOT.
- . ALL ELEVATIONS SHOWN ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88, CORS-GEOID 03).
- BENCHMARK: FORT BEND COUNTY MARKER NO. 251 BRONZE DISK IN CONCRETE LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CARDINAL SKY WA' AND AMBER POINT. ELEVATION = 93.26 FEE

LEGEND

FND = IR = IP = U.E. = A.E. =	FOUND IRON ROD IRON PIPE UTILITY EASEMENT AERIAL EASEMENT
VOL. =	VOLUME
PG. =	PAGE
ESMT. =	EASEMENT
STM.SWR.ESMT. =	STORM SEWER EASEMENT
SAN.SWR.ESMT. =	SANITARY SEWER EASEMENT



PROJECT TITL	E: STELLA ROAD	
	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
SHEET DESCR	IPTION: TOPOGRAPHIC AND RIGHT OF WAY SURVEY	
DRAWN BY:		DATE:
CK'D BY:	SCALE: AS SHOWN	SHEET NO: 36 / 133

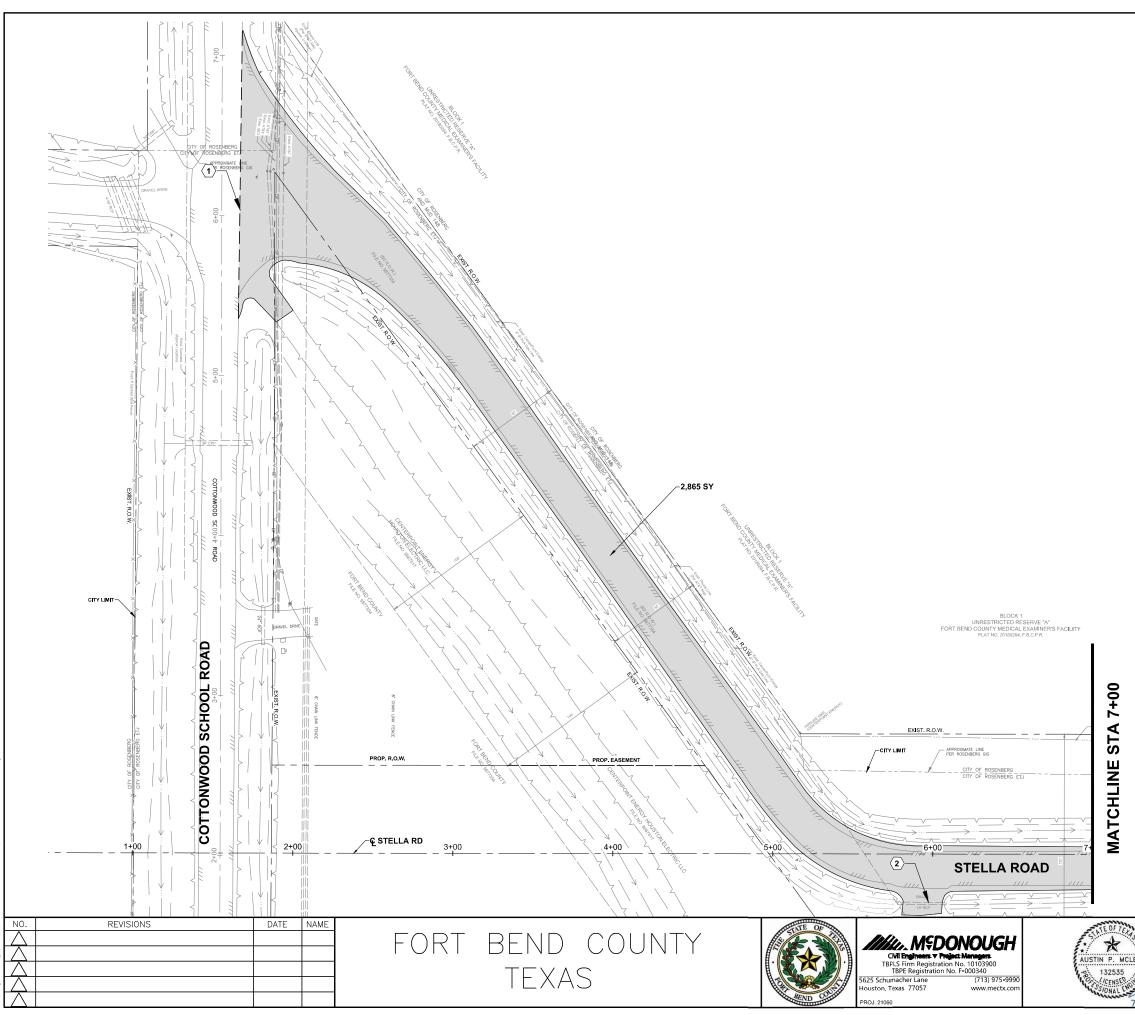
Charlie Valenzuela	
X:\Engineering\2021\21060 - Stella Road\37 HORIZONTAL ALIGNMENT DATA SHEET.dwg Charlie Volenzuela	
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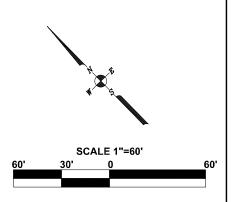
Э.	REVISIONS	DATE	NAME		TATE OF	
\mathbf{Z}				FORT REND COUNTY		MEDONOUGH
				I ONI DEND COONTI		Civil Engineers V Project Managers
						TBPLS Firm Registration No. 10103900 TBPE Registration No. F-000340
				IEXAS		5625 Schumacher Lane(713) 975-9990Houston, Texas 77057www.mectx.com
\mathbf{A}					SO SEND USO	PROJ. 21060



Horizontal Alignment Report Alignment Name: Stella Rd Station Range: Start: 0+00.00, End: 49+99.95 Description: Begin Stella Rd N 13,754,244.1303 E 2,977,397.5658 0+00.00 Line (1) S47° 20' 34.99"E 2,397.992' N 13,752,619.2334 E 2,979,161.1068 23+97.99 Line (1) Curve (2) BC N 13,752,619.2334 E 2,979,161.1068 23+97.99 CTR N 13,753,244.3437 E 2,979,737.0730 PI N 13,752,178.3639 E 2,979,639.5934 Direction Back S47° 20' 34.99"E Radius 850.000' Delta 74°51'50"(LT) Length 1,110.630' Tangent 650.627' Chord Direction S84° 46' 30,22"E Distance 1,033,293' Direction Ahead N57° 47' 34,54"E EC N 13,752,525.1355 E 2,980,190.1066 35+08.62 Curve (2) Line (3) N57° 47' 34.54"E 215.685' N 13,752,640.0913 E 2,980,372.6034 37+24.31 Line (3) Curve (4) BC N 13,752,640.0913 E 2,980,372.6034 37+24.31 CTR N 13,751,920.8830 E 2,980,825.6370 PI N 13,752,987.4921 E 2,980,924.1155 Direction Back N57° 47' 34.54"E Radius 850.000' Delta 74°57'52"(RT) Length 1,112,118' Tangent 651,808' Chord Direction S84° 43' 29.69"E Distance 1,034.474' Direction Ahead S47° 14' 33.92"E EC N 13,752,544.9842 E 2,981,402.6965 48+36.43 Curve (4) Line (5) S47° 14' 33.92"E 163.527' N 13,752,433.9667 E 2,981,522.7641 49+99.95 Line (5) N 13,752,433.9667 E 2,981,522.7641 49+99.95 End Stella Rd Alignment Length: 4,999.952'

` h.	PROJECT TITL	E: STELLA ROAD	
(*),	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: HORIZONTAL ALIGNMENT	
Con and	SCALE:	DATA SHEET	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	37 / 133





LEGEND



REMOVE EXIST. ASPHALT PAVEMENT AND BASE (ALL DEPTHS)

REMOVE EXIST. CONCRETE PAVEMENT AND BASE (ALL DEPTHS)



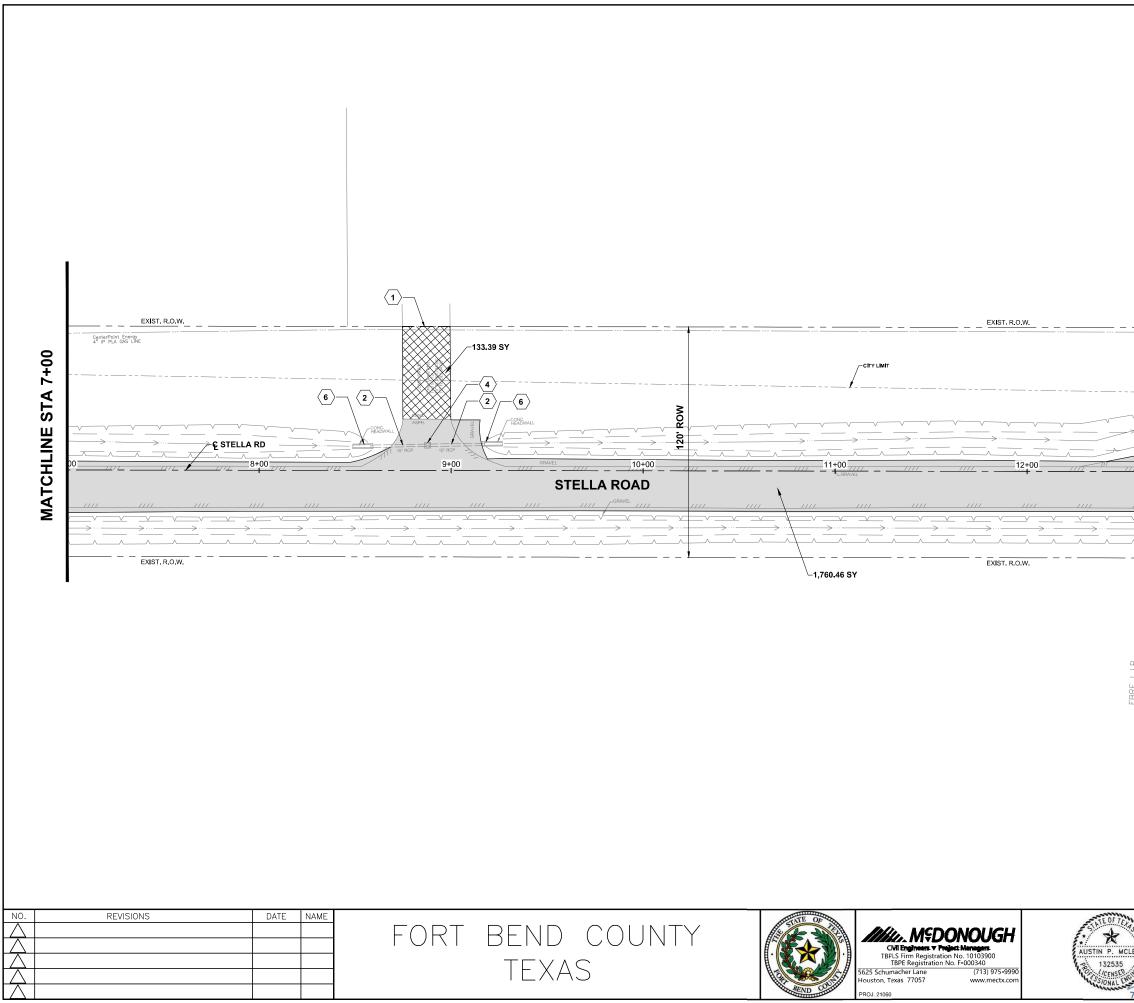
REMOVE EXIST. GRAVEL AND BASE (ALL DEPTHS)

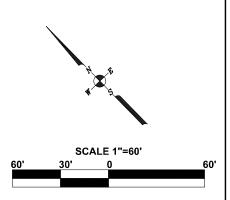


--- SAWCUT

- (1) SAWCUT PAVEMENT
- $\langle 2 \rangle$ REMOVE STORM SEWER (ALL SIZES)
- (3) REMOVE METAL BEAM GUARD FENCE
- $\langle \overline{4} \rangle$ REMOVE INLET OR MANHOLE (ALL SIZES)
- 6 REMOVE S.E.T OR HEADWALL

').	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: REMOVAL PLAN	
1 10	SCALE: 1" = 60	STA. 0+00 TO STA. 7+00	SHEET NO:
1/2/2024	DATE: 1/16/2023	APPROVED BY:	38 / 133







REMOVE EXIST. ASPHALT PAVEMENT AND BASE (ALL DEPTHS)

REMOVE EXIST. CONCRETE PAVEMENT AND BASE (ALL DEPTHS)

REMOVE EXIST. GRAVEL AND BASE (ALL DEPTHS)

FILL EXIST. DITCH

—— SAWCUT

(1) SAWCUT PAVEMENT

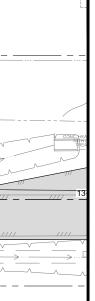
2 REMOVE STORM SEWER (ALL SIZES)

(3) REMOVE METAL BEAM GUARD FENCE

 $\langle 4 \rangle$ REMOVE INLET OR MANHOLE (ALL SIZES)

REMOVE FENCE AND/OR GATE (ALL TYPES)

6 REMOVE S.E.T OR HEADWALL



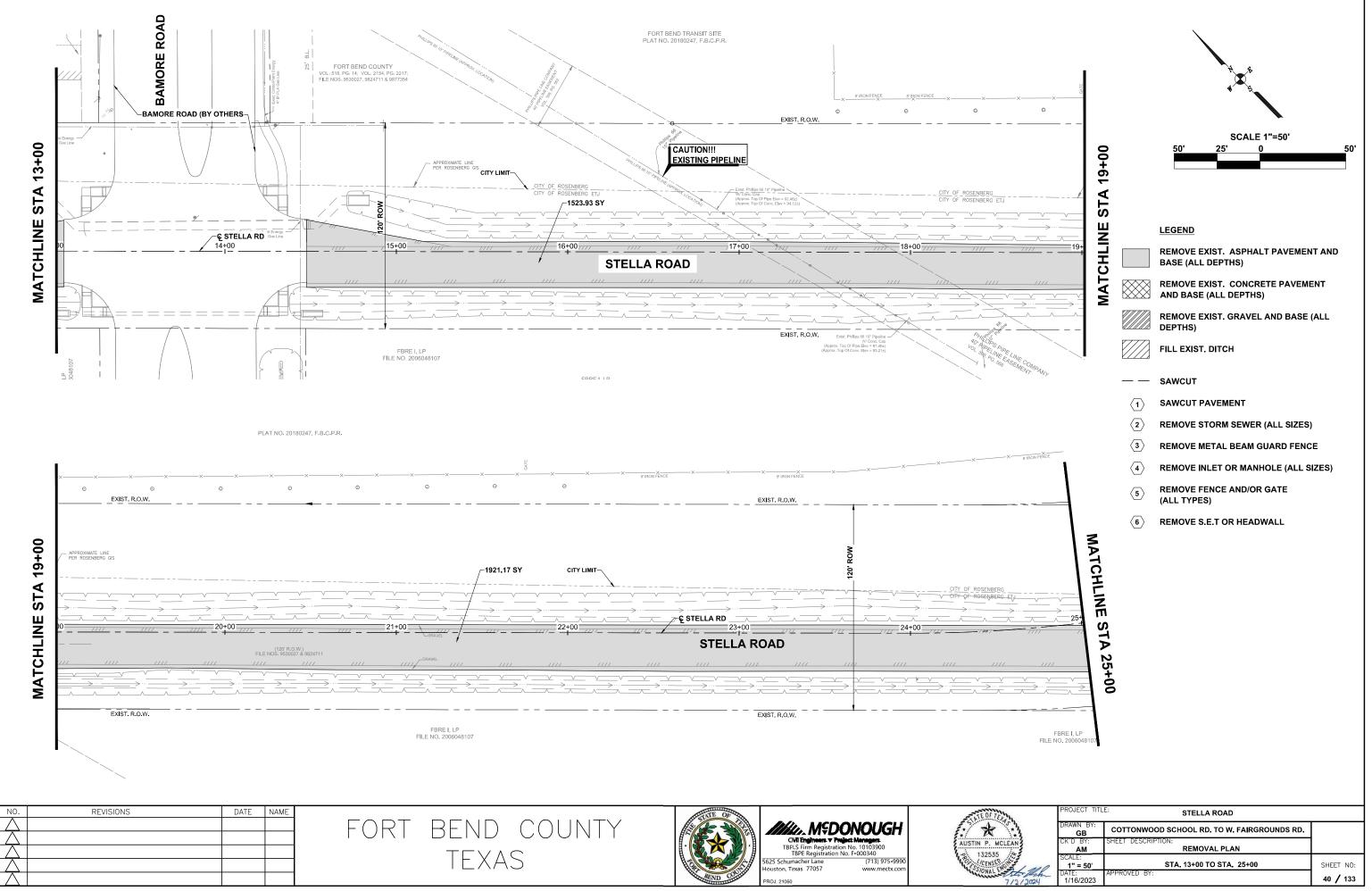
STA 13+00

MATCHLINE

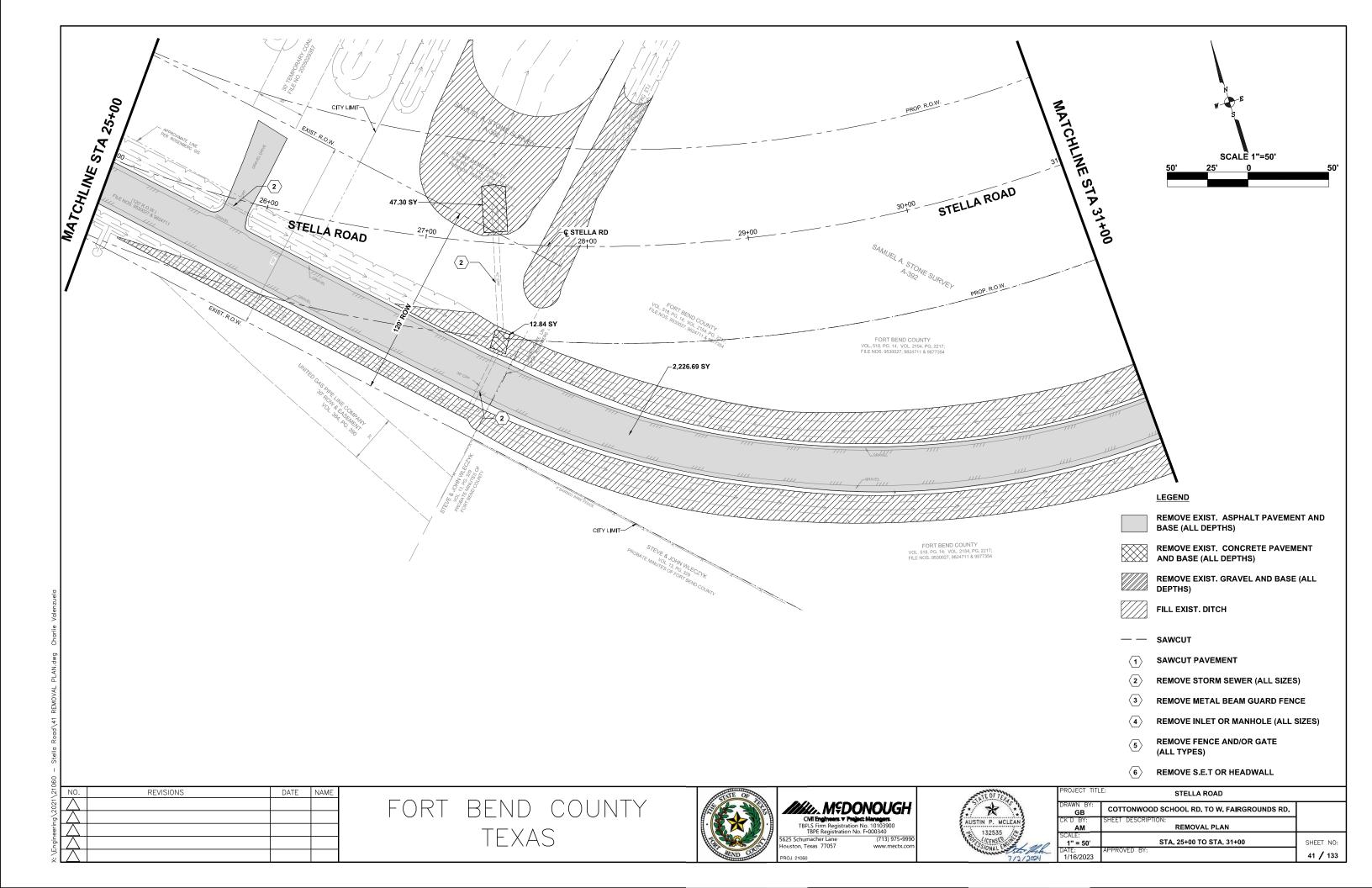
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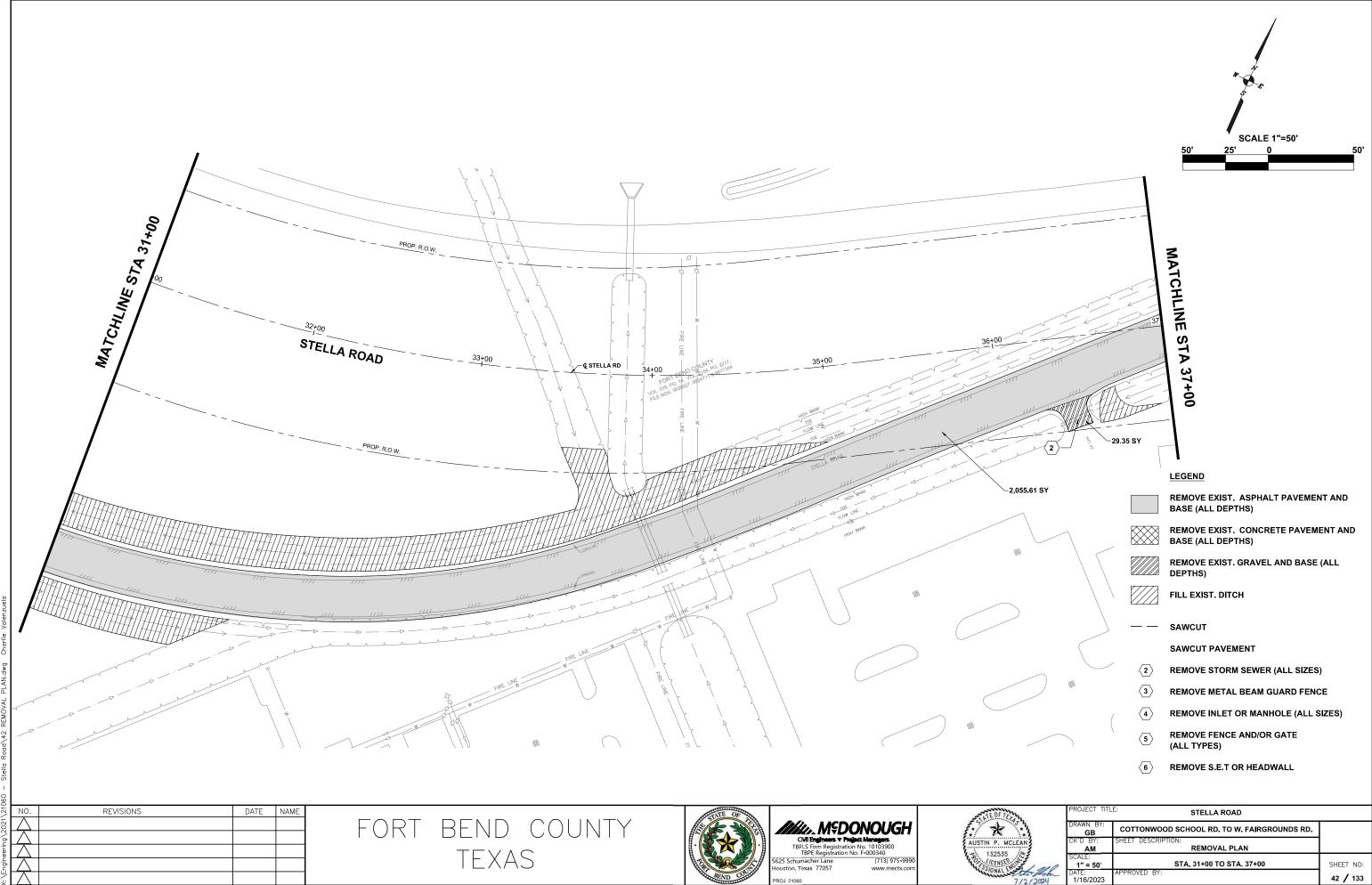
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	PROJECT TITL		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: REMOVAL PLAN	
	SCALE: 1" = 60'	STA. 0+00 TO STA. 7+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	39 / 133

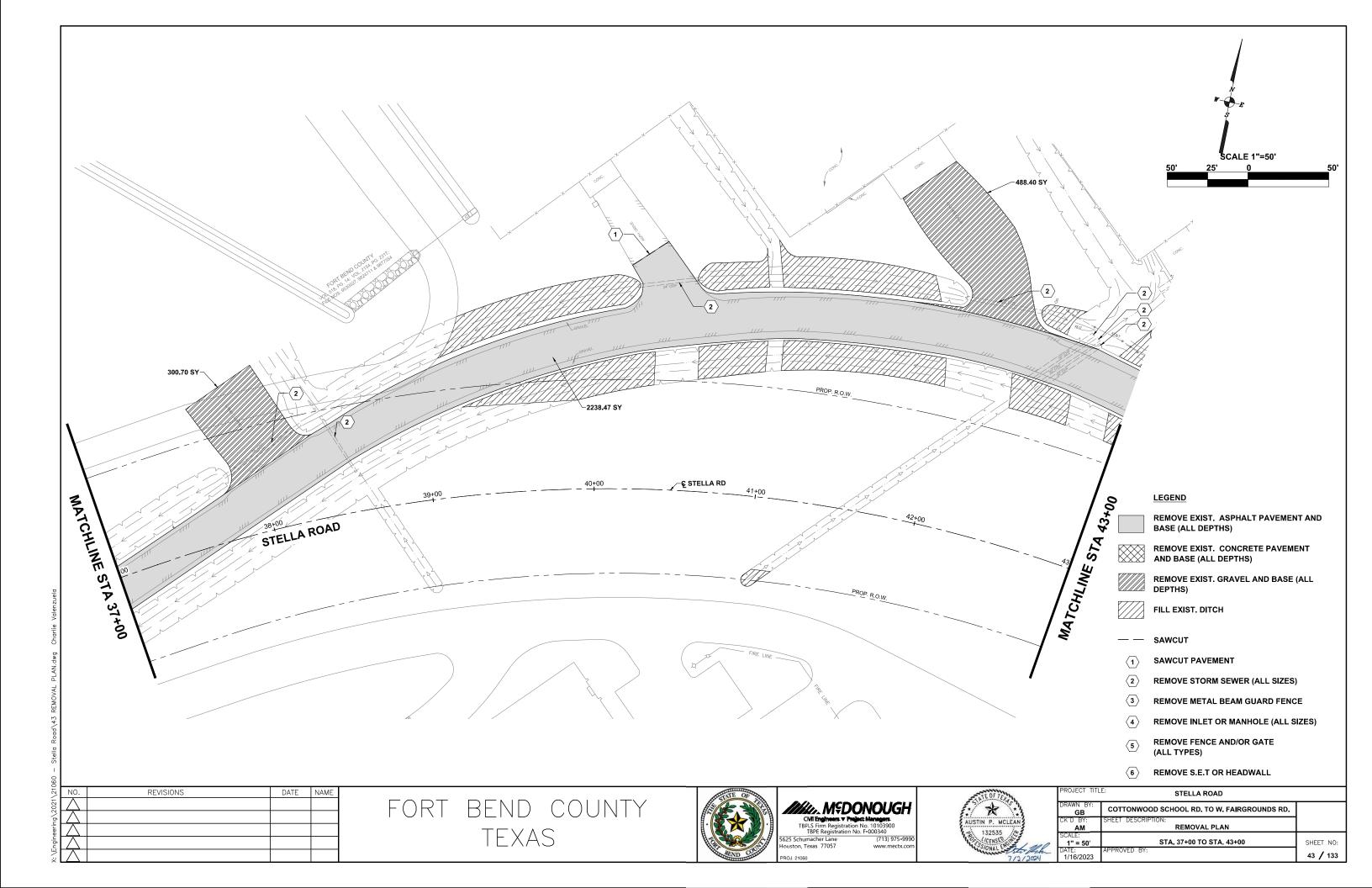


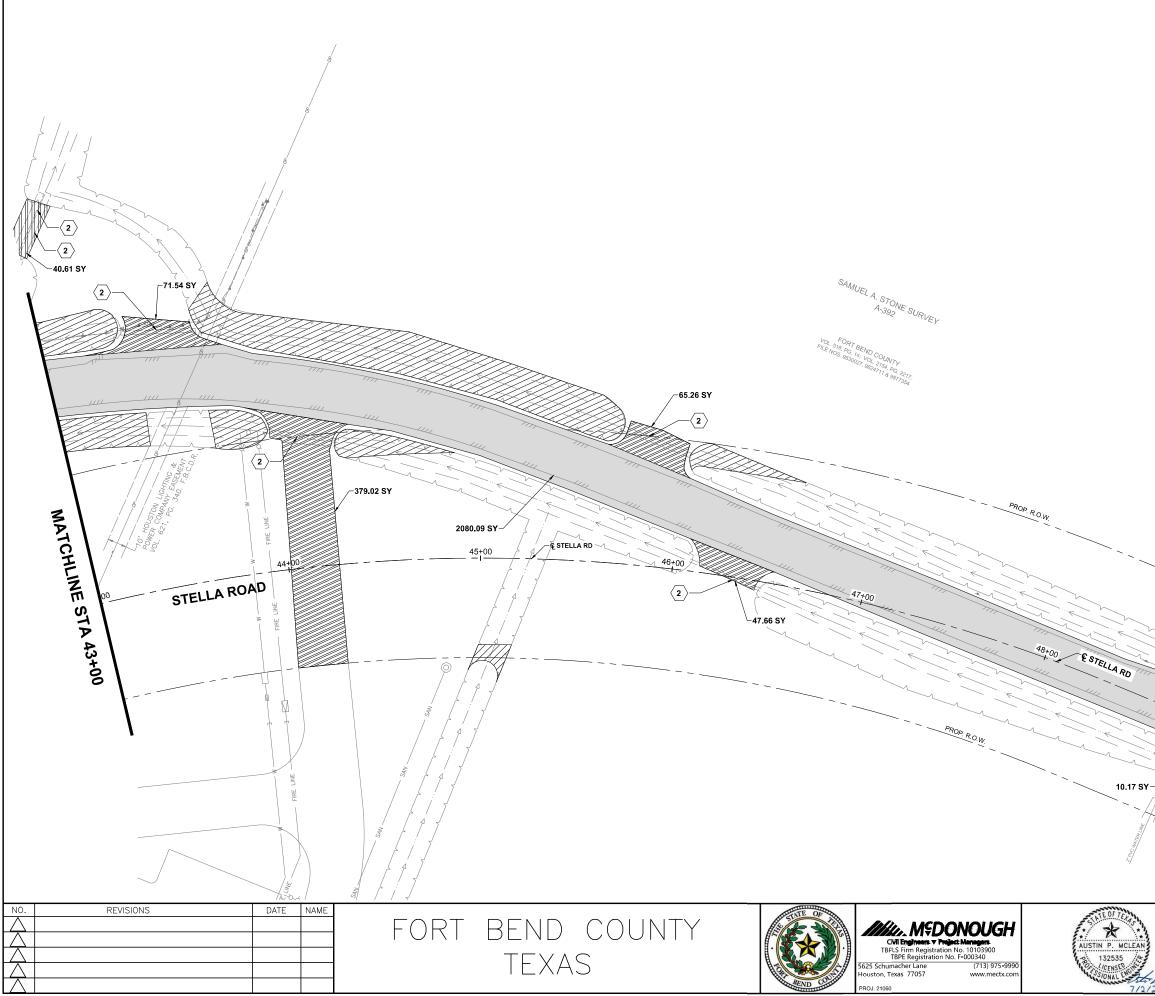
äh.	PROJECT TITLE: STELLA ROAD		
****	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
ICLEAN	CK'D BY: AM	SHEET DESCRIPTION: REMOVAL PLAN	
NO HUNDER	SCALE: 1" = 50'	STA. 13+00 TO STA. 25+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	40 / 133

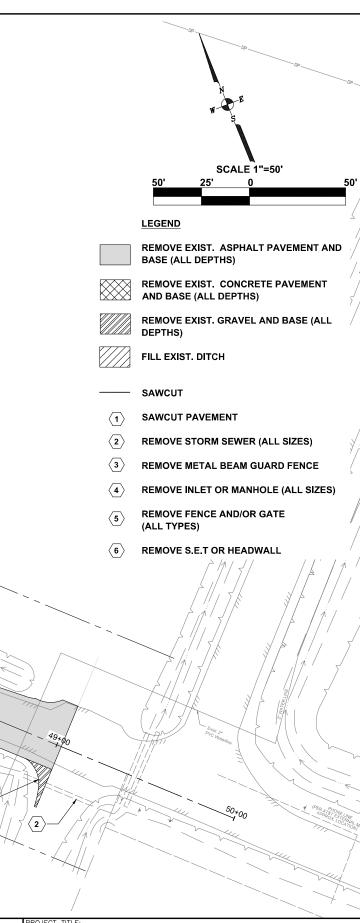




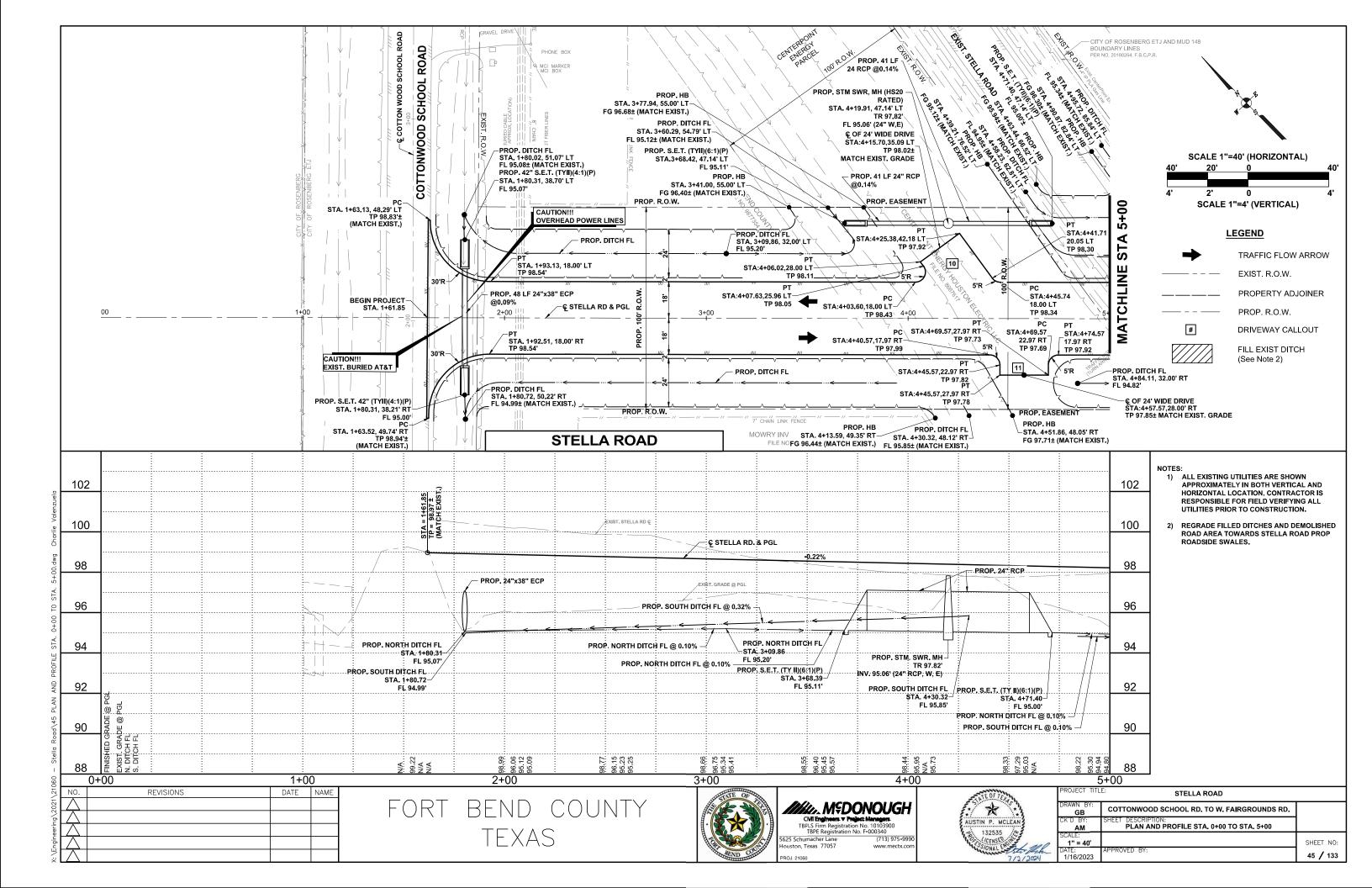
	GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AN	CK'D BY: AM	SHEET DESCRIPTION: REMOVAL PLAN	
	SCALE: 1" = 50'	STA. 31+00 TO STA. 37+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	42 / 133

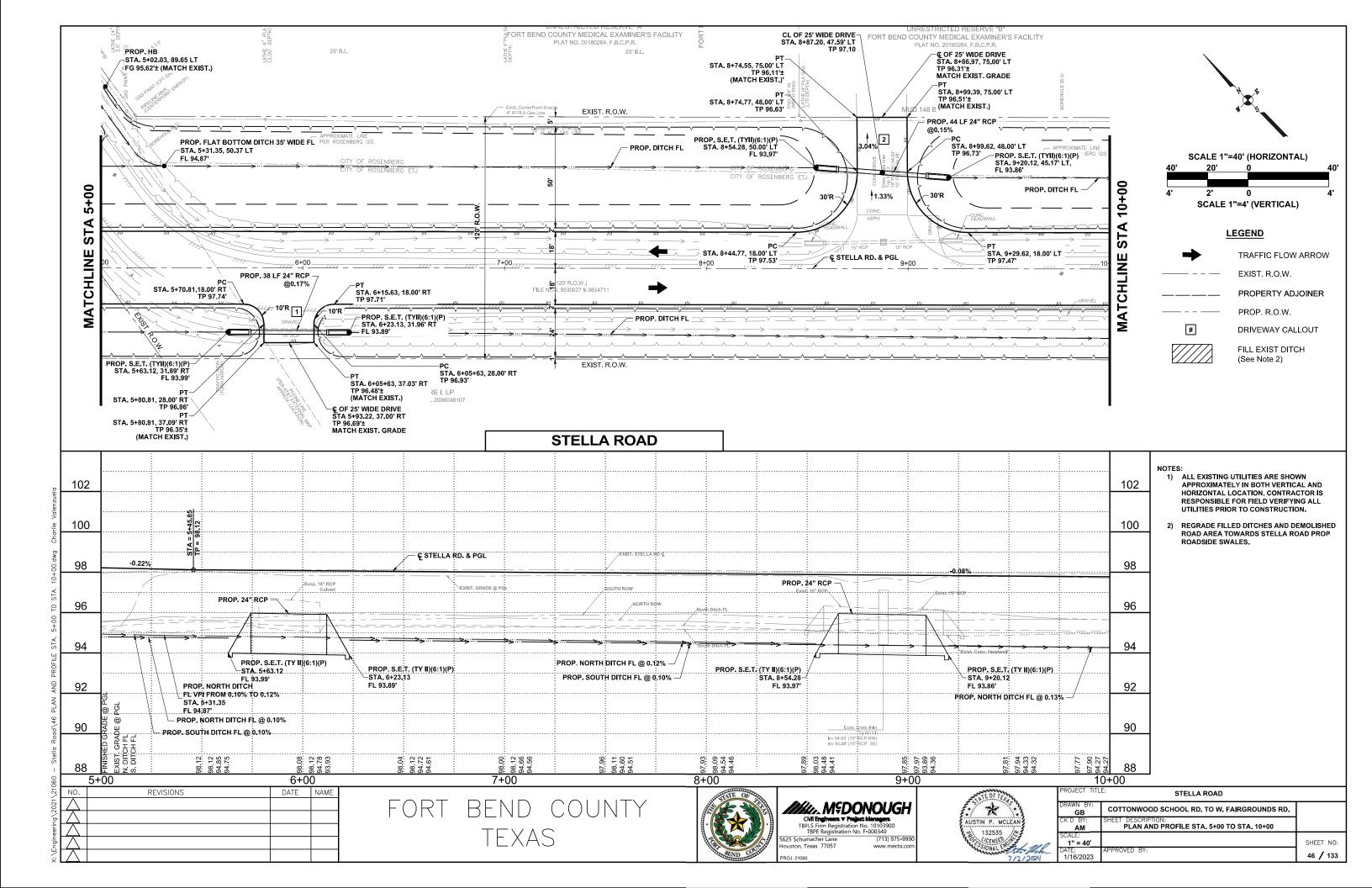


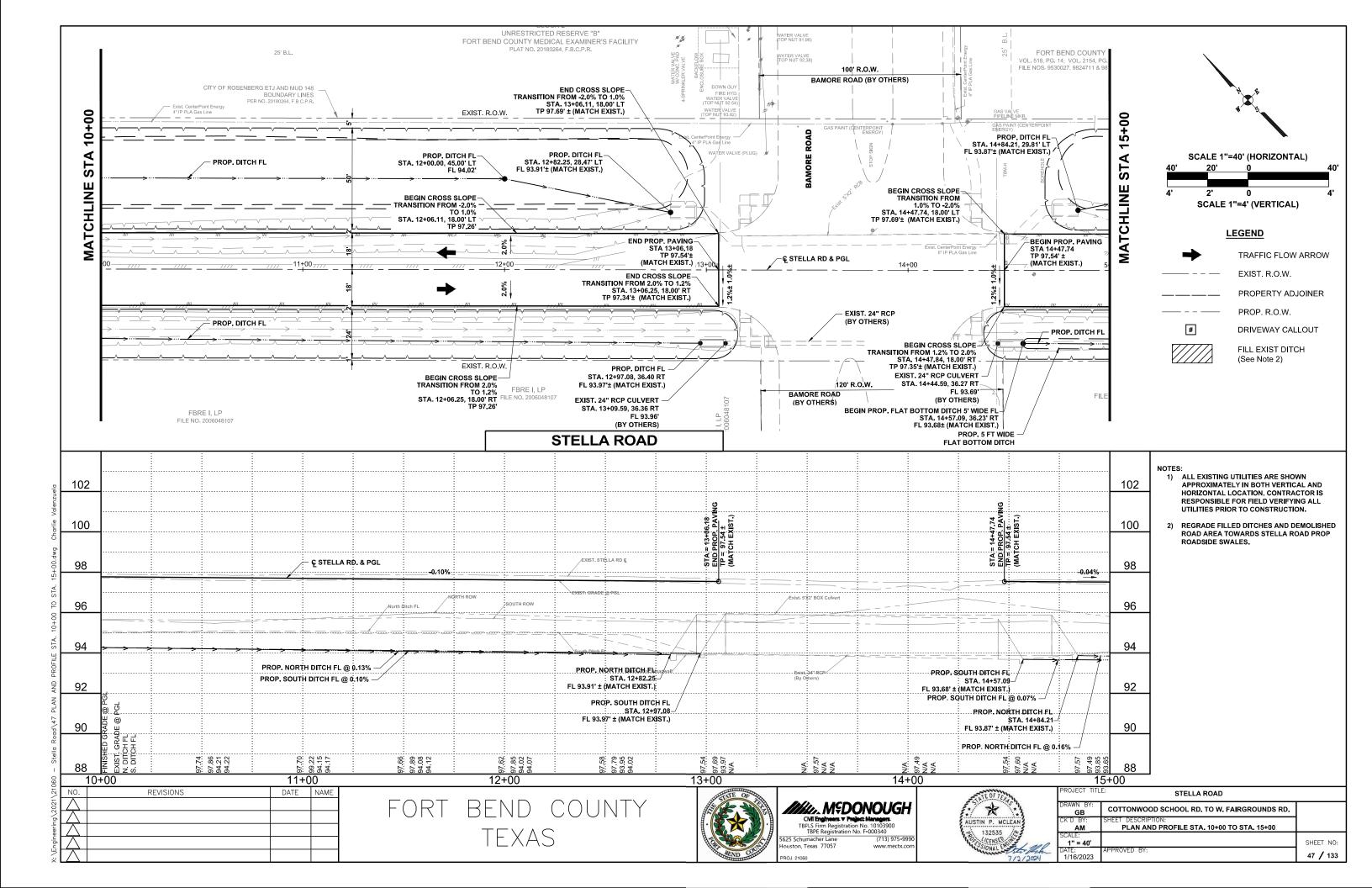


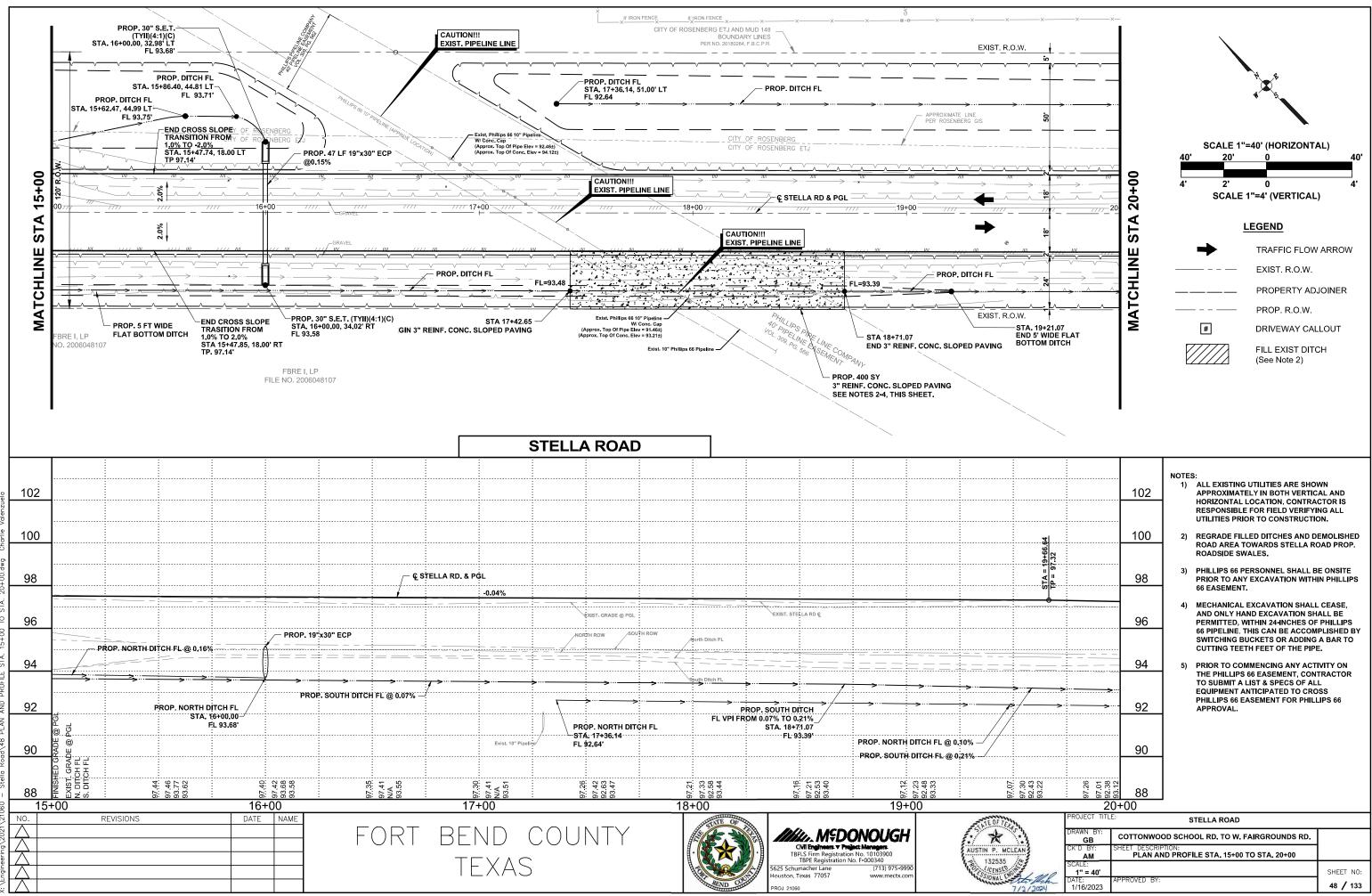


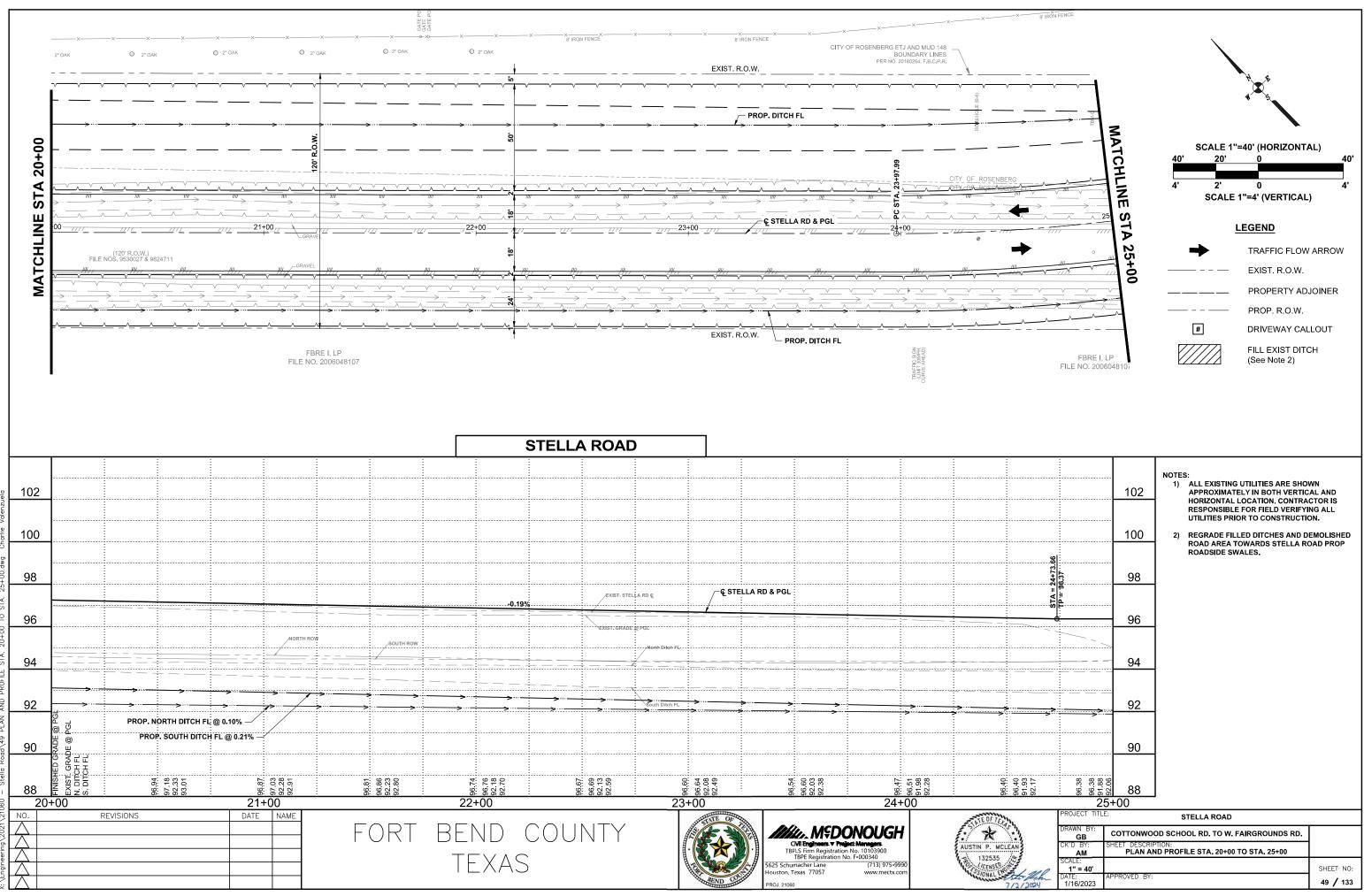
1	STELLA ROAD			
LEAN	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.		
	CK'D BY: AM	SHEET DESCRIPTION: REMOVAL PLAN		
	SCALE: 1" = 50'	STA. 43+00 TO END	SHEET NO:	
7/2/2024	DATE: 1/16/2023	APPROVED BY:	44 / 133	



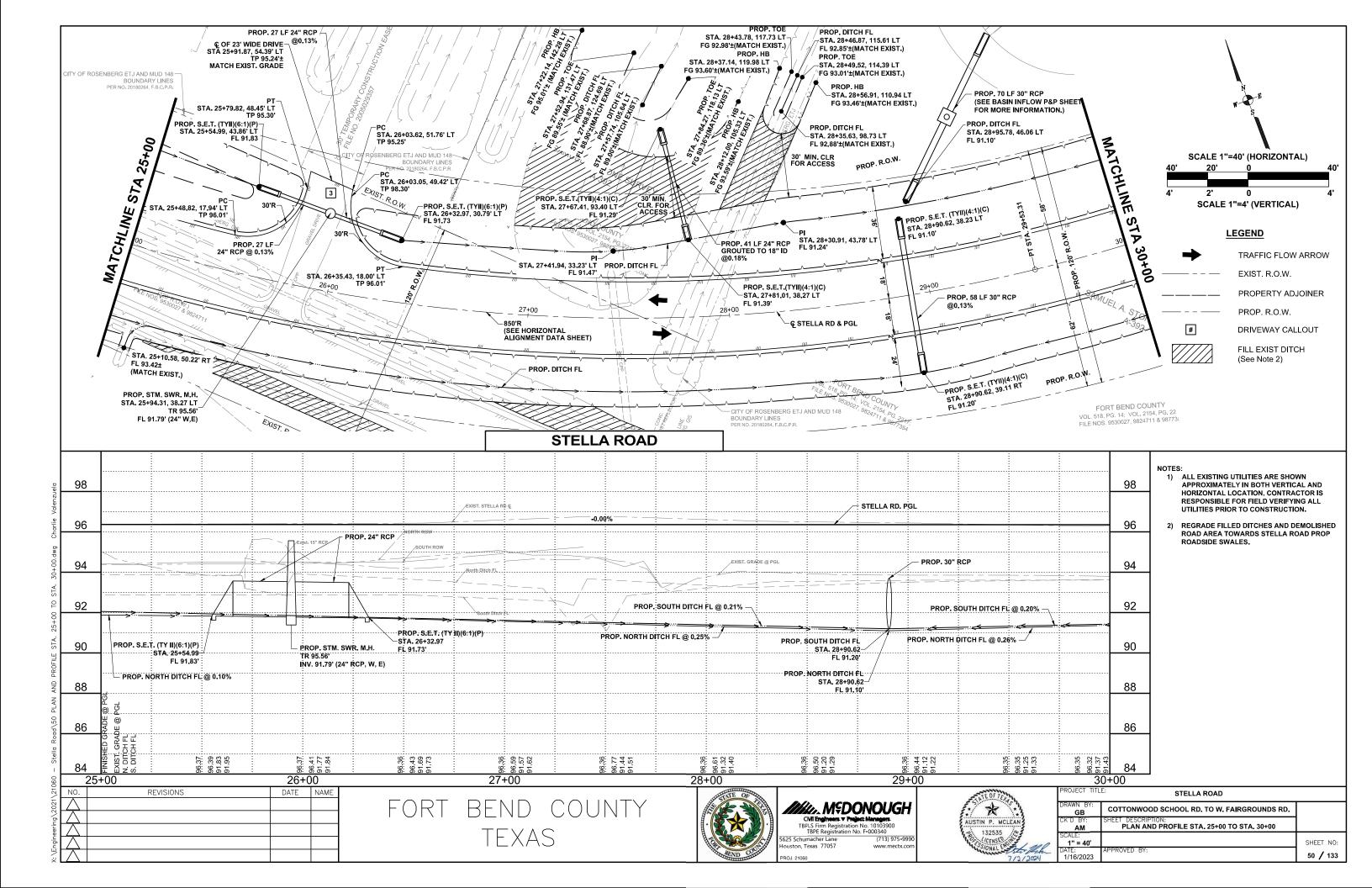


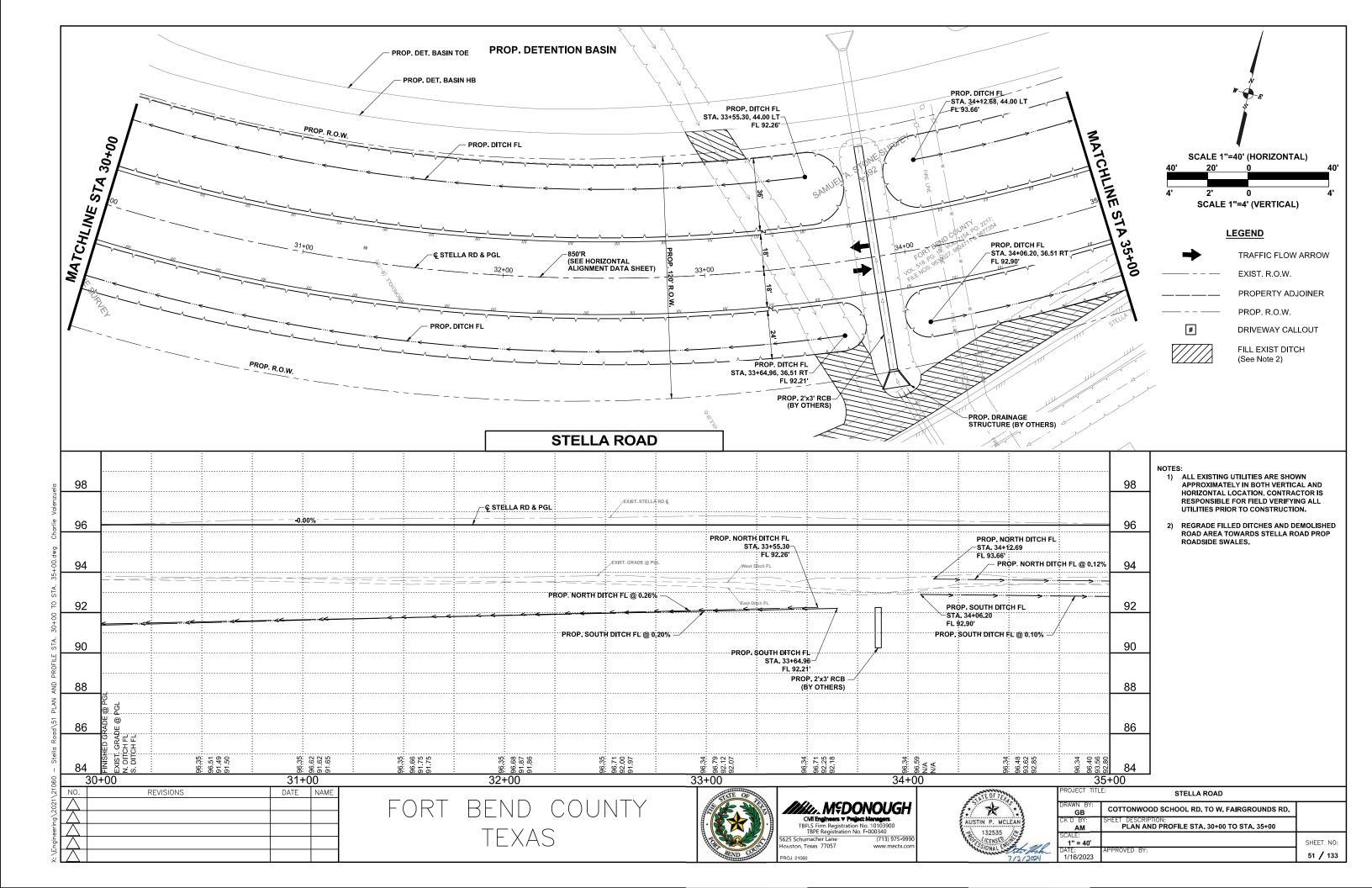


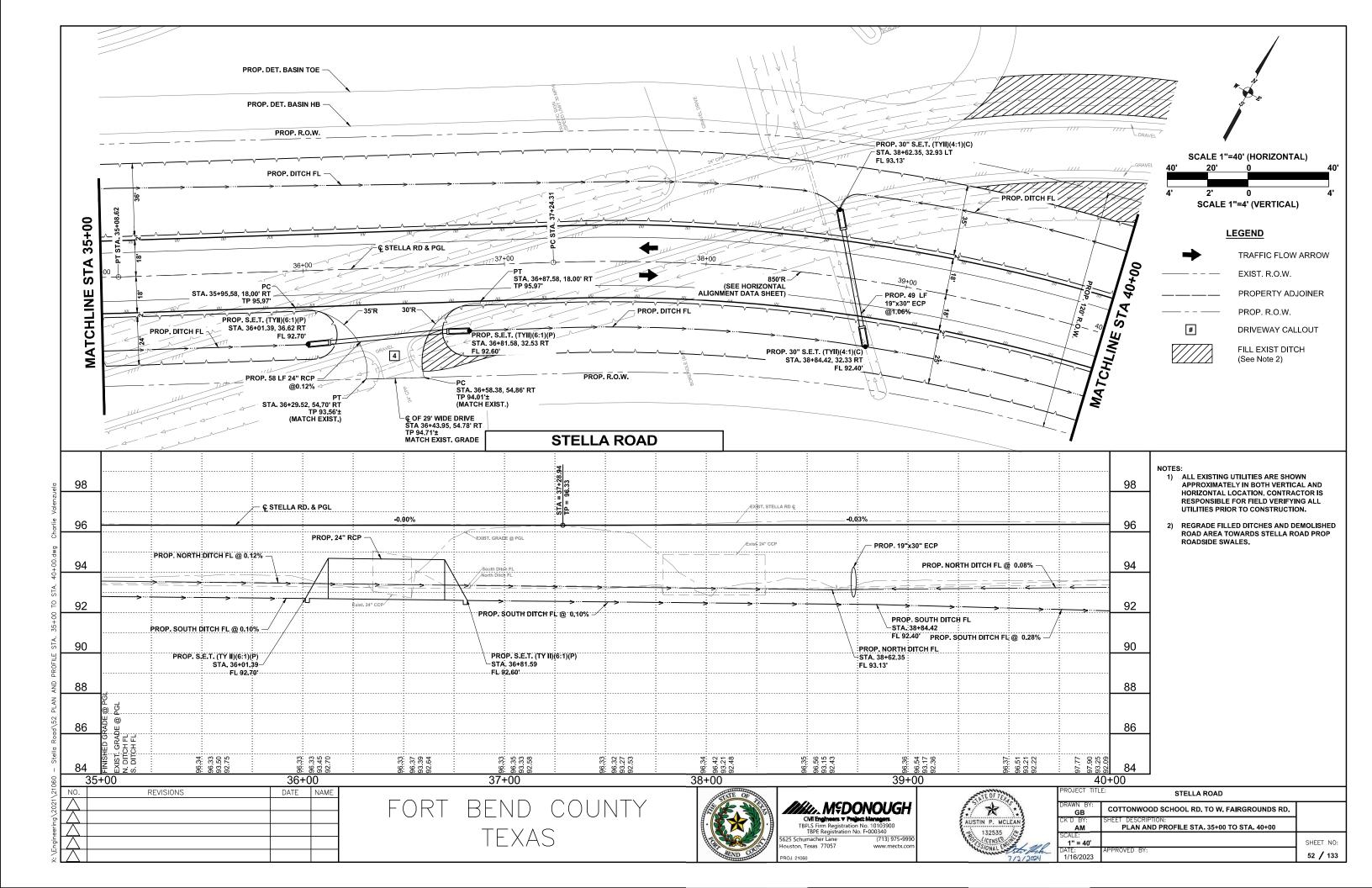


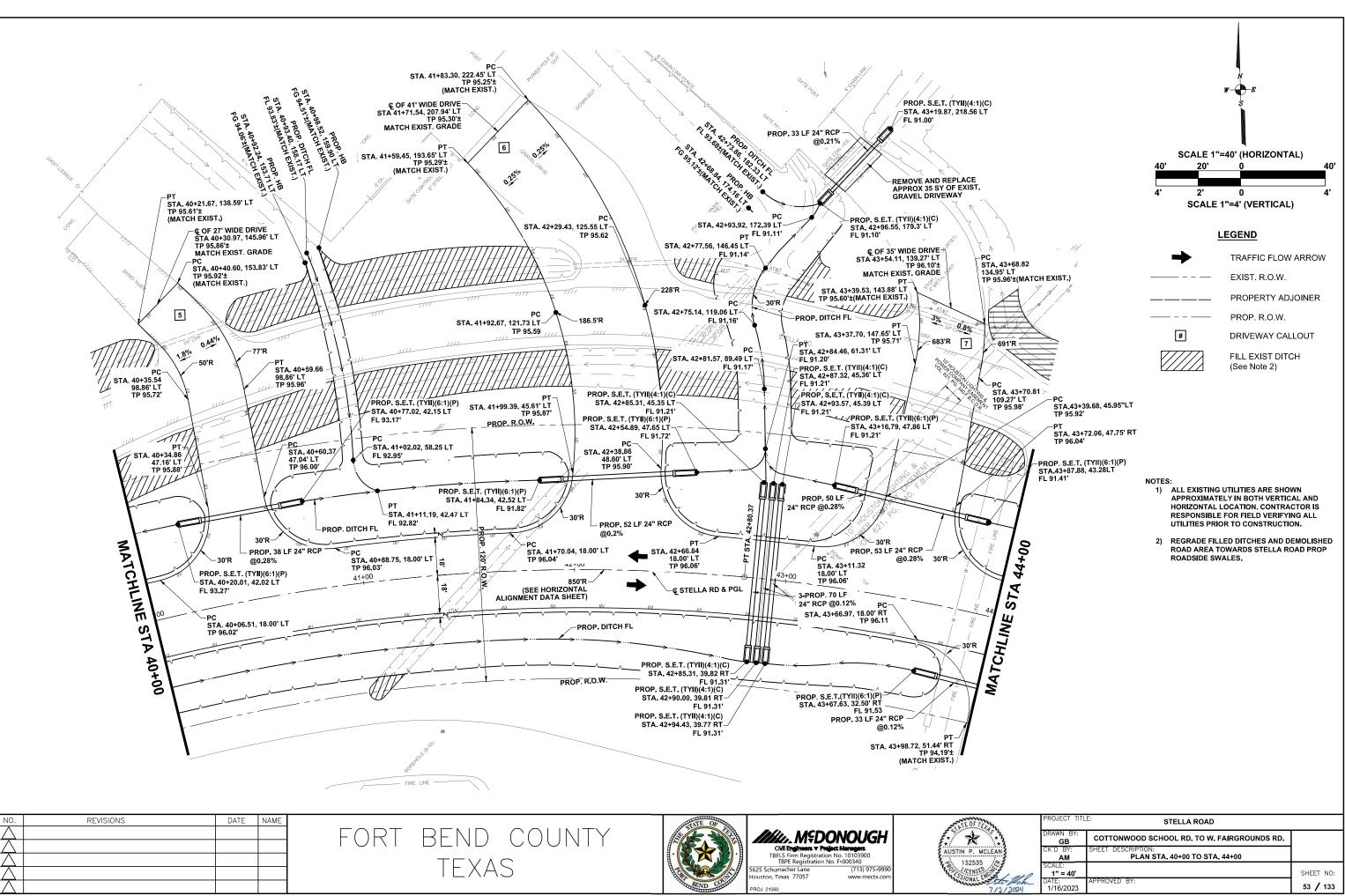




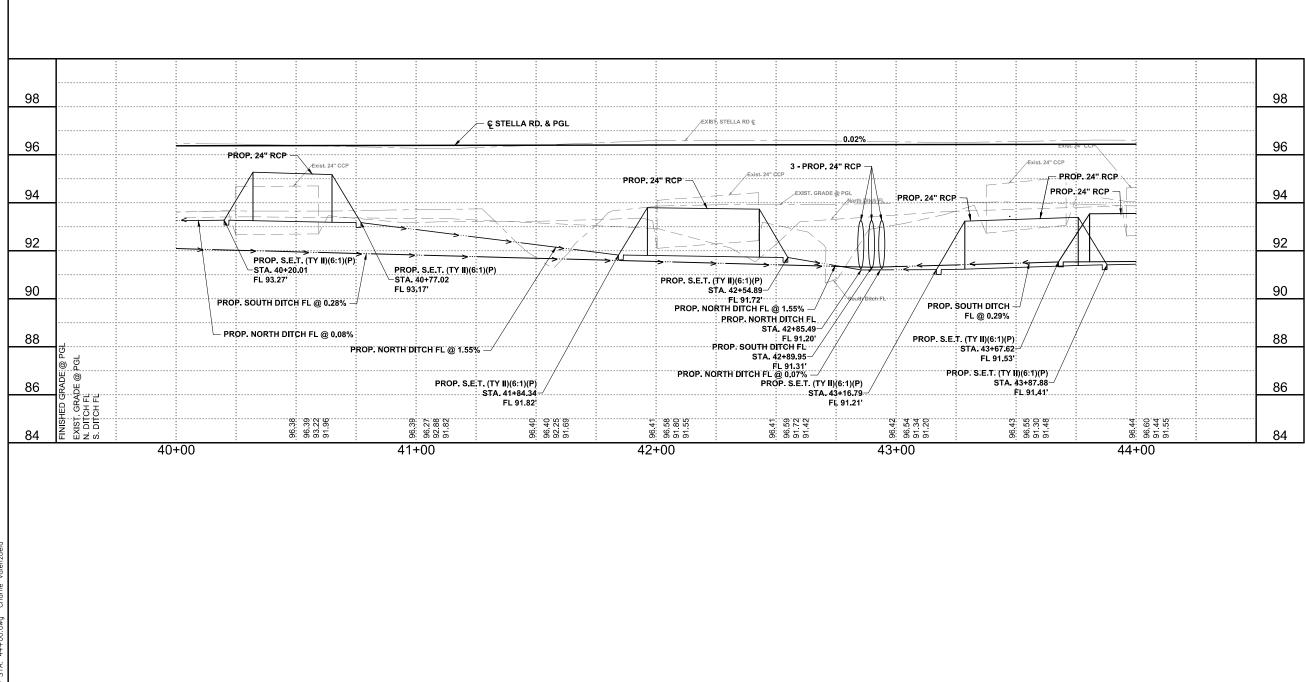








53	1	133
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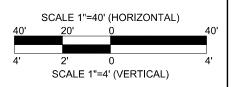
NO.

Image: Contract of the contra	REVISIONS DA	I NAME	TEXAS		TBPLS Firm Registration No. 10103900 TBPE Registration No. F-000340 5625 Schumacher Lane (713) 975-9990 Houston, Texas 77057 www.mectx.com	5
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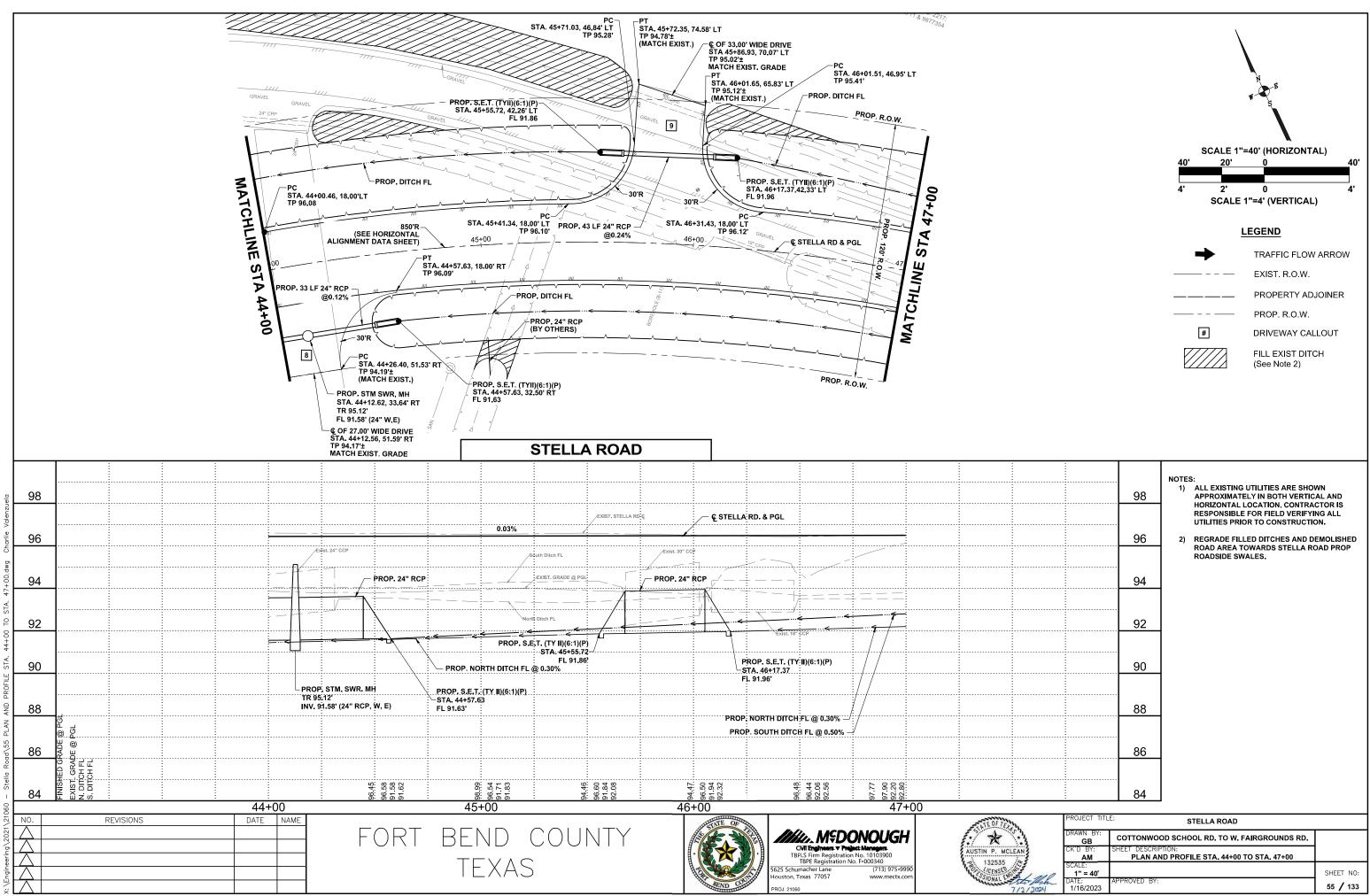


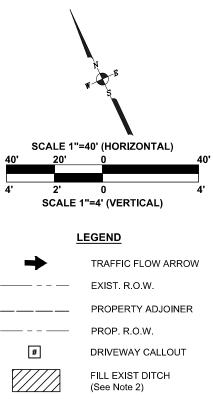
NOTES:

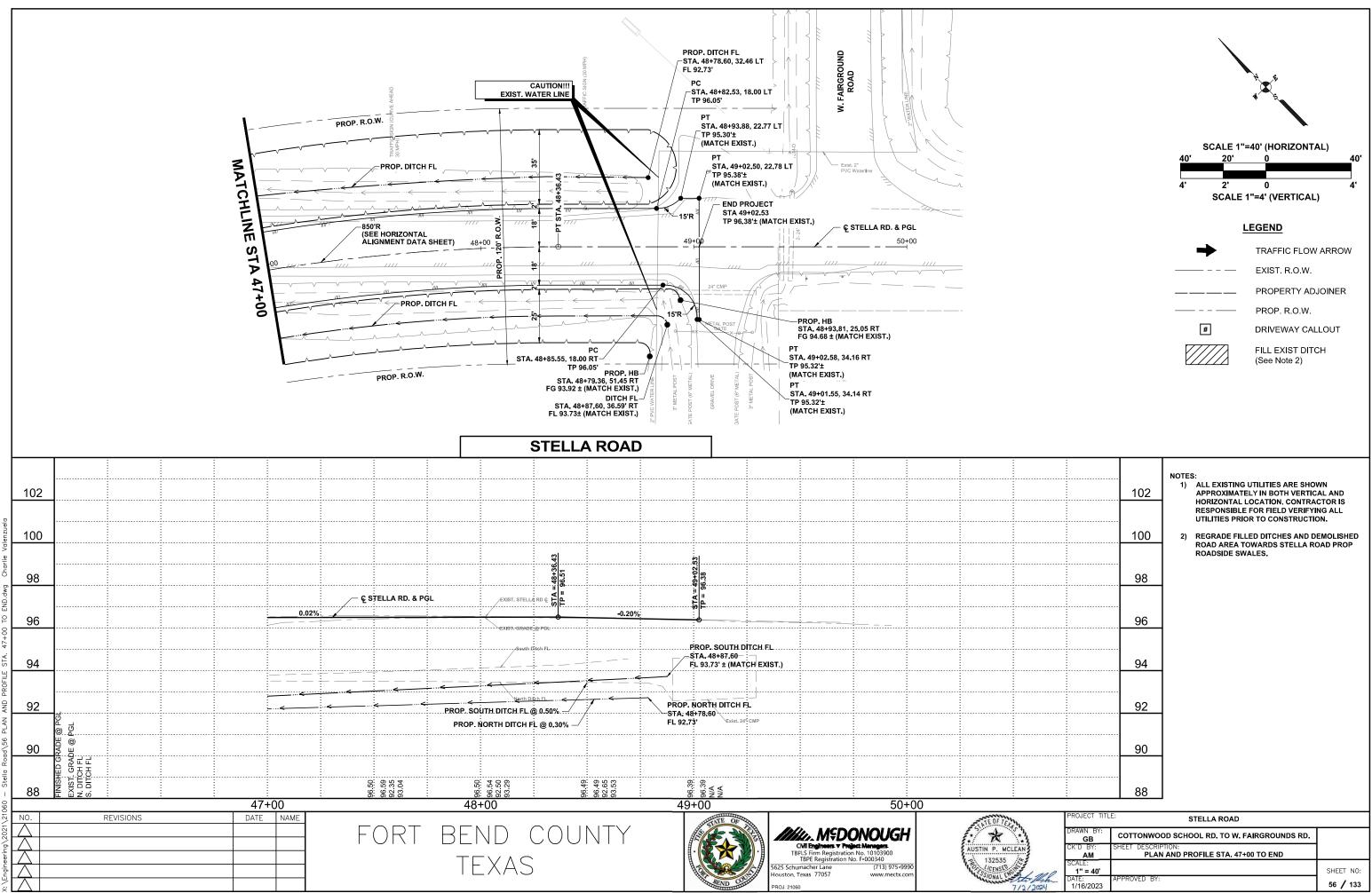
- ALL EXISTING UTILITIES ARE SHOWN APPROXIMATELY IN BOTH VERTICAL AND HORIZONTAL LOCATION, CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL UTILITIES PRIOR TO CONSTRUCTION.
- 2) REGRADE FILLED DITCHES AND DEMOLISHED ROAD AREA TOWARDS STELLA ROAD PROP ROADSIDE SWALES.

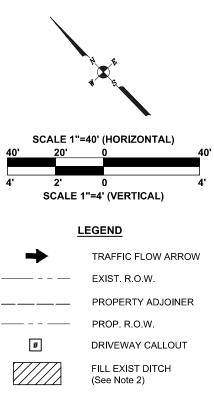


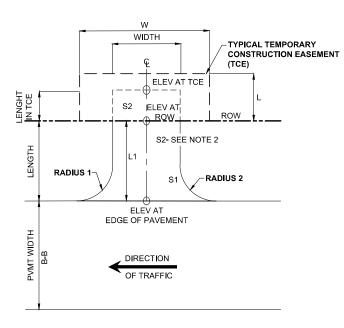
PROJECT 1	ITLE: STELLA ROAD	
DRAWN BY GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CK'D BY: AM	SHEET DESCRIPTION: PROFILE STA. 40+00 TO STA. 44+00	
SCALE: 1" = 40'		SHEE
DATE: 12/9/2022	APPROVED BY:	54











DRIVEWAY DETAIL NTS

- NOTES: 1. CONTRACTOR TO FIELD VERIFY EXIST. PAVING ELEVATION AND NOTIFY ENGINEER IF IT DOES NOT MATCH ELEV. SHOWN IN PLANS.
- 2. GRADE S2 ONLY NEEDED FOR DRIVEWAY NO. 2.

			PROP.	PROP.	PROP.				EXIST.				
DRIVEWAY	SHEET		WIDTH	LENGTH	LENGTH IN	RADIUS 1	RADIUS 2	EL. AT	PAVING EL.	LENGTH	GRADE	TCE	GRADE
NO.	NO	CL STATION	(FT)	(FT)	TCE (FT)	(FT)	(FT)	EOP	@ TIE IN	L1 (FT)	S1(%)	(LxW)	S2(%)
1	45	STA 5+93.22, 37.00' RT	25	19	N/A	10	10	97.73	96.69 +/-	19	5.47	N/A	
2	45	STA 8+86.97, 75.00' LT	25	57	N/A	30	30	97.50	96.31 +/-	57	1.35	N/A	2.88
3	49	STA 25+92.16,54.37' LT	23	36.4	N/A	30	30	96.01	95.24 +/-	36.4	2.12	N/A	
4	51	STA 36+43.95, 54.78' RT	29	36.8	N/A	35	30	95.97	94.71 +/-	36.8	3.42	N/A	
5	52	STA 40+30.97, 145.96' LT	27	132.8	N/A	30	30	96.03	95.86 +/-	132.8	0.13	N/A	
6	52	STA 41+71.54, 207.94' LT	41	204.1	N/A	30	30	96.05	95.30 +/-	204.1	0.37	N/A	
7	52	STA 43+54.11, 139.27' LT	35	121.3	N/A	30	30	96.07	96.10+/-	121.1	-0.02	N/A	
8	54	STA 44+12.56, 51.59' RT	27	33.6	N/A	30	30	96.10	94.17 +/-	33.6	5.74	N/A	
9	54	STA 45+86.93, 70.07' RT	33	52.1	N/A	30	30	96.11	95.02 +/-	52.1	2.09	N/A	
10	44	STA 4+15.70, 35.09' LT	24	21.2	N/A	5	5	98.38	98.02 +/-	21.2	1.70	N/A	
11	44	STA 4+57.57, 28.00' RT	24	10	N/A	5	5	97.96	97.85 +/-	10	1.10	N/A	

FORT BEND COUNTY TEXAS

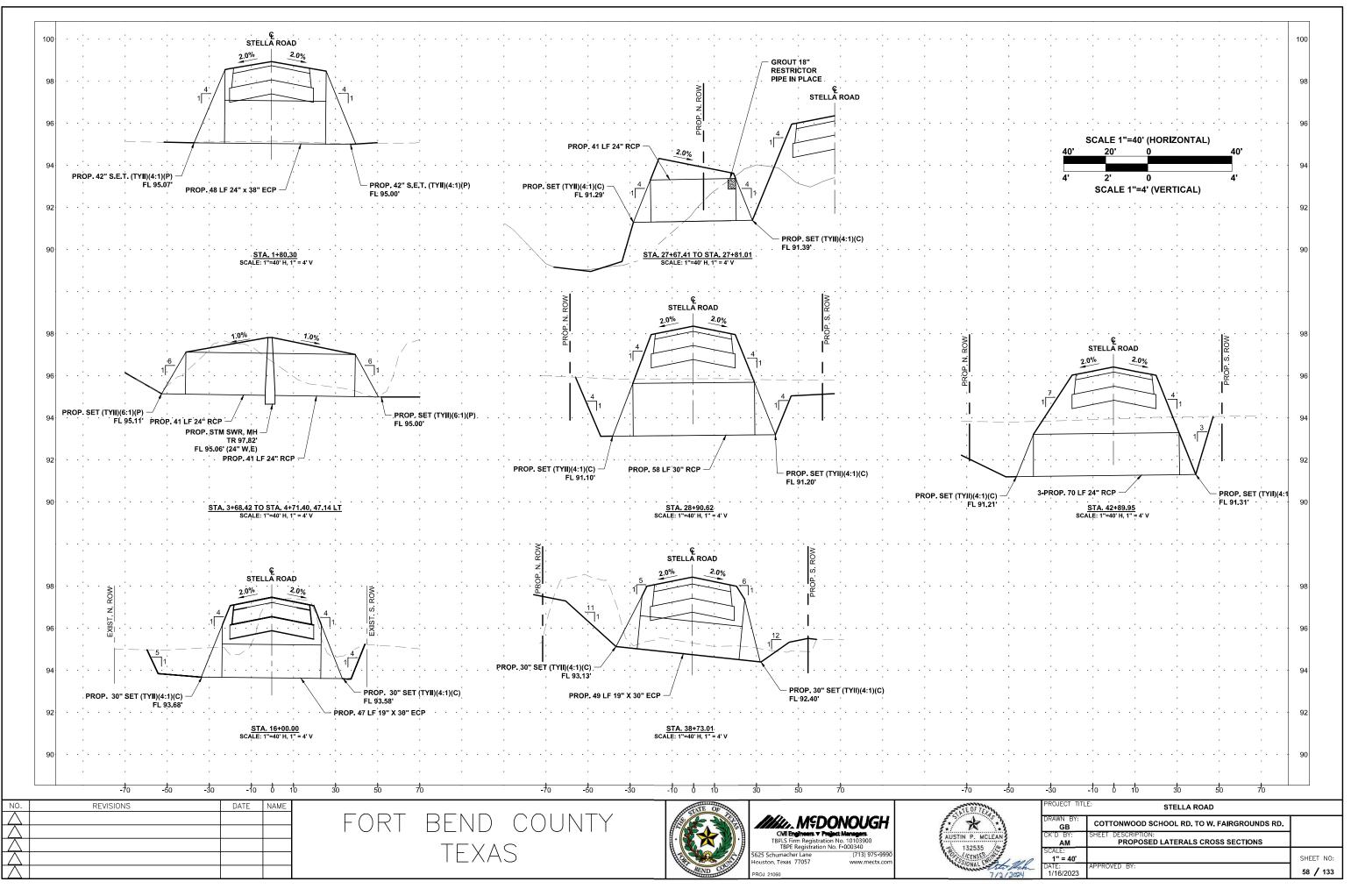
NO.	REVISIONS	DATE	NAME
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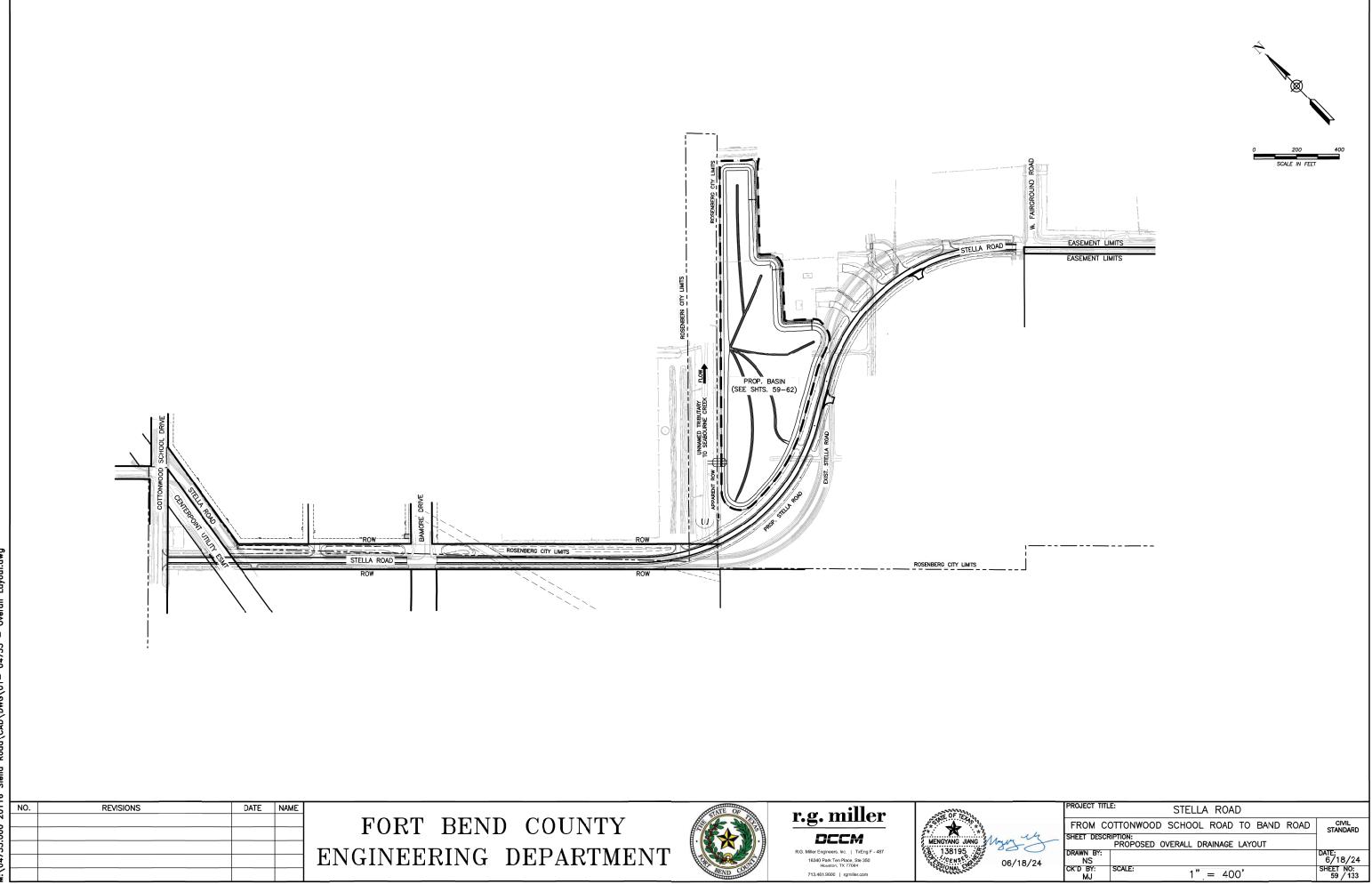




2	PROJECT TITLE: STELLA ROAD				
****	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.			
CLEAN	CK'D BY: AM				
CHOSE AND	SCALE:		SHEET NO:		
7/2/2024	DATE: 1/16/2023	APPROVED BY:	57 / 133		

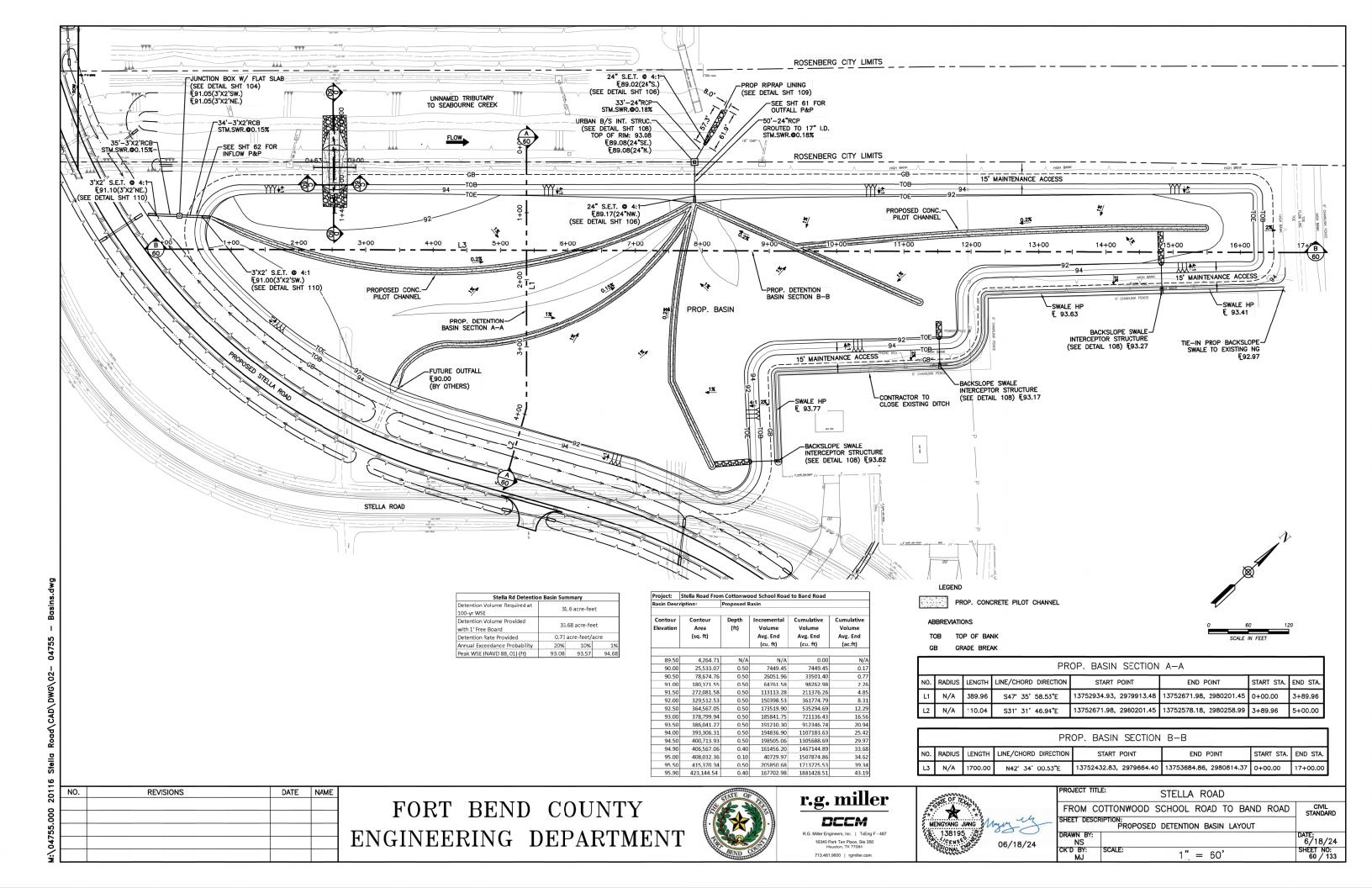


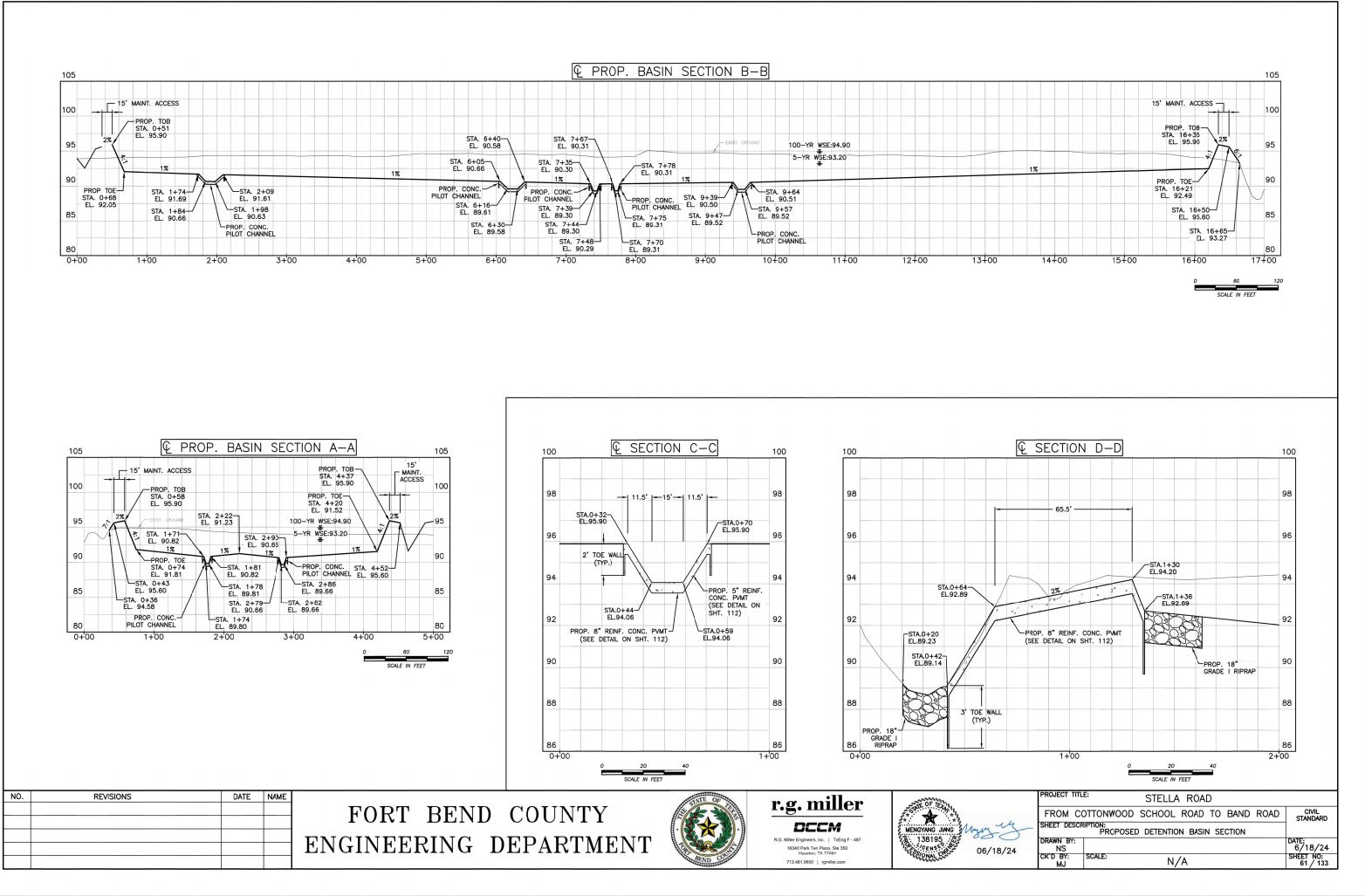
\Engineering\2021\21060 - Stella Road\58 PROPOSED LATERALS CROSS SECTIONS.dwg Charlie Valer

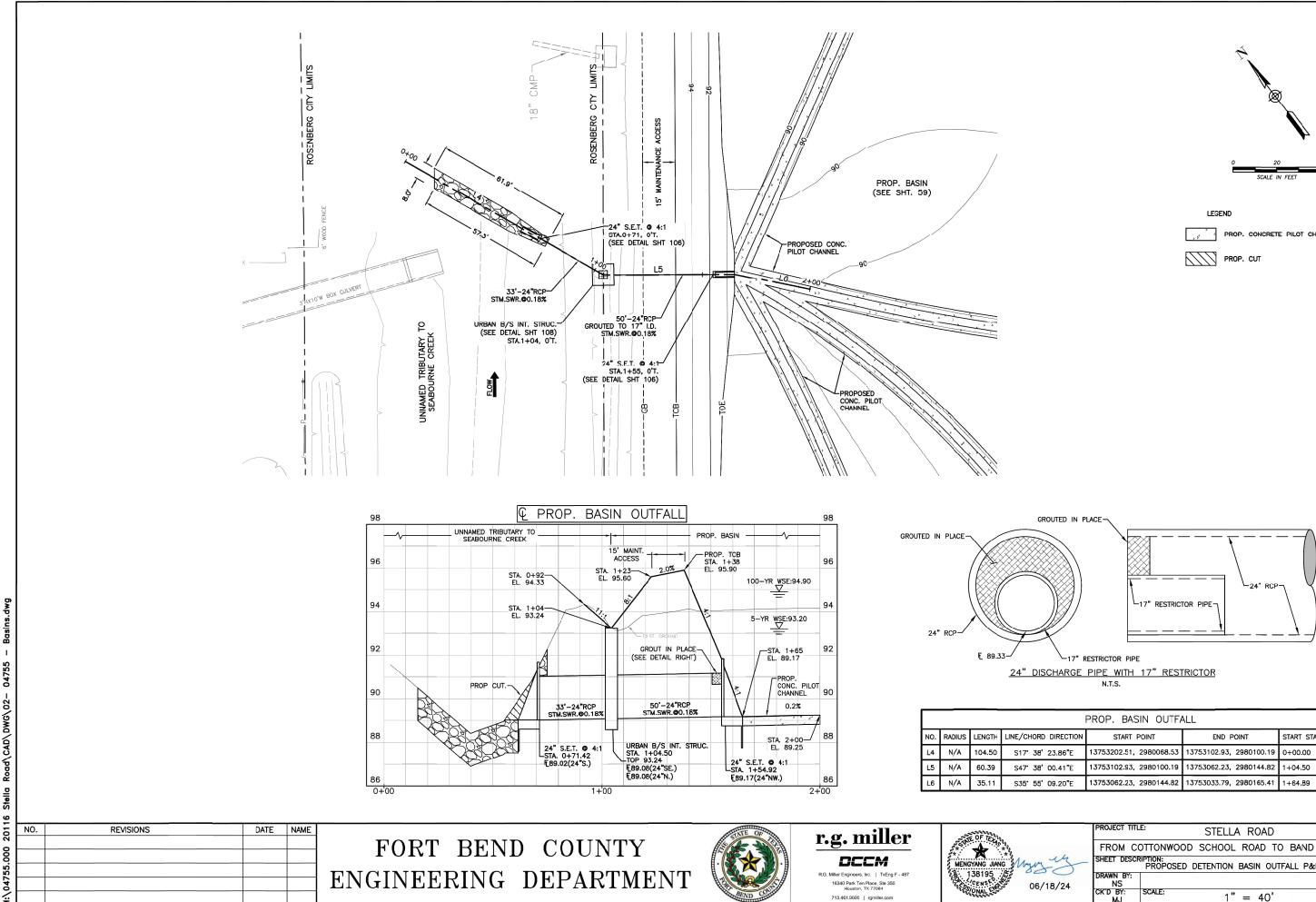


Stella Road/CAD/DWG/01- 04755 - Overall Layout.dwg 20116 M:\04755.000

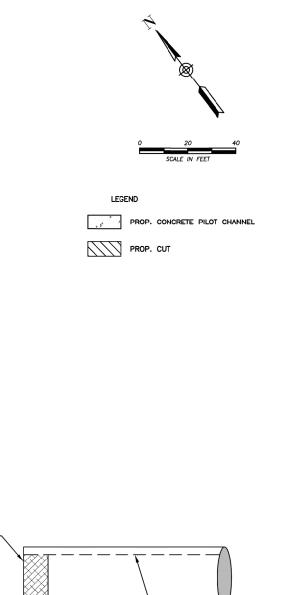
NS CK'D BY: MJ	SCALE:	1" = 400'	





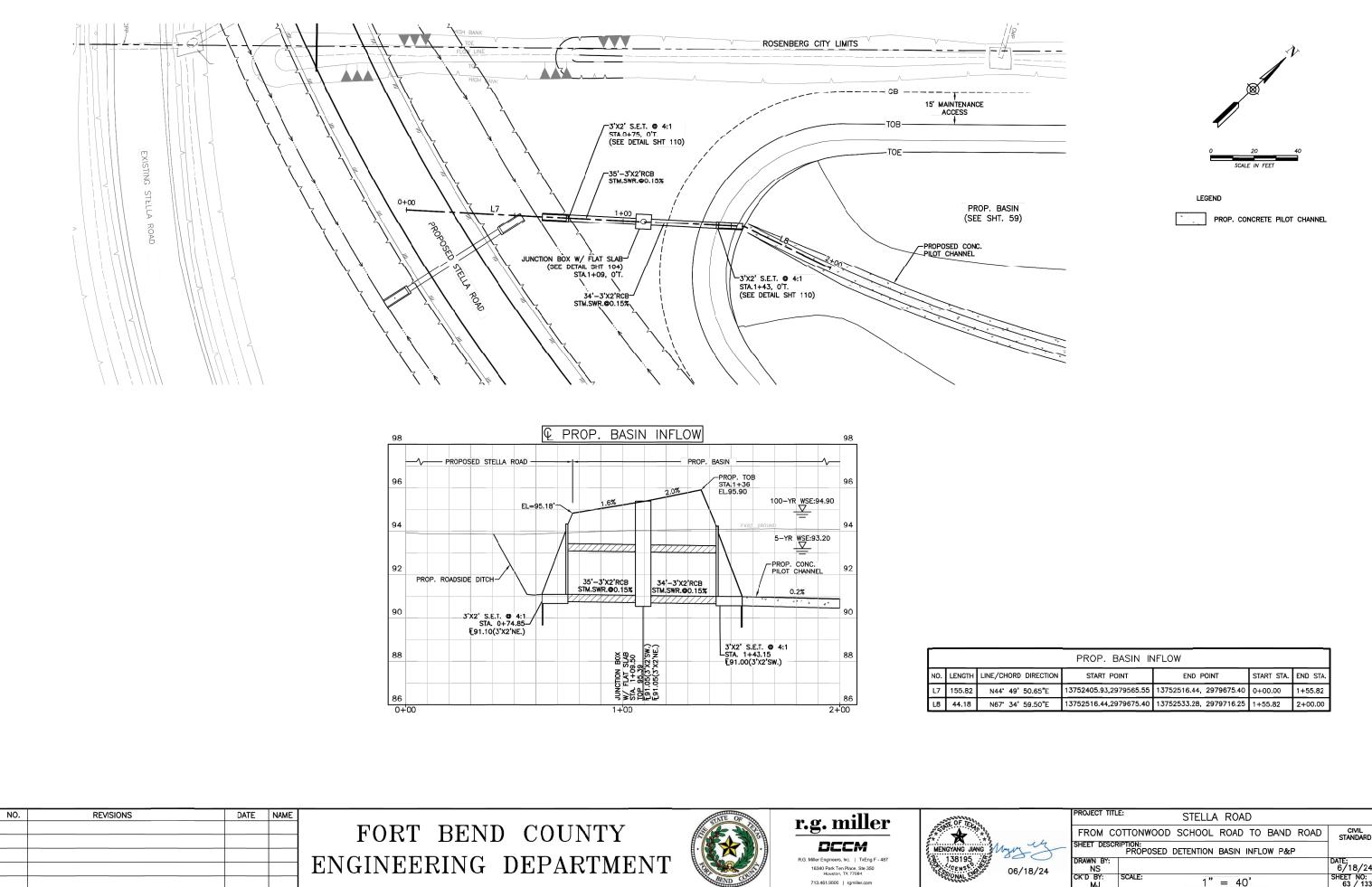


Basins Т Road\CAD\DWG\02- 04755 Stella 20116 800 M:\04755.



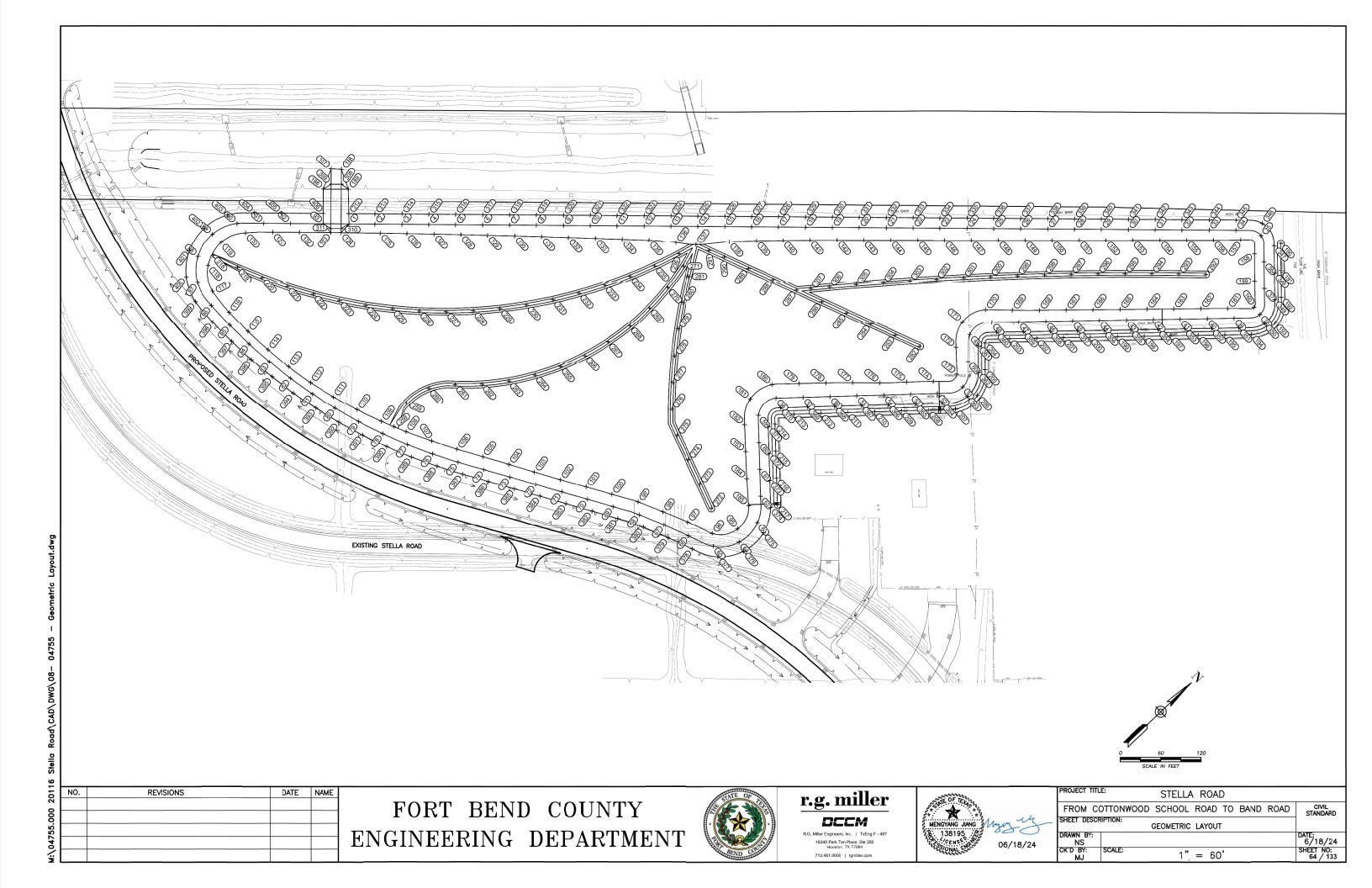
PROP. BASIN OUTFALL							
ORD DIRECTION	START POINT	END POINT	START STA.	END STA.			
38' 23.86"E	13753202.51, 2980068.53	13753102.93, 2980100.19	0+00.00	1+04.50			
58'00.41"E	13753102.93, 2980100.19	13753062.23, 2980144.82	1+04.50	1+64.89			
55' 09.20"E	13753062.23, 2980144.82	13753033.79, 2980165.41	1+64.89	2+00.00			

	PROJECT TITL	^{E:} STELLA ROAD	
	FROM CO	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
yung	SHEET DESCR	RIPTION: PROPOSED DETENTION BASIN OUTFALL P&P	
6/18/24	DRAWN BY: NS		DATE: 6/18/24
	CK'D BY: MJ	scale: 1 " = 40'	SHEET NO: 62 / 133



PROP. BASIN INFLOW							
DIRECTION	START POINT	END POINT	START STA.	END STA.			
9'50.65"E	13752405.93,2979565.55	13752516.44, 2979675.40	0+00.00	1+55.82			
↓'59.50"E	13752516.44,2979675.40	13752533.28, 2979716.25	1+55.82	2+00.00			

	PROJECT TITLE: STELLA ROAD	
	FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
yung	SHEET DESCRIPTION: PROPOSED DETENTION BASIN INFLOW P&P]
6/18/24	DRAWN BY: NS	DATE: 6/18/24
	$\begin{array}{c} \text{CK'D BY:} \\ \text{MJ} \end{array} \begin{array}{c} \text{SCALE:} \\ 1 \\ \end{array} 1 \\ \end{array} = 40'$	SHEET NO: 63 / 133



	1					
				POINT T		
2.		POINT NO.	ELEV.	NORTHING	EASTING	DESC.
E		282	89.90	13753208.97	2980479.72	PILOT CHANNEL
.E		283	89.82	13753192.92	2980443.09	PILOT CHANNEL
E		284	89.74	13753176.87	2980406.45	PILOT CHANNEL
.E		285	89.66	13753160.81	2980369.81	PILOT CHANNEL
.E		286	89.58	13753144.76	2980333.17	PILOT CHANNEL
.E		287	89.50	13753128.19	2980295.36	PILOT CHANNEL
.E		288	89.42	13753112.65	2980259.90	PILOT CHANNEL
E		289	89.34	13753096.60	2980223.26	PILOT CHANNEL
E		290	89.26	13753080.55	2980186.62	PILOT CHANNEL
.E		291	89.21	13753070.78	2980164.33	PILOT CHANNEL
.E		292	90.72	13753596.68	2980683.97	PILOT CHANNEL
.E.		293	90.64	13753567.36	2980656.77	PILOT CHANNEL
.E		293	90.57	13753537.83	2980629.79	PILOT CHANNEL
		295				PILOT CHANNEL
.E			90.50	13753508.08	2980603.04	
.E		296	90.40	13753478.13	2980576.53	PILOT CHANNEL
.E		297	90.34	13753447.98	2980550.25	PILOT CHANNEL
E		298	90.25	13753417.62	2980524.20	PILOT CHANNEL
.E		299	90.17	13753387.05	2980498.40	PILOT CHANNEL
.E		300	90.10	13753356.29	2980472.83	PILOT CHANNEL
.E		301	90.00	13753325.33	2980447.51	PILOT CHANNEL
.E		302	89.93	13753294.17	2980422.43	PILOT CHANNEL
.E		303	89.86	13753262.81	2980397.59	PILOT CHANNEL
ANNEL		304	89.76	13753231.26	2980373.00	PILOT CHANNEL
ANNFI		305	89.70	13753199.52	2980348.66	PILOT CHANNEL
ANNEL		306	89.61	13753167.60	2980324.56	PILOT CHANNEL
ANNEL		307	89.56	13753145.55	2980308.15	PILOT CHANNEL
ANNEL		308	92.80	13752730.12	2979725.55	WEIR
ANNEL		309	92.84	13752718.95	2979715.55	WEIR
ANNEL		310	92.70	13752685.82	2979774.37	WEIR
ANNEL		310	92.70	13752674.71	2979764.31	WEIR
ANNEL		312	95.93	13752708.73	2979765.67	GB
ANNEL		313	95.60	13752737.66	2979792.02	GB
ANNEL		314	95.60	13752767.23	2979818.96	GB
ANNEL		315	95.60	13752796.80	2979845.90	GB
ANNEL		316	95.60	13752826.36	2979872.84	GB
ANNEL		317	95.60	13752855.93	2979899.78	GB
ANNEL		318	95.60	13752885.50	2979926.72	GB
ANNEL		319	95.60	13752915.07	2979953.66	GB
ANNEL		320	95.60	13752944.64	2979980.60	GB
ANNEL		321	95.60	13752974.21	2980007.54	GB
ANNEL		322	95.60	13753003.77	2980034.48	GB
ANNEL		323	95.60	13753033.34	2980061.42	GB
ANNEL		324	95.60	13753062.91	2980088.36	GB
ANNEL		325	95.60	13753092.48	2980115.29	GB
ANNEL		325		13753122.05	2980142.23	GB
			95.60		2980142.23	
		327	95.60	13753151.61 13753181.18		GB
		328	95.60		2980196.11	GB
ANNEL		329	95.60	13753210.75	2980223.05	GB
ANNEL		330	95.60	13753240.32	2980249.99	GB
ANNEL		331	95.60	13753269.89	2980276.93	GB
ANNEL		332	95.60	13753299.46	2980303.87	GB
ANNEL		333	95.60	13753329.02	2980330.81	GB
ANNEL		334	95.60	13753358.59	2980357.75	GB
ANNEL		335	95.60	13753388.16	2980384.69	GB
ANNEL		336	95.60	13753417.73	2980411.63	GB
ANNEL		337	95.60	13753447.30	2980438.56	GB
ANNEL		338	95.60	13753476.87	2980465.50	GB
ANNEL		339	95.60	13753506.43	2980492.44	GB
ANNEL		340	95.60	13753536.00	2980519.38	GB
ANNEL		341	95.60	13753565.57	2980546.32	GB
ANNEL		342		13753595.14	2980548.32	GB
			95.60			
ANNEL		343	95.60	13753624.71	2980600.20	GB
ANNEL		344	95.60	13753654.28	2980627.14	GB
ANNEL		345	95.60	13753683.84	2980654.08	GB
ANNEL		346	95.60	13753709.76	2980683.85	GB

		POINT T	ABLE	
POINT NO.	ELEV.	NORTHING	EASTING	DESC.
196	95.00	13753527.30	2980744.04	SWALE
197	95.00	13753497.07	2980717.85	SWALE
198	95.90	13753466.83	2980691.66	SWALE
199	95.90	13753436.60	2980665.47	SWALE
200	95.90	13753406.36	2980639.28	SWALE
201	95.90	13753376.13	2980613.09	SWALE
202	95.90 95.90	13753345.90 13753315.66	2980586.90 2980560.71	SWALE
203 204	95.90	13753284.36	2980560.71	SWALE
204	95.90	13753257.60	2980570.66	SWALE
206	95.00	13753224.18	2980591.11	SWALE
207	95.00	13753185.64	2980584.34	SWALE
208	95.00	13753154.68	2980559.07	SWALE
209	95.90	13753124.18	2980533.19	SWALE
210	95.90	13753093.67	2980507.32	SWALE
211	95.90	13753063.17	2980481.44	SWALE
212	95.90	13753032.67	2980455.57	SWALE
213	95.90	13753002.16	2980429.70	SWALE
214	95.90	13752971.61	2980421.73	SWALE
215	95.90	13752945.78	2980452.27	SWALE
216	95.90	13752919.95	2980482.81	SWALE
217	95.90	13752894.27	2980513.18	SWALE
218	91.24	13752517.15	2979677.26	PILOT CHANNEL
219	91.15	13752532.38	2979714.24	PILOT CHANNEL
220	91.03	13752549.56	2979750.36	PILOT CHANNEL
221	90.93	13752568.64	2979785.51	PILOT CHANNEL
222	90.80	13752589.55	2979819.60	PILOT CHANNEL
223	90.70	13752612.25	2979852.54	PILOT CHANNEL
224 225	90.59 90.47	13752636.67 13752662.73	2979884.21 2979914.55	PILOT CHANNEL PILOT CHANNEL
225	90.47	13752690.37	2979914.55	PILOT CHANNEL PILOT CHANNEL
220	90.26	13752719.51	2979970.85	PILOT CHANNEL
228	90.15	13752750.06	2979996.66	PILOT CHANNEL
229	90.04	13752781.94	2980020.81	PILOT CHANNEL
230	89.92	13752815.06	2980043.23	PILOT CHANNEL
231	89.82	13752849.33	2980063.86	PILOT CHANNEL
232	89.71	13752884.64	2980082.64	PILOT CHANNEL
233	89.58	13752920.90	2980099.52	PILOT CHANNEL
234	89.49	13752958.01	2980114.44	PILOT CHANNEL
235	89.37	13752995.86	2980127.37	PILOT CHANNEL
236	89.25	13753034.34	2980138.27	PILOT CHANNEL
258	91.83	13752547.44	2980045.85	PILOT CHANNEL
259	91.24	13752584.75	2980032.39	PILOT CHANNEL
260		13752623.99		PILOT CHANNEL
261	90.28	13752656.82	2980060.52	PILOT CHANNEL
262	90.26	13752686.93	2980086.84	PILOT CHANNEL
263	90.23	13752719.58	2980109.90	PILOT CHANNEL
264	90.19	13752754.45	2980129.47 2980145.34	PILOT CHANNEL
265 266	90.13 90.06	13752791.15 13752829.29	2980145.34	PILOT CHANNEL PILOT CHANNEL
266	89.98	13752868.47	2980157.33	PILOT CHANNEL
267	89.90	13752908.26	2980169.21	PILOT CHANNEL
269	89.80	13752948.24	2980168.97	PILOT CHANNEL
270	89.69	13752987.99	2980164.61	PILOT CHANNEL
271	89.53	13753027.07	2980156.17	PILOT CHANNEL
272	90.04	13752817.23	2980451.54	PILOT CHANNEL
273	89.96	13752828.90	2980413.28	PILOT CHANNEL
274	89.88	13752840.56	2980375.02	PILOT CHANNEL
275	89.80	13752852.22	2980336.76	PILOT CHANNEL
276	90.72	13752871.69	2980302.87	PILOT CHANNEL
277	90.79	13752899.39	2980274.02	PILOT CHANNEL
278	90.71	13752928.39	2980246.48	PILOT CHANNEL
	90.56	13752958.63	2980220.30	PILOT CHANNEL
279	30.50	10/02000.00		
279	90.36	13752990.04	2980195.53	PILOT CHANNEL

POINT NO.	ELEV.	POINT TABLE	EASTING	DESC.
131	ELEV. 91.70	13752899.45	2979982.46	TOE
132	91.70	13752899.45	2979982.46	TOE
133	91.21	13752957.27	2980037.75	TOE
134	90.95	13752986.11	2980065.46	TOE
135	90.67	13753014.91	2980093.22	TOE
136	89.79	13753041.98	2980122.63	TOE
137	89.17	13753062.23	2980144.82	TOE
138	90.29	13753102.17	2980174.77	TOE
139	90.70	13753132.80	2980200.45	TOE
140	90.95	13753163.02	2980226.66	TOE
141	91.10	13753193.01	2980253.14	TOE
142	91.20	13753222.84	2980279.78	TOE
143	91.25	13753252.55	2980306.56	TOE
144	91.31	13753282.27	2980333.34	TOE
145	91.37	13753311.99	2980360.10	TOE
146	91.43	13753341.73	2980386.86	TOE
147	91.49	13753371.47	2980413.61	TOE
148	91.56	13753401.22	2980440.35	TOE
149	91.63	13753430.97	2980467.08	TOE
150	91.70	13753460.73	2980493.81	TOE
150	91.78	13753490.50	2980520.52	TOE
152	91.86	13753520.28	2980520.32	TOE
152	91.86	13753550.07	2980547.25	TOE
153	91.94	13753550.07	2980573.93	TOF
155	92.11	13753609.67	2980627.30	TOF
		13753639.45	2980654.00	TOE
156	92.19			
157	92.40	13753669.57	2980680.32	TOE TOE
158	92.46	13753671.23	2980710.13	
159	92.40	13753644.63	2980740.00	TOE
160	92.54	13753617.51	2980768.76	TOE
161	92.27	13753587.09	2980743.55	TOE TOE
162	92.17	13753557.14	2980717.05	
163	92.09	13753527.10	2980690.63	TOE
164	92.01	13753497.07	2980664.21	TOE
165	91.94	13753467.04	2980637.79	TOE
166	91.85	13753437.03	2980611.34	TOE
167	91.77	13753407.02	2980584.89	TOE
168	91.68	13753377.02	2980558.44	TOE
169	91.44	13753347.41	2980531.55	TOE
170	91.13	13753317.91	2980504.53	TOE
1 71	91.02	13753280.29	2980494.99	TOE
172	91.27	13753247.78	2980516.52	TOE
173	91.62	13753221.95	2980547.05	TOE
174	91.44	13753189.11	2980532.11	TOE
175	91.42	13753158.65		TOE
176	91.39	13753128.25	2980480.20	TOE
177	91.07	13753098.57	2980453.38	TOE
178	90.84	13753068.67	2980426.81	TOE
179	90.80	13753038.28	2980400.81	TOE
180	90.77	13753007.50	2980375.30	TOE
181	90.78	13752969.02	2980369.31	TOE
182	90.85	13752937.32	2980392.29	TOE
183	90.93	13752911.73	2980423.03	TOE
184	91.06	13752886.30	2980453.90	TOE
185	91.42	13752861.60	2980485.36	TOE
186	95.82	13752705.50	2979713.25	WEIR
187	92.88	13752734.06	2979698.84	WEIR
188	92.82	13752744.72	2979709.42	WEIR
189	95.93	13752732.78	2979737.78	WEIR
190	95.00	13753710.26	2980724.48	SWAL
191	95.00	13753683.82	2980754.49	SWAL
192	95.00	13753657.38	2980784.51	SWAL
193	95.00	13753625.13	2980806.50	SWAL
194	95.00	13753587.77	2980796.42	SWAL
134				

FORT BEND COUNTY

ENGINEERING DEPARTMENT

POINT NO.	ELEV.	NORTHING	EASTING	DESC
66	95.88	13752764.97	2980459.11	тов
67	95.8 9	13752748.81	2980422.53	тов
68	95.88	13752731.21	2980386.61	тов
69	95.90	13752710.77	2980352.26	тов
70	95.90	13752689.50	2980318.38	тов
71	95.90	13752668.23	2980284.50	тов
72	95.90	13752646.96	2980250.63	тов
73	95.90	13752625.70	2980216.75	TOB
74	95.90	13752604.43	2980182.87	TOB
75	95.90	13752583.17	2980148.99	TOB
76	95.90	13752563.00	2980114.45	TOB
77	95.90	13752544.62	2980078.93	TOB
78	95.90 95.90	13752528.06 13752513.38	2980042.52 2980005.32	TOB TOB
80	95.90	13752500.61	2979967.41	тов
81	95.90	13752489.79	2979928.91	тов
82	95.90	13752480.94	2979889.91	тов
83	95.90	13752474.08	2979850.50	тов
84	95.90	13752469.24	2979810.80	тов
85	95.90	13752466.42	2979770.91	тов
86	95.90	13752465.63	2979730.92	тов
87	95.90	13752472.97	2979692.03	тов
88	95.90	13752499.71	2979663.02	тов
89	95.90	13752537.89	2979653.07	TOR
90	95.90	13752575.39	2979665.36	тов
91	95.90	13752605.37	2979691.79	тов
92	95.90	13752634.94	2979718.73	тов
93	95.90	13752670.85	2979751.45	WEIR
94	91. 4 2	13752861.51	2980485.47	TOE
95	91.47	13752826.19	2980500.46	TOE
96	91.36	13752793.60	2980479.96	TOE
97	91.39	13752777.81	2980443.23	TOE
98	91.59	13752760.59	2980407.13	TOE
99	91.59	13752742.70	2980371.36	TOE
100	91.62 91.65	13752721.64 13752700.25	2980337.37 2980303.57	TOE
102	91.69	13752678.86	2980269.77	TOE
103	91.59	13752657.93	2980235.68	TOE
104	91. 4 7	13752637.07	2980201.55	TOE
105	91.39	13752616.08	2980167.50	TOE
106	91.36	13752595.00	2980133.50	TOE
107	91.36	13752575.16	2980098.78	TOE
108	91.67	13752556.05	2980063.64	TOE
109	92.00	13752538.77	2980027.57	TOE
110	92.30	13752523.42	2979990.64	TOE
111	92.59	13752510.11	2979952.93	TOE
112	92.85	13752498.84	2979914.55	TOE
113	92.90	13752490.40	2979875.47	TOE
114	92.84	13752484.42	2979835.92	TOE
115	92.77	13752480.54	2979796.12	TOE
116	92.69	13752478.72	2979756.16	TOE
117	92.16	13752481.10	2979716.30	TOE
118	91.41	13752502.10	2979683.31 2979670.49	TOE
	91.57	13752539.33		TOE
120	92.33 92.68	13752575.96 13752606.44	2979684.32 2979710.18	TOE TOE
121	92.00	13752636.08	2979737.05	TOE
122	92.69	13752670.75	2979768.68	WEIR
123	92.69	13752681.80	2979778.80	WEIR
125	92.60	13752724.51	2979818.16	TOE
126	92.52	13752753.83	2979845.36	TOE
127	92.40	13752783.09	2979872.64	TOE
128	92.26	13752812.28	2979899.99	TOE
		-		-
129	92.10	13752841.41	2979927.40	TOE

DATE NAME

DOINT NO.ELEV.NORTHINGEASTING195.901375298.93297977.03295.901375275.13297983.05495.9013752875.40297983.05595.9013752875.402979987.10695.9013752875.402979987.10795.9013752875.402979981.61995.9013752875.402979981.61995.901375293.672980045.771295.901375302.242980045.711195.901375302.24298018.301195.901375302.24298018.311195.901375302.242980281.011195.901375311.19298018.021195.901375320.252980241.011195.901375320.252980241.011195.901375320.252980241.011295.901375320.252980241.011395.901375328.93298034.101495.901375348.49298034.101595.901375348.49298034.101295.901375348.49298034.101395.901375348.49298034.101495.901375348.49298034.101595.901375348.49298034.101495.901375348.49298054.131595.901375348.49298054.141695.901375346.472980545.141795.9013753				POINT TABLE	
295.901375275.03297983.05395.901375275.13297983.03695.9013752816.26297983.93695.9013752815.23297991.87795.901375293.45.3297991.63995.901375293.45.3297991.631095.901375293.672980045.571295.901375303.24298007.201395.901375302.342980045.571295.901375302.34298007.201395.901375302.34298007.201495.901375311.942980163.321595.901375317.08298023.141995.901375317.08298023.141995.901375329.352980314.962295.901375318.22298034.902395.901375347.63298042.712595.901375347.63298042.712595.901375347.63298042.712595.901375347.63298045.742695.901375347.63298045.742795.901375346.74298053.472895.901375355.47298053.472895.901375364.74298053.172595.901375364.74298055.742895.901375364.74298053.173695.901375364.74298058.133795.901375364.74298078.103795.901375364.74 <td>P0</td> <td>INT NO.</td> <td>ELEV.</td> <td>NORTHING</td> <td>EASTING</td>	P0	INT NO.	ELEV.	NORTHING	EASTING
395.901375275.132979830.05495.9013752816.262979883.93595.9013752815.26297983.73795.9013752875.402979964.75995.9013752934.532979916.871095.901375293.672980045.571195.901375302.242980074.101195.901375302.342980045.571295.901375302.342980074.101305.001375311.942980173.201495.901375311.942980180.261795.9013753171.08298027.201895.9013753171.08298027.201895.9013753171.08298027.201895.901375318.222980314.962295.901375318.222980341.902395.901375347.63298042.712595.901375347.63298042.712695.901375347.632980453.012795.901375346.74298057.412895.901375346.75298057.412995.901375355.47298057.413195.901375364.74298057.113195.901375364.74298057.123295.901375364.74298057.123395.901375364.74298057.123495.901375364.74298057.133595.901375364.72298078.133695.9013		1	95.90	13752698.93	2979777.03
4 95.90 13752786.68 297985.99 5 95.90 13752816.26 297983.93 6 95.90 13752815.23 2979910.87 7 95.90 13752815.23 2979916.475 9 95.90 1375293.67 2980045.57 12 95.90 1375293.67 2980045.57 12 95.90 1375302.24 2980072.50 13 05.00 13753082.38 298016.33 15 95.90 1375311.94 298016.38 15 95.90 1375320.65 298024.14 19 95.90 1375320.22 2980261.08 20 95.90 1375328.35 298014.96 21 95.90 1375348.49 298038.84 24 95.90 1375348.49 298038.84 24 95.90 1375348.49 298038.84 24 95.90 1375346.76 298042.71 25 95.90 1375346.76 298078.13 23 95.90 </td <td></td> <td></td> <td>95.90</td> <td></td> <td></td>			95.90		
595.9013752816.26297983.93695.9013752875.40297991.87795.901375293.672979964.75995.901375293.672980045.571295.901375293.672980045.571295.901375302.242980072.501305.001375302.842980126.381595.9013753082.382980126.381595.901375311.942980126.321695.901375311.08298023.141995.901375320.652980234.141995.901375320.522980261.082095.901375329.35298014.962195.901375329.352980314.962295.901375348.49298038.842495.901375347.002980422.712595.901375347.202980449.652795.901375347.202980449.652795.901375346.76298073.172895.901375365.47298053.473095.901375365.47298053.143195.901375367.74298068.123395.901375367.74298078.233495.901375367.72298078.233595.901375367.72298078.233695.901375367.72298078.233795.901375367.72298078.233895.901375367.72298078.233995.9013753					
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795.9013752875.40297993.781895.901375293.453297991.69995.901375293.4532970991.631095.901375293.672980045.571295.901375302.322980072.501305.001375302.322980126.381595.901375311.942980153.321695.901375311.942980126.381795.901375311.082980207.201895.901375320.652980231.411995.901375320.222980281.022195.9013753289.352980341.902295.901375334.492980388.422495.901375346.762980422.712595.901375346.762980435.772595.901375346.762980449.652795.901375346.762980449.652895.901375355.47298053.473095.901375364.72298053.473195.901375364.72298053.473395.901375364.72298053.1293495.901375364.72298053.1293595.901375364.722980782.1293395.901375364.722980782.133495.901375364.722980782.1293595.901375364.722980782.133695.901375364.722980782.133795.901375364.722980782.133695.9		-			
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42 95.90 13753493.76 2980681.91 43 95.90 13753463.52 2980655.72 44 95.90 1375343.3.29 2980655.72 44 95.90 1375343.3.29 2980629.53 45 95.90 1375343.22 298057.15 47 95.90 13753342.59 2980550.96 48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 1375314.75 298054.45 51 95.90 1375316.05 298054.70 52 95.90 1375316.05 298054.71 54 95.90 1375316.02 298054.33 54 95.90 1375309.19 2980471.83 55 95.90 1375309.19 2980471.58 56 95.90 1375309.19 2980445.71 57 95.90 13752091.63 2980395.52 60 95.90 13752091.53 2980395.52 60		40	95.90	13753554.23	2980734.29
43 95.90 13753463.52 2980655.72 44 95.90 13753433.29 2980629.53 45 95.90 1375343.329 2980629.53 46 95.90 13753372.82 2980507.15 47 95.90 13753312.36 298050.96 48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 1375312.36 298054.45 51 95.90 1375314.75 298054.45 51 95.90 1375316.05 298054.33 54 95.90 1375311.70 2980547.16 55 95.90 1375309.19 298047.16 55 95.90 1375309.19 298047.16 57 95.90 1375309.19 298047.13 58 95.89 1375298.62 298039.63 59 95.90 1375298.62 298039.52 60 95.90 1375298.62 2980395.52 60 <td< td=""><td></td><td>41</td><td>95.90</td><td>13753523.99</td><td>2980708.10</td></td<>		41	95.90	13753523.99	2980708.10
44 95.90 13753433.29 2980629.53 45 95.90 13753403.06 2980603.34 46 95.90 13753372.82 2980577.15 47 95.90 13753312.36 2980550.96 48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 1375316.05 2980544.45 51 95.90 1375316.05 2980547.01 52 95.90 13753181.71 2980542.33 54 95.90 1375310.70 2980497.46 55 95.90 1375309.19 298047.15 57 95.90 1375309.19 298047.15 57 95.90 1375309.18 298045.11 57 95.90 1375298.62 2980395.52 60 95.90 1375298.62 298035.52 60 95.90 1375299.60 298045.34 61 95.90 1375299.62 298035.52 60		42	95.90	13753493.76	2980681.91
45 95.90 13753403.06 2980603.34 46 95.90 13753372.82 2980577.15 47 95.90 13753342.59 2980550.96 48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 13753216.05 298054.45 51 95.90 13753181.71 298054.20 52 95.90 13753181.71 298054.33 54 95.90 1375312.070 2980497.46 55 95.90 1375309.19 298047.15 54 95.90 1375309.19 298047.16 55 95.90 1375309.19 298047.16 56 95.90 1375209.62 298039.62 57 95.90 1375298.62 2980394.51 58 95.89 1375298.62 2980354.52 60 95.90 1375299.60 2980455.80 61 95.90 1375299.62 2980456.34 62		43	95.90	13753463.52	2980655.72
46 95.90 13753372.82 2980577.15 47 95.90 13753342.59 2980550.96 48 95.90 13753312.36 2980550.96 48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 13753216.05 2980544.45 51 95.90 13753151.20 2980542.01 52 95.90 13753151.20 2980542.03 53 95.90 13753120.70 2980474.56 55 95.90 13753090.19 2980471.58 56 95.90 13753090.19 2980471.58 56 95.90 13753091.82 2980497.46 57 95.90 13752091.82 2980497.46 57 95.90 13752091.82 2980445.71 57 95.90 13752991.62 2980395.52 60 95.90 13752991.62 2980395.52 60 95.90 13752991.62 2980395.52		44	95.90	13753433.29	2980629.53
47 95.90 13753342.59 2980550.96 48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 1375321.36 2980544.45 51 95.90 13753216.05 2980544.45 51 95.90 13753181.71 2980549.20 53 95.90 13753151.20 2980547.66 55 95.90 13753151.20 2980543.33 54 95.90 13753090.19 2980471.68 55 95.90 13753090.19 2980445.71 57 95.90 13753092.18 2980445.31 58 95.89 13752998.62 2980394.03 59 95.90 1375291.55 2980395.52 60 95.90 1375291.55 2980395.52 60 95.90 1375290.60 2980456.34 61 95.90 1375290.60 2980456.34 62 95.90 13752854.04 2980516.53 63 <td></td> <td>45</td> <td>95.90</td> <td>13753403.06</td> <td>2980603.34</td>		45	95.90	13753403.06	2980603.34
48 95.90 13753312.36 2980524.77 49 95.88 13753275.89 2980516.58 50 95.90 13753247.55 2980544.45 51 95.90 13753216.05 2980547.01 52 95.90 13753181.71 2980547.01 53 95.90 13753151.20 2980549.20 53 95.90 13753120.70 2980497.46 55 95.90 13753090.19 298047.158 56 95.90 13753029.18 2980445.71 57 95.90 13753029.18 2980445.71 57 95.90 13752098.62 2980394.03 58 95.89 13752998.62 2980395.52 60 95.90 13752996.62 2980395.52 60 95.90 13752904.60 2980456.34 62 95.90 13752904.60 2980456.34 62 95.90 13752884.04 2980512.89 63 95.90 13752854.04 2980512.98		46	95.90	13753372.82	2980577.15
49 95.88 13753275.89 2980516.58 50 95.90 13753247.55 2980544.45 51 95.90 13753216.05 2980547.01 52 95.90 13753181.71 2980547.01 53 95.90 13753151.20 2980497.46 55 95.90 1375309.19 2980497.46 55 95.90 1375309.19 2980471.58 56 95.90 1375309.19 2980445.71 57 95.90 1375309.18 2980445.71 57 95.90 1375209.18 2980445.31 58 95.89 1375299.62 2980394.03 59 95.90 1375291.55 2980395.52 60 95.90 13752904.60 2980458.34 61 95.90 13752904.60 2980458.34 62 95.90 13752854.04 2980516.34 62 95.90 13752854.04 2980512.98 64 95.90 13752814.94 2980516.53					
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		63	95.90	13752854.04	2980512.98
65 95.90 13752782.02 2980495.14		64	95.90	13752814.94	2980516.53
		65	95.90	13752782.02	2980495.14
	Τ			REVISION	IS

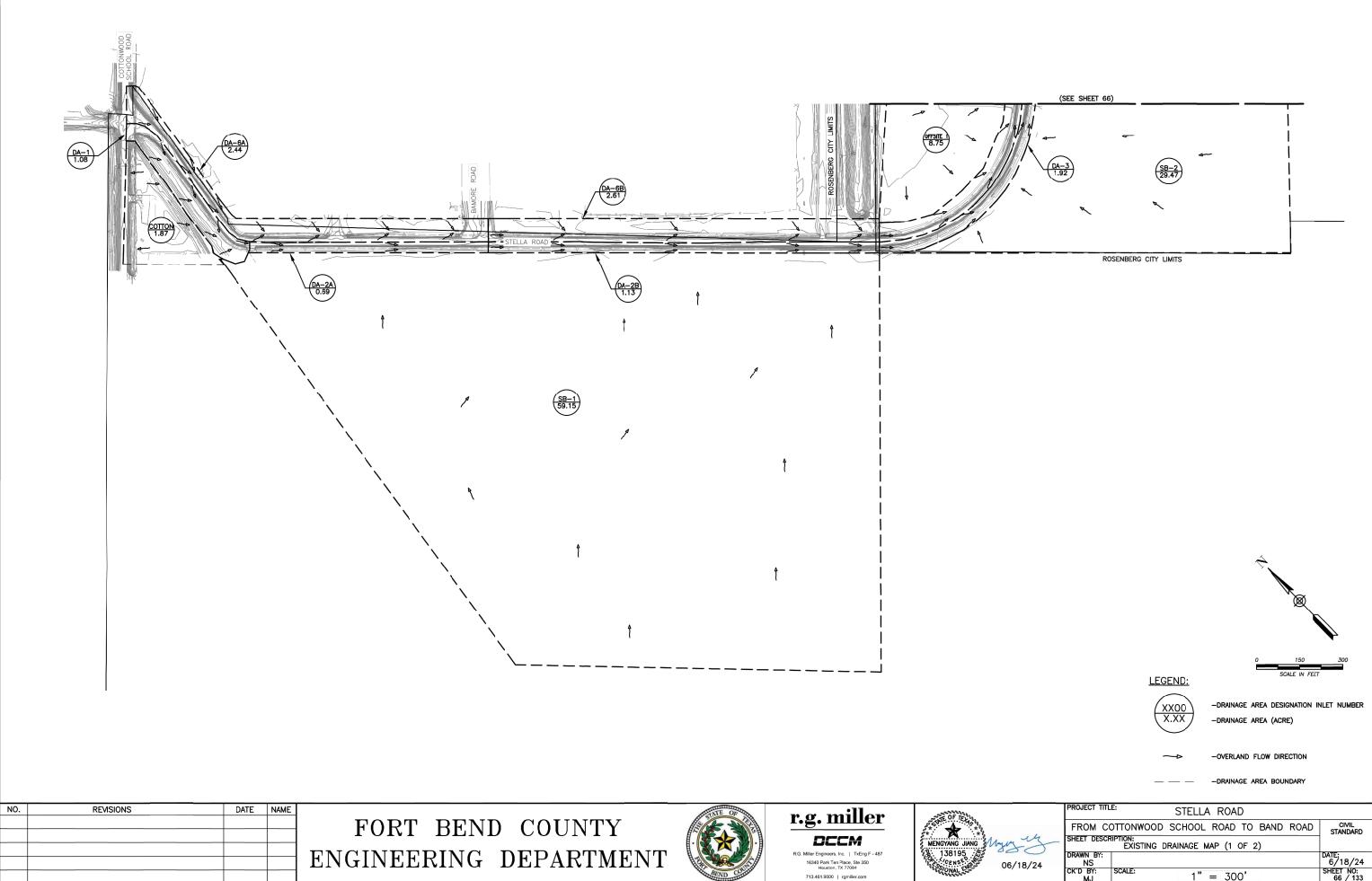
R.G. Miller Engineers, Inc. | TxEng F - 487 16340 Park Ten Place, Ste 350 Houston, TX 77084 713.461.9600 | rgmiller.com

r.g. miller



	I	POINT TABLE		
POINT NO.	ELEV.	NORTHING	EASTING	DESC.
347	95.60	13753700.34	2980720.61	GB
348	95.60	13753673.90	2980750.63	GB
349	95.60	13753647.45	2980780.64	GB
350	95.60	13753613.04	2980797.00	GB
351	95.60	13753579.74	2980776.23	GB
352	95.60	13753549.50	2980750.04	GB
353	95.90	13753519.27	2980723.85	GB
354	95.90	13753489.04	2980697.66	GB
355	95.90	13753458.80	2980671.47	GB
356	95.90	13753428.57	2980645.28	GB
357	95.90	13753398.34	2980619.09	GB
358	95.90	13753368.10	2980592.90	GB
350	95.90	13753337.87	2980566.71	СВ
360	95.90	13753307.64	2980540.52	GB
361	95.90	13753274.41	2980537.04	GB
362	95.90	13753247.64	2980566.77	GB
363	95.60	13753212.10	2980582.06	GB
364	95.60	13753177.03	2980564.91	GB
365	95.90	13753146.53	2980539.03	GB
366	95.90	13753116.02	2980513.16	GB
367	95.90	13753085.52	2980487.29	GB
368	95.90	13753055.01	2980461.41	GB
369	95.90	13753024.51	2980435.54	GB
370	95.90	13752994.00	2980409.66	GB
371	95.90	13752961.75	2980417.91	GB
372	95.90	13752935.91	2980448.46	GB
373	95.90	13752910.0B	2980479.00	GB
374	95.90	13752884.19	2980509.48	GB
375	95.60	13752850.55	2980530.24	GB
376	95.60	13752811.03	2980530.24	GB
370	95.60	13752776.60	2980511.59	
378	95.60	13752756.32	2980311.59	GB GB
379		13752740.65	2980440.74	GB
	95.60			
380	95.60	13752723.51	2980404.60	GB
381	95.60	13752704.02	2980369.71	GB
382	95.60	13752682.75	2980335.84	GB
383	95.60	13752661.48	2980301.96	GB
384	95.60	13752640.21	2980268.08	GB
385	95.60	13752618.94	2980234.20	GB
386	95.60	13752597.67	2980200.33	GB
387	95.60	13752576.41	2980166.45	GB
388	95.60	13752555.75	2980132.20	GB
389	95.60	13752536.82	2980096.97	GB
390	95.60	13752519.67	2980060.84	GB
391	95.60	13752504.34	2980023.89	GB
392	95.60	13752490.88	2979986.23	GB
393	95.60	13752479.31	2979947.95	GB
394	95.60	13752469.67	2979909.13	GB
395	95.60	13752461.97	2979869.88	GB
396	95.60	13752456.25	2979830.30	GB
397	95.60	13752452.51	2979790.48	GB
398	95.60	13752450.76	2979750.52	GB
399	95.60	13752451.65	2979710.57	GB
400	95.60	13752466.44	2979673.80	GB
401	95.60	13752496.30	2979647.73	GB
402	95.60	13752534.73	2979638.04	GB
403	95.60	13752573.37	2979646.83	GB
404	95.60	13752604.94	2979671.11	GB
405	95.60	13752634.51	2979698.05	GB
				GB

	PROJECT TITL				
	FROM C	OTTONW	OOD SCHOOL ROAD TO BAND	ROAD	CIVIL STANDARD
ing	SHEET DESCR				
18/24	DRAWN BY: NS				DATE: 6/18/24
	CK'D BY: MJ	SCALE:	N/A		SHEET NO: 65 / 133

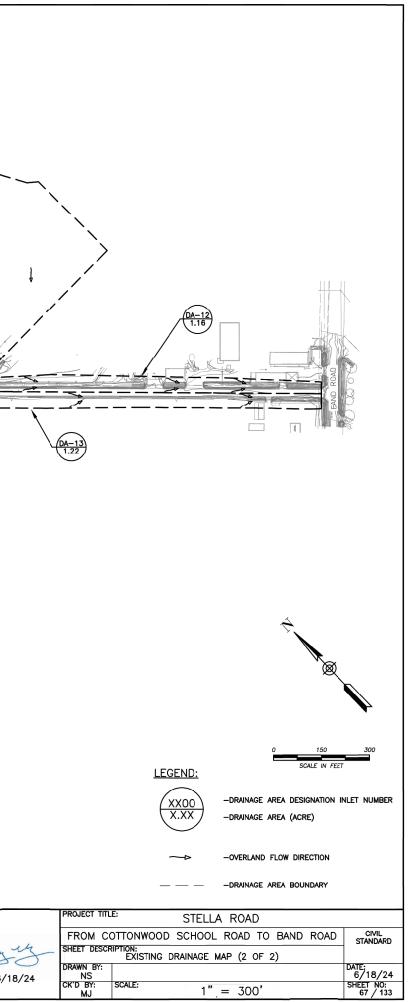


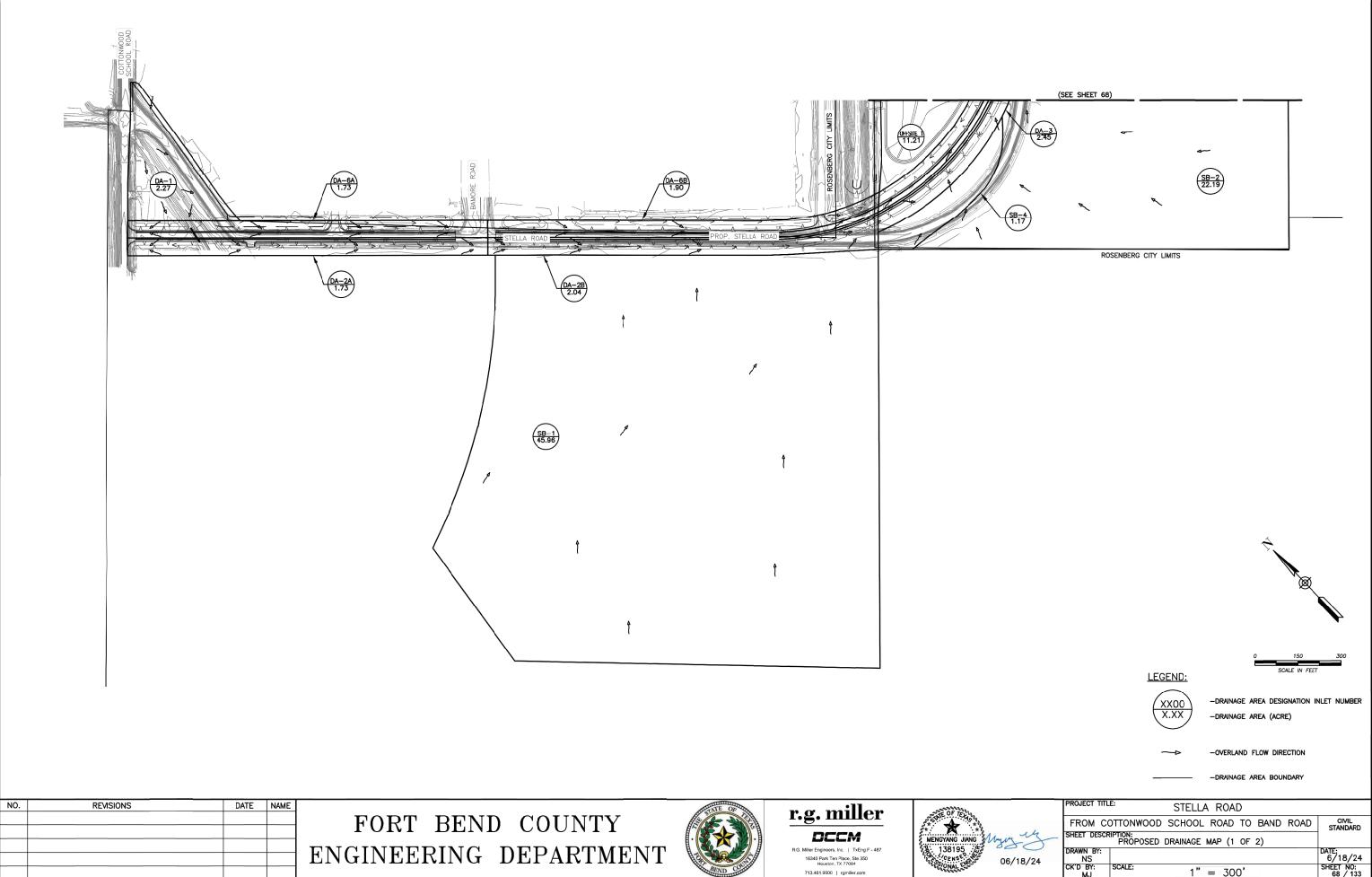
Maps Area – Drainage Stella Road\CAD\DWG\05- 04755 20116 M:\04755.000

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		OTEED T TROTE	
	FROM CO	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
yung	SHEET DESCR	RIPTION: EXISTING DRAINAGE MAP (1 OF 2)	
6/18/24	DRAWN BY: NS		DATE: 6/18/24
,, 10, 21	CK'D BY: MJ	scale: 1" = 300'	SHEET NO: 66 / 133

20116 Stella Road\CAD\DWG\05- 04755 - Drainage Area Maps.dwg		E SHEET 65	
M:\04755.000 20116	NO. REVISIONS DATE NAME	FORT BEND COUN INEERING DEPAR	r.g. miller DECEM RG. Miller Engineers, Inc. TKEng F-487 15340 Park Ten Place, Ste 350 Houston, TX 77084 713.461.9800 gmiller.com



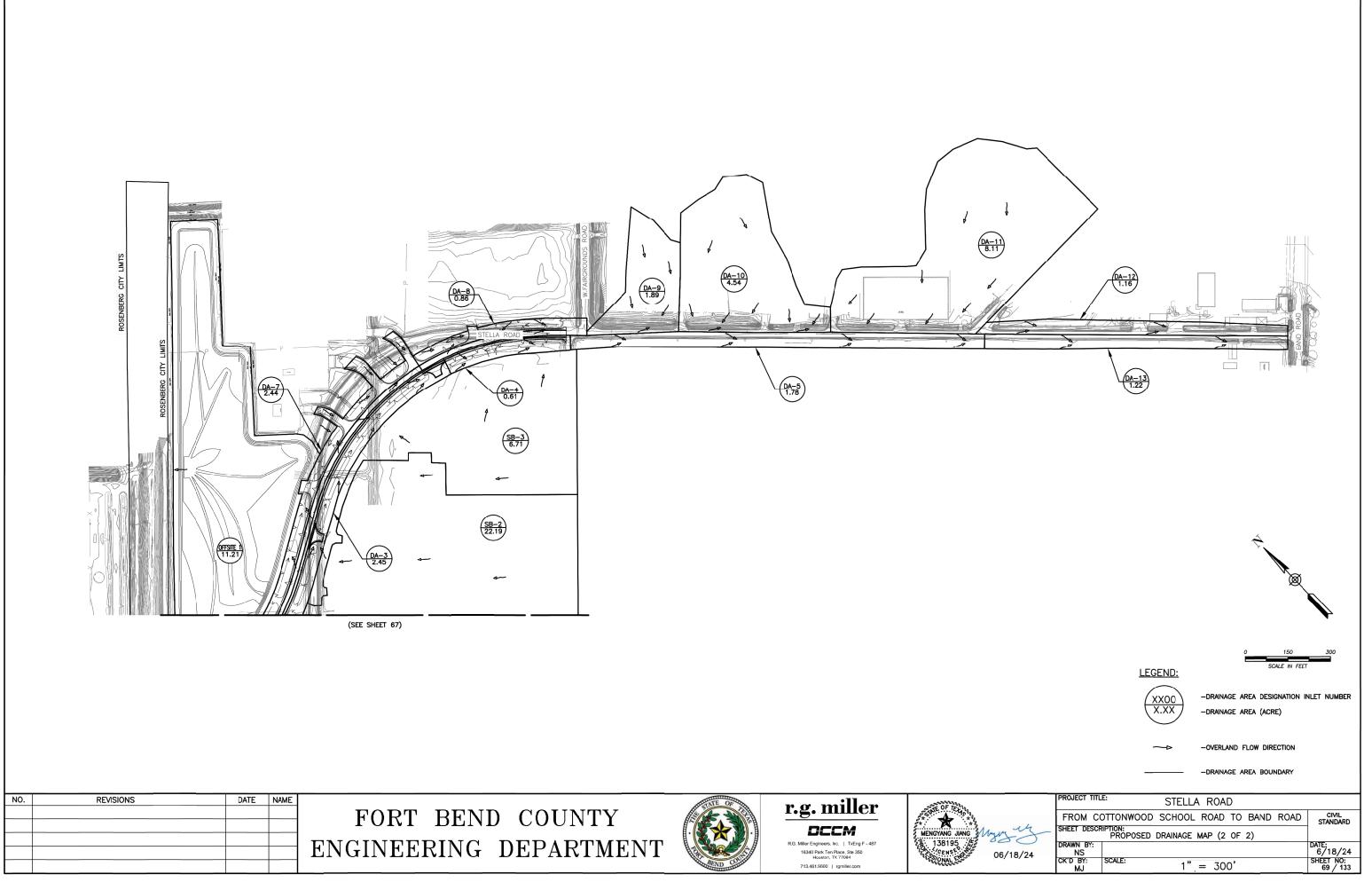


Stella Road/CAD/DWG/05- 04755 - Drainage Area Maps.dwg

20116

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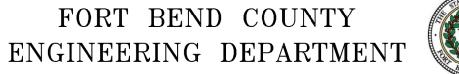
	PROJECT TITL	.E:	STELLA	ROAD			
		OTTONWOOD	SCHOOL	ROAD TO	BAND	ROAD	CIVIL STANDARD
ying	SHEET DESCR	PROPOSED	DRAINAGE	MAP (1 OF	2)		
6/18/24	DRAWN BY: NS						DATE: 6/18/24
,, 10, 21	CK'D BY: MJ	SCALE:	1	= 300'			SHEET NO: 68 / 133



	Rational Method & TC&R Parameters Existing Condition - Atlas 14										
Sub-Basin	Ar	rea	Impervious	С	Тс	Тс	I ₁₀₀	Q ₁₀₀	Storage		
	acre	mi ²	%		min	hr	in/hr	cfs	hr		
DA_1	1.08	0.0017	44.4%	0.54	12.65	0.21	10.80	6.4	0.37		
DA_2A	0.69	0.0011	76.4%	0.72	11.38	0.19	11.37	5.6	0.12		
DA_2B	1.13	0.0018	76.7%	0.72	17.38	0.29	9.24	7.5	0.25		
DA_3	1.92	0.0030	79.7%	0.74	19.78	0.33	8.66	12.3	0.26		
DA_4	0.76	0.0012	57.9%	0.62	10.70	0.18	11.71	5.5	0.18		
DA_5	1.83	0.0029	44.3%	0.54	16.53	0.28	9.47	9.4	0.49		
DA_6A	2,44	0.0038	41.6%	0.49	18.58	0.31	8.94	10.8	0.69		
DA_6B	2.61	0.0041	33.3%	0.48	18.91	0.32	8.86	11.2	0.76		
DA_7	2.34	0.0037	65.8%	0.66	13.95	0.23	10.29	15.9	0.24		
DA_8	0.74	0.0012	79.7%	0.74	10.99	0.18	11.56	6.3	0.11		
DA_9	1.84	0.0029	88.8%	0.78	12.86	0.21	10.71	15.3	0.09		
DA_10	4.54	0.0071	76.0%	0.64	17.62	0.29	9.17	26.5	0.33		
DA_11	8.11	0.0127	86.4%	0.79	21.90	0.36	8.23	52.6	0.24		
DA_12	1.16	0.0018	78.4%	0.73	11.44	0.19	11.33	9.5	0.10		
DA_13	1.22	0.0019	44.7%	0.55	11.77	0.20	11.18	7.4	0.34		
Cotton	1.87	0.0029	11.4%	0.36	11.32	0.19	11.40	7.7	0.83		
Offsite_1	8.75	0.0137	0.0%	0.30	65.02	1.08	4.71	12.4	5.21		
\$B-1	59.15	0.0924	0.0%	0.30	166.15	2.77	2.89	51.3	10.297		
SB-2	29.47	0.0460	30.2%	0.43	182.79	3.05	2.75	34.9	6.770		

DATE NAME

							TIME OF				IS USING T		AND METH	OD								
									-	-	s Training F						0.1.10		0.0.4			1.0%1
	-Area	_		DA_1	DA_2A	DA_2B	DA_3	DA_4	DA_5	DA_6A	DA_6B	DA_7	DA_8	DA_9	DA_10	DA_11	DA_12	DA_13	SB-1	SB-2	Cotton	Offsite
Basin Drainage Area				1.00	0.00	4.42	1.02	0.70	1.02	2.44	2.64	2.24	0.74	1.04	4.5.4	0.11	1.10	1.00	50.45	20.47	1.07	0.75
Desiness Ares		acres		1.08	0.69	1.13	1.92	0.76	1.83	2.44	2.61	2.34	0.74	1.84	4.54	8.11	1.16	1.22	59.15	29.47	1.87	8.75
Drainage Area		sq. mi.		0.002	0.001	0.002	0.003	0.001	0.003	0.004	0.004	0.004	0.001	0.003	0.007	0.013	0.002	0.002	0.092	0.046	0.003	0.014
mpervious Cover		_																				
Description		c	,							Are	a of Land (a	icres) Beloi	nging to Ea	ch Develop	ment Cate	gory						
Road/Lake	W	0.85	100%	0.48	0.53	0.87	1.53	0.44	0.81	0.86	0.87	1.54	0.59	0.46	0.65	0.80	0.74	0.52	0.00	1.04	0.00	0.00
Commercial	С	0.75	85%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.38	0.00	7.30	0.20	0.03	0.00	0.00	0.25	0.00
Industrial	1	0.6	72%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.89	0.00	0.00	0.00	0.00	10.90	0.00	0.00
Single-Family	SF	0.5	66%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Residential larger Lot	LR	0.3	25%	0.00	0.00	0.00	0.00	0.00	0.00	0.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Open Space/Undeveloped	OS	0.3	0%	0.60	0.16	0.26	0.40	0.33	1.02	0.96	1.74	0.80	0.15	0.00	0.00	0.80	0.21	0.67	59.15	17.53	1.62	8.75
Runoff Coefficient				0.54	0.72	0.72	0.74	0.62	0.54	0.49	0.48	0.66	0.74	0.78	0.64	0.79	0.73	0.55	0.3	0.4	0.4	0.30
Impervious Area		acres		0.5	0.5	0.9	1.5	0.4	0.8	1.0	0.9	1.5	0.6	1.6	3.5	7.0	0.9	0.5	0.0	8.9	0.2	0.0
Impervious Cover		%		44.4%	76.4%	76.7%	79.7%	57.9%	44.3%	41.6%	33.3%	65.8%	79.7%	88.8%	76.0%	85.4%	78.4%	44.7%	0.0%	30.2%	11.4%	0.0%
									Tir	ne of Conc	entration											
							:	SCS Upland	s Method	Curve C - O	verland Flo	w in Grassy	Areas									
Distance		feet		50.0	30.0	30.0	0.0	50.0	0.0	40.0	65.0	20.0	30.0	0.0	0.0	0.0	0.0	0.0	300.0	300.0	77.8	300
Slope		percent		0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.07	0.01	0.05	0.0
Velocity		ft/sec		0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.18	0.08	0.16	0.1
Travel Time		minutes		3.78	2.27	2.27	0.00	3.78	0.00	3.02	4.91	1.51	2.27	0.00	0.00	0.00	0.00	0.00	27.09	59.63	8.32	32.0
							SCS Uplan	ds Method	Curve F - S	hallow Cor	ncentrated	low in Gra	ssed Wate	rway								
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2776.0	1222.0	0.0	653.
Slope		percent		0.00	0.11	0.11	0.00	0.00	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.01	0.07	0.05
Velocity		ft/sec		0.00	0.49	0.49	0.00	0.00	0.47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.39	0.18	0.39	0.33
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	118.19	115.24	0.00	32.9
							SCS Upla	ands Meth	od Curve G	- Paved Ar	eas (Sheet	Flow) and L	Jpland Gull	ies								
Distance		feet		50	0.0	0.0	10.0	25.0	0.0	0.0	0.0	25.0	25.0	341.0	370.0	233.0	0.0	0.0	0.0	0.0	0.0	0.0
Slope		percent		0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.1
Velocity		ft/sec		0.69	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.5
Travel Time		minutes		1.22	0.00	0.00	0.32	0.81	0.00	0.00	0.00	0.81	0.81	11.06	12.00	7.56	0.00	0.00	0.00	0.00	0.00	0.0
									Flow in St	orm Sewer	/Roadside	Ditch										
Distance		feet		0.0	0.0	0.0	46.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
/elocity		ft/sec		3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00
ravel Time		minutes		0.00	0.00	0.00	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
									Flo	w in Roads	ide Ditch											
Distance		feet		689.0	820.0	1360.0	1728.0	550.0	1488.0	1400.0	1260.0	1047.0	712.0	162.0	506.0	1291.0	1030.0	1059.0	1879.0	712.0	270.0	0.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.5
Travel Time		minutes		7.66	9.11	15.11	19.20	6.11	16.53	15.56	14.00	11.63	7.91	1.80	5.62	14.34	11.44	11.77	20.88	7.91	3.00	0.0
									Flo	w in Chan	nel/Ditch											
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.5
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0
TC			minutes	12.65	11.38	17.38	19.78	10.70	16.53	18.58	18.91	13.95	10.99	12.86	17.62	21.90	11.44	11.77	166.15	182.79	11.32	65.0
тс			hours	0.21	0.19	0.29	0.33	0.18	0.28	0.31	0.32	0.23	0.18	0.21	0.29	0.36	0.19	0.20	2.77	3.05	0.19	1.0







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NO.

REVISIONS

	PROJECT TITL	E: STELLA ROAD								
FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD										
yung	SHEET DESCRIPTION: EXISTING DRAINAGE CALCULATIONS									
6/18/24	DRAWN BY: NS		DATE: 6/18/24							
0,10,21	CK'D BY: MJ	SCALE: N/A	SHEET NO: 70 / 133							

	TIME OF CONCENTRATION CALCULATIONS USING THE SCS UPLAND METHOD																						
								TIME OF C		TION CALC				ND METHO	D								
Suk	o-Area			DA 1	DA 2A	DA 2B	DA 3	DA 4	DA 5	DA 6A	DA 6B	DA 7	DA 8	DA 9	DA 10	DA 11	DA 12	DA 13	Offsite 1	SB-1	SB-2	SB-3	SB-4
Jul	FArea			UA_I	UA_ZA	DA_2D	DA_3	UA_4	DA_5		n Drainage		DA_0	DA_9	DA_10	DA_11	DA_12	DA_13	Unsite_1	30-1	50-2	50-5	3D-4
		acres		2.27	1.73	2.04	2.45	0.61	1.78	1.73	1.90	2.44	0.86	1.89	4.54	8.11	1.16	1.22	11.21	45.95	22.19	6.71	1.17
Drainage Area		sg. mi.		0.004	0.003	0.003	0.004	0.001	0.003	0.003	0.003	0.004	0.001	0.003	0.007	0.013	0.002	0.002	0.018	0.072	0.035	0.010	0.002
oraniage Area	I	3q. m.		0.004	0.005	0.005	0.004	0.001	0.005		pervious C		0.001	0.005	0.007	0.015	0.001	0.001	0.010	0.072	0.035	0.010	0.001
Description		c	,													ent Catego							
Road/Lake	W	0.85	100%	0.00	1.21	1.30	1.61	0.43	0.81	1.46	1.90	1.74	0.58	0.46	0.65	0.80	0.74	0.52	9.67	0.00	0.00	0.00	0.00
Commercial	С	0.75	85%	0.21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.38	0.00	7.30	0.20	0.03	0.00	0.00	22.19	0.00	0.00
Industrial	1	0.6	72%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.89	0.00	0.00	0.00	0.00	0.00	0.00	5.94	0.00
Single-Family	SF	0.5	66%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Residential larger Lot	LR	0.3	25%	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Open Space/Undeveloped	OS	0.3	0%	2.06	0.52	0.74	0.84	0.18	0.97	0.27	0.00	0.70	0.28	0.05	0.00	0.80	0.21	0.67	1.54	45.95	0.00	0.77	1.17
Runoff Coefficient				0.3	0.7	0.7	0.7	0.7	0.6	0.8	0.9	0.7	0.7	0.8	0.6	0.8	0.7	0.5	0.8	0.3	0.8	0.6	0.3
Impervious Area		acres		0.2	1.2	1.3	1.6	0.4	0.8	1.5	1.9	1.7	0.6	1.6	3.5	7.0	0.9	0.5	9.7	0.0	18.9	4.3	0.0
Impervious Cover		%		7.9%	69.9%	63.7%	65.7%	70.5%	45.5%	84.4%	100.0%	71.3%	67.4%	86.4%	76.0%	86.4%	78.4%	44.7%	86.3%	0.0%	85.0%	63.7%	0.0%
										Time	of Concent	tration											
SCS Uplands Method Curve C - Overland Flow in Grassy Areas																							
Distance feet 77.8 60.0 60.0 60.0 60.0 60.0 60.0 60.0 60																							
Slope		percent		0.05	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.07	0.10	0.10	0.10
Velocity		ft/sec		0.16	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.22	0.18	0.22	0.22	0.22
Travel Time		minutes		8.32	4.53	4.53	4.53	4.53	0.00	4.53	4.53	4.53	4.53	0.00	0.00	0.00	0.00	0.00	22.65	27.09	4.53	22.65	7.55
							sc	S Uplands	Method Cu	rve F - Sha	llow Conce	entrated Flo	w in Grass	ed Waterv	vay								
Distance		feet		521.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	140.0	2224.0	0.0	0.0	0.0
Slope		percent		0.05	0.11	0.11	0.00	0.00	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.10	0.07	0.00	0.00	0.00
Velocity		ft/sec		0.33	0.49	0.49	0.00	0.00	0.47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.47	0.39	0.00	0.00	0.00
Travel Time		minutes		26.29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.98	94.69	0.00	0.00	0.00
								SCS Unland	ds Method	Curve G - F	aved Area	s (Sheet Fl	w) and Up	and Gullie	25					1 1			
Distance		feet		50	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	341.0	370.0	233.0	0.0	0.0	0.0	0.0	300.0	0.0	0.0
Slope		percent		0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
Velocity		ft/sec		0.69	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51	0.51
Travel Time		minutes		1.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	11.06	12.00	7.56	0.00	0.00	0.00	0.00	9.73	0.00	0.00
										ow in Stor													
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	738.0	0.0	0.0
Velocity		ft/sec		3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.10	0.00	0.00
				0.00	0.00	0.00	0.00	0.00	0.00		in Roadsid		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-1140	0.00	0.00
Distance		feet		689.0	1253.0	1359.0	1786.0	446.0	1488.0	1253.0	1359.0	1563.0	627.0	162.0	506.0	1291.0	1030.0	1059.0	450.0	0.0	1732.0	906.0	500.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Travel Time		minutes		7.66	13.92	15.10	19.84	4.96	16.53	13.92	15.10	17.37	6.97	1.80	5.62	14.34	11.44	11.77	5.00	0.00	19.24	10.07	5.56
Traver Time	L	in lates		7.00	13.52	10.10	13.04	4.50	10.55		in Channel		0.97	1.00	5.02	14.34	11.44	11.77	5.00	0.00	13.24	10.07	5.50
Distance		feet		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	80.0
Velocity		ft/sec		1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Travel Time		minutes		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.89
TC		innutes	minutes	43.48	18.45	19.63	24.38	9.49	16.53	18.45	19.63	21.90	11.50	12.86	17.62	21.90	11.44	11.77	32.63	121.77	37.60	32.72	14.00
TC			hours			0.33	24.38 0.41	9.49	0.28	18.45 0.31	19.63 0.33	0.36	0.19	0.21	0.29	0.36	0.19	0.20	32.63 0.54	2.03	0.63	0.55	0.23
			nours	0.72	0.31	0.33	0.41	0.16	0.28	0.31	0.33	0.36	0.19	0.21	0.29	0.36	0.19	0.20	0.54	2.03	0.63	0.55	0.23

Rational Method & TC&R Parameters Proposed Condition - Atlas 14											
Sub-Basin	Ar	rea	Impervious	С	Тс	Тс	I ₁₀₀	Q ₁₀₀	Storage		
	acre	mi ²	%		min	hr	in/hr	cfs	hr		
DA_1	2.27	0.0035	7.9%	0.34	43.48	0.72	5.80	4.5	3.000		
DA_2A	1.73	0.0027	69.9%	0.68	18.45	0.31	8.97	10.6	0.303		
DA_2B	2.04	0.0032	63.7%	0.65	19.63	0.33	8.69	11.5	0.373		
DA_3	2.45	0.0038	65.7%	0.66	24.38	0.41	7.80	12.6	0.460		
DA_4	0.61	0.0010	70.5%	0.69	9.49	0.16	12.40	5.2	0.118		
DA_5	1.78	0.0028	45.5%	0.55	16.53	0.28	9.47	9.3	0.490		
DA_6A	1.73	0.0027	84.4%	0.76	18.45	0.31	8.97	11.9	0.208		
DA_6B	1.90	0.0030	100.0%	0.85	19.63	0.33	8.69	14.0	0.165		
DA_7	2.44	0.0038	71.3%	0.69	21.90	0.36	8.23	13.9	0.343		
DA_8	0.86	0.0013	67.4%	0.67	11.50	0.19	11.31	6.5	0.120		
DA_9	1.89	0.0030	86.4%	0.76	12.86	0.21	10.71	15.4	0.097		
DA_10	4.54	0.0071	76.0%	0.64	17.62	0.29	9.17	26.5	0.33		
DA_11	8.11	0.0127	86.4%	0.79	21.90	0.36	8.23	52.6	0.24		
DA_12	1.16	0.0018	78.4%	0.73	11.44	0.19	11.33	9.5	0.10		
DA_13	1.22	0.0019	44.7%	0.55	11.77	0.20	11.18	7.4	0.34		
Offsite_1	11.21	0.0175	86.3%	0.77	32.63	0.54	6.72	58.4	0.500		
SB-1	45.95	0.0718	0.0%	0.30	121.77	2.03	3.40	46.9	8.270		
SB-2	22.19	0.0347	85.0%	0.75	37.60	0.63	6.25	104.1	0.540		
SB-3	6.71	0.0105	63.7%	0.57	32.72	0.55	6.71	25.5	0.940		
SB-4	1.17	0.0018	0.0%	0.30	14.00	0.23	10.28	3.6	1.400		

DATE NAME



DCCM



FORT BEND COUNTY ENGINEERING DEPARTMENT

REVISIONS

	PROJECT TITL	E:	STELLA I	ROAD			
	FROM C	DOOWNOTTC	SCHOOL R	OAD TO	BAND	ROAD	CIVIL STANDARD
yung	SHEET DESCR		DRAINAGE CA	LCULATION	IS		
	DRAWN BY: NS						DATE: 6/18/24
0,10,21	CK'D BY: MJ	SCALE:	N/A				SHEET NO: 71 / 133
6/18/24	NS CK'D BY:						6/18/24 SHEET NO:

PROPOSED CULVERT NODE INPUT DATA

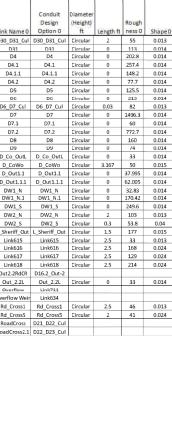
Node Inputs

Name Scenario

NO.		REVI	SIONS			DA	TE NA
	D13_DS.1 D13_DS.2	100-Year Storm 100-Year Storm	D13_DS.1 D13_DS.2	96.664 97.16	92.258 93.66	0.25	Allowed None
	D3_US	100-Year Storm	D3_US	96.511 96.664	92.9 92.258	0.25	None
	D3_DS.1	100-Year Storm	D3_DS.1	96.511	92.21	0	Allowed
	D12_US.1.1 Sheriff_Outfall	100-Year Storm 100-Year Storm	D12_US.1.1 Sheriff_Outfall	97.47 96.511	92.64 90.3	0	Allowed
	D2B_US.1	100-Year Storm	D2B_US.1	97.48	93.58		Allowed
	DW2N_US.1 D12_US.1	100-Year Storm 100-Year Storm	DW2N_US.1 D12_US.1	99.36 97.48	95.45 93.68	0.17	None Allowed
	Out_2.2	100-Year Storm	Out_2.2	95.28	87.21		None
	D2B_US D_Co_Out	100-Year Storm 100-Year Storm	D2B_US D_Co_Out	97.54 100.5	93.68 94.89	0.17	Allowed None
	D2_US.2	100-Year Storm	D2_US.2	97.7	93.97	0.25	Allowed
	DW2S_DS DW1S_Out	100-Year Storm 100-Year Storm	DW2S_DS DW1S_Out	99.36 100.5	95.24 94.99	0	None
	DW1N_Out	100-Year Storm	DW1N_Out	100.5	95.07		None
	DW2N_US	100-Year Storm	DW2N_US	95.929	95.3	0.17	None
	Node653 D7 US.1	100-Year Storm 100-Year Storm	Node653 D7_US.1	92.94 95.929	89.44 92.579	0.17	Allowed
	Node652	100-Year Storm	Node652	93.38	89.43		Allowed
	Node650 Node651	100-Year Storm 100-Year Storm	Node650 Node651	93.45 96.03	89.2 89.1		Allowed
	D28_US	100-Year Storm	D28_US	95.691	90.6		Allowed
	D31_DS	100-Year Storm	D31_DS	95.52	91.16		Allowed
	D30_DS D31_US	100-Year Storm 100-Year Storm	D30_DS D31_US	94.939 94.9	91.46 91.52		Allowed
	D30_US	100-Year Storm	D30_US	95.187	91.42		Allowed
	D29_DS	100-Year Storm	D29_DS	95.2	91.51		Allowed
	D28_DS D29_US	100-Year Storm 100-Year Storm	D28_DS D29_US	95.22	91.05 91.019		Allowed
	D28.1_US D28_DS	100-Year Storm	D28.1_US D28_DS	95.971 95.22	92.437 91.05	0.17	Allowed
	D27_DS	100-Year Storm	D27_DS	95	92.4	a :-	Allowed
	D20_03	100-Year Storm	D20_03 D27_US	95.09	93.435		Allowed
	D21_DS D20_US	100-Year Storm 100-Year Storm	D21_DS D20_US	96.59 96.24	89.26 89.5	0	Allowed
	D21_US	100-Year Storm	D21_US	97.803	87.73		Allowed
	D22_US D22_DS	100-Year Storm	D22_03 D22_DS	96.11	92.46 88.49		Allowed
	D23_DS	100-Year Storm 100-Year Storm	D23_DS D22_US	96.496 95.94	92.79 92.46		Allowed
	D23_US	100-Year Storm	D23_US	96.03	89.2	0.17	Allowed
	D24_US D24_DS	100-Year Storm 100-Year Storm	D24_US D24_DS	95.769 96.01	92.51 92.01		Allowed
	D25_DS	100-Year Storm	D25_DS	95.91	92.38		Allowed
	D26_D3 D25_US	100-Year Storm	D26_D3 D25_US	95.911	92.89		Allowed
	D26.1_US D26_DS	100-Year Storm 100-Year Storm	D26.1_US D26_DS	95.911 95.911	89.26 92.99		Allowed
	D26_US	100-Year Storm	D26_US	95.09	94.114		Allowed
	D8_DS Out_4	100-Year Storm 100-Year Storm	D8_DS Out_4	95.949 95.838	90.85		Allowed
	D8_US	100-Year Storm	D8_US	96 95.949	91.52 90.85		Allowed
	D7_DS	100-Year Storm	D7_DS	96	91.67		Allowed
	D7.1_US	100-Year Storm	D7.1_US	95.146	92.5		Allowed
	Out_1.1 Out_1.2	100-Year Storm 100-Year Storm	Out_1.1 Out_1.2	95 95.146	88.95 89.15		Allowed
	Basin1_Opt2	100-Year Storm	Basin1_Opt2	96	89.2	0.17	Allowed
	D4.1_US.1	100-Year Storm	D4.1_US.1	96.6	91.41	0.17	Allowed
	D13_US.1 D2_US.1	100-Year Storm 100-Year Storm	D13_US.1 D2_US.1	96.7 96.5	91.47 91.37	0.25	Allowed
	D15_DS	100-Year Storm	D15_DS D13_US.1	96.7 96.7	91.82 91.47		Allowed
	D14_DS	100-Year Storm	D14_DS	96.78	93.27	0.17	Allowed
	Outs.1 Out_3	100-Year Storm	Outs.1 Out_3	96.381	90.113		Allowed
	D19_US Out3.1	100-Year Storm 100-Year Storm	D19_US Out3.1	96.31 95.407	92.594 91.93		Allowed
	D18_DS	100-Year Storm	D18_DS	96.6	92.73	0.17	Allowed
	D17_DS D18_US	100-Year Storm 100-Year Storm	D17_DS D18_US	96.6 96.5	91.86 91.96		Allowed
	D17_US	100-Year Storm	D17_US	96.7	91.41		Allowed
	D16_DS	100-Year Storm	D16_D5	96.517	91.21		Allowed
	D15_US D16_US	100-Year Storm 100-Year Storm	D15_US D16_US	96.68 96.7	93.17 91.72		Allowed
	D14_US	100-Year Storm	D14_US	96.64	93.13		Allowed
	D13_03	100-Year Storm	D13_03 D13_DS	96.8	93.3	0.25	Allowed
	D12_DS D13_US	100-Year Storm 100-Year Storm	D12_DS D13_US	96.3 96.4	91.83 91.73		Allowed
	D12_US	100-Year Storm	D12_US	97.54	93.87	0.17	Allowed
	D11_US D11_DS	100-Year Storm 100-Year Storm	D11_US D11_DS	98 97.9	93.86		Allowed Allowed
	D10_DS	100-Year Storm	D10_DS	98.1	93.81 93.86		Allowed
	Out_2 D10_US	100-Year Storm 100-Year Storm	Out_2 D10_US	98.23	95	0.17	Allowed
	D16.2_DS	100-Year Storm	D16.2_DS	95.39 95.28	91.1 87.31		Allowed
	D16.1_US	100-Year Storm	D16.1_US	96.6	91.2		Allowed
	D6_DS	100-Year Storm 100-Year Storm	D6_DS D7_US	96.5 96.32	92.58 92.73	U.1/	Allowed
	D6_US	100-Year Storm	D6_US	96.5	92.09		Allowed
	D5_US D5_DS	100-Year Storm 100-Year Storm	D5_US D5_DS	96.7 96.6	91.63 91.99	0.17	Allowed
	D4_DS	100-Year Storm	D4_DS	96.7	91.53	_	Allowed
	D4.1_US D4.1_D3	100-Year Storm 100-Year Storm	D4.1_US D4.1_D3	96.6 96.7	92.4 91.31		Allowed
	D4_US	100-Year Storm	D4_US	96.3	92.6		Allowed
	D3_DS	100-Year Storm	D3_DS	96.4	92.7	0.17	Allowed
	D13.1_US Out_1	100-Year Storm 100-Year Storm	D13.1_US Out_1	96.8 95.269	91.1 89.274		Allowed
	D2_05	100 Year Storm	D2_05	96.8	91.2	0	Allowed
	D2_US	100-Year Storm	D1_DS D2_US	98.11	93.99 93.89	0.25	Allowed
	D1_DS	100-Year Storm					
	D1_US D1_DS	100-Year Storm 100-Year Storm	Node Name 0 D1_US	98.33 98.11	Elevation ft 94.82	Hydrograph 0.25	Ponding Type Allowed

		Conduit Design	Diameter	Length			Upstream Invert Elevation	Downstream Invert	Natural Section Shape	Initial Water Surface	Vertical		Cross-section Identification	Left Channel	Right Channel	User- defined Conduit using		
Name	Link Name 0	Option 0	(Height) ft	ft	Foughness 0	Shape 0	ft	Elevation ft	GLDB Reference 0	Slope 0	Shift ft	Factor ft	Number 0	Length ft	Length ft	GLDB	Roughness	
1311.1	D1_D2_Cul	Circular	2	60	0.013	Circular	93.99	93.89		0.001	0	0	0	0	0	D	0.013	Link
1317.1	D13_Out_CUI	Circular	1.5	81	0.013	Circular	91.1	89.67		0.001	0	0	0	0	0	D	0.013	D30
1322.1 1330.1	D3_D4_Cul D4_D5_Cul	Circular	2	80.2 90	0.013	Circular	92.7 91.63	92.6 91.53		0.001	0	0	0	0	0	D	0.013	
1330.1	D4_D5_Cul D5_D6_Cul	Circular Circular	2	91.5	0.013	Circular Circular	92.09	91.99		0.001	0	0	0	0	0	D	0.013	
1344.1	D7_D8_Cul	Circular	2	45	0.013	Circular	91.84	91.52		0.001	0	0	0	0	0	D	0.013	
1344.2	D7_D8_Cul	Circular	2	45	0.013	Circular	91.67	91.57		0.001	0	0	0	0	0	D	0.013	D
1350.1	Rd_Cross3	Circular	2	42	0.013	Circular	91.31	91.21		0.001	0	0	0	0	0	D	0.013	
1350.2	Rd_Cross3	Circular	2	42	0.013	Circular	91.31	91.21		0.001	0	0	0	0	0	D	0.013	
1350.3	Rd_Cross3	Circular	2	42	0.013	Circular	91.31	91.21		0.001	0	0	0	0	0	D	0.013	D6_
1354.1	D16.2_Out-2	Circular	1.67	51	0.013	Circular	91.1	91		0.001	0	0	0	0	0	D	0.013	
1359.1	D10_D11_Cul	Circular	2	65.8	0.013	Circular	93.97	93.86		0.001	0	0	0	0	0	D	0.013	
1367.1	D12_D13_CUI	Circular	2	78	0.013	Circular	91.83	91.73		0.001	0	0	0	0	0	D	0.013	
1372.1	D13_D14_Cul	Circular	2	79.1	0.013	Circular	93.3	93.13		0.001	0	0	0	0	0	D	0.013	
1381.1 1385.1	D16 D17 Cul D17_D18_Cul	Circular Circular	2	71.1 61.7	0.013	Circular Circular	91.41 91.96	91.21 91.86		0.001	0	0	0	0	0	D	0.013	
1389.1	Rd_Cross4	Circular	2	83	0.013	Circular	92.89	92.83		0.001	0	0	0	0	0	D	0.013	D_C
1389.2	Rd_Cross4	Circular	2	83	0.013	Circular	92.87	92.65		0.001	0	0	0	0	0	D	0.013	D_
1410.1	D22_D23_Cul	Circular	1	24	0.024	Circular	92.79	92.46		0.001	0	0	0	0	0	D	0.024	
1414.1	D21_D22_Cul	Circular	3	81	0.024	Circular	88.49	87.83		0.001	0	0	0	0	0	D	0.024	_D_C
1442.1	D14_D15_Cul	Circular	2	57	0.013	Circular	93.27	93.17		0.001	0	0	0	0	0	D	0.013	D
1444.1	D15_D16_Cul	Circular	2	70.6	0.013	Circular	91.82	91.72		0.001	0	0	0	0	0	D	0.013	DV D
1445.1	Rd_Cross2	Circular	2.5	44	0.013	Special	93.13	92.4		0.001	0	0	0	0	0	D	0.013	D
1487.1	Link533	Circular	2.5	100	0.013	Circular	91.1	91		0.001	0	0	0	0	0	D	0.013	D
1488.1	Link634	Circular	1.417	70	0.013	Circular	89.2	89.15		0.001	0	0	0	0	0	D	0.013	L_Sh
Coss_Cul	D10_D11_Cul																	L_SITE
Cross_Cul	D14_D15_Cul																	Lir
Cross_Cul1	D12_D13_CUI D13 D14 Cul													<u> </u>				Lir
Cross_Cul2.1																		Lir
Cross_Cul3 Cross_Cul4	D15_D16_Cul D16_D17_Cul		-	-														Out
CrossCul4	CrossCul	Circular	2.5	64.7	0.014	Special	93.68	93.58		0.001	0	0	0	0	0	D	0.014	Ou
Cul_Cross	D1_D2_Cul		2.5	0.4.7	0.014	opeerd	55.00	55.50		0.004						5	0.047	0~
Cul_Cross2	D3_D4_Cul																	Overf
Cul_Cross3	D17_D18_Cul																	Rd_
Cul_Cross4	D5_D6_Cul																	Rd
Cul_Cross5	D4_D5_Cul																	Roa
D1	D1	Circular	0	80	0.014	Natural	94.82	93.99	Stella_A_S_XS_O240	0.001	0	0	0	80	80	D	0.014	Road
D10	D10	Circular	0	382.88	0.014	Natural	95	93.97	Stella_A_N_XS_O2_60'	0.001	0	0	0	382.88	382.88	D	0.014	
D11	D11	Circular	0	362.1	0.014	Natural	93.86	93.91	Stella_N_50RD	0.001	0	0	0	362.1	362.1	D	0.014	
	D11_D12_Cul	Circular	2	200	0.015	ectangul		93.87		0.001	0	0	0	0	0	D	0.015	
D12.1	D12.1	Circular	0	115.8	0.014	Natural	93.87	93.68	Stella_N_50RD	0.001	0	0	0	115.8	115.8	D	0.014	
D12.1.1.1	D12.1.1.1	Circular	0	818.9	0.014	Natural	92.64	91.83	Stella_N_50RD	0.001	0	0	0	818.9	818.9	D	0.014	
D13 D13.1	D13 D13.1	Circular Circular	0	109 358.59	0.014	Natural Natural	91.73 93.66	91.47 93.3	Stella_A_N_XS_O2_40' Stella_N_36_3ft	0.001	0	0	0	109 876.2	109 876.2	n D	0.014	
D13.1.1	D13.1.1	Circular	0	517.61	0.014	Natural	92.258	91.1	Stella_N_36_3ft	0.001	0	0	0	876.2	876.2	D	0.014	
D13.2	D13.2	Circular	0	148.5	0.014	Natural	91.47	91.1	Stella N 36	0.001	0	0	0	148.5	148.5	D	0.014	
D14	D14	Circular	0	174.2	0.014	Natural	93.27	93.13	Stella N 36 3ft	0.001	0	0	0	174.2	174.2	D	0.014	
D15	D15	Circular	0	107.3	0.014	Natural	93.17	91.82	Stella_N_36_3ft	0.001	0	0	0	107.3	107.3	D	0.014	
D16	D16	Circular	0	29.6	0.014	Natural	91.72	91.2	Stella_N_36	0.001	0	0	0	29.6	29.6	D	0.014	
D16.1	D16.1	Circular	0	32.3	0.014	Natural	91.21	91.2	Stella_N_36	0.001	0	0	0	32.3	32.3	D	0.014	
D16.2	D16.2	Circular	0	118	0.014	Natural	91.2	91.1	D16.2_XS	0.001	0	0	0	118	118	D	0.014	
D17	D17	Circular	0	167.8	0.014	Natural	91.86	91.41	Stella_N_36	0.001	0	0	0	157.8	167.8	D	0.014	
D17_Out-2	D17_Out-2	Circular	0	115	0.014	Natural	93.06	92.8	D17_Out-2_XS	0.001	0	0	0	115	115	D	0.014	
D18	D18	Circular	0	261.2	0.014	Natural	92.73	91.96	Stella_N_36_3ft	0.001	0	0	0	251.2	261.2	D	0.014	
D19	D19	Circular	0	129	0.014	Natural	92.594	91.93	D19_XS	0.001	0	0	0	129	129 674	D	0.014	
D2 D2.1	D2 D2.1	Circular Circular	0	674 80	0.014	Natural Natural	93.89 91.37	93.97 91.2	Stella_A_S_XS_O240 Stella_A_S_XS_O240	0.001	0	0	0	674 80	674 80	D	0.014	
D2.1 D2.2	D2.1 D2.2	Circular	2	160	0.014	Circular	93.97	91.2	Stella_A_S_XS_O2_34'	0.001	0	0	0	79.123	79.123	D	0.014	
D2.2.1	D2.2 D2.2.1	Circular	0	142.9	0.013	Natural	93.68	93.58	Stella_A_S_XS_O240	0.001	0	0	0	142.9	142.9	D	0.013	
D2.2.1.1	D2.2.1.1	Circular	0	1210.6	0.014	Natural	93.58	91.37	Stella A S XS O2 .40	0.001	0	0	0	1210.6	1210.6	D	0.014	
D20	D20	Circular	0	400	0.014	Natural	89.956	90.113	D20_XS	0.001	0	0	0	400	400	D	0.014	
	D20_D21_Cul	Circular	2.5	81	0.024	Circular	89.26	89.5	-	0.001	0	0	0	0	0	D	0.024	
D21	D21	Circular	0	251	0.014	Natural	91.024	89.811	D21_XS	0.001	0	0	0	251	251	D	0.014	
D22	D22	Circular	0	78	0.014	Natural	93.12	89.034	D22_XS	0.001	0	0	0	78	78	D	0.014	
D23	D23	Circular	0	89	0.014	Natural	92.599	93.235	D23_XS	0.001	0	0	0	89	89	D	0.014	
	D23_D24_Cul	Circular	1	67	0.009	Circular	92.01	91.12		0.001	0	0	0	0	0	D	0.009	
D24	D24	Circular	0	199	0.014	Natural	93.06	92.591	D24_XS	0.001	0	0	0	199	199	D	0.014	
	D24_D25_Cul	Circular	1.5	80	0.013	Circular	92.38	92.51		0.001	0	0	0	0	0	D	0.013	
D25	D25	Circular	0	179	0.014	Natural	92.811	93.038	D25_XS	0.001	0	0	0	179	179	D	0.014	
	D25_D26_Cul	Circular	1.25	37	0.012	Circular	92.99	92.85	D00.10	0.001	0	0	0	0	0	D	0.012	
D26 D26.1	D26 D26.1	Circular	0	132.5 25	0.014	Natural Natural	94.114 92.782	92.814	D26_XS D26.1_XS	0.001	0	0	0	25	0 25	D	0.014	
	D26.1 D26.1_D28_Cul		2	25 82	0.014	Circular	92.782	93.082 89.34	12/0.1_XS	0.001	0	0	0	25	25	D	0.014	
D26.1_D28_CU D27	D26.1_D28_CU D27	Circular	0	36	0.024	Natural	93.435	92.97	D27_XS	0.001	0	0	0	0	0	D	0.024	
	D27 D27_D28.1_Cul		2	63	0.014	Circular	93.433	92.97	01/05	0.001	0	0	0	0	0	D	0.014	
D28	D27_D28.1_Cul D28	Circular	0	15	0.024	Natural	92.157	92.44	D28_XS	0.001	0	0	0	15	15	D	0.024	
D28	D28	Circular	0	463	0.014	Natural	92.437	92.092	D28_X3 D28.1_XS	0.001	0	0	0	463	463	D	0.014	
	D28_D29_Cul	Circular	2	123	0.009	Circular	91.05	91.019		0.001	0	0	0	0	0	D	0.009	
D29	D29	Circular	0	62	0.014	Natural	92.47	92.253	D29_XS	0.001	0	0	0	62	62	D	0.014	
	D29_D30_Cul	Circular	2	122	0.009	Circular	91.51	91.42		0.001	0	0	0	0	0	D	0.009	
		Circular	0	197.91	0.014	Natural	92.9	92.7	Stella_A_S_XS_O240	0.001	0	0	0	710.8	710.8	D	0.014	
D3	D3																	
D3 D3.1	D3	Circular	0	512.89	0.014	Natural	92.21	91.2	Stella_A_S_XS_O240	0.001	0	0	0	710.8	710.8	D	0.014	

PROPOSED CULVERT LINK INPUT DATA



Conduit

iamete







PROPOSED CULVERT LINK INPUT DATA

						Horizo						
	Upstrea					ntal	Cross-			User-		
	m	Downstre	Natural			Distorti	section		Right	defined		
	Invert	am Invert	Section			on	Identifica		Channel	Conduit		
	Elevatio	Elevation	Shape GLDB	Initial Water Surface		Factor	tion	Left Channel	Length	using	Rough	
e0	n ft	ft	Reference 0	Slope 0	Shift ft	ft	Number 0	Length ft	ft	GLDB	ness	Roughness
3	Circular	91.46	91.52		0.001	0	0	0	0	0	0	0.013
4	Natural	92.14	91.886	D31_XS	0.001	0	0	0	113	113	0	0.014
4	Natural	92.6	92.4	Stella_A_S_XS_O240	0.001	0	0	0	202.8	202.8	0	0.014
4	Natural	92.4	91.41	Stella_A_S_XS_O240	0.001	0	0	0	257.4	257.4	0	0.014
4	Natural	91.41	91.31	Stella_A_S_XS_O240	0.001	0	0	0	148.2	148.2	0	0.014
4	Natural	91.53	91.31	Stella_A_S_XS_O240	0.001	0	0	0	77.7	77.7	0	0.014
4	Natural	91.99	91.63	Stella_A_S_XS_O240	0.001	0	0	0	125.5	125.5	0	0.014
4	Natural	92.99	92.09	Stella_A_S_XS_O240	0.001	0	0	0	213	213	0	0.014
3	Circular	92.58	92.73		0.001	0	0	0	0	0	0	0.013
4	Natural	92.73	92.703	D7_XS	0.001	0	0	0	1496.31	1496.31	0	0.014
4	Natural	92.5	92.354	D7.1_XS	0.001	0	0	0	60	60	0	0.014
4	Natural	92.703	92.5	D7_XS	0.001	0	0	0	772.695	772.695	0	0.014
4	Natural	92.14	91.503	D8_XS	0.001	0	0	0	160	160	0	0.014
4	Natural	91.503	91.392	DA ⁷²	0.001	U	U	U	/4	74	U	0.014
4	Natural	94.99	94.89	Cotton	0.001	0	0	0	33	33	0	0.014
5	Special	95.07	95		0.001	0	0	0	0	0	0	0.015
4	Natural	89.274	89.15	Out_1.1_XS	0.001	0	0	0	37.995	37.995	0	0.014
4	Natural	89.151	88.95	Out 1.1 XS	0.001	0	0	0	0	0	0	0.014
4	Natural	95.45	95.3	Stella A N XS O2 40'	0.001	0	0	0	32.83	32.83	0	0.014
4	Natural	95.45	95.07	Stella_A_N_XS_O2_40'	0.001	0	0	0	170.42	170.42	0	0.014
4	Natural	95.85	94.99	Stella_A_S_XS_O240	0.001	0	0	0	249.6	249.6	0	0.014
3	Circular	95.3	95	Stella_A_N_XS_O2_35'	0.001	0	0	0	102	102	0	0.013
i.	rapezoida	98.16	98		0.001	0	0	0	0	0	0	0.04
5	ectangula	90.3	90		0.001	0	0	0	0	0	0	0.015
3	Circular	89.2	89.1		0.001	0	0	0	0	0	0	0.013
4	Circular	89.43	89.2		0.001	0	0	0	0	0	0	0.024
4	Circular	89.44	89.43		0.001	0	0	0	0	0	0	0.024
4	Circular	89.44	89.26		0.001	0	0	0	0	0	0	0.024
4	Natural	87.31	87.28	D_Out_2.2	0.001	0	0	0	33	33	0	0.014
3	Circular	91.2	91.1		0.001	0	0	0	0	0	0	0.013
4	Circular	91.16	90.85		0.001	0	0	0	0	0	0	0.024
-												

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

	PROJECT TITL		
	FROM C	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
ying	SHEET DESCR	RIPTION: PROPOSED CULVERT INPUT DATA	
6/18/24	DRAWN BY: NS		DATE: 6/18/24
,, 10, 21	CK'D BY: MJ	SCALE: N/A	SHEET NO: 72 / 133

EXISTING CULVERT LINK INPUT DATA

EXISTING	CULVERT	NODE	INPUT	DATA	

		Ground			
Name	Node Name 0	Elevation (Spill Crest) ft	Invert Elevation ft	User Inflow Hydrograph	Ponding Type
Node653	Node653	92.94	89.44	0.17	Allowed
Node652	Node652	93.38	89.43	0.17	Allowed
Node650	Node650	93.45	89.2		Allowed
D31_US	D31_US	94.9	91.52	0.17	Allowed
030_05	D30_DS	94.939	91.46	0.17	Allowed
D27_DS	D27_DS	95	92.4		Allowed
Out_1.1	Out_1.1	95	88.95		Allowed
D26_US	D26_US	95.09	94.114		Allowed
D27_US	D27_US	95.09	93.435	0.17	Allowed
D30 US	D30 US	95.187	91.42		Allowed
D29_DS	D29_DS	95.2	91.51		Allowed
D29_US	D29_US	95.213	91.019	0.17	Allowed
D28_DS	D28_DS	95.22	91.05		Allowed
Out_1	Out_1	95.269	89.274		Allowed
Out_2	Out_2	95.28	87.31	0.17	Allowed
Out_2.2	Out_2.2	95.28	87.21		None
D16.2_DS	D16.2_DS	95.39	87.33		Allowed
Out3.1	Out3.1	95.407	91.93	0.17	Allowed
D31_DS	D31_DS	95.52	91.16	0.17	Allowed
D28_US	D28_US	95.691	90.6		Allowed
D25 US	D25 US	95.71	92.811		Allowed
D7.1_US	D7.1_US	95.725	92.5		Allowed
D24_US	D24_US	95.769	92.51		Allowed
Out_4	Out_4	95.838	91.39		Allowed
D25_DS	D25_DS	95.91	92.38		Allowed
D26.1_US	D26.1_US	95.911	89.26		Allowed
D26_D5	D26_D5	95.911	92.99	0.17	Allowed
D7_US.1	D7_US.1	95.928	92.578		Allowed
D22_US	D22_US	95.94	92.46		Allowed
D8_DS	D8_DS	95.949	90.85		Allowed
D28.1_US	D28.1_US	95.971	92.437	0.17	Allowed
D7 DS	D7 DS	96	91.67		Allowed
D8_US	D8_US	96	91.52		Allowed
D24_DS	D24_DS	96.01	92.01		Allowed
D23_US	D23_US	96.03	89.2		Allowed
Node651	Node651	96.03	89.1		Allowed
D22_DS	D22_DS	96.11	83.49		Allowed
D20_US	D20_U5	96.24	89.5		Allowed
D19_US	D19_US	96.31	92.594		Allowed
D7_US	D7_US	96.32	92.73		Allowed
D13_US	D13_US	96.366	92.84	0.17	Allowed
D18_US	D18_US	96.37	92.73		Allowed
Out 3	Out 3	95.381	90.113		Allowed
D6_DS	D6_DS	95.419	92.58		Allowed
D6_US	D6_US	96.43	92.89		Allowed
D15_US	D15_US	96.44	92.7		Allowed
D18_DS	D18_DS	95.451	94.389		Allowed
D23_DS	D23_DS	95.496	92.79		Allowed
D16_DS	D16_D5	95.517	92.73		Allowed
D3_DS	D3_DS	96.524	93.06		Allowed
D14_US	D14_US	96.56	91.9	0.17	Allowed
D5_US	D5_US	95.566	92.97		Allowed
D4.1_DS	D4.1_DS	96.58	90.7		Allowed
D16.1_US	D16.1_US	95.587	90.65		Allowed
D16_US	D16_US	95.587	92.42		Allowed
D21_DS	D21_DS	96.59	89.26		Allowed
D15_DS	D15_DS	96.59	92.1	0.17	Allowed
D17_US	D17_US	95.645	93.06		Allowed
D17_DS	D17_DS	95.645	92.25		Allowed
D5_DS	D5_DS	95.682	92.87		Allowed
D2_DS	D2_DS	96.756	91.45	0.17	Allowed
D3_US	D3_US	95.767	93.66		Allowed
D13.1_US	D13.1_US	96.78	91.1		Allowed
D14_DS	D14_DS	97.078	92.67		Allowed
D12_DS	D12_DS	97.098	93.2		Allowed
D11_DS	D11_DS	97.134	93.95		Allowed
D13_DS	D13_DS	97.435	92.89		Allowed
D4.1_US	D4.1_US	97.519	92.04		Allowed
D12_US	D12_US	97.625	93.91	0.17	Allowed
D4_US	D4_US	97.745	92.73		Allowed
D21_US	D21_US	97.803	87.73		Allowed
D11_US	D11_US	97.98	94.848		Allowed
D2B_US	D2B_US	98.785	93.671		Allowed
D4_DS	D4_DS	98.816	92.61		Allowed
D1_DS	D1_DS	100	95.26	0.17	Allowed
D1_US	D1_03	100	95	0.17	Allowed
D1_US	D1_US	100.113	95.748	0.17	Allowed
D10_DS	D10_DS	100.229	95.08	0.17	Allowed
	010_03	100.223	55.00	0.17	- COLONE CO

	Conduit Design	Diameter (Height)		Roughness		Upstream Invert Elevation	Downstre am Invert Elevation	Natural Section Shape GLDB	Initial Water Surface	Vertical	Horizontal Distortion	Cross- section Identification	Left Channel	Right Channel	User- defined Conduit using	
Link Name 0	Option 0	(Height)	Length ft	0	Shape 0	ft	ft	Reference 0	Slope 0	Shift ft	Factor ft	Number 0	Length ft	Length ft	GLDB	Roug
D7_D8_Cul	Circular	2	45	0.013	Circular	91.84	91.52		0.001	0	0	0	0	0	0	0.0
D7_D8_Cul	Circular	2	45	0.013	Circular	91.67	91.57		0.001	0	0	0	0	0	0	0.0
Rd Cross3	Circular	2.5	104	0.009	Circular	90.7	90.65		0.001	0	0	0	0	0	0	0.0
Rd_Cross3 Rd_Cross3	Circular Circular	2.5	104	0.009	Circular	90.75	90.77		0.001	0	0	0	0	0	0	0.0
D16.2_Out-2	Circular	2.5	104 99	0.009	Circular Circular	90.89 87.35	90.86 87.44		0.001	0	0	0	0	0	0	0.
D16.2_Out-2	Circular	3	99	0.009	Circular	87.33	87.31		0.001	0	0	0	0	0	0	0.
Rd_Cross4	Circular	2	83	0.013	Circular	92.89	92.83		0.001	0	0	0	0	0	0	0.
Rd_Cross4	Circular	2	83	0.013	Circular	92.87	92.65		0.001	0	0	0	0	0	0	n.
022_D23_Cul	Circular	1	24	0.024	Circular	92.79	92.46		0.001	0	0	0	0	0	0	0.
021_D22_Cul	Circular	3	81	0.024	Circular	88.49	87.83		0.001	0	0	0	0	0	0	0.
D1	Circular	0	550	0.014	Natural	95.748	95.33	D1_XS	0.001	0	0	0	550	550	0	0.
D10	Circular	0	906	0.014	Natural	95.18	95.131	D10_XS	0.001	0	0	0	906	906	0	0.
10_D11_Cul	Circular	1.25	60	0.013	Circular	95.08	94.89		0.001	0	0	0	0	0	0	0
D11 D12_Cul	Circular Circular	2	383 131	0.014	Natural	94.848 93.95	94.03 93.91	D11_XS	0.001	0	0	0	383	383 383	0	0
D12_Cui	Circular	0	1106	0.013	Rectangula Natural	94.356	93.829	D12_XS	0.001	0	0	0	1106	1106	0	0
12_D13_CUI	Circular	1.5	46	0.009	Circular	93.2	92.84	012_73	0.001	0	0	0	0	0	0	0
D13	Circular	0	170	0.014	Natural	93.675	91.296	D13_XS	0.001	0	0	0	170	170	0	0.
D13.1	Circular	0	1174	0.014	Natural	92.601	93.268	D13.1_XS	0.001	0	0	0	1174	1174	0	0.
13_D14_Cul	Circular	2	118	0.002	Circular	92.89	92.86		0.001	0	0	0	0	0	0	0.
13_Out_CUI	Circular	3.5	81	0.009	Circular	91.1	89.67		0.001	0	0	0	0	0	0	0.
D14	Circular	0	214	0.014	Natural	93.128	93.657	D14_XS	0.001	0	0	0	214	214	0	0
14_D15_Cul	Circular	2	84	0.009	Circular	92.67	92.7		0.001	0	0	0	0	0	0	0
D15	Circular	0	158	0.014	Natural	93.458	93.301	D15_XS	0.001	0	0	0	158	158	0	0.
15_D16_Cul	Circular	2	103	0.009	Circular	92.1	92.42		0.001	0	0	0	0	0	0	0.
D16 D16.1	Circular	0	37 68	0.014	Natural Natural	93.185 93.62	91.898 91.898	D16_X3 D16.1_XS	0.001	0	0	0	37 68	37 68	0	0
D16.1 D16.2	Circular	0	43	0.014	Natural	93.62	91.898	D16.1_XS D16.2_XS	0.001	0	0	0	43	43	0	0
16_D17_Cul	Circular	2	82	0.014	Circular	93.06	92.73	D10.2_V3	0.001	0	0	0	43	45	0	0
D17	Circular	0	218	0.009	Natural	94.236	94.284	D17_XS	0.001	0	0	0	218	218	0	0
17_D18_Cul	Circular	2.5	83	0.009	Circular	92.73	92.25		0.001	0	0	0	0	0	0	0
D1/_Out-2	Circular	U	115	0.005	Natural	94.463	92.8	D1/_Out-2_XS	0.001	U	U	U	115	115	U	U
D18	Circular	0	278	0.014	Natural	94.389	94.028	D18_XS	0.001	0	0	0	278	278	0	0
D19	Circular	0	129	0.014	Natural	92.594	91.93	D19_XS	0.001	0	0	0	129	129	0	0
D1_D2_Cul	Circular	1.5	63.19	0.013	Circular	95.26	95		0.001	0	0	0	0	0	0	0
D2	Circular	0	806.98	0.014	Natural	95	94.259	D2_XS	0.001	0	0	0	806.98	806.98	0	0
D20	Circular	0	400	0.014	Natural	89.956	90.113	D20_XS	0.001	0	0	0	400	400	0	0.
20_D21_Cul	Circular	2.5	81	0.024	Circular	89.26	89.5		0.001	0	0	0	0	0	0	0.
D21	Circular	0	251	0.014	Natural	91.024	89.811	D21_XS	0.001	0	0	0	251	251	0	0.
D22	Circular	0	78	0.014	Natural	93.12	89.034	D22_XS	0.001	0	0	0	78	78	0	0.
D23 D23_D24_Cul	Circular Circular	0	89 67	0.014	Natural Circular	92.599 92.01	93.235 91.12	D23_XS	0.001	0	0	0	89 0	89 0	0	0
D24_Cur	Circular	0	199	0.003	Natural	93.06	92.591	D24_XS	0.001	0	0	0	199	199	0	0
24_D25_Cul	Circular	1.5	80	0.013	Circular	92.38	92.51	024_10	0.001	0	0	0	0	0	0	0
D25	Circular	0	179	0.014	Natural	92.811	93.038	D25_XS	0.001	0	0	0	179	179	0	0.
25_D26_Cul	Circular	1.25	37	0.012	Circular	92.99	92.85		0.001	0	0	0	0	0	0	0
D26	Circular	0	132.5	0.014	Natural	94.114	92.814	D26_XS	0.001	0	0	0	132.5	132.5	0	0.
D26.1	Circular	0	25	0.014	Natural	92.782	93.082	D26.1_XS	0.001	0	0	0	25	25	0	0.
26.1_D28_Cul	Circular	2	82	0.024	Circular	90.6	89.34		0.001	0	0	0	0	0	0	0
D27	Circular	0	36	0.014	Natural	93.435	92.97	D27_XS	0.001	0	0	0	36	36	0	0.
27_D28.1_Cul	Circular	2	63	0.024	Circular	92.4	92.44	D28 VC	0.001	0	0	0	0	0	0	0
D28 D28.1	Circular Circular	0	15 463	0.014	Natural Natural	92.157 92.437	92.437 92.092	D28_XS D28.1_XS	0.001	0	0	0	15 463	15 463	0	0
28_D29_Cul	Circular	2	123	0.014	Circular	91.05	92.092	D20.1_A3	0.001	0	0	0	463	463	0	0
D29_Cui	Circular	0	62	0.009	Natural	91.05	92.253	D29_XS	0.001	0	0	0	62	62	0	0
29_D30_Cul	Circular	2	122	0.009	Circular	91.51	91.42	225_10	0.001	0	0	0	0	0	0	0
D2B	Circular	0	1348.02	0.014	Natural	94.259	91.45	D2_XS	0.001	0	0	0	1348.02	1348.02	0	0
D3	Circular	0	1055	0.014	Natural	93.66	93.417	D3_XS	0.001	0	0	0	1055	1055	0	0
D30	Circular	0	232	0.014	Natural	92.458	92.21	D30_XS	0.001	0	0	0	232	232	0	0
30_D31_Cul	Circular	2	55	0.013	Circular	91.46	91.52		0.001	0	0	0	0	0	0	0
D31	Circular	0	113	0.014	Natural	92.14	91.886	D31_XS	0.001	0	0	0	113	113	0	0
D3_D4_Cul D4	Circular	2	33	0.009	Circular	93.06	92.73	D4 KC	0.001	0	0	0	0	0	0	0
	Circular	0	213	0.014	Natural	93.38	93.154	D4_XS	0.001	0	0	0	213	213	0	0
D4.1 D4.2	Circular Circular	0	425 173	0.014	Natural Natural	93.154 91.545	91.545 93.781	D4.1_XS D4.2_XS	0.001	0	0	0	425 173	425 173	0	0
D4.2 D4_D5_Cul	Circular	2	82	0.014	Circular	91.545	93.781	U4.2_A3	0.001	0	0	0	0	0	0	0
D5	Circular	0	192	0.005	Natural	93.492	93.608	D5_XS	0.001	0	0	0	192	192	0	0
D5_D6_Cul	Circular	1.5	82	0.009	Circular	92.87	92.89		0.001	0	0	0	0	0	0	0
D6	Circular	0	238	0.014	Natural	93.515	92.918	D6_XS	0.001	0	0	0	238	238	0	0
D6_D7_Cul	Circular	2	82	0.024	Circular	92.58	92.73		0.001	0	0	0	0	0	0	0
D7	Circular	0	1496.305	0.014	Natural	92.73	92.703	D7_XS	0.001	0	0	0	1496.305	1496.305	0	0
D7.1	Circular	0	60	0.014	Natural	92.5	92.354	D7.1_XS	0.001	0	0	0	60	60	0	0.
D7.2	Circular	0	772.695	0.014	Natural	92.703	92.5	D7_XS	0.001	0	0	0	772.695	772.695	0	0
D8	Circular	0	160	0.014	Natural	92.14	91.503	D8_XS	0.001	0	0	0	160	160	0	0
D9	Circular	0	74	0.014	Natural	91.503	91.392	D9_XS	0.001	0	0	0	74	74	0	0.
D_Out1.1	Circular	0	100	0.014	Natural	89.274	88.95	D_Out1.1_XS	0.001	0	0	0	0	0	0	0
Link615	Circular	2.5	33	0.013	Circular	89.2	89.1		0.001	0	0	0	0	0	0	0
Link616	Circular	2.5	168	0.024	Circular	89.43	89.2		0.001	0	0	0	0	0	0	0
Link617	Circular	2.5	129	0.024	Circular	89.44	89.43		0.001	0	0	0	0	0	0	0
Link618 Out_2.2L	Circular Circular	2.5	214 33	0.024	Circular	89.44 87.31	89.26 87.28	D_Out_2.2	0.001	0	0	0	33	0 33	0	0.
Rd_Cross1	Circular	2.5	33 84	0.014	Natural Circular	87.31 91.45	91.23	0_001_2.2	0.001	0	0	0	33	33	0	0
	Circular	1.5	84	0.009	Circular	91.45	91.23		0.001	0	0	0	0	0	0	0
	uncular															0
Rd_Cross2 Rd_Cross5	Circular	2	41	0.024	Circular	91.16	90.85		0.001	0	0	0	0	0	0	

REVISIONS DATE NA	AME		TATE OF	r.g. miller
		FORT BEND COUNTY		r.g. miller
		FORT DEND COURT		DCCM
		ENGINEERING DEPARTMENT		R.G. Miller Engineers, Inc. TxEng F - 487
		ENGINEERING DEPARTMENT		16340 Park Ten Place, Ste 350 Houston, TX 77084
			BEND COULD	713.461.9600 rgmiller.com



NO.

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND. TEXAS", EXHIBIT 8A.

	PROJECT TITL	E: STELLA ROAD	
		OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
ying	SHEET DESCR	RIPTION: EXISTING CULVERT INPUT DATA	
6/18/24	DRAWN BY: NS		DATE: 6/18/24
,, 10, 21	CK'D BY: MJ	SCALE: N/A	SHEET NO: 73 / 133

PROPOSED CULVERT NODE 100-YR RESULT

Name Basin1_Opt2	Scenario 100-Year Storm	Node Name 0 Basin1_Opt2	Max Water Elevation ft 94.825	Max Volume ft^3 1381275.708	Node Inflow ft^ 656456.154
D10_DS	100-Year Storm	D10_DS	96.711	36.456	0
D10_US	100-Year Storm	D10_US	96.711	21.501	100815.313
D11_DS	100-Year Storm	D11_DS	96.731	35.451	0
D11_US	100-Year Storm	 D11_US	96,731	36.078	0
D12 DS	100-Year Storm	D12 DS	95.214	42.529	0
D12_US	100-Year Storm	D12_US	96.734	35.99	114980.557
D12_US.1	100-Year Storm	D12_US.1	96.734	38.377	0
D12_US.1.1	100-Year Storm	D12_US.1.1	95.215	32.351	0
D13.1_US	100-Year Storm	D13.1_US	95.214	51.698	0
D13_DS	100-Year Storm	D13_DS	96.086	35.004	0
D13_05.1	100-Year Storm	D13_DS.1	95.214	37.147	0
D13_DS.2	100-Year Storm	D13_DS.2	96.086	30.481	0
D13_US	100-Year Storm	D13_US	95.214	43.782	0
D13_US.1	100-Year Storm	D13_US.1	95.214	47.049	0
D14_DS	100-Year Storm	D14_DS	96.084	35.365	138723.595
D14_US	100-Year Storm	D14_US	96.085	37.139	0
D15_D5	100 Year Storm	D15_D5	95.967	52.117	0
D15_US	100-Year Storm	D15_US	95.968	35.158	0
D16.1_US	100-Year Storm	D16.1_US	95.768	57.399	0
D16.2_DS	100-Year Storm	D16.2_DS	95.766	2338.4	0
D16_DS	100-Year Storm	D16_DS	95.768	57.274	0
D16_US	100-Year Storm	D16_US	95.768	50.865	0
DT1_D2	100-rear Storm	DT1_D2	95.77	49.136	Ŭ
D17_US	100-Year Storm	D17_US	95.77	54.791	0
D18_DS	100-Year Storm	D18_DS	95.771	38.215	47104.329
D18_US	100-Year Storm	D18_US	95.771	47.89	0
D19_US	100-Year Storm	D19_US	93.39	10.006	0
D1_DS	100-Year Storm	D1_DS	97.141	39.594	0
D1_US	100-Year Storm	D1_US	97.141	29.166	0
D20_US	100-Year Storm	D20_US	92.192	33.832	107948.85
D21_DS	100-Year Storm		93.901	59.078	0
D21_US	100-Year Storm	D21_US	94.124	80.417	0
D22_DS	100-Year Storm	D22_D5	94.93	83.779	0
D22_US	100-Year Storm	D22_US	95.245	34.997	0
D23_DS	100-Year Storm	D23_DS	95.265	31.176	0
D23_US	100-Year Storm	D23_US	95.285	76.736	261742.407
D24_DS	100-Year Storm	D24_DS	95.211	40.225	0
D24_US	100-Year Storm	D24_US	95.211	33.956	0
D25_DS	100-Year Storm	D25_DS	95.218	35.657	0
D25 US	100-Year Storm	D25 US	95.218	30.245	0
D26.1_US	100-Year Storm	D26.1_US	95.288	75.752	0
D26_DS	100-Year Storm	D26_DS	95.288	28.88	0
D26_US	100-Year Storm	D26_US	95.288	1108.697	0
D27_DS	100-Year Storm	D27_DS	94.739	29.393	0
D27_US	100-Year Storm	D27_US	94.739	16.387	0
D28 1_US	100-Year Storm	D28.1_LK	94 738	28 918	66905 239
D28_DS	100-Year Storm	D28_DS	94.725	46.186	0
D28_US	100-Year Storm	D28_US	94.74	52.024	0
D29_DS	100-Year Storm	D29_DS	94.347	35.646	0
D29_US	100-Year Storm	D29_US	94.353	41.896	0
D2_DS	100-Year Storm	D2_DS	96.718	69.344	2221285.356
D2_US	100 Year Storm	D2_US	97.14	40.842	98049.784
D2_US.1	100-Year Storm	D2_US.1	96.718	1285.002	0
D2_US.2	100-Year Storm	D2_US.2	97.137	39.8	107282.869
D2B_US	100-Year Storm	D2B_US	96.737	38.413	115393.415
D2B_US.1	100-Year Storm	D2B_US.1	96.734	39.636	115393.415
D30_DS	100-Year Storm	D30_D5	93.885	30.477	0
D30_US	100-Year Storm	D30_US	93.973	32.087	0
D31_DS	100-Year Storm	D31_DS	93.342	27.413	0
D31_US	100-Year Storm	D31_US	93.536	25.335	0
D3_DS	100-Year Storm	D3_DS	96.259	44.717	137463.164
D3_DS.1	100-Year Storm	D3_DS.1	96.716	1191.691	57717.88
D3_US	100-Year Storm	D3_US	96.277	42.439	0
D4.1_DS	100-Year Storm	D4.1_D5	96.1	60.185	0
D4.1_US	100-Year Storm	D4.1_US	96.109	46.608	0
D4.1_US.1	100-Year Storm	D4.1_US.1	96.102	58.964	379091.294
D4_DS	100-Year Storm	D4_DS	96.1	57.421	0
D4_US	100-Year Storm	D4_US	96.129	44.347	0
D5_DS	100-Year Storm	D5_DS	96.1	51.649	36543.347
D5_US	100-Year Storm	D5_US	96.1	56.172	0
D6_DS	100-Year Storm	D6_DS	96.1	44.237	0
D6_US	100-Year Storm	D6_US	96.1	50.394	0
D7.1_US	100-Year Storm	D7.1_US	93.303	10.095	0
D7_DS	100-Year Storm	D7_DS	93.124	18.272	0
D7_US	100-Year Storm	D7_US	93.848	14.05	101348.761
D7_US.1	100-Year Storm	D7_US.1	93.846	15.92	66303.446
D8_DS	100-Year Storm	D8_DS	92.833	24.914	0
D8_US	100-Year Storm	D8_US	93.097	19.813	0
D_Co_Out	100-Year Storm	D_Co_Out	95.556	8.37	0
DW1N_Out	100-Year Storm	DW1N_Out	96.03	12.068	0
DW15_Out	100-Year Storm	DW15_Out	95.915	11.628	0
DW2N_US	100-Year Storm	DW2N_US	96.663	17.132	0
DW2N_US.1	100-Year Storm	DW2N_US.1	96.647	15.038	113943.611
DW25_DS	100-Year Storm	DW2S_DS	95.915	8.487	0
Node650	100-Year Storm	Node650	89.2	0	0
Node651	100-Year Storm	Node651	89.1	0	0
Nodc652	100 Year Storm	Node652	95.449	34623.146	0
Node653	100-Year Storm	Node653	95.853	87123.784	476578.977
Out3.1	100-Year Storm	Out3.1	92.426	6.233	0
Out_1	100-Year Storm	Out_1	90.374	13.827	0
Out_1.1	100-Year Storm	Out_1.1	89.757	10.138	0.041
Out_1.2	100-Year Storm	Out_1.2	90.349	15.069	0
Out_112	100-Year Storm	Out_11	88.679	17.206	0
	100-Year Storm	Out_2.2	88.211	12.582	0
Out 2.2					
Out_2.2 Out 3	100-Year Storm	Out 3	91.257		
Out_2.2 Out_3 Out_4	100-Year Storm 100-Year Storm	Out_3 Out_4	91.257 92.279	14.399 11.173	0.021

DATE NAME

FORT BEND COUNTY

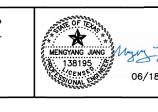
ENGINEERING DEPARTMENT

aini_opt2 10-YearStorm Basini_opt2 93.648 912239.284 2 D10_DS 10-YearStorm D10_DS 95.974 27.195 D10_US 10-YearStorm D10_DS 95.979 12.302 D11_US 10-YearStorm D11_US 95.921 25.264 D11_US 10-YearStorm D11_US 95.921 25.901 D12_US 10-YearStorm D12_US 93.899 25.602 D12_US.1 10-YearStorm D12_US.1 95.912 28.602 D13_US.1 10-YearStorm D12_US.1 93.899 35.173 D13_US.1 10-YearStorm D13_US 93.899 20.622 D13_US.1 10-YearStorm D13_DS 94.658 12.57 D13_DS.1 0-YearStorm D13_US 93.899 20.622 D13_DS.1 10-YearStorm D13_US 93.899 20.622 D13_DS.1 10-YearStorm D13_US 93.899 20.622 D13_DS.1 10-YearStorm D14_DS 94.658	de Inflow ft*3 384337.348 0 51371.947 0 0 59597.205 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
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D11_DS 10-YearStorm D11_US 95.921 25.264 D11_US 10-YearStorm D11_US 95.921 25.901 D12_DS 10-YearStorm D12_US 95.921 25.901 D12_US 10-YearStorm D12_US 95.912 25.659 D12_US.1 10-YearStorm D12_US 95.912 28.602 D12_US.1 10-YearStorm D12_US.1 93.899 35.173 D13_DS 10-YearStorm D13_US 94.658 20.076 D13_DS.1 0-YearStorm D13_DS.1 93.899 35.173 D13_DS.1 0-YearStorm D13_DS.2 94.658 12.537 D13_DS.1 0-YearStorm D13_US 93.899 30.524 D14_DS 10-YearStorm D13_US 93.899 30.524 D14_DS 10-YearStorm D14_US 94.657 17.424 D14_DS 10-YearStorm D14_US 94.657 17.424 D14_DS 10-YearStorm D15_US 94.641 18.15	0 0 55597.205 0 0 0 0 0 0 0 0 0 0 0 0 0
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D12_D5 10-YearStorm D12_DS 99.899 26.002 D12_US 10-YearStorm D12_US.1 95.912 28.602 D12_US.1 10-YearStorm D12_US.1 95.912 28.602 D12_US.1.1 10-YearStorm D12_US.1.1 95.912 28.042 D12_US.1.1 0-YearStorm D13_US.1 0.YearStorm D13_US.1 0.YearStorm D13_US.1 93.899 35.173 D13_DS.1 10-YearStorm D13_DS.2 94.658 12.537 D13_DS.2 0.4658 12.537 D13_DS.1 10-YearStorm D13_US.1 93.899 30.524 D14_DS 10-YearStorm D13_US.1 0.YearStorm D13_US.1 93.899 30.524 D14_DS 10-YearStorm D13_US.1 0.YearStorm D14_DS 94.657 17.424 D14_US 10-YearStorm D14_US 94.657 17.424 D14_US 10-YearStorm D16_US 10-YearStorm D16_US 10-YearStorm D16_US 10-YearStorm D16_US 94.585 42.408 D16_US 10-YearStorm </td <td>0 59597.205 0 0 0 0 0 0 69840.797 0 69840.797 0 69840.797 0 0 0 0 0 0 0 0 2 4905.078 0 0 0 0 0 0 0 0 0 0 55417.52</td>	0 59597.205 0 0 0 0 0 0 69840.797 0 69840.797 0 69840.797 0 0 0 0 0 0 0 0 2 4905.078 0 0 0 0 0 0 0 0 0 0 55417.52
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D12_US.1 10-YearStorm D12_US.1 95.912 28.042 D13_US.1 10-YearStorm D12_US.1 93.899 15.824 D13.1_US 10-YearStorm D13.1_US 93.899 35.173 D13_DS 10-YearStorm D13.1_US 93.899 35.173 D13_DS.1 0-YearStorm D13.DS 94.658 20.076 D13_DS.1 0-YearStorm D13_DS.2 94.658 12.537 D13_US.1 0-YearStorm D13_US.9 93.899 27.257 D13_US.1 0-YearStorm D13_US.9 94.657 17.424 D14_US 10-YearStorm D14_US 94.657 19.194 D15_US 10-YearStorm D15_US 94.614 18.15 D16_US 10-YearStorm D16_US 94.585 42.532 D16_US 10-YearStorm D16_US 94.585 42.408 D16_US 10-YearStorm D16_US 94.585 36 D16_US 10-YearStorm D16_US 94.586 39.908 <	0 0 0 0 0 669640,797 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Dial_US 10:YearStorm Dial_DS 93.899 35.173 Dia_DS 10:YearStorm Dial_DS 94.658 20.076 Dia_DS.1 10:YearStorm Dial_DS.2 94.658 20.076 Dia_DS.1 10:YearStorm Dial_DS.2 94.658 12.527 Dia_US 10:YearStorm Dial_DS.1 93.899 20.622 Dia_US 10:YearStorm Dial_US 94.658 12.537 Dia_US 10:YearStorm Dial_US 93.899 20.527 Dia_US 10:YearStorm Dial_US 94.657 17.424 Di4_US 10:YearStorm Di4_US 94.657 19.194 Di5_US 10:YearStorm Di4_US 94.657 19.194 Di5_US 10:YearStorm Di5_US 94.585 42.532 Di6_US 10:YearStorm Di6_US 94.585 42.408 Di6_US 10:YearStorm Di6_US 94.585 36 Di7_US 10:YearStorm Di6_US 94.587 23.008	0 0 0 669840.797 0 0 0 0 0 0 0 24905.078 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
D13_DS 10-Year Storm D13_DS.1 93,658 20.076 D13_DS.1 10-Year Storm D13_DS.2 94,658 21.537 D13_DS.2 10-Year Storm D13_DS.2 94,658 12.537 D13_DS.2 10-Year Storm D13_DS.2 94,658 12.537 D13_US.1 0-Year Storm D13_US.1 93,899 20,524 D14_DS 10-Year Storm D14_US 94,657 17,424 D14_DS 10-Year Storm D14_US 94,657 19,194 D15_DS 10-Year Storm D15_US 94,614 18,15 D15_DS 10-Year Storm D16,1_US 10-Year Storm D16,2_DS 94,583 42,532 D16_US 10-Year Storm D16,2_DS 94,585 36 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,746 10,74	0 0 0 69840,797 0 0 0 0 0 0 0 24905,078 0 24905,078 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
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D13_DS.2 10-Year Storm D13_DS.2 93.658 12.537 D13_US.1 0-Year Storm D13_US 93.899 27.257 D13_US.1 0-Year Storm D13_US 93.899 30.524 D14_US 10-Year Storm D14_US 94.657 17.424 D14_US 10-Year Storm D14_US 94.657 19.194 D14_US 10-Year Storm D14_US 94.657 19.194 D15_US 10-Year Storm D15_US 94.614 18.15 D16_US 10-Year Storm D16.1_US 94.585 42.532 D16_US 10-Year Storm D16.2_US 94.585 42.608 D16_US 10-Year Storm D16.2_US 94.585 36 D17_US 10-Year Storm D16.2_US 94.585 36 D17_US 10-Year Storm D17_US 94.586 39.098 D18_US 10-Year Storm D18_US 94.587 23.344 D18_US 10-Year Storm D19_US 10-Year Storm D19_US <td>0 0 69840.797 0 0 0 0 0 0 0 24905.078 0 0 0 24905.078 0 0 0 0 0 0 55417.52</td>	0 0 69840.797 0 0 0 0 0 0 0 24905.078 0 0 0 24905.078 0 0 0 0 0 0 55417.52
D13_US 10-Year Storm D13_US.1 93.899 27.257 D13_US.1 10-Year Storm D13_US.1 93.899 30.524 D14_DS 10-Year Storm D14_US 94.657 17.424 D14_US 10-Year Storm D14_US 94.657 17.424 D14_US 10-Year Storm D14_US 94.657 19.194 D15_US 10-Year Storm D15_US 94.614 18.15 D16_US 10-Year Storm D15_US 94.583 43.771 D16_DS 10-Year Storm D16_US 94.583 42.632 D16_DS 10-Year Storm D16_US 94.583 42.408 D16_US 10-Year Storm D16_US 94.585 36 D17_US 10-Year Storm D17_US 94.587 23.334 D18_US 10-Year Storm D18_US 94.587 33.008 D13_US 10-Year Storm D19_US 93.246 8.192 D1_DS 10-Year Storm D1_US 94.587 23.344	0 69840.797 0 0 0 0 0 0 0 0 0 24905.078 0 0 0 0 0 0 55417.52
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D15_DS 10.YearStorm D15_US 94.612 25.004 D15_US 10.YearStorm D15_US 94.614 18.15 D16_US 10.YearStorm D15_US 94.585 42.532 D16_US 10.YearStorm D16_L_DS 94.585 42.532 D16_DS 10.YearStorm D16_DS 94.585 42.408 D16_DS 10.YearStorm D16_DS 94.585 36 D17_DS 10.YearStorm D16_DS 94.585 36 D17_DS 10.YearStorm D17_US 94.586 39.908 D18_DS 10.YearStorm D18_US 94.587 23.334 D18_US 10.YearStorm D18_US 94.587 33.008 D19_US 10.YearStorm D19_US 93.246 8.192 D1_US 10.YearStorm D19_US 93.246 8.192 D1_US 10.YearStorm D19_US 93.246 8.192 D1_US 10.YearStorm D20_US 91.824 29.201 D1_US<	0 0 0 0 0 24905.078 0 0 0 0 55417.52
D15 US 10-Year Storm D15 US 94.614 18.15 D16.1_US 10-Year Storm D16.1_US 94.585 42.532 D16.2_DS 10-Year Storm D16.2_DS 94.583 43.771 D16.5_DS 10-Year Storm D16.1_DS 94.583 42.408 D16_DS 10-Year Storm D16_DS 94.585 36 D17_D3 10-Year Storm D16_US 94.585 36 D17_D3 10-Year Storm D17_US 94.587 33.34 D18_DS 10-Year Storm D18_DS 94.587 33.008 D18_DS 10-Year Storm D18_US 94.587 33.008 D19_US 10-Year Storm D19_US 93.246 8.192 D1_DS 10-Year Storm D1_US 94.587 33.008 D12_US 10-Year Storm D1_US 94.587 33.046 D10_US 10-Year Storm D1_US 94.454 21.057 D20_US 10-Year Storm D2_US 93.012	0 0 0 0 24905.078 0 0 0 0 55417.52
D16.1_US 10-Year Storm D16.1_US 94.585 42.532 D16.2_DS 10-Year Storm D16.2_DS 94.583 43.771 D16.0_S 10-Year Storm D16.0_S 94.585 42.408 D16_US 10-Year Storm D16_US 94.585 36 D17_US 10-Year Storm D16_US 94.585 36 D17_US 10-Year Storm D17_US 94.586 39.908 D18_US 10-Year Storm D18_US 94.587 23.334 D18_US 10-Year Storm D18_US 94.587 33.008 D19_US 10-Year Storm D18_US 94.587 33.008 D19_US 10-Year Storm D18_US 94.587 33.008 D10_US 10-Year Storm D19_US 93.246 8.192 D1_US 10-Year Storm D10_US 93.246 8.192 D1_US 10-Year Storm D20_US 91.824 29.201 D1_US 10-Year Storm D21_US 93.291 66.372	0 0 0 24905.078 0 0 0 0 0 0 0 0 0 0 0 55417.52
Di62_DS 10-YearStorm D16_DS 94.583 43.771 Di6_DS 10-YearStorm D16_DS 94.585 42.408 D16_US 10-YearStorm D16_DS 94.585 36 D17_US 10-YearStorm D16_US 94.585 36 D17_US 10-YearStorm D17_US 94.586 39.908 D18_US 10-YearStorm D18_US 94.587 23.334 D18_US 10-YearStorm D18_US 94.587 23.334 D18_US 10-YearStorm D18_US 94.587 33.008 D19_US 10-YearStorm D19_US 93.246 8.192 D1_US 10-YearStorm D19_US 94.587 33.008 D1_US 10-YearStorm D19_US 94.587 33.008 D1_US 10-YearStorm D19_US 94.587 33.008 D1_US 10-YearStorm D10_US 94.84 21.047 D20_US 10-YearStorm D21_US 93.012 66.327 D21_US	0 0 0 24905.078 0 0 0 0 0 0 0 0 55417.52
D16_DS 10-YearStorm D16_DS 94.585 42.408 D16_US 10-YearStorm D16_US 94.585 36 D17_DS 10-YearStorm D17_US 94.586 39.908 D17_US 10-YearStorm D17_US 94.587 23.334 D18_DS 10-YearStorm D17_US 94.587 23.334 D18_DS 10-YearStorm D18_US 94.587 23.334 D18_US 10-YearStorm D19_US 93.246 8.192 D1_DS 10-YearStorm D1_US 96.494 31.468 U1_US 10-YearStorm D2_US 90.4944 21.037 D20_US 10-YearStorm D2_US 90.4944 21.037 D20_US 10-YearStorm D21_US 93.012 66.372 D21_US 10-YearStorm D21_US 93.752 16.234 D23_DS 10-YearStorm D22_US 93.752 16.234 D23_DS 10-YearStorm D23_US 94.879 25.873 D	0 0 24905.078 0 0 0 0 55417.52
D16_US 10-YearStorm D16_US 94,585 36 D17_DS 10-YearStorm D17_DS 94,586 39,908 D17_US 10-YearStorm D17_US 94,586 39,908 D18_DS 10-YearStorm D17_US 94,586 39,908 D18_US 10-YearStorm D18_US 94,587 23,334 D19_US 10-YearStorm D19_US 93,246 8,192 D1_DS 10-YearStorm D1_DS 96,494 31,468 D1_US 10-YearStorm D1_DS 96,494 21,1047 D20_US 10-YearStorm D20_US 91,824 29,201 D21_US 10-YearStorm D20_US 93,246 60,327 D21_US 10-YearStorm D21_US 93,212 66,372 D22_US 10-YearStorm D22_US 93,752 16,234 D23_DS 10-YearStorm D23_US 94,849 25,873 D23_US 10-YearStorm D23_US 94,849 25,873 D	o 0 24905.078 0 0 0 0 0 55417.52
D17_US 10-YearStorm D17_US 94.586 39.908 D18_DS 10-YearStorm D18_DS 94.587 23.334 D18_DS 10-YearStorm D18_US 94.587 23.308 D19_US 10-YearStorm D18_US 94.587 33.008 D19_US 10-YearStorm D19_US 93.246 8.192 D1_DS 10-YearStorm D1_US 96.494 31.468 D2_US 10-YearStorm D1_US 96.494 21.047 D20_US 10-YearStorm D21_US 91.84 29.201 D21_US 10-YearStorm D21_US 93.216 66.372 D22_DS 10-YearStorm D22_US 93.721 66.327 D23_DS 10-YearStorm D23_US 94.849 25.873 D23_DS 10-YearStorm D23_US 94.849 25.873 D24_US 10-YearStorm D24_US 94.832 35.467 D24_US 10-YearStorm D24_US 94.832 29.189	0 24905.078 0 0 0 0 0 55417.52
D18_DS 10-Year Storm D18_US 94.587 23.334 D18_US 10-Year Storm D18_US 94.587 33.008 D19_US 10-Year Storm D19_US 93.246 8.192 D1_DS 10-Year Storm D1_D_US 96.494 31.468 D1_US 10-Year Storm D1_D_US 96.494 21.047 D20_US 10-Year Storm D20_US 91.824 29.201 D21_US 10-Year Storm D20_US 91.824 29.201 D21_US 10-Year Storm D21_US 93.012 66.372 D22_US 10-Year Storm D22_US 93.291 60.327 D22_US 10-Year Storm D22_US 93.752 16.234 D23_DS 10-Year Storm D23_US 94.849 25.873 D23_US 10-Year Storm D24_US 94.832 35.467 D24_US 10-Year Storm D24_US 94.832 29.189	24905.078 0 0 0 0 0 55417.52
D18_US 10-Year Storm D18_US 94.587 33.008 D19_US 10-Year Storm D19_US 93.246 8.192 D1_DS 10-Year Storm D1_DS 96.494 31.468 U_U_US 10-Year Storm D1_US 96.494 31.468 U_U_US 10-Year Storm D20_US 90.7487 20.01 D20_US 10-Year Storm D20_US 90.484 29.201 D21_US 10-Year Storm D21_US 93.012 66.372 D22_US 10-Year Storm D22_US 93.752 16.234 D32_DS 10-Year Storm D23_US 94.849 25.873 D23_DS 10-Year Storm D23_US 94.849 25.873 D24_US 10-Year Storm D24_US 94.832 35.467 D24_US 10-Year Storm D24_US 94.832 29.189	0 0 0 0 55417.52
D19_US 10-Year Storm D19_US 93,246 8,192 D1_DS 10-Year Storm D1_DS 96,494 31,468 D1_US 10-Year Storm D1_US 96,494 21,037 D20_US 10-Year Storm D21_US 91,484 29,201 D21_DS 10-Year Storm D21_DS 92,485 40,531 D21_US 10-Year Storm D21_US 93,012 66,372 D22_US 10-Year Storm D22_US 93,752 16,234 D23_DS 10-Year Storm D23_US 94,849 25,873 D23_DS 10-Year Storm D23_US 94,847 71,252 37 D24_US 10-Year Storm D23_US 94,847 71,252 37 D24_DS 10-Year Storm D24_US 94,842 35,867 37 D24_US 10-Year Storm D24_US 94,833 29,189 37	0 0 0 55417.52
D1_DS 10-YearStorm D1_DS 96,494 31.468 U1_US 10-YearStorm D1_US 96,494 21.057 D20_US 10-YearStorm D20_US 91.824 29.201 D21_DS 10-YearStorm D21_US 92.485 40.531 D21_US 10-YearStorm D21_US 93.012 66.327 D22_DS 10-YearStorm D22_US 93.291 60.327 D23_DS 10-YearStorm D23_US 93.752 16.234 D23_DS 10-YearStorm D23_US 94.849 25.873 D23_DS 10-YearStorm D24_US 94.87 71.252 3 D24_US 10-YearStorm D24_DS 94.832 25.467 D24_US 10-YearStorm D24_DS 94.832 23.467	0 U 55417.52
D20_US 10-Year Storm D20_US 91.824 29.201 D21_DS 10-Year Storm D21_DS 92.485 40.531 D21_US 10-Year Storm D21_US 93.012 66.372 D22_DS 10-Year Storm D22_DS 93.291 66.327 D22_US 10-Year Storm D22_US 93.752 16.234 D23_DS 10-Year Storm D23_DS 94.849 25.873 D24_US 10-Year Storm D24_US 94.87 71.252 3 D24_US 10-Year Storm D24_DS 94.832 35.467 D24_US 10-Year Storm D24_DS 94.832 35.467	55417.52
D21_DS 10-Year Storm D21_DS 92.485 40.531 D21_US 10-Year Storm D21_US 93.012 66.372 D22_DS 10-Year Storm D22_US 93.291 66.372 D22_DS 10-Year Storm D22_US 93.752 16.234 D23_DS 10-Year Storm D23_US 94.849 25.873 D24_US 10-Year Storm D24_US 94.87 71.252 1 D24_US 10-Year Storm D24_US 94.832 23.6467 1	
D21_US 10-Year Storm D21_US 93.012 66.372 D22_DS 10-Year Storm D22_DS 93.291 60.327 D22_US 10-Year Storm D22_US 93.752 16.234 D23_DS 10-Year Storm D23_DS 94.849 25.873 D24_DS 10-Year Storm D23_US 94.87 71.252 21 D24_DS 10-Year Storm D24_US 94.83 25.647 D24_DS 10-Year Storm D24_US 94.833 29.189	0
D22_DS 10-Year Storm D22_DS 93.291 60.327 D22_US 10-Year Storm D22_US 93.752 16.234 D23_DS 10-Year Storm D23_DS 94.849 25.873 D24_US 10-Year Storm D23_US 94.849 25.873 D24_DS 10-Year Storm D24_US 94.87 71.252 3 D24_US 10-Year Storm D24_US 94.832 35.467 D24_US 10-Year Storm D24_US 94.833 29.189	-
D22_US 10-Year Storm D22_US 93.752 16.234 D23_DS 10-Year Storm D23_DS 94.849 25.873 D23_US 10-Year Storm D23_US 94.87 71.252 12 D24_DS 10-Year Storm D24_DS 94.832 35.467 D24_US 10-Year Storm D24_US 94.833 29.189	0
Dzs_us Di-rear storm Dzs_us 94.87 71.252 1 Dz4_DS 10-Year storm D24_DS 94.82 35.467 Dz4_US 10-Year Storm D24_US 94.832 35.467 D24_US 10-Year Storm D24_US 94.833 29.189	0
Dzs_us Di-rear storm Dzs_us 94.87 71.252 1 Dz4_DS 10-Year storm D24_DS 94.82 35.467 Dz4_US 10-Year Storm D24_US 94.832 35.467 D24_US 10-Year Storm D24_US 94.833 29.189	0
D24_US 10-Year Storm D24_US 94.833 29.189	131418.505
	0
	0
D25_DS 10-Year Storm D25_DS 94.834 30.837	0
D25_US 10-Year Storm D25_US 94.834 25.422 D26.1_US 10-Year Storm D26.1_US 94.835 70.051	0
D26_DS 10-Year Storm D26_DS 94.835 23.185	0
D26_US 10-Year Storm D26_US 94.835 9.056	0
D27_DS 10-Year Storm D27_DS 94.313 24.039	0
D27_US 10-Year Storm D27_US 94.313 11.034	0
	33545 764 0
D28_DS 10-Year Storm D28_DS 94.282 40.613 D28_US 10-Year Storm D28_US 94.316 46.695	0
D29_DS 10-Year Storm D29_DS 94.003 31.329	0
D29_US 10-Year Storm D29_US 94.014 37.64	0
D2_DS 10-Year Storm D2_DS 95.419 53.019 5	990486.574
	48665.824
D2_US.1 10-Year Storm D2_US.1 95.42 50.896	0
	107284.182 56584.135
	115395.149
D30_DS 10-Year Storm D30_DS 93.624 27.189	0
D30_US 10-Year Storm D30_US 93.755 29.336	0
D31_DS 10-Year Storm D31_DS 93.062 23.906	0
D31_US 10-Year Storm D31_US 93.374 23.3	0
	68524.407 25563.083
D3_US 10-Year Storm D3_US 94.739 23.109	0
D4.1_DS 10-Year Storm D4.1_DS 94.648 41.944	0
D4.1_US 10-Year Storm D4.1_US 94.658 28.373	0
	187624.549
D4_DS 10-Year Storm D4_DS 94.648 39.181 D4_US 10-Year Storm D4_US 94.666 25.961	0
D4_US 10-Year Storm D4_US 94.666 25.961 D5_DS 10-Year Storm D5_DS 94.649 33.412	17763.76
D5_US 10-Year Storm D5_US 94.649 37.935	0
D6_DS 10-Year Storm D6_DS 94.649 25.999	0
D6_US 10-Year Storm D6_US 94.649 32.156	0
D7.1_US 10-Year Storm D7.1_US 93.106 7.612	0
D7_DS 10-Year Storm D7_DS 92.886 15.282 D7_US 10-Year Storm D7_US 93.63 11.31	0 49266.88
	32824.989
D8_DS 10-Year Storm D8_DS 92.692 23.147	0
D8_US 10-Year Storm D8_US 92.874 17.018	0
D_Co_Out 10-Year Storm D_Co_Out 95.207 3.984	0
DW1N_Out 10-Year Storm DW1N_Out 95.536 5.86	0
DW15_Out 10-Year Storm DW15_Out 95.487 6.245 DW2N_US 10-Year Storm DW2N_US 96 8.797	0
	51122.415
DW2S_DS 10-Year Storm DW2S_DS 95.24 0	0
Node650 10-Year Storm Node650 89.2 0	0
Node651 10-Year Storm Node651 89.1 0	0
	0
Node652 10-Year Storm Node652 95.032 21145.097	242745.684 0
Node653 10-Year Storm Node653 95.221 43967.968 2	2
Node653 10-Year Storm Node653 95.221 43967.968 2 Out3.1 10-Year Storm Out3.1 92.335 5.088 2	0
Node653 10-Year Storm Node653 95.221 43967.968 2 Out3.1 10-Year Storm Out3.1 92.335 5.088 2	0
Node653 10-Year Storm Node653 95.221 43967.968 2 Out3.1 10-Year Storm Out3.1 92.335 5.088 9 Out_1 10-Year Storm Out_1 90.102 10.4 Out_1.1 10-Year Storm Out_1.1 89.502 6.948 Out_1.2 10-Year Storm Out_1.2 90.04 11.189	
Node653 10-Year Storm Node653 95.221 43967.968 2 Out3.1 10-Year Storm Out3.1 92.335 5.088 0 Out_1 10-Year Storm Out_1 90.102 10.4 0 Out_1.1 10-Year Storm Out_1.1 89.502 6.948 0 Out_1.2 10-Year Storm Out_1.2 90.04 11.189 0 Out_2.1 10-Year Storm Out_2.2 88.295 12.372 0	0
Node653 10-Year Storm Node653 95.221 43967.968 2 Out3.1 10-Year Storm Out3.1 92.335 5.088 0 Out_1 10-Year Storm Out_1 90.102 10.4 0 Out_1.1 10-Year Storm Out_1.1 89.502 6.948 0 Out_1.2 10-Year Storm Out_1.2 90.04 11.189 0 Out_2.2 10-Year Storm Out_2.2 87.893 8.582 0	0 0 0 0
Node653 10-Year Storm Node653 95,221 43967,968 2 Out3.1 10-Year Storm Out3.1 92,335 5.088 0 Out_1 10-Year Storm Out_1 90,102 10.4 0 Out_1.1 10-Year Storm Out_1.1 89,502 6.548 0 Out_2.2 10-Year Storm Out_2.2 90,40 11,189 0 Out_2.2 10-Year Storm Out_2.2 88,295 12,372 0 Out_2.2 10-Year Storm Out_2.2 87,893 8,582 0 Out_3.1 10-Year Storm Out_2.3 91,005 11,207 0	0 0 0 0 0.044
Node653 10-Year Storm Node653 95.221 43967.968 2 Out3.1 10-Year Storm Out3.1 92.335 5.088 0 Out_1 10-Year Storm Out_1 90.102 10.4 0 Out_1.1 10-Year Storm Out_1.1 89.502 6.948 0.94 Out_2.1 10-Year Storm Out_1.2 90.04 11.189 0.04 0.042 10-Year Storm 0.01_2.2 88.295 12.372 0.01_2.2 10-Year Storm 0.01_2.2 87.893 8.582 0.01_3.1 10-Year Storm 0.01_3.3 91.005 11.207 0.01_4 10-Year Storm 0.01_4.4 92.175 9.87	0 0 0 0

PROPOSED CULVERT NODE 10-YR RESULT

r.g. miller





 D12_US1
 SYear Storm

 D12_US1.1
 Syear Storm

 D13_1_US
 Syear Storm

 D13_0S
 Syear Storm

 D13_0S
 Syear Storm

 D13_0S.2
 Syear Storm

 D13_US
 Syear Storm

 D13_US
 Syear Storm

 D13_US1
 Syear Storm

 D14_US1
 Syear Storm

 D14_US
 Syear Storm

 D14_US
 5-Year Storm

 D15_DS
 5-Year Storm

 D15_US
 5-Year Storm

 D16_L_US
 5-Year Storm
 D16.2_DS 5-Year Storm D16_DS 5-Year Storm
 D16.2.05
 5-Year Storm

 D16.05
 5-Year Storm

 D17.05
 5-Year Storm

 D17.05
 5-Year Storm

 D17.05
 5-Year Storm

 D17.05
 5-Year Storm

 D18.05
 5-Year Storm

 D18.05
 5-Year Storm

 D18.05
 5-Year Storm

 D19.05
 5-Year Storm

 D10.05
 5-Year Storm

 D20.05
 5-Year Storm

 D21_05
 5-Year Storm

 D22_05
 5-Year Storm

 D22_05
 5-Year Storm

 D22_05
 5-Year Storm

 D23_05
 5-Year Storm

 D24_05
 5-Year Storm

 D25_05
 5-Year Storm

 D26_105
 5-Year Storm
 D29_DS
 S-Year Storm

 D29_US
 S-Year Storm

 D2_DS
 S-Year Storm

 D2_US.
 S-Year Storm

 D3_US
 S-Year Storm

 D30_US
 S-Year Storm

 D31_US
 S-Year Storm

 D3_US
 S-Year Storm

 D4.L_US
 S-Year Storm

 D4.1_US
 5-Year Storm

 D4.1_US.1
 5-Year Storm

 D4_US.1
 5 Year Storm

 D4_DS
 5 Year Storm

 D5_US
 5 Year Storm

 D5_US
 5 Year Storm

 D6_DS
 5 Year Storm

 D6_US
 5 Year Storm

 D6_US
 5 Year Storm

 D6_US
 5 Year Storm

 D7_U_US
 5 Year Storm

 D7_US
 5 Year Storm

 D8_US
 5 Year Storm

 D8_US
 5 Year Storm

 D8_US
 5 Year Storm

 D8_US
 5 Year Storm

 DW1N_0out
 5 Year Storm
 DW15_Out 5-Year Storm DW2N_US 5-Year Storm DW2N_US.1 5-Year Storm DW2N_US.1 5-Year Storm DW2S_DS 5-Year Storm
 Node650
 5-Year Storm

 Node651
 5-Year Storm

 Node652
 5-Year Storm

 Node652
 5-rear Storm

 Node653
 5-rear Storm

 Out_1
 5-rear Storm

 Out_1.1
 5-rear Storm

 Out_1.2
 5-rear Storm

 Out_2.2
 5-rear Storm

 Out_2.2
 5-rear Storm

 Out_2.2
 5-rear Storm

 Out_3
 5-rear Storm

 Out_4
 5-rear Storm

 Strear Storm
 S-rear Storm

Scenario

 Name
 Scenario

 Basin1_Opt2
 5-Year Storm

 D10_DS
 5-Year Storm

 D10_US
 5-Year Storm

 D11_DS
 5-Year Storm

 D11_US
 5-Year Storm

 D12_DS
 5-Year Storm

 D12_US
 5-Year Storm

 D12_US
 5-Year Storm

 D12_US.1
 5-Year Storm

Name

ewb. Maps Area Drainage 1 04755 Road\CAD\DWG\05-Stella 9 201 8 M:\04755

NO.

REVISIONS

PROPOSED CULVERT NODE 5-YR RESULT

Node Name 0	Max Water Elevation ft	Max Volume ft^3	Node Inflow ft^3
Basin1_Opt2	93.134	714598.167	264979.813 0
D10_DS D10_US	95.844 95.85	25.562 10.681	40327.028
D11_DS	95.804	23.806	0
D11_US	95.805	24.44	0
D12_DS	93.555	21.674	0
D12_US	95.798	24.228	47774.674
D12_US.1	95.798	26.612	0
D12_US.1.1	93.555	11.497	0
D13.1_US	93.554	30.84	0
D13_DS D13_D3.1	94.329 93.554	15.941 16.289	0
D13 DS.2	94.329	8.405	0
D13_US	93.554	22.926	0
D13_US.1	93.554	26.191	0
D14_DS	94.328	13.29	54121.439
D14_US	94.328	15.055	0
D15_DS	94.273	30.828	0
D15_US	94.277	13.91	0
D16.1_US	94.256	38.407	0
D16.2_DS	94.255	39.641	0
D16_DS	94.257	38.283	0
D16_US	94.256	31.875	0
D17_DS	94.258	30.135	0
D17_US D18 DS	94.258 94.259	35.785 19.212	0 21002.807
D18_US	94.259	28.886	0
D18_05	93.201	7.63	0
D1_D5	96.36	29.782	0
D1_US	96.36	19.351	0
D20_US	91.686	27.473	43206.374
D21_DS	92.085	35.499	0
D21_US	92.782	63.49	0
D22_DS	92.963	56.206	0
D22_US	93.687	15.42	0
D23_DS	94.784	25.057	0 103999.185
D23_US	94.8	70.369	0
D24_DS D24_US	94.719 94.719	27.764	0
D25_DS	94.718	29.384	0
D25_US	94.718	23.966	0
D26.1_US	94.707	68.448	0
D26_DS	94.707	21.577	0
D26_US	94.707	7.453	0
D27_DS	94.218	22.849	0
D27_US	94.219	9.853	0
D28.1_US	94.215	22.349	27303.584
D28_DS	94.177	39.298	0
D28_US	94.22	45.488	0
D29_DS	93.925 93.938	30.349 36.681	0
D29_US D2_DS	95.042	48.276	735952.327
D2_03	96.359	31.022	38528.549
D2_US.1	95.045	46.175	0
D2_US.2	96.355	29.967	107284.325
D2B_US	95.813	26.801	44644.8
D2B_US.1	95.759	27.378	115396.078
D30_DS	93.567	26.473	0
D30_US	93.706	28.73	0
D31_DS	92.991	23.011	0
D31_US	93.339	22.858	0
D3_D5	94.382 95.042	21.14	53040.641 19019.565
D3_DS.1 D3_US	95.042	35.586 18.628	19019.565
D3_05 D4.1_DS	94.382	37.754	0
D4.1_US	94.328	24.229	0
D4.1_US.1	94.323	36.608	146708.465
D4_DS	94.315	34.991	0
D4_US	94.338	21.84	0
D5_DS	94.316	29.224	13681.371
D5_US	94.316	33.748	0
D6_DS	94.316	21.814	0
D6_US	94.316	27.97	0
D7.1_US	93.054	6.965 14.452	0
D7_DS D7_US	92.82	14.452	38583.613
D7_US.1	93.566	12.415	24185.091
D8_DS	92.654	22.668	0
D8_US	92.812	16.229	0
D_Co_Out	95.156	3.341	0
DW1N_Out	95.473	5.063	0
DW15_Out	95.429	5.511	0
DW2N_US	95.884	7.344	0
DW2N_US.1	96.004	6.957	38101.185
DW2S_DS	95.85	0	0
Node650 Node651	89.2 89.1	0	0
Node651 Node652	94.91	18151.529	0
	95.035	35657.035	192104.64
Node653		4.647	0
Node653 Out3.1			0
Out3.1	92.3 90.07	10.003	
		10.003 6.701	0
Out3.1 Out_1	90.07		
Out3.1 Out_1 Out_1.1 Out_1.2 Out_2	90.07 89.483 90.008 88.267	6.701 10.779 12.022	0 0 0
Out3.1 Out_1 Out_1.1 Out_1.2 Out_2 Out_2.2	90.07 89.483 90.008 88.267 87.872	6.701 10.779 12.022 8.316	0 0 0 0
Out3.1 Out_1 Out_1.1 Out_1.2 Out_2	90.07 89.483 90.008 88.267	6.701 10.779 12.022	0 0 0

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

	PROJECT T	ITLE: STELLA ROAD						
	FROM	COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD					
fig	SHEET DESCRIPTION: PROPOSED CULVERT NODE RESULTS							
/18/24	DRAWN BY: NS		DATE: 6/18/24					
,	CK'D BY: MJ	SCALE:N/A	SHEET NO: 74 / 133					

Name	Scenario	Link Name
1311.1 1317.1	5-Year Storm 5-Year Storm	D1_D2_Cu D13 Out C
1322.1 1330.1	5-Year Storm 5-Year Storm	D13_Out_C D3_D4_Cu D4_D5_Cu
1334.1	5-Year Storm	D5_D6_Cu
1344.1	5-Year Storm 5-Year Storm	D7_D8_Cu D7_D8_Cu
1350.1	5-Year Storm	Rd_Cross3
1350.2 1350.3	5-Year Storm 5-Year Storm	Rd_Cross3 Rd_Cross3
1354.1	5-Year Storm	D16.2_Out-
1359.1 1367.1	5-Year Storm 5-Year Storm	D10_D11_C D12_D13_C
1372.1 1381.1	5-Year Storm 5-Year Storm	D13 D14 C
1385.1	5-Year Storm	D16_D17_C D17_D18_C
1389.1 1389.2	5-Year Storm 5-Year Storm	Rd Cross4
1410.1	5-Year Storm	Rd_Cross4 D22_D23_C
1414.1 1442.1	5 Year Storm 5-Year Storm	D21_D22_C D14_D15_C D15_D16_C
1444.1	5-Year Storm	D15_D16_C Rd_Cross2
1445.1 1487.1	5-Year Storm 5-Year Storm	Rd_Cross2 Link633
1488.1	5-Year Storm	
Coss_Cul Cross_Cul	5-Year Storm 5-Year Storm	D10_D11_C D14_D15_C
Cross_Cul1 Cross_Cul2.1	5-Year Storm 5-Year Storm	D14_D15_C D12_D13_C D13_D14_C
Cross_Cul3	5-Year Storm	D15_D16_C
Cross_Cul4 CrossCul	5-Year Storm 5-Year Storm	D16_D17_C CrossCul
Cul_Cross	5-Year Storm	D1_D2_Cu
Cul_Cross2 Cul_Cross3	5-Year Storm 5-Year Storm	D3 D4 Cu D17_D18_C
Cul_Cross4	5-Year Storm	D5_D6_Cu
Cul_Cross5 D1	5-Year Storm 5-Year Storm	D4_D5_Cu D1
D10	5-Year Storm	D10
D11 D11_D12_Cul	5-Year Storm 5-Year Storm	D11 D11_D12_C
D12.1 D12.1.1.1	5-Year Storm 5-Year Storm	D12.1
D13	5-Year Storm	D12.1.1.1 D13
D13.1 D13.1.1	5-Year Storm 5-Year Storm	D13.1 D13.1.1
D13.2	5-Year Storm	D13.2
D14 D15	5-Year Storm 5-Year Storm	D14 D15
D16	5-Year Storm 5-Year Storm	D16
D16.1 D16.2	5-Year Storm 5-Year Storm	D16.1 D16.2
D17	5-Year Storm	D17
D17_Out-2 D18	5-Year Storm 5-Year Storm	D17_Out-2 D18
D19 D2	5-Year Storm 5-Year Storm	D19 D2
D2.1	5-Year Storm	D2.1
D2.2 D2.2.1	5-Year Storm 5-Year Storm	D2.2 D2.2.1
D2.2.1.1	5-Year Storm	D2.2.1.1
D20 D20_D21_Cul	5-Year Storm 5-Year Storm	D20 D20_D21_C
D21 D22	5-Year Storm 5-Year Storm	D21 D22
D23	5-Year Storm 5-Year Storm	0.33
D23_D24_Cul D24	5-Year Storm 5-Year Storm	D23_D24_C D24
024_025_Cul	5-Year Storm	D24_D25_C
D25 D25_D26_Cul	5-Year Storm 5-Year Storm	D25 D25_D26_C
D26	5-Year Storm	D26
D26.1 D26.1_D28_Cul	5-Year Storm 5-Year Storm	D26.1 D26.1_D28_0
D27 D27_D28.1_Cul	5-Year Storm	D27
D28	5-Year Storm 5-Year Storm	D27_D28.1_0 D28
D28.1	5-Year Storm 5-Year Storm	D28.1
D28_D29_Cul D29	5-Year Storm	D28_D29_C D29
029_D30_Cul D3	5-Year Storm 5-Year Storm	D29_D30_C D3
D3.1	5-Year Storm	D3.1
D30 D30_D31_Cul	5-Year Storm 5-Year Storm	D30 D30_D31_C
D31	5-Year Storm 5-Year Storm	D31
D4 D4.1	5-Year Storm 5-Year Storm	D4 D4.1
D4.1.1 D4.2	5-Year Storm 5-Year Storm	D4.1.1 D4.2
D4.2 D5	5-Year Storm	D4.2 D5
D6_D7_Cul	5-Year Storm 5-Year Storm	D6 D6D7Cu
D7	5-Year Storm	D7
D7.1 D7.2	5-Year Storm 5-Year Storm	D7.1 D7.2
D8	5-Year Storm	D8
D9	5-Year Storm 5-Year Storm	D9 D_Co_Out
D_Co_OutL	5-Year Storm	D_CoWo
D_CoWo		D_Out1.1
D_CoWo D_Out1.1 D_Out1.1.1	5-Year Storm 5-Year Storm	D_Out1.1.1
D_CoWo D_Out1.1 D_Out1.1.1 DW1 N	5-Year Storm 5-Year Storm 5-Year Storm	DW1 N
D_CoWo D_Out1.1 D_Out1.1.1 DW1_N DW1_N.1 DW1_S	5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm	DW1_N DW1_N1 DW1_S
D_CoWo D_Out1.1 D_Out1.1.1 DW1_N DW1_N.1 DW1_S DW2_N	5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm	DW1_N DW1_N1 S DW1_S
D_CoWo D_Out1.1 D_Out1.1.1 DW1_N DW1_N.1 DW2_N DW2_N DW2_S Emergency Overflow	5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm	DW1_N DW1_N.1 DW1_S DW2_N DW2_S Link634
D_CoWo D_Out1.1 DW1_N DW1_N.1 DW1_S DW2_N DW2_S Emeigency Overflow L_Sheriff_Out	5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm	DW1_N DW1_N1 DW1_S DW2_N DW2_S Link634 L_Sheriff_0
D_COWO D_Out1.1 D_Out1.1.1 DW1_N DW1_N.1 DW1_S DW2_N DW2_S Emergency Overflow	5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm 5-Year Storm	DW1_N DW1_N.1 DW1_S DW2_N DW2_S Link634
D_CoWo D_Out1.1 D_Out1.1.1 DW1_N DW1_N.1 DW1_S DW2_N DW2_S Emergency Overflow LSheriff_Out Link615 Link616 Link617	5-Year Storm 5-Year Storm	DW1_N DW1_N1 DW1_S DW2_N DW2_S Link634 L_Sheriff_O Link615 Link616 Link616
D_CoWo D_Out1.1 D_Out1.1.1 DW1_N DW1_N.1 DW1_S DW2_N DW2_S Emergency Overflow Link615 Link616 Link617 Link617 Link617 Out2_2RdCR	5-Year Storm 5-Year Storm	DW1_N DW1_N1 DW1_S DW2_N DW2_S Link634 L_Sheriff_D Link615 Link616 Link617 Link618 D16.2_Out-
D_COW0 D_Out1.1 D_Out1.1.1 D_V1.N.1 DW1_N_DW1_S DW2_N_DW1_S DW2_N_DW2_S Emergency Overflow L_sheriff_Out Link615 Link616 Link617 Link618 Out2_ZRCR Out_2.21	5-Year Storm 5-Year Storm	DW1_N DW1_N1 DW2_N DW2_S Link634 L_Sheriff_O Link615 Link616 Link616 Link618 D16.2_Out Out_2.2L
D.CoWo D.Out1.1 D.Out1.1 D.W1.N DW1.N DW2.N DW2.S DW2.N DW2.S Imegency Overflow Link615 Link615 Link616 Out2.2RdCR Out2.2RdCR Out2.2.1 Overflow Rd_Cross1	S-Year Storm S	DW1_N DW1_N1 DW1_S DW2_N DW2_S Link634 L_Sheriff_D Link616 Link616 Link616 D16.2_Out Out_2.2L Link711 Rd_Cross1
D_COW0 D_Out1.1 D_Out1.1.1 DW1_N DW1_N_1 DW1_S DW2_N DW2_S Emergency Overflow L_sherff_Out Link615 Link615 Link617 Link618 Out2_ZRCR Out_2.2L	S-Year Storm S	DW1_N DW1_N1 DW1_S DW2_N DW2_S Link634 L_Sheriff_O Link616 Link616 Link616 Link618 D16.2_Out_ Out_2.2L Link711

PROPOSED	CULVERT	LINK	10–YR	RESULT

	PROPOSEI	CULVE	RT LINK	100)—YR	RESU	LT					PROPOSED	CULVER	3
		Upstrcam	Pownatrcam	Max Flow	Max X-	Max Velocity	Maximum Water Elevation	Maximum Water Ecvation					Upstream	
1	Scenario Link Name	0 Node Name	0 Node Name 0 D2 US	-0.994	Area ft^2 3.291		(US) ft 97.141	[DS] ft 97.14			Name 1311.1	Scenario Link Name 0 10-Year Storm D1_D2_Cui	Node Name 0 D1_DS	r
00-Year Storm D13 00-Year Storm D3	Out	UI D13.1_US		16.813 11.23	1.796	9.434 3.563	95.214 96.259	91.106 96.129			1317.1 1322.1	10-Year Storm D13_Out_CUI 10-Year Storm D3_D4_CuI	D13.1_US D3_DS	_
				1.6	3.291 3.291	0.683	96.1 96.1	96.1 96.1			1330.1 1334.1	10-Year Storm D4_D5_Cul 10-Year Storm D5_D6_Cul	DS_US D6_US	
00 Vear Storm D7_D9_Cul 00-Year Storm D7_D8_Cul	ul	D7_DS D7_DS	D9_US D8_US	2.507 2.438	2.192	1.149	93.124 93.124	97.097 93.097			1344.1 1344.2	10 Year Storm D7_D8_Cui 10-Year Storm D7_D8_Cui	D7_DS D7_DS	Ξ
00-Year Storm Rd_Cross3 D4.:	3 D4.:	1_DS 1_DS	D16.1_US D16.1_US	10.414 10.414	3.293	3.286	96.1 96.1	95.768 95.768			1350.1 1350.2	10-Year Storm Rd_Cross3 10-Year Storm Rd_Cross3	D4.1_DS D4.1_DS	D
00-Year Storm Rd_C 00-Year Storm D16.2	Ou	-2 D16.2_DS		10.414 20.775	3.293	3.286	96.1 95.766	95.768 92.578			1350.3 1354.1	10-Year Storm Rd_Cross3 10-Year Storm D16.2_Out-2	D4.1_DS D16.2_DS	D:
00-Year Storm D10 00-Year Storm D12 00-Year Storm D13	D13_	THE_D12_D5	D11_US D13_US D14_US	5.438 -10.087 -3.861	3.293 3.291 3.286	1.702 -3.185 -1.34	96.711 95.214 96.086	96.731 95.214 96.085			1359.1 1367 1 1372.1	10-Year Storm D10_D11_Cul 16-Year Storm D12_D13_CUI 10-Year Storm D13_D14_Cul	D10_DS D12_DS D13_DS	
00-Year Storm D16_E 00-Year Storm D17_E	017	ul D17_US	D16_DS D17_DS	-4.401 2.137	3.285 3.291	-1.392 0.892	95.77 95.771	95.768 95.77			1381.1 1385.1	10-Year Storm D16_D17_Cul 10-Year Storm D17 D18 Cul	D17_US D18_US	0
00-Year Storm 00-Year Storm	Rd_Cross Rd_Cross		D19_US D19_US	3.419 4.219	1.426 1.48	2.398 2.851	93.848 93.848	93.476 93.39			1389.1 1389.2	10-Year Storm Rd_Cross4 10-Year Storm Rd_Cross4	D7_US D7_US	C
00-Yea	r Storm D22_D23_ r Storm D21 D22	ul D22 D5	D22_US D21_US	3.091 31.226	0.816	3.899 4.387	95.271 95.159	95.245 94.13			1410.1 1414.1	10-Year Storm D22_D23_Cul 10-Year Storm D21 D22 Cul	D23_D5 D22_D5	C
00-Year Storm 00-Year Storm 00-Year Storm		Cul D15_D5	D15_US D16_US D4.1_US	6.897 7.397 5.988	3.293 3.292 3.326	2.436 2.339 2.974	96.084 95.967 96.085	95.968 95.768 96.109			1442.1 1444.1	10-Year Storm D14_D15_Cul 10-Year Storm D15_D16_Cul	D14_DS D15_DS	
00-Year Storm 00-Year Storm	Link633	D13.1_US	Basin1_Opt2	16.449 15.704	5.145	3.338	95.214 94.825	96.109 94.825 90.349			1445.1 1487.1 1488.1	10-Year Storm Rd_Cross2 10-Year Storm Link633 10-Year Storm Link634	D14_US D13.1_US Basin1_Opt2	Bas
00-Year Storm D10_ 00-Year Storm D14	D11_	ul D10_D5	D11_US D15_US	0 2.631	0	0	-9.00E+99	-9.00E+99 -9.00E+99			Coss_Cul Cross_Cul	10-Year Storm D10_D11_Cul 10-Year Storm D14 D15 Cul	D10_DS D14_DS	0
00-Year Storm D12 00-Year Storm D13			D13_US D14_US	0	0	C	-9.00E+99 -9.00E+99	-9.00E+99 -9.00E+99			Cross_Cul1 Cross_Cul2.1	10-Year Storm D12_D13_CUI 10-Year Storm D13_D14_CuI	D12_DS D13_DS	0
00-Year Storm 00-Year Storm	D15_D16_ D16_D17_	Cul D15_D5 Cul D17_U5	D16_US D16_DS	1.051	0	0	-9.00E+99 -9.00E+99	-9.00E+99 -9.00E+99			Cross_Cul3 Cross_Cul4	10-Year Storm D15_D16_Cul 10-Year Storm D16_D17_Cul	D15_DS D17_US	0
	form D1_D2_C	ul D1_DS	D2_US	9.458	3.32	2.854	96.734 -9.00E+99	96.734 -9.00E+99			CrossCul Cul_Cross	10-Year Storm CrossCul 10-Year Storm D1_D2_Cul 10-Year Storm D2_D4_Cul	D12_US.1 D1_DS	
	storm D3_D4_C storm D17_D18_ storm D5 D6 C	Cul D18_US	D4_US D17_DS D5 DS	17.435 -0.447 -1.596	0	C C	-9.00E+99 -9.00E+99 -9.00E+99	-9.00E+99 -9.00E+99 -9.00E+99			Cul_Cross2 Cul_Cross3 Cul_Cross4	10-Year Storm D3_D4_Cui 10-Year Storm D17_D13_Cui 10-Year Storm D5_D6_Cui	D3_DS D18_US D6_US	
	torm D4_D5_C		D3_D3 D4_DS D1_DS	-2 008	0 38.177	-0.057	-9.00E+99 97.141	-9.00E+99 97.141			Cul_Cross5 D1	10-Year Storm D3_D8_Cul 10-Year Storm D4_D5_Cul 10-Year Storm D1	DS_US D1_US	
00-Year Stor 00-Year Stor	m D10	D10_US D11_US	D10_DS D11_DS	10.173 3.675	98.523 102.445	0.375	96.711 96.731	96.711 96.731			D10 D11	10-Year Storm D10 10-Year Storm D11	D10_US D11_US	с С
UU-Year Stor		D12_05	D12_US D12_US.1	-3257 10.154	10.087 112.895	0.351	96.731 96.734	96.734 96.734			D11_D12_Cul D12.1	10-Year Storm D11_D12_Cul 10-Year Storm D12.1	D11_DS D12_US	D D
00-Year Storm 00-Year Storm 00-Year Storm	D12.1.1. D13 D13.1	D12_US.1. D13_US D13_DS.2	D13_US.1	-4 846 -10.45 -1.905	129.795 62.541 44.184	-0.093 -0.462 0.186	95.215 95.214 96.086	95.214 95.214 96.086			D12.1.1.1 D13 D13.1	10-Year Storm D12.1.1.1 10-Year Storm D13 10-Year Storm D13.1	D12_US.1.1 D13_US D13_DS.2	D: D: C
00-Year Storm 00-Year Storm 00-Year Storm	D13.1 D13.1.1 D13.2	D13_DS.2 D13_DS.1 D13_US.1	D13.1_US	-1 905 -2 197 -11.423	44.184 102.324 72.855	0.186 0.064 -0.339	96.086 95.214 95.214	96.086 95.214 95.214			D13.1 D13.1.1 D13.2	10-Year Storm D13.1.1 10-Year Storm D13.1.1 10-Year Storm D13.2	D13_DS.2 D13_DS.1 D13_US.1	D: D:
00-Year Stor 00-Year Stor	rm D14	D13_03.1 D14_D5 D15_03	D13.1_05	-11.425 7.316 8.450	50.491 52.085	0.741 3.904	96.084 95.908	95.214 96.085 95.967			D13.2 D14 D15	10-Year Storm D14 10-Year Storm D15	D14_DS D15_U3	C
00-Year Storm 00-Year Storm	D16 D16.1	D16_U5 D16_D5	D16.1_US D16.1_US	8.42 -4.591	74.128 94.984	4.034 -0.185	95.768 95.768	95.768 95.768			D16 D16.1	10-Year Storm D16 10-Year Storm D16.1	D16_US D16_DS	D
00-Year Storn 00-Year Storn	m D17	D16.1_US D17_DS	D17_US	39.905 -1 809	156.397 79.672	0.437	95.768 95.77	95.766 95.77			D16.2 D17	10-Year Storm D16.2 10-Year Storm D17	D16.1_US D17_DS	D
00-Year Storm	D17_Out D18	D18_D5	Out_2 D18_US	0 4.56 7.637	98.967 57.719 4.661	0.596	95.77 95.771	92.8 95.771 92.426			D17_Out-2 D18	10-Year Storm D17_Out-2 10-Year Storm D18 10 Year Storm D19	D17_US D18_DS D10_US	0
00-Year Storm 00-Year Storm 00-Year Storm	D19 D2 D2.1	D19_US D2_US D2_US.1	0ut3.1 D2_US.2 D2_DS	7.637 5.895 16.426	4.661 42.799 136.662	1.639 0.261 1.734	93.39 97.14 96.718	97.137 96.718			D10 D2 D2.1	10 Year Storm D 10 10-Year Storm D 2 10-Year Storm D 2.1	D10_US D2_US D2_US.1	D
00-Year Sto 00-Year Sto 00-Year Sto	rm D2.2	D2_US.2 D2B_US	D2B_US D2B_US.1	10.420 19.265	3.293 37.719	3.325	97.137 96.737	96.737 96.734			D2.2 D2.2.1	10-Year Storm D2.2 10-Year Storm D2.2.1	D2_US.2 D2B_US	D
00-Year Storm 00-Year Storm	D2.2.1.1 D20	D2B_US.1 D20_US	D2_US.1 Out_3	33.947 28.391	49.706 18.483	1.102 1.538	96.734 92.192	96.718 91.259			D2.2.1.1 D20	10-Year Storm D2.2.1.1 10-Year Storm D20	D2B_US.1 D20_US	D
00-Year Storn 00-Year Storn	n D21	D21_U5	D20_US D21_DS	25.978 36.215	5.102 23.373	5.251 1.753	93.961 94.13	92 192 93.961			D20_D21_Cul D21	10-Year Storm D20_D21_Cul 10-Year Storm D21 10-Year Storm D21	D21_DS D21_US	0
00-Year Storm 00-Year Storm	D22 D23	D22_US D23_US	D22_DS D23_DS	68.435 29.436	112.389 42.262	3.143 0.783	95.245 95.307	95.157 95.271 95.307			D22 D23 D23_D24_Cul	10-Year Storm D22 10-Year Storm D23 10-Year Storm D23_D24_Cul	D22_US D23_US D24_DS	C
00-Year Storm D23 00-Year Storm D24	D24	D24_U5	D23_US D24_DS D24_US	2.017 2.002 1.858	0.82 44.551 1.848	2.532 0.754 1.045	95.211 95.212 95.218	95.307 95.211 95.212			D23_D24_Cul D24 D24_D25_Cul	10-Year Storm D23_D24_Cul 10-Year Storm D24 10-Year Storm D24_D25_Cul	D24_DS D24_US D25_DS	0
	D25	D25_U5	D24_0S D25_DS D25_US	1.858 1.854 1.798	1.848 34.776 1.282	-0.258 1.457	95.218 95.288	95.212 95.218 95.218			D25_D25_Cul	10-Year Storm D25 10-Year Storm D25 10-Year Storm D25_D25_Cul	D25_US D26_DS	0
00-Year Storm 00-Year Storm	D26 D26.1	D26_US D26.1_US	D26.1_US D26_DS	-1 473 -3 356	76.207 64.772	0.031	95.288 95.288	95.288 95.288			D26 D26.1	10-Year Storm D26 10-Year Storm D26.1	D26_US D26.1_US	D
00-Year Storm D 00-Year Storm	D27	D27_U5	D26.1_US D27_DS	-8457	3.28 40.442	9.72 0.057	94.74 94.739	95.288 94.739			D26.1_D28_Cul D27	10-Year Storm D26.1_D28_Cul 10-Year Storm D27	D28_US D27_US	Di
00-Year	r Storm D27_D28.1 r Storm D28 r Storm D28.1	D28_U5	D28.1_US D28.1_US	-0.477 8.456	3.293 30.57 43.507	-0.245 -11.219 2.202	94.739 94.74	94.738 94.738 94.725			D27_D28.1_Cul D28 D28.1	10-Year Storm D27_D28.1_Cul 10-Year Storm D28 10-Year Storm D28.1	D27_DS D28_US D28.1_US	D) D) D
	torm D28_D29	D28.1_US Cul D28_DS D29_US	D28_DS D29_US D29_DS	12.756 10.449 10.378	43.507 3.293 22.345	3.303 3.31 0.7	94.738 94.725 94.353	94.725 94.353 94.347			D28_D29_Cul D29	10-Year Storm D28_D29_Cul 10-Year Storm D28_D29_Cul 10-Year Storm D29	D28_DS D29_US	C
00-Yea	ar Storm D29_D30_ ar Storm D3		D30_US D3_DS	10.378 10.347 20.357	3.292 48.384	3.285	94.353 94.347 96.277	94.347 93.973 96.259			D29_D30_Cul D3	10-Year Storm D29_D30_Cul 10-Year Storm D3	D29_DS D3_US	0
00-Year 00-Year	Storm D3.1 Storm D30	D3_DS.1 D30_U5	D2_DS D30_DS	-18.449 10.357	138.99 14.893	0.246	96.716 93.973	96.718 93.885			D3.1 D30	10-Year Storm D3.1 10-Year Storm D30	D3_DS.1 D30_US	C
00-Year St 00-Year St	torm D30_D31_ torm D31	Cul D30_D5 D31_U5	D31_US D31_DS	10.375 10.383	3.275 8.691	3.211 1.351	93.885 93.536	93.536 93.342			D30_D31_Cul D31	10-Year Storm D30_D31_Cul 10-Year Storm D31	D30_DS D31_US	0
00	-Year Storm D4 -Year Storm D4.1	D4_US D4.1_US	D4.1_US D4.1_US.1	23.231 18.901	54.839 66.551	0.696	96.129 96.109	96.109 96.102			D4 D4.1 D4.1.1	10-Year Storm D4 10-Year Storm D4.1 10-Year Storm D4.1.1	D4_US D4.1_US	D4
00-Y	ear Storm D4.1.1 ear Storm D4.2 ear Storm D5	D4.1_US.1 D4_DS D5_DS	D4.1_DS D4.1_DS D5_US	32.114 -3 744 -1 833	106.813 108.929 92.828	0.824 1.191 0.476	96.102 96.1 96.1	96.1 96.1 96.1			D4.1.1 D4.2 D5	10-Year Storm D4.1.1 10-Year Storm D4.2 10-Year Storm D5	D4.1_US.1 D4_DS D5 DS	D
00-Y	Year Storm D5 Year Storm D6 Year Storm D6 D7 C	D6_DS	D5_US D6_US D7_US	-1 833 -1 528 0.001	92.828 72.004 0.001	0.476 0.183 0.727	96.1 96.1 96.1	96.1 96.1 93.848			D6 D6_D7_Cul	10-Year Storm DS 10-Year Storm D6 10-Year Storm D6_D7_Cul	D6_DS D6_DS	
00-Year 00-Year	Storm D7	D7_US D7.1_US	D7_03 D7_05.1 D7_D5	0.602	10.545	0.06	93.848 93.303	93.846 93.124			D7 D7.1	10-Year Storm D7 10-Year Storm D7.1	D7_US D7.1_US	D
00-Yea 00-Yea	ar Storm D7.2 ar Storm D8	D7_US.1 D8_US	D7.1_US D8_DS	5.16 4.938	10.347 4.455	0.501	93.846 93.097	93.303 92.833			D7.2 D8	10-Year Storm D7.2 10-Year Storm D8	D7_US.1 D8_US	D
00-Year Sto 00-Year Sto	orm D_Co_Ou			15.067 7.431	8.137 4.068	1.852	92.833 95.915	92.281 95.556			D9 D_Co_OutL D_CoWo	10-Year Storm D9 10-Year Storm D_Co_OutL 10-Year Storm D_CoWo	D8_DS DW15_Out DW1N_Out	D
00-Year S 00-Year S	torm D_Out1.	1 Out_1	Out_1.2	7.431 16.816 54.685	2.43 21.997	3.058	96.03 90.374	95.915 90.349 89.757			D_CoWo D_Out1.1 D_Out1.1.1	10-Year Storm D_CoWo 10-Year Storm D_Out1.1 10-Year Storm D_Out1.1.1	DW1N_Out Out_1 Out_1.2	DV C
00-Ye	ear Storm D_Out1.1 ear Storm DW1_N ear Storm DW1 N.	DW2N_US		54.685 -4 129 7.432	23.684 7.28 5.543	2.309 1.367 1.341	90.349 9.66E+01 96.647	89.757 9.67E+01 96.03			D_0011.1.1 DW1_N DW1_N.1	10-Year Storm D_001.1.1 10-Year Storm DW1_N 10-Year Storm DW1_N.1	DW2N_US.1 DW2N_US.1	D1 DV
00-Year Storr 00-Year Storr	m DW1_S m DW2_N	DW25_D5	DW15_Out	-0.029 -4.126	3.103	-0.(45	9.59E+01 9.67E+01	9.59E+01 9.67E+01			DW1_S DW2_N	10-Year Storm DW1_S 10-Year Storm DW2_N	DW2S_DS DW2N_US	DV
00-Year Storr 00-Year Storr	m DW2_S m L_Sheriff_(DW25_05 Dut Sheriff_Out	D1_US all Basin1_Opt2	0 104.024	0 7.606	(6.838	-9.00E+99 96.279	-9.00E+99 94.825			DW2_S L_Sheriff_Out		DW2S_DS Sheriff_Outfall	Bas
00-Year St 00-Year St	orm Link616	Node652	D23_US	0 10.7	0 5.134	0	-9.00E+99 9.54E+01	-9.00E+99 9.53E+01			Link615 Link616	10-Year Storm Link615 10-Year Storm Link616	Node650 Node652	D
00-Year St 00-Year St	orm Link618		D26.1_US	11.658 10.843	5.137 5.13	2.352	95.853 9.59E+01	95.449 9.53E+01			Link617 Link618 Out2.2RdCR	10-Year Storm Link617 10-Year Storm Link618 10-Year Storm D16.2_Out-2	Node653 Node653 D16.2_DS	N D
00-Yea	ar Storm D16.2_Ou ar Storm Out_2.2 ar Storm Link711		Out_2 Out_2.2 D3 DS.1	19.314 39.88 -20.286	0 14.587 0	C 2.734	9.00E+99 88.679 -9.00E+99	9.00E+99 88.281 -9.00E+99			Out_2.2kdCk Out_2.2L Overflow	10-Year Storm D16.2_Dut-2 10-Year Storm Out_2.2L 10-Year Storm Link711	D16.2_DS Out_2 D3_US	C p
0	IO-Year Storm Link711 IO-Year Storm Link634 IO-Year Storm Rd Cross	Basin1_Op		-20.286 23.815 42.003	0 5.145	C 8.474	-9.00E+99 -9.00E+99 96.718	-9.00E+99 -9.00E+99 95.214			Overflow Weir Rd_Cross1	10-Year Storm Link634 10-Year Storm Rd_Cross1	Basin1_Opt2 D2_DS	C D
00-Year Sto 00-Year Sto	orm Rd_Cross orm D21_D22_	5 D31_D5 Cul D22_D5	D8_D5 D21_U5	10.4 12.838	3.252 0	3.217	93.342 -9.00E+99	92.833 -9.00E+99			Rd_Cross5 RoadCross	10-Year Storm Rd_Cross5 10-Year Storm D21_D22_Cul	D31_DS D22_DS	0
	m D22_D23_		D22_US	26.619	0	C	-9.00E+99				RoadCross2.1	10-Year Storm D22_D23_Cul	D23_DS	C
				D.47	rc	NALA	- 1							
ONS				DA		NAME	-					OTINT	TI X 7	
						<u> </u>	1		FORT	BF.	ND C	JUUN	ΙY	
							-							

Name	Scenario	Link Name O	Upstream Node Name 0	pownstream Node Name 0	Max Flow cfs	Max X- Sectional Area ft^2	Max Velocity ft/s	Maximum Water Elevation (U3) ft	Maximun Water Elevation (DS) ft
1311.1	Scenario 10-Year Storm	D1_D2_Cul	D1_DS	D2_US	+0.567	3.291	-0.233	π 96.494	(DS) H 96.493
1317.1	10-Year Storm	D13_Out_CUI	D13.1_US	Out_1	13.936	1.796	7.945	93.899	90.899
1322.1 1330.1	10-Year Storm 10-Year Storm	D3_D4_Cul D4 D5 Cul	D3_DS D5_US	D4_US D4 DS	7.274	3.171 3.291	2.561 0.591	94.735 94.649	94.666 94.648
1334.1	10-Year Storm	D5_D6_Cul	D6_US	D5_DS	-1.924	3.291	-0.631	94.649	94.649
1344.1 1344.2	10 Vear Starm	D7_D8_Cul	D7_D5	DD_US	1.41	1.723	0.819	92.886	92.874 92.874
1344.2	10-Year Storm 10-Year Storm	D7_D8_Cul Rd_Cross3	D7_DS D4.1_DS	D8_US D16.1_US	6.107	2.016	0.664	92.886	92.874
1350.2	10-Year Storm	Rd_Cross3	D4.1_DS	D16.1_US	6.107	3.293	1.938	94.648	94.585
1350.3	10-Year Storm	Rd_Cross3	D4.1_DS	D16.1_US	6.107	3.293	1.938	94.648	94.585
1354.1 1359.1	10-Year Storm 10-Year Storm	D16.2_Out-2 D10_D11_Cul	D16.2_DS D10 DS	Out_2 D11 US	16.615 4.043	2.232 3.149	7.583	94.583 95.974	92.49 95.921
1367 1	16-Vear Storm	D12_D13_CUI	D12_D5	D13_US	-E 052	3 253	.7 300	02 200	03 800
1372.1 1381.1	10-Year Storm 10-Year Storm	D13_D14_Cul D16_D17_Cul	D13_DS D17_US	D14_US D16_DS	-1.552 -3.442	2.544 3.285	-1.168 -1.093	94.658 94.586	94.657 94.585
1385.1	10-Year Storm	D17_D13_Cul	D18_US	D17_DS	2.066	3.291	0.921	94.587	94.586
1389.1	10-Year Storm	Rd_Cross4	D7_US	D19_US	2.065	1.013	2.039	93.63	93.328
1389.2 1410.1	10-Year Storm 10-Year Storm	Rd_Cross4 D22_D23_Cul	D7_US D23_DS	D19_US D22_US	2.684 3.155	1.065 0.816	2.52 4.007	93.63 94.849	93.246 93.752
1410.1	10-Year Storm	D22_D23_Cul D21 D22 Cul	D23_D3 D22 DS	D22_05 D21 US	16.273	7.378	2.293	93.291	93.732
1442.1	10-Year Storm	D14_D15_Cul	D14_DS	D15_US	4.177	2.334	2.782	94.657	94.614
1444.1 1445.1	10-Year Storm 10-Year Storm	D15_D15_Cul Rd Cross2	D15_DS D14_US	D16_US D4.1_US	3.186 6.88	3.292	1.246 2.446	94.613 94.657	94.585 94.658
1445.1	10-Year Storm	Link633	D13.1_US	Basin1_Opt2	19.121	5.114	3.825	93.899	93.648
1488.1	10-Year Storm	Link634	Basin1_Opt2	Out_1.2	12.629	1.547	8.187	93.648	90.04
Coss_Cul Cross_Cul	10-Year Storm 10-Year Storm	D10_D11_Cul D14_D15_Cul	D10_DS D14_DS	D11_US D15_US	0	0	0	-9.00E+99 -9.00E+99	-9.00E+9
Cross_Cul1	10-Year Storm	D12_D13_CU	D12_DS	D13_US	0	0	0	-9.00E+99	-9.00E+9
Cross_Cul2.1	10-Year Storm	D13_D14_Cul	D13_DS	D14_US	0	0	0	-9.00E+99	-9.00E+9
Cross_Cul3 Cross_Cul4	10-Year Storm 10-Year Storm	D15_D15_Cul D16_D17_Cul	D15_DS D17_US	D16_US D16_DS	0	0	0	-9.00E+99 -9.00E+99	-9.00E+9
CrossCul	10-Year Storm	CrossCul	D17_03	D16_03 D28_US.1	6.307	3.308	1.906	95.912	95.858
Cul_Cross	10-Year Storm	D1_D2_Cul	D1_DS	D2_US	0	0	0	-9.00E+99	-9.00E+9
Cul_Cross2 Cul_Cross3	10-Year Storm 10-Year Storm	D3_D4_Cul D17_D13_Cul	D3_DS D18_US	D4_US D17_DS	0	0	0	-9.00E+99 -9.00E+99	-9.00E+9
Cul_Cross4	10-Year Storm	D5_D6_Cul	D6_US	D5_DS	0	0	0	-9.00E+99	-9.00E+9
Cul_Cross5	10-Year Storm	D4_D5_Cul	DS_US	D4_DS	0	0	0	-9.00E+99	-9.00E+9
D1 D10	10-Year Storm 10-Year Storm	D1 D10	D1_US D10_US	D1_DS D10_DS	-0.73 6.69	23.671 29.54	-0.077 0.407	96.494 95.979	96.494 95.974
D11	10-Year Storm	D10	D10_03	D10_D3	3.902	67.534	0.059	95.921	95.921
D11_D12_Cul	10-Year Storm	D11_D12_Cul	D11_DS	D12_US	-4.323	10.047	-0.558	95.921	95.912
D12.1 D12.1.1.1	10-Year Storm	D12.1 D12.1.1.1	D12_US D12_US.1.1	D12_05.1 D12_DS	6.383 -2.896	64.936	0.144 -0.1	95.912 93.899	93.912 93.899
D12.1.1.1	10-Year Storm	D12.111.1	D13_US	D13_US.1	-6.177	23.142	-0.502	93.899	93.899
D13.1	10-Year Storm	D13.1	D13_D5.2	D13_DS	-0.721	10.28	-0.194	94.658	94.658
D13.1.1 D13.2	10-Year Storm 10-Year Storm	D13.1.1 D13.2	D13_DS.1 D13_US.1	D13.1_US D13.1_US	-0.95 -6.479	42.707 33.466	0.058	93.899 93.899	93.899 93.899
D13.2 D14	10-Year Storm	D13.2 D14	D14_DS	D13.1_05 D14_US	5.03	33.466 13.379	0.326	94.657	94.657
D15	10-Year Storm	D 15	D15_U3	D15_D5	3.927	15.525	3.964	94.614	94.013
D16 D16.1	10-Year Storm 10-Year Storm	D16 D16.1	D16_US D16_DS	D16.1_US D16.1_US	2.964	37.337 49.883	5.692 0.226	94.585 94.585	94.585 94.585
D16.2	10-Year Storm	D16.2	D16.1_US	D16.2_DS	16.717	92.177	0.441	94.585	94,583
D17	10-Year Storm	D17	D17_DS	D17_US	1.598	41.805	0.432	94.586	94.586
D17_Out-2 D18	10-Year Storm 10-Year Storm	D17_Out-2 D18	D17_US	Out_2 D18_US	0 3.673	43.753 22.149	0	94.586 94.587	92.8 94.587
D10	10 Year Storm	D 18 D 19	D18_DS D10_UE	Out3.1	4.740	3.44	1.38	94.367 03.246	94.387
D2	10-Year Storm	D2	D2_US	D2_US.2	3.743	26.947	0.246	96.493	96.489
D2.1 D2.2	10-Year Storm 10-Year Storm	D2.1 D2.2	D2_US.1 D2_US.2	D2_DS D2B_US	19.764 10.643	78.243	0.957 3.368	95.42 96.489	95.419 95.907
D2.2.1	10-Year Storm	D2.2.1	D2B_US	D2B_US.1	15.171	19.932	0.882	95.907	95.858
D2.2.1.1	10-Year Storm	D2.2.1.1	D2B_US.1	D2_US.1	28.821	25.512	1.376	95.858	95.42
D20 D20_D21_Cul	10-Year Storm 10-Year Storm	D20 D20_D21_Cul	D20_US D21_DS	Out_3 D20_US	17.022	13.24	1.286	91.824 92.485	91005 91824
D21	10-Year Storm	D21	D21_US	D21_DS	16.119	9.492	1.85	93.012	92.485
D22	10-Year Storm	D22	D22_US	D22_DS	16.505	11.397	1.486	93.752	93.291
D23 D23_D24_Cul	10-Year Storm 10-Year Storm	D23 D23_D24_Cul	D23_US D24_DS	D23_DS D23_US	16.51 -2.551	32.112 0.82	0.514	94.87 94.832	94.849 94.87
D24	10-Year Storm	D24	D24_US	D24_DS	-1.648	35.763	0.669	94.833	94.832
D24_D25_Cul	10-Year Storm	D24_D25_Cul	D25_DS	D24_US	-1.12	1.848	-0.632	94.834	94.833
D25 D25_D26_Cul	10-Year Storm 10-Year Storm	D25 D25_D25_Cul	D25_US D26_DS	D25_DS D25_US	-1.264	25.922 1.282	-0.221 -1.144	94.834 94.835	94.834 94.834
D26	10-Year Storm	D26	D26_US	D26.1_US	-0.665	56.356	0.03	94.835	94.835
D26.1	10-Year Storm	D26.1	D26.1_US	D26_DS	-1.455	46.549	-0.137	94.835	94.835
D26.1_D28_Cul D27	10-Year Storm 10-Year Storm	D26.1_D28_Cul D27	D28_US D27_US	D26.1_US D27_DS	-8.353 -0.544	3.28 23.266	-2.631 0.084	94.316 94.313	94.835
027_D28.1_Cul	10-Year Storm	D27_D28.1_Cul	D27_DS	D28.1_US	-0.322	3.088	-0.231	94.313	94.312
D28	10-Year Storm	D28	D28_US	D28.1_US	8.343	20.199	0.601	94.316	94.312
D28.1 D28_D29_Cul	10-Year Storm 10-Year Storm	D28.1 D28_D29_Cul	D28.1_US D28_DS	D28_DS D29_US	9.788 8.767	29.92 3.293	0.467	94.312 94.282	94.282 94.014
D29	10-Year Storm	D29	D29_US	D29_DS	8.762	16.558	0.599	94.014	94.003
D29_D30_Cul	10-Year Storm	D29_D3)_Cul	D29_DS D3_US	D30_US D3 D5	8.759 2.871	3.292 15.512	2.763	94.003 94.739	93.755 94.735
D3 D3.1	10-Year Storm 10-Year Storm	D3 D3.1	D3_05 D3_05.1	D3_DS D2_DS	-1.929	15.512 80.744	0.237	94.739 9.54E+01	94.735 9.54E+0
D30	10-Year Storm	D 30	D30_US	D30_DS	8.754	11.529	0.79	9.38E+01	9.36E+0
D30_D31_Cul D31	10-Year Storm 10-Year Storm	D30_D31_Cul D31	D30_DS D31_US	D31_US D31_DS	8.756	3.234 6.636	2.707	9.36E+01 9.34E+01	9.34E+0
D31 D4	10-Year Storm	D3. D4	D4_US	D4.1_US	6.724	17.418	0.804	9.47E+01	9.47E+0
D4.1	10-Year Storm	D4.1	D4.1_US	D4.1_US.1	8.463	22.647	0.934	9.47E+01	9.47E+0
D4.1.1 D4.2	10-Year Storm 10-Year Storm	D4.1.1 D4.2	D4.1_US.1 D4_DS	D4.1_DS D4.1_DS	20.43	43.19 43.946	0.929	94.655 9.46E+01	94.648 9.46E+0
D4.2 D5	10-Year Storm	D4.2 D5	D4_DS D5_DS	D4.1_DS D5_US	1.316	43.946 33.165	0.493	9.46E+01 9.46E+01	9.468+0
D6	10-Year Storm	D6	D6_DS	D6_US	-0.785	24.69	0.163	9.46E+01	9.468+0
D6_D7_Cul D7	10-Year Storm 10-Year Storm	D6_D7_Cul D7	D6_DS D7_US	D7_US D7_US.1	0.001	0.001 7.524	0.511 0.067	9.46E+01 9.36E+01	9.368+0
D7.1	10-Year Storm	D7.1	D7_US	D7_05.1 D7_DS	2.751	2.74	1.005	93.106	92.886
D7.2	10-Year Storm	D7.2	D7_US.1	D7.1_US	2.886	7.154	0.404	93.632	93106
D8 D9	10-Year Storm 10-Year Storm	D8 D9	D8_US D8_DS	D8_DS Out_4	2.742	3.191 6.519	0.86	92.874 92.692	92.692 92.177
D_Co_OutL	10-Year Storm	D_Co_OutL	DW15_Out	D_Co_Out	1.186	1.168	1.016	95.487	95.207
D_CoWo	10-Year Storm	D_CoWo	DW1N_Out	DW1S_Out	1.187	0.816	1.455	95.536	95.487
D_Out1.1 D_Out1.1.1	10-Year Storm 10-Year Storm	D_Out1.1 D_Out1.1.1	Out_1 Out_1.2	Out_1.2 Out_1.1	13.936 26.385	13.999 14.601	1.001	90.102 90.04	90.04 89.503
DW1_N	10-Year Storm	DW1_N	DW2N_US.1	DW2N_US	1.372	1.608	1.095	9.61E+01	9.6CE+0
DW1_N.1	10-Year Storm	DW1_N.1	DW2N_US.1	DW1N_Out	1.188	1.501	0.791	96.072	95.536
DW1_S DW2_N	10-Year Storm 10-Year Storm	DW1_S DW2_N	DW2S_DS DW2N_US	DW1S_Out D10 US	0	0.101	0 2.199	-9.00E+99 9.60E+01	-9.00E+9 9.6CE+0
DW2_N DW2_S	10-Year Storm	DW2_N DW2_S	DW2N_DS DW2S_DS	D10_05	0	0	0	-9.00E+99	-9.00E+9
L_Sheriff_Out	10-Year Storm		Sheriff_Outfall	Basin1_Opt2	67.443	7.595	4.474	96.511	93.648
Link615	10-Year Storm	Link615	Node650	Node651	0	0	0	-9.00E+99	-9.00E+9
Link616 Link617	10-Year Storm 10-Year Storm	Link616 Link617	Node652 Node653	D23_US Node652	7.046	5.136 5.143	1.424	9.50E+01 95.221	9.45E+0 95.032
Link618	10-Year Storm	Link618	Node653	D26.1_US	9.631	5.137	1.946	9.52E+01	9.488+0
Out2.2RdCR	10-Year Storm	D16.2_Out-2	D16.2_DS	Out_2	0	0	0	-9.00E+99	-9.00F+9
Out_2.2L Overflow	10-Year Storm 10-Year Storm	Out_2.2L Link711	Out_2 D3_US	Out_2.2 D3_DS.1	16.615 0	8.385	1.982	88.295 0.00E+00	87.963 0.00E+0
Overflow Weir	10-Year Storm	Link634	Basin1_Opt2	Out_1.2	0	0	0	-9.00E+99	-9.00E+9
	10-Year Storm	Rd Cross1	D2_DS	D13.1_US	38.67	5.13	7.859	95.419	93.899
Rd_Cross1 Rd_Cross5	10-Year Storm	Rd_Cross5	D31_DS	D8 DS	8.768	3.077	2.858	93.062	92.692

ENGINEERING DEPARTMENT

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713.461.9600 | rgmiller.com



Drainage Т Road\CAD\DWG\05- 04755 Stella 20116 NO. M:\04755.000

Area Maps.dwg

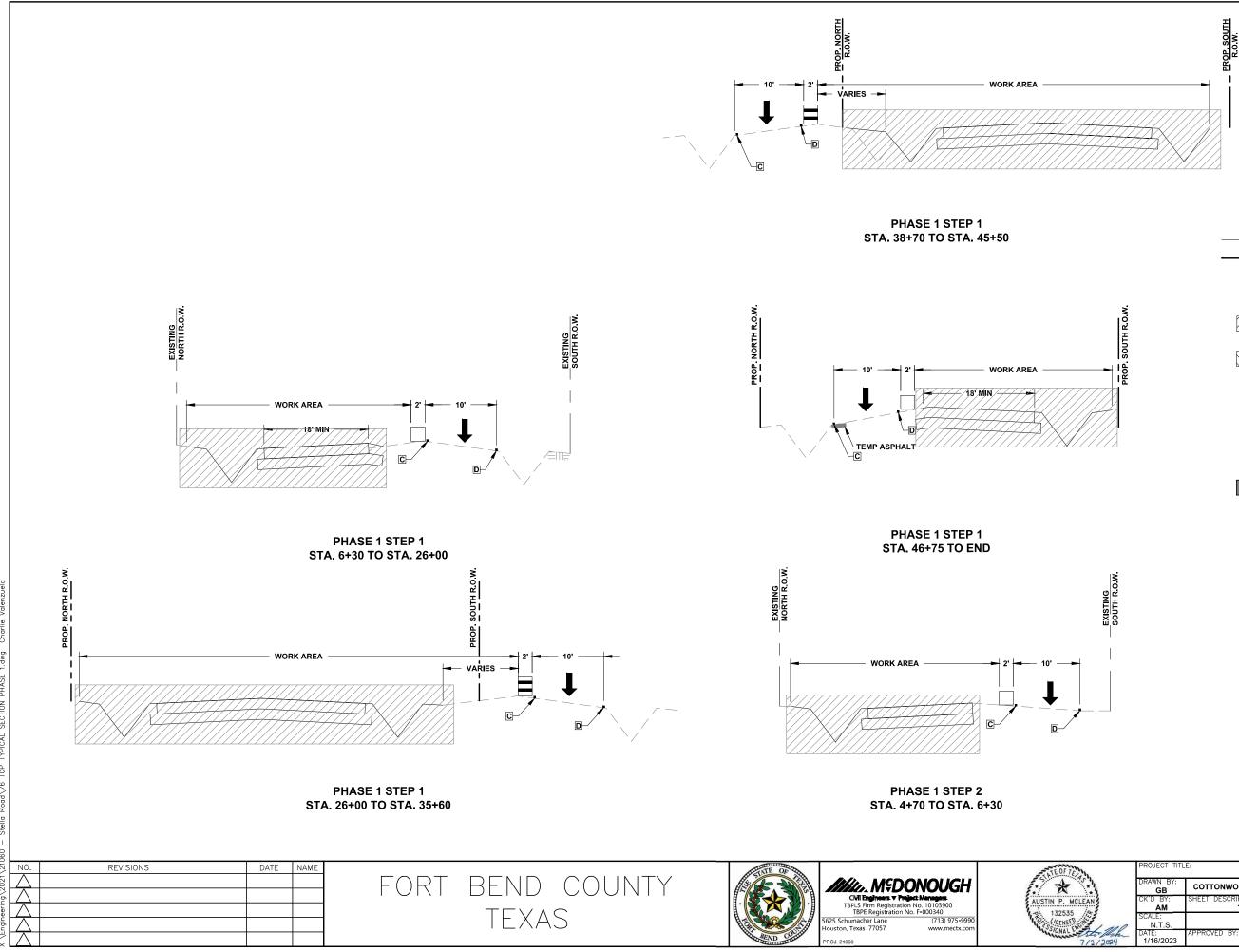
PROPOSED CULVERT LINK 5-YR RESULT

		10000		Max X-	Max	Maximum Water	Maximum Water Elevation
ne O	upstream Node Name 0	Node Name 0	Max How cfs	Sectional Area ft^2	velocity ft/s	Elevation (US) ft	(DS) ft
Cul	D1_DS D13.1 US	D2_US Out 1	-0.583 13.078	3.291 1.796	-0.216 7.505	96.36 93.554	96.359 90.823
_CUI Cul	D13.1_05 D3_D5	D4_US	6.496	2.827	2.467	93.554 94.382	90.823
Cul	D5_US	D4_DS	1.135	3.291 3.291	0.608	94.316	94.315
Cul	D6_US	D5_DS D8_US	-1.538	3.291	-0.586 0.717	94.316 92.82	94.316 92.812
Cul	D7_DS D4.1 DS	D8_US D16.1 US	1.069	1.888 3.292	0.567	92.82 94.314	92.812 94.256
ss3 ss3	D4.1_DS D4.1_DS	D16.1_US	5.558	3.292	1.765	94.314	94.256
553	D4.1_DS	D16.1_US	5.558 15.326	3.292	1.765	94.314 94.255	94.256 92.448
ut-2 _Cul	D16.2_DS D10_DS	Out_2 D11_US	3.536	3.062	1.16	94.255 95.844	92.448
_cui _Cul	D 12_DS D 13_DS	D13_US D14_US	-5.201	2.993	-2.265	93.555 94.329	93.554 94.328
Cul	D15_US	D14_05	-2.856	3.284	-0.907	94.258	94.328
_Cul	D18_US	D17_DS	1.83	3.291	0.93	94.259	94.258
ss4 ss4	D7_US D7_US	D19_US	1.724	0.895	1.926	93.566 93.566	93.283 93.201
Cul	D23_DS	D22_US	3.124	0.816	3.967	94.784	93.687
_Cul	D22_DS D14_DS	D11_US D15_US	12.999 3.693	7.279	1.932	94.328	92.792 94.277
_Cul	D15_DS	D16_US	2.88	3.292	1.262	94.273	94.257
ss2 3	D14_US D13.1_US	D4.1_US Basin1_Opt2	6.652 19.128	2.8 4.799	2.394	94.328 93.554	94.328 93.134
4	Basin1_Opt2	Out_1.2	11.511	1.549	7.504	93.134	90.008
Cul	D10_DS D14_DS	D11_US	0	0	0	-9.00E+99 -9.00E+99	-9.00E+99
CUL	D12_DS	D15_US D13_US	0	0	0	-9.00E+99	-9.00E+99
Cul Cul	D13_DS D15_DS	D14_US D16_US	0	0	0	-9.00E+99 -9.00E+99	-9.00E+99
Cul	D17_US	D16_DS	0	0	0	-9.00E+99	-9.00E+99
ul Cul	D12_US.1	D2B_US.1	5.422	3.307	1.64	95.798 -9.00E+99	95.759 -9.00E+99
Cul	D1_DS D3 DS	D2_US D4_US	0	0	0	-9.00E+99	-9.00E+99
<u>Cul</u> Cul	D18_US	D17_DS D5_DS	0	0	0	-9.00E+99 -9.00E+99	-9.00E+99
Cul	D6_US D5_US	D4_DS	0	0	0	-9.00E+99 -9.00E+99	-9.00E+99
	D1_US	D1_DS	-0.742	21.136	0.089	96.36	96.36
_	D10_US D11_US	D10_DS D11_DS	5.95 3.432	25.367 62.686	0.752	95.85 95.805	95.844 95.804
Cul	D11_D5	D12_US	-4.358	9.489	-0.605	95.804	95.798
.1	D12_US D12_US.1.1	D12_US.1 D12_DS	5.544	62.998 51.12	0.114	95.798 93.555	95.798 93.555
	D13_US	D13_US.1	-5.299	16.978	-0.491	93.554	93.554
1	D13_DS.2 D13_DS.1	D13_DS D13.1_US	-0.43 -0.828	5.84 32.629	-0.168 -0.054	94.329 93.554	94.329 93.554
	D13_US.1	D13.1_US	-5.539	25.65	-0.312	93.554	93.554
	D14_DS D15_US	D14_US D15_DS	4.333 3.529	8.201 9.955	0.947 4.818	94.328 94.277	94.328 94.273
	D16_US	D16.1_US	2.67	29.429	3.024	94.257	94.256
_	D16_DS D16.1_US	D16.1_US D16.2_DS	-2.995 15.437	40.685 75.916	0.256	94.257 94.256	94.256 94.255
	D17 DS	D17_US	1.48	33.893	0.508	94.258	94.258
1-2	D17_US D18_DS	Out_2 D18 US	0	31.93 15.364	0.731	94.258 94.259	92.8 94.259
	D19_US	Out3.1	4.01	3.085	1.3	93.201	92.3
_	D2_US D2_US.1	D2_US.2 D2_DS	3.108	24.231 61.327	0.225	96.359 95.045	96.355 95.042
	D2_US.2	D2B_US	10.372	3.287	3.178	96.355	95.813
1.1	D2B_US D2B_US.1	D2B_US.1 D2_US.1	13.957 26.972	18.289 22.158	0.862	95.813 95.759	95.759 95.045
	D20_US	Out_3	13.626	11.472	1.188	91.686	90.918
Cul	D21_D5 D21_US	D20_U5 D21 D5	12.846 12.937	5.083 7.299	2.527	92.085 92.782	91.686 92.085
	D22_US	D22_DS	13.127	9.993	1.332	93.687	92.963
Cul	D23_US D24_DS	D23_DS D23_US	13.132 -2.566	30.457 0.82	0.431	94.8 94.719	94.784 94.8
	D24_US	D24_DS	-2.506	32.72	0.49	94.719	94.719
Cul	D25_D5 D25_U5	D24_US D25_DS	-1.126	1.848 19.478	-0.636 -0.221	94.718 94.718	94.719 94.718
Cul	D26_D5	D25_US	-1.441	1.281	1.578	94.707	94.718
	D26_US D26.1 US	D26.1_US D26_DS	-0.738	50.979 38.363	0.031	94.707 94.707	94.707 94.707
Cul	D28_U5	D26_DS D26.1_US	-1.486	38.363	-5.798	94.707 94.22	94.707
	D27_US D27_DS	D27_DS	-0.392	19.784	0.08	94.219	94.218
_Cul	D27_DS D28_US	D28.1_US D28.1_US	-0.377 8.137	2.991 18.457	-0.404 1.378	94.218 94.22	94.216 94.216
Cul	D28.1_U5	D28_DS	9.125	26.893	0.472	94.216	94.177
cul	D28_DS D29_US	D29_US D29_DS	8.288	3.293 15.309	2.629 0.605	94.177 93.938	93.938 93.925
Cul	D29_D5	D30_US	8.28	3.292	2.547	93.925	93.706
	D3_US D3_DS.1	D3_DS D2_DS	-0.514 1.276	11.071 53.252	0.105	94.382 95.042	94.382 95.042
.	D30_US	D30_DS	8.273	10.838	0.787	93.706	93.567
Cul	D30_DS D31_US	D31_US D31_DS	8.272 8.273	3.199 6.212	2.586	93.567 93.339	93.339 92.991
_	D4_US	D4.1_US	6.237	12.379	0.832	94.338	94.328
	D4.1_US D4.1_US.1	D4.1_US.1 D4.1_DS	8.298 18.585	16.791 34.18	0.994 0.983	94.328 94.323	94.323 94.314
	D4_DS	D4.1_DS	1.303	34.819	0.314	94.315	94.314
	D5_DS D6_DS	D5_US D6_US	1.236 -0.642	25.697 18.547	0.512 0.251	94.316 94.316	94.316 94.316
ul	D6_DS	D7_US	0	0.001	0.429	94.316	93.566
_	D7_US D7.1_US	D7_US.1 D7_DS	0.293	6.57 2.337	0.071	93.566 93.054	93.567 92.82
	D7_US.1	D7.1_US	2.321	6.232	0.373	93.567	93.054
_	D8_US D8_DS	D8_DS Out 4	2.205	2.873 6.128	0.769	92.812 92.654	92.654 92.149
utL	DW15_Out	D_Co_Out	0.843	0.919	0.917	95.429	95.156
0 1	DW1N_Out Out_1	DW15_Out Out_1.2	0.843 13.078	0.675 13.207	1.25 0.994	95.473 90.07	95.429 90.008
1.1	Out_1.2	Out_1.1	24.303	13.826	1.758	90.008	89.483
.1	DW2N_US.1 DW2N_US.1	DW2N_US DW1N_Out	1.243 0.845	1.24 1.172	1.095	9.60E+01 96.004	9.59E+01 95.473
	DW25_DS	DW15_Out	0	0.078	0	-9.00E+99	-9.00E+99
N .	DW2N_US DW2S_DS	D10_US D1_US	1.229	0.816	2.292	9.59E+01 -9.00E+99	9.59E+01 -9.00E+99
1	Basin1_Opt2	Out_1.2	0	0	0	-9.00E+99	-9.00E+99
Out 5	Sheriff_Outfall	Basin1_Opt2 Node651	57.745	7.567	3.839	93.877	93.134
5	Node650 Node652	D23_US	0 5.923	0 5.135	0 1.198	-9.00E+99 9.49E+01	-9.00E+99 9.48E+01
7	Node653	Node652	6.337	5.139	1.281	95.035	94.91
8 ut-2	Node653 D16.2_DS	D26.1_US Out_2	9.135	5.136 0	1.847	95.035 -9.00E+99	94.707 -9.00E+99
21	Out_2	Out_2.2	15.326	7.976	1.922	8.83E+01	8.79E+01
1 is1	D3_US D2_DS	D3_D5.1 D13.1 US	0 36.6	0 5.102	0	0.00E+00 95.042	0.00E+00 93.554
555	D31_DS	D8_DS	8.28	3.01	2.756	92.991	92.654
Cul	D22_DS	D21_US	0	0	0	-9.00E+99	-9.00E+99

NOTE:

NODES LOCATION CAN BE FOUND IN THE DRAINAGE REPORT TITLED "DRAINAGE IMPACT ANALYSIS FOR STELLA ROAD EXPANSION WITHIN SEABOURNE CREEK WATERSHED FORT BEND, TEXAS", EXHIBIT 8A.

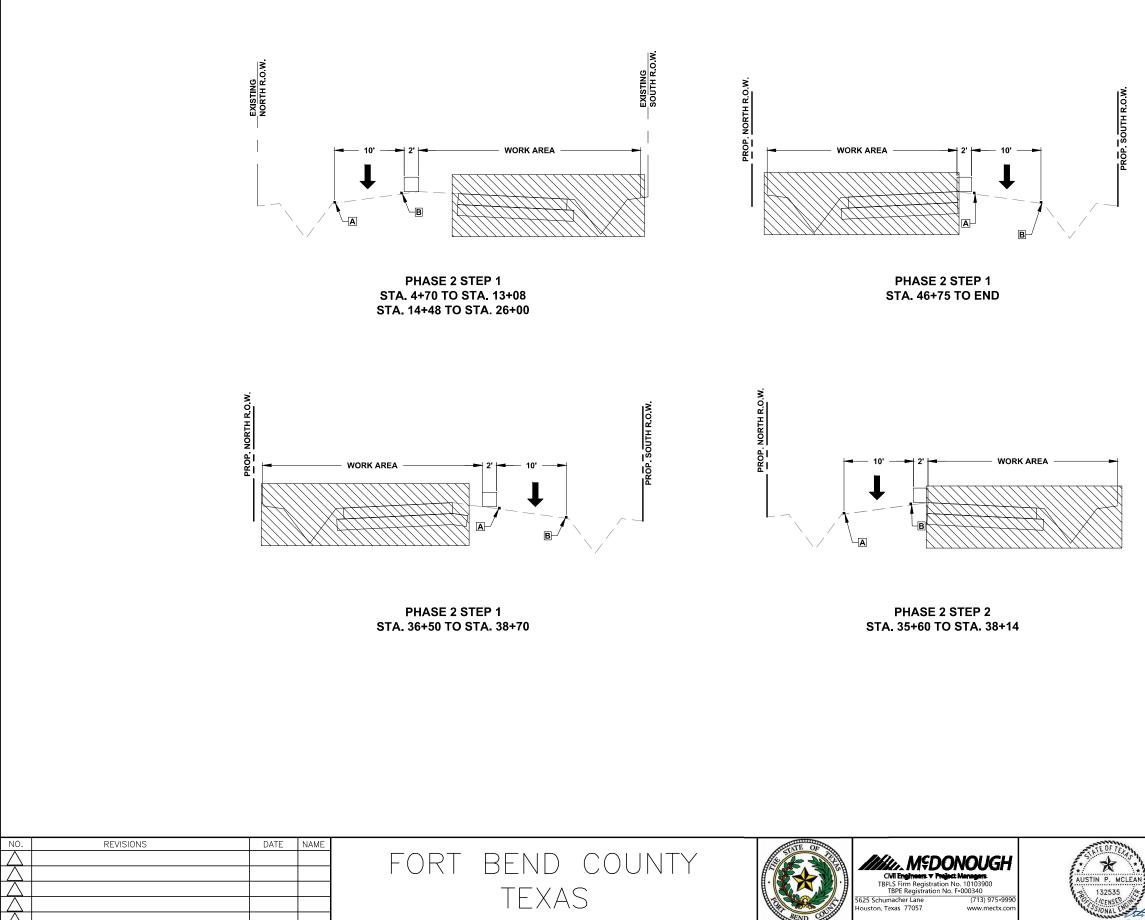
	PROJECT TITL	E: STELLA ROAD	
	FROM CO	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
yung	SHEET DESCR	PROPOSED CULVERT LINK RESULTS	
6/18/24	DRAWN BY: NS		DATE: 6/18/24
,, 10, 21	CK'D BY: MJ	SCALE: N/A	SHEET NO: 75 / 133



<u></u>	EGEND
	EXIST. R.O.W.
	PROP. R.O.W.
→	PROP. TRAFFIC FLOW
\rightarrow	EXIST. TRAFFIC FLOW
	CONSTRUCTION PHASE 1
	CONSTRUCTION PHASE 2
	PORT CTB LOW PROFILE (LPCB) TY 1
	PORT CTB LOW PROFILE (LPCB) TY 2
Ľ	TYPE III BARRICADE
•	CHANNELIZING DEVICE
	TEMP ASPHALT
Α	4" WHITE SOLID REMOVABLE
В	4" YELLOW SOLID REMOVABLE
С	4" WHITE SOLID NON-REMOVABLE
D	4" YELLOW SOLID NON-REMOVABLE
E	24" WHITE SOLID

h.	PROJECT TITL		
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: TCP TYPICAL SECTION PHASE 1	
	SCALE: N.T.S.		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	76 / 133

LEGEND



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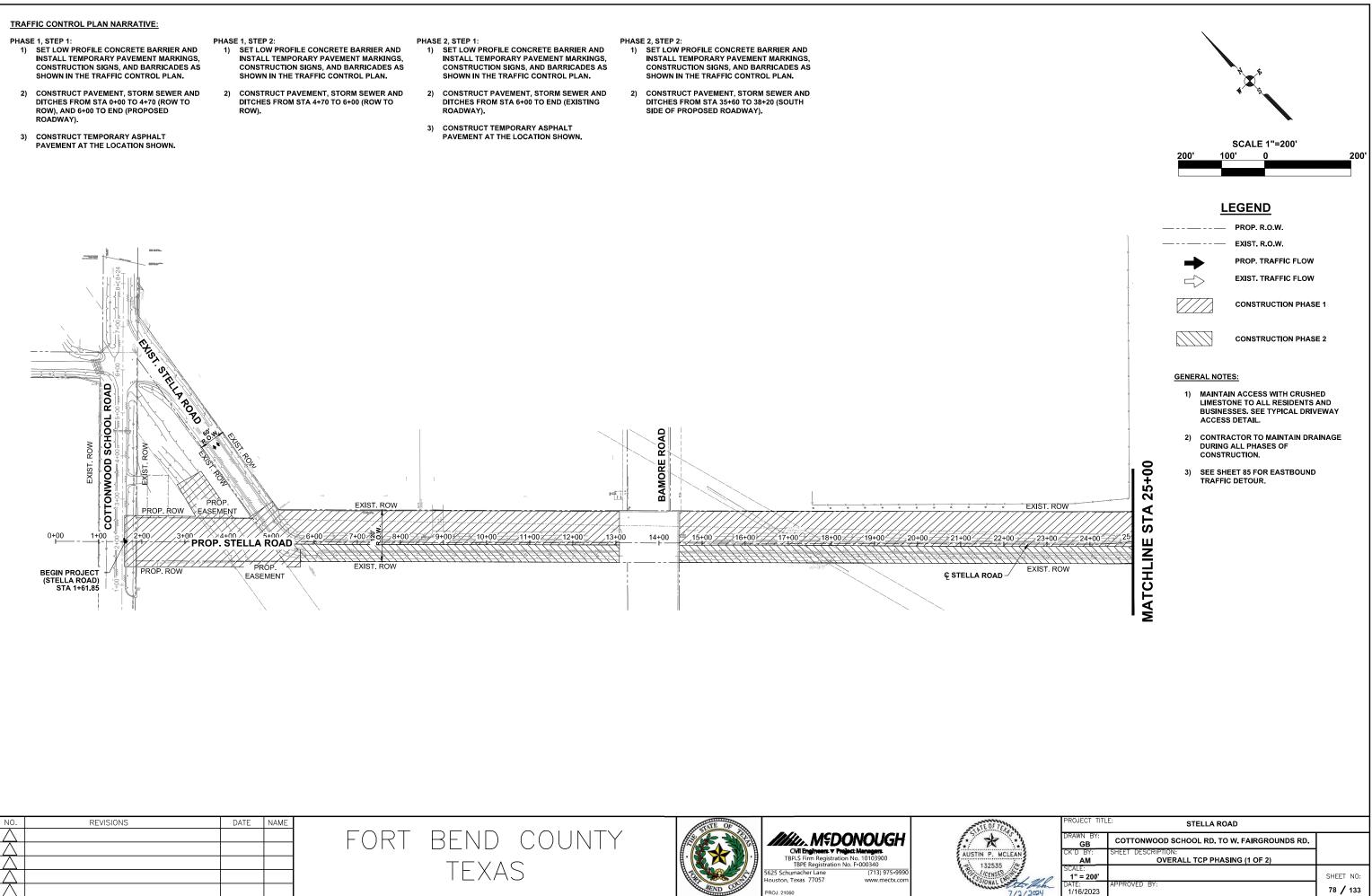
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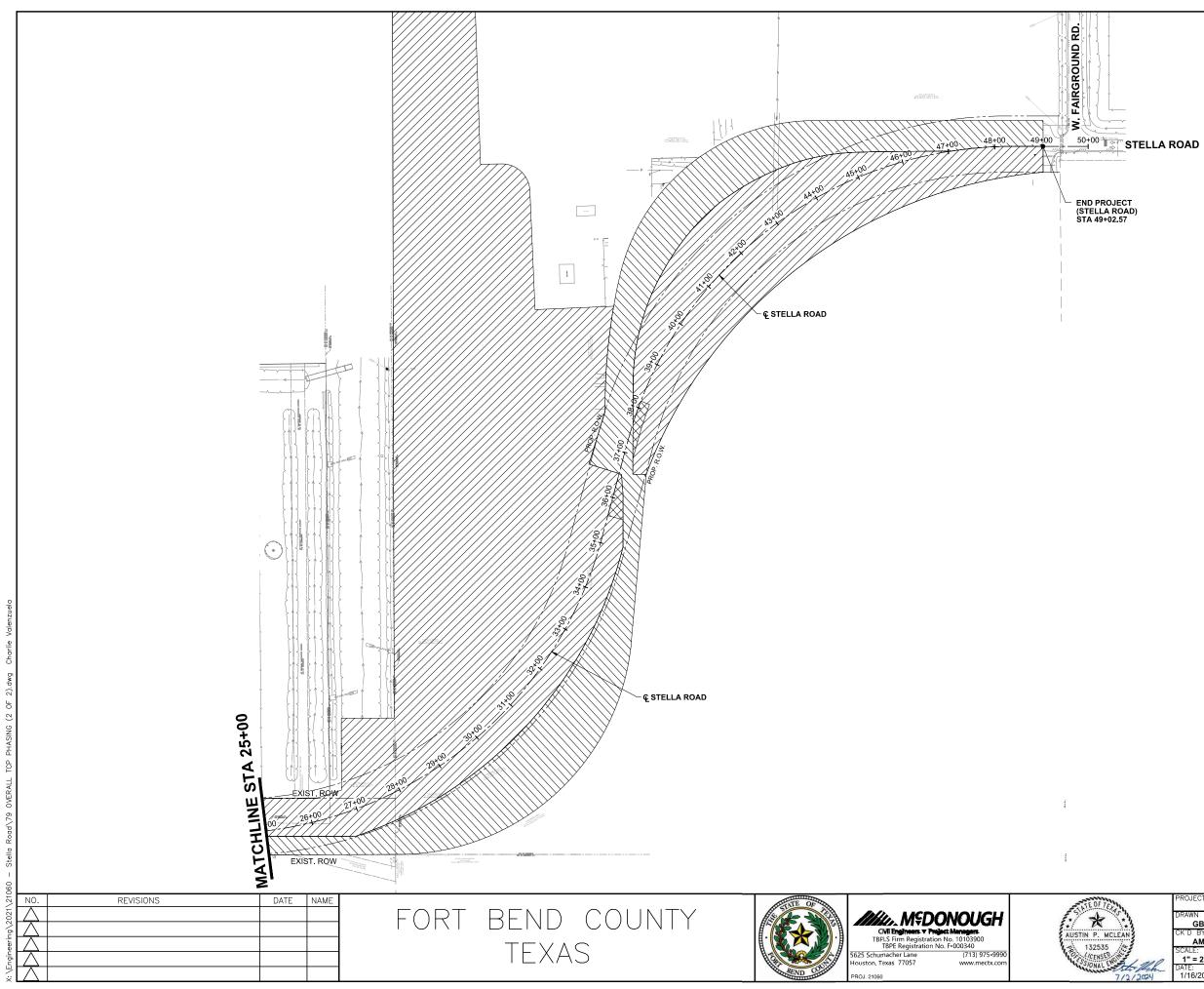
	EXIST. R.O.W.
	PROP. R.O.W.
→	PROP. TRAFFIC FLOW
\leq	EXIST. TRAFFIC FLOW
	CONSTRUCTION PHASE 1
	CONSTRUCTION PHASE 2
	PORT CTB LOW PROFILE (LPCB) TY 1
	PORT CTB LOW PROFILE (LPCB) TY 2
ľ:	TYPE III BARRICADE
•	CHANNELIZING DEVICE
	TEMP ASPHALT
A	4" WHITE SOLID REMOVABLE
В	4" YELLOW SOLID REMOVABLE
С	4" WHITE SOLID NON-REMOVABLE
D	4" YELLOW SOLID NON-REMOVABLE
E	24" WHITE SOLID

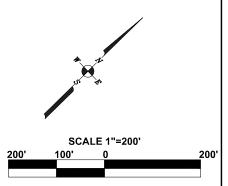
h),	PROJECT TITL	E: STELLA ROAD	
(°, ***)	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: TCP TYPICAL SECTION PHASE 2	
	SCALE: N.T.S.		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	77 / 133

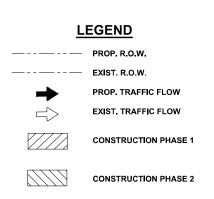
- INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS
- CONSTRUCT TEMPORARY ASPHALT

- SHOWN IN THE TRAFFIC CONTROL PLAN.
- - SHOWN IN THE TRAFFIC CONTROL PLAN.
- DITCHES FROM STA 6+00 TO END (EXISTING ROADWAY)
- PAVEMENT AT THE LOCATION SHOWN.
- INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS SHOWN IN THE TRAFFIC CONTROL PLAN.

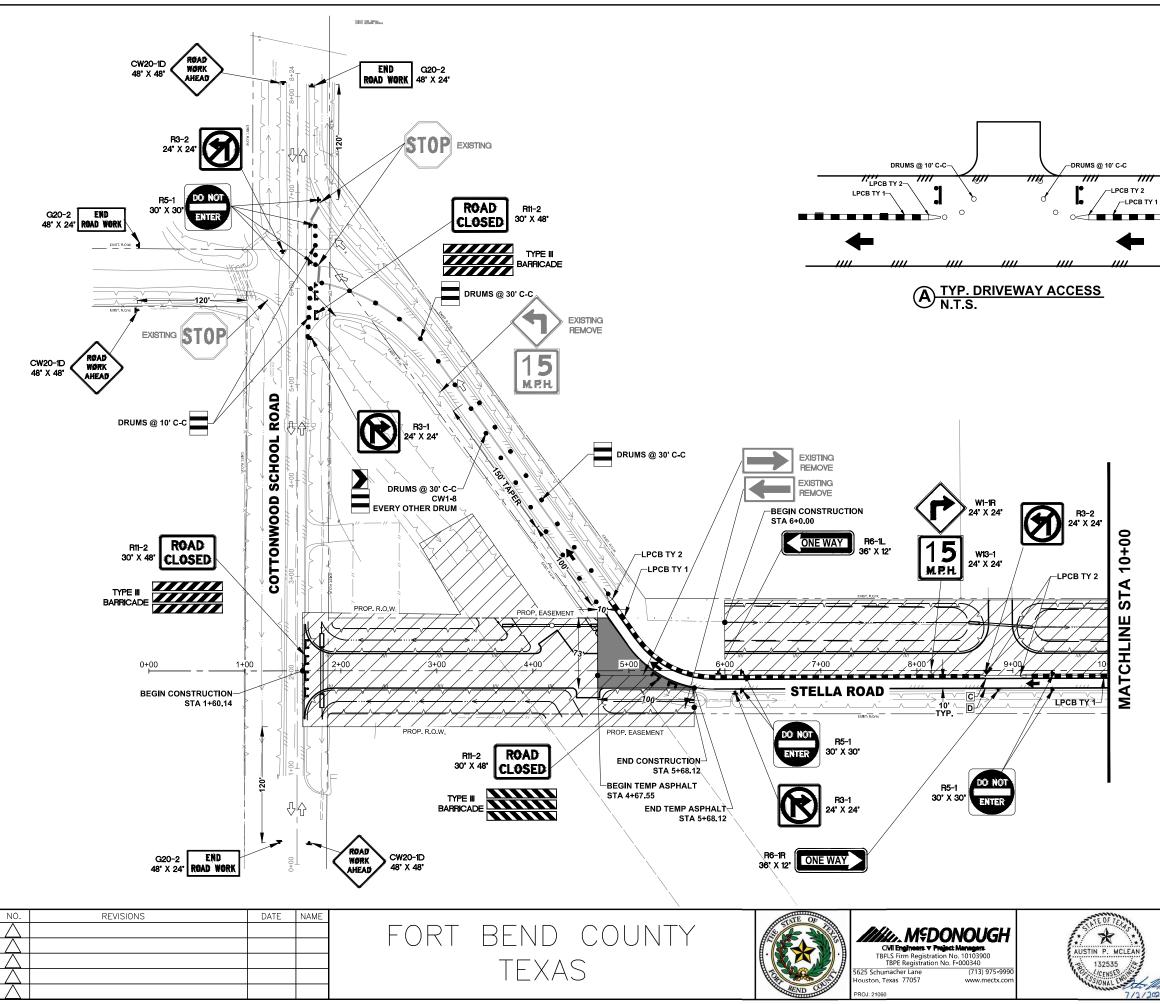






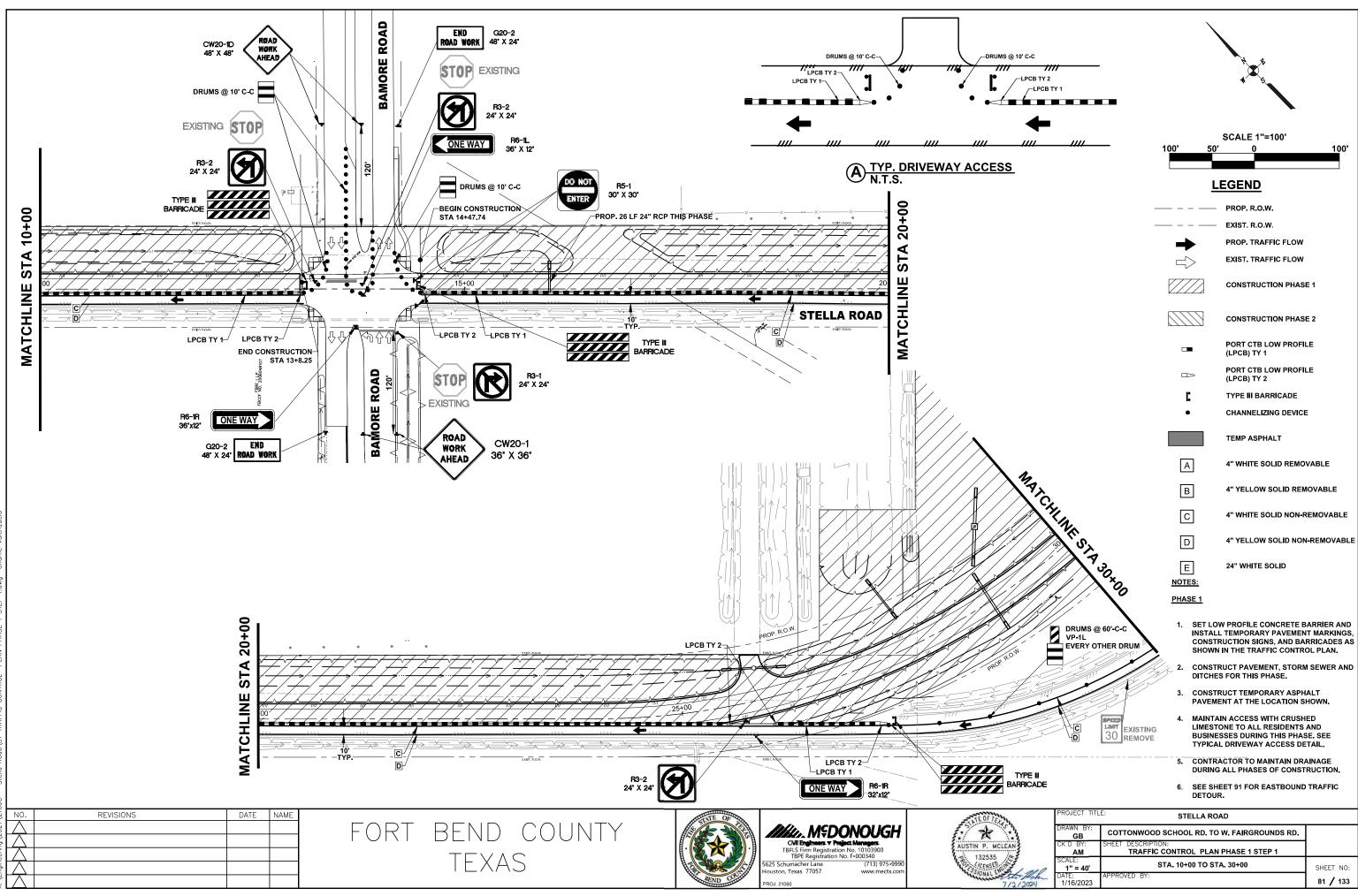


	PROJECT TITL	E: STELLA ROAD	
***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: OVERALL TCP PHASING (2 OF 2)	
New I MA	SCALE: 1" = 200'		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	79 / 133

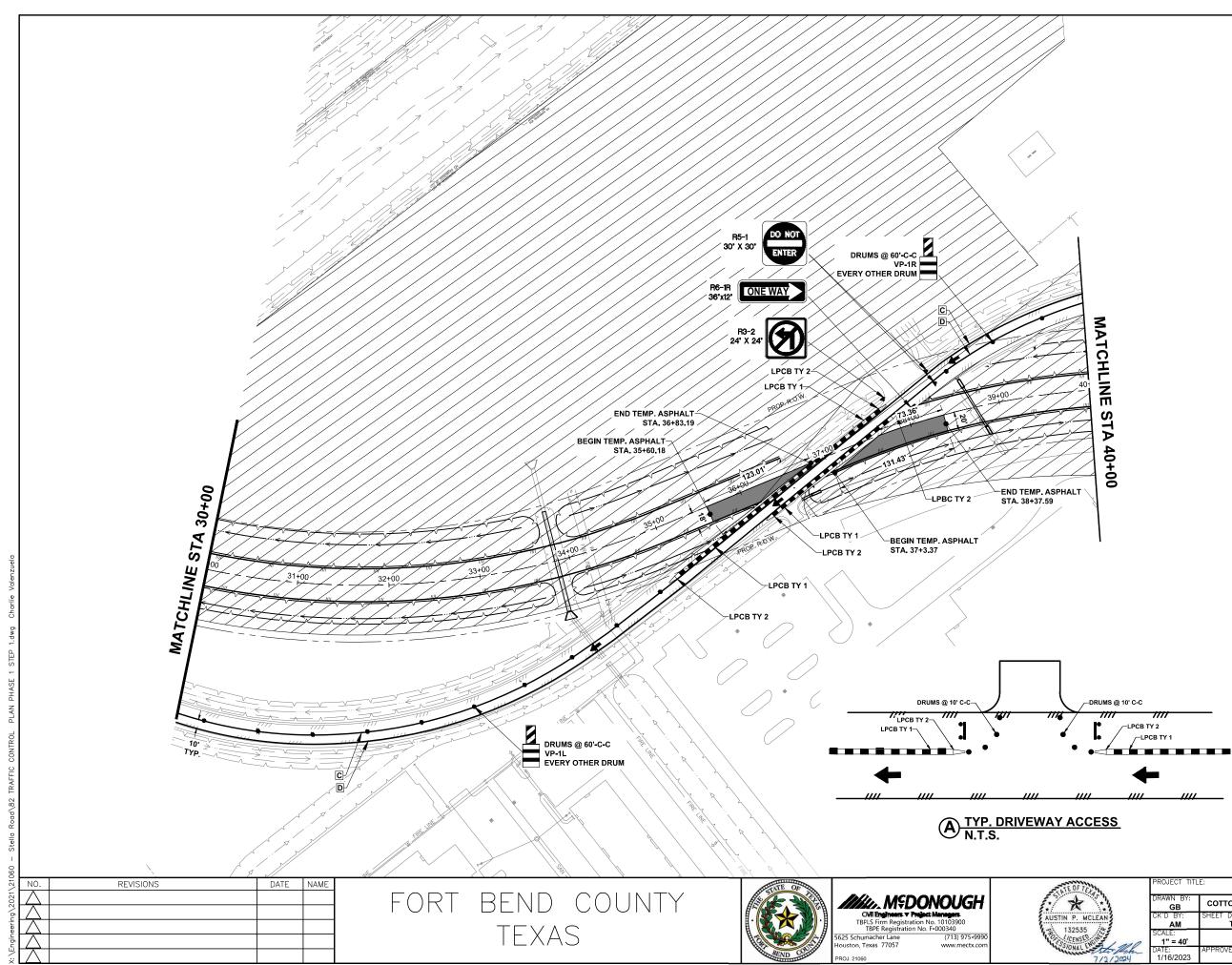


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			SCALE 1"=100'
		10 <u>0' 50'</u>	0 100'
-		<u>L</u>	EGEND
			PROP. R.O.W.
	_		EXIST. R.O.W PROP. TRAFFIC FLOW
			EXIST. TRAFFIC FLOW
			CONSTRUCTION PHASE 1
			CONSTRUCTION PHASE 2
			PORT CTB LOW PROFILE (LPCB) TY 1
			PORT CTB LOW PROFILE (LPCB) TY 2
		Ľ	TYPE III BARRICADE
		•	CHANNELIZING DEVICE
			TEMP ASPHALT
		Α	4" WHITE SOLID REMOVABLE
		В	4" YELLOW SOLID REMOVABLE
		С	4" WHITE SOLID NON-REMOVABLE
		D	4" YELLOW SOLID NON-REMOVABLE
		E	24" WHITE SOLID
	<u>NOT</u>	ES:	
	PHA:	<u>SE 1</u>	
	1.	INSTALL TEMP	FILE CONCRETE BARRIER AND ORARY PAVEMENT MARKINGS, N SIGNS, AND BARRICADES AS TRAFFIC CONTROL PLAN.
	2.	CONSTRUCT PA	AVEMENT, STORM SEWER AND THIS PHASE.
	3.	CONSTRUCT TI THE LOCATION	EMPORARY ASPHALT PAVEMENT AT I SHOWN.
	4.	ALL RESIDENT	ESS WITH CRUSHED LIMESTONE TO S AND BUSINESSES DURING THIS /PICAL DRIVEWAY ACCESS DETAIL.
	5.		TO MAINTAIN DRAINAGE DURING OF CONSTRUCTION.
	6.	SEE SHEET 91	FOR EASTBOUND TRAFFIC DETOUR.
TITLE:		STELLA RO	AD

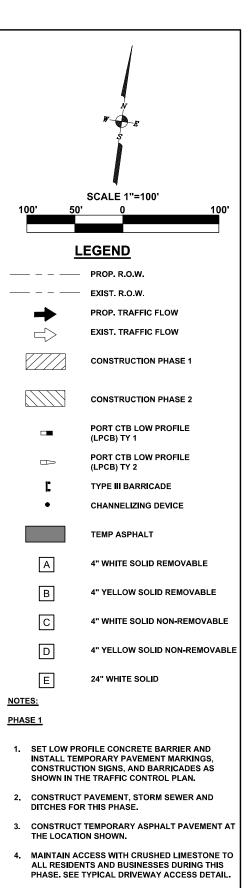
h.	PROJECT THE	STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 1 STEP 1	
	SCALE: 1" = 40'	STA. 0+00 TO STA. 10+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	80 / 133



1	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 1 STEP 1	
CHARTER L THE	SCALE: 1" = 40'	STA. 10+00 TO STA. 30+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	81 / 133

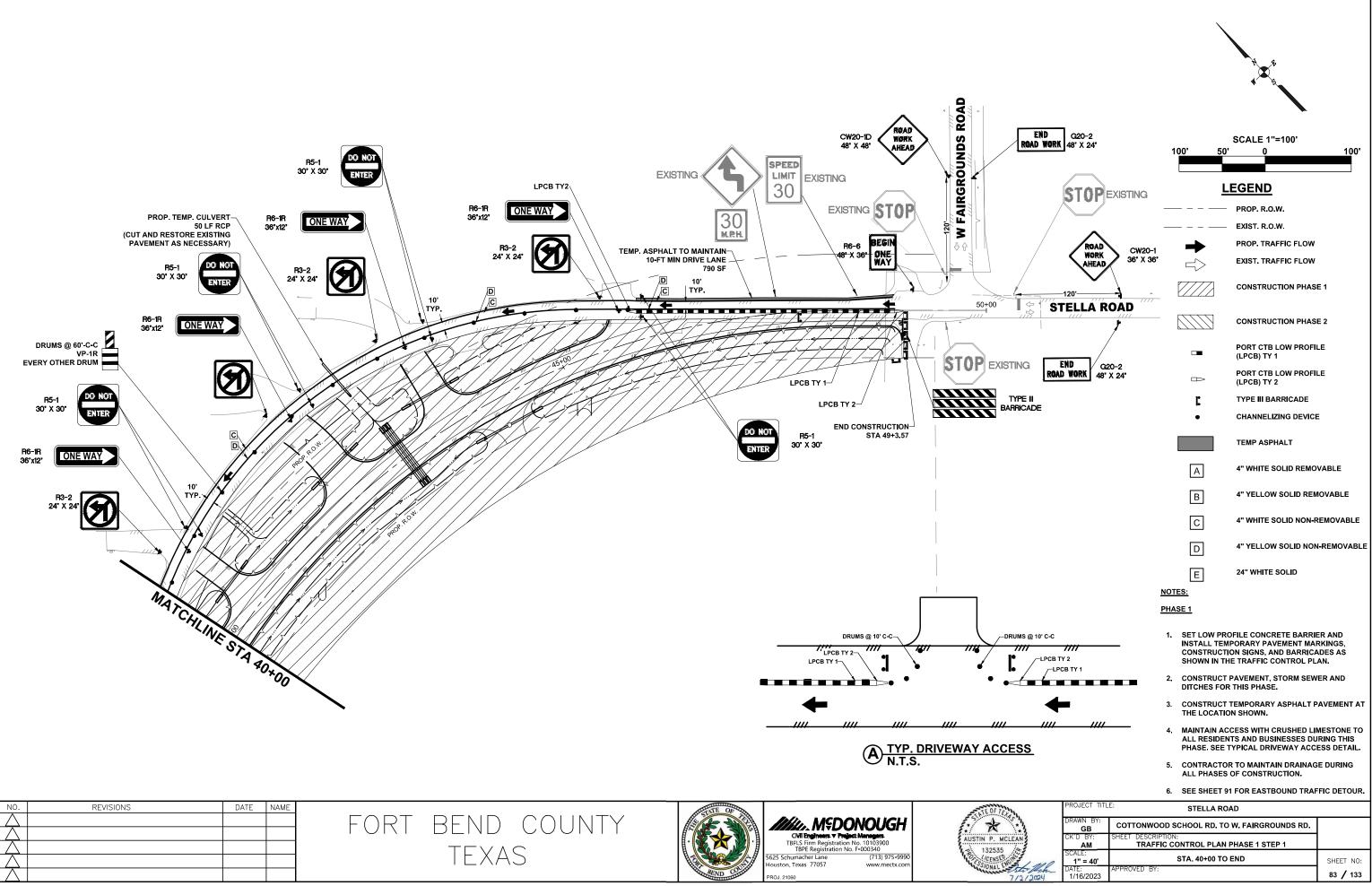


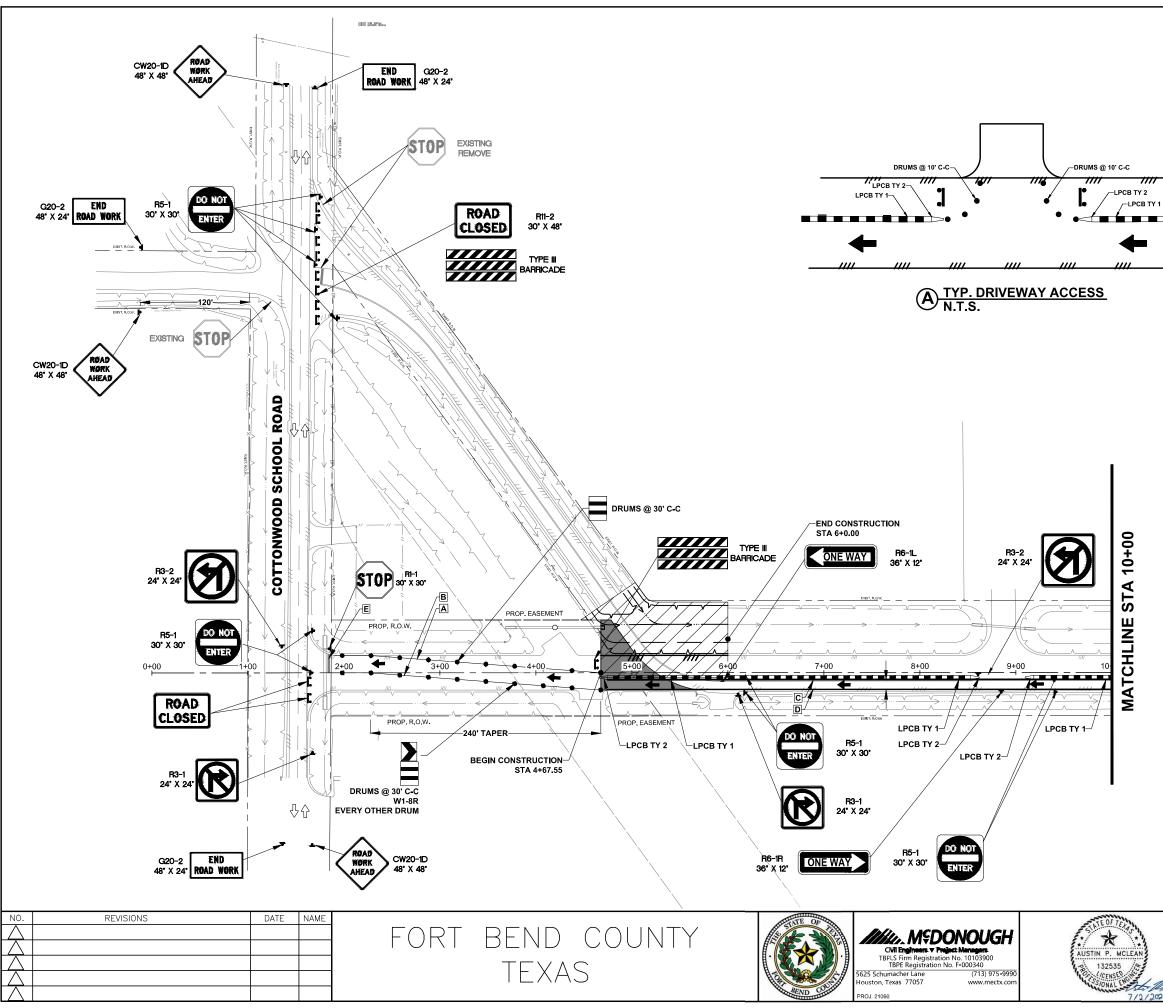
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- 5. CONTRACTOR TO MAINTAIN DRAINAGE DURING ALL PHASES OF CONSTRUCTION.
- 6. SEE SHEET 91 FOR EASTBOUND TRAFFIC DETOUR.

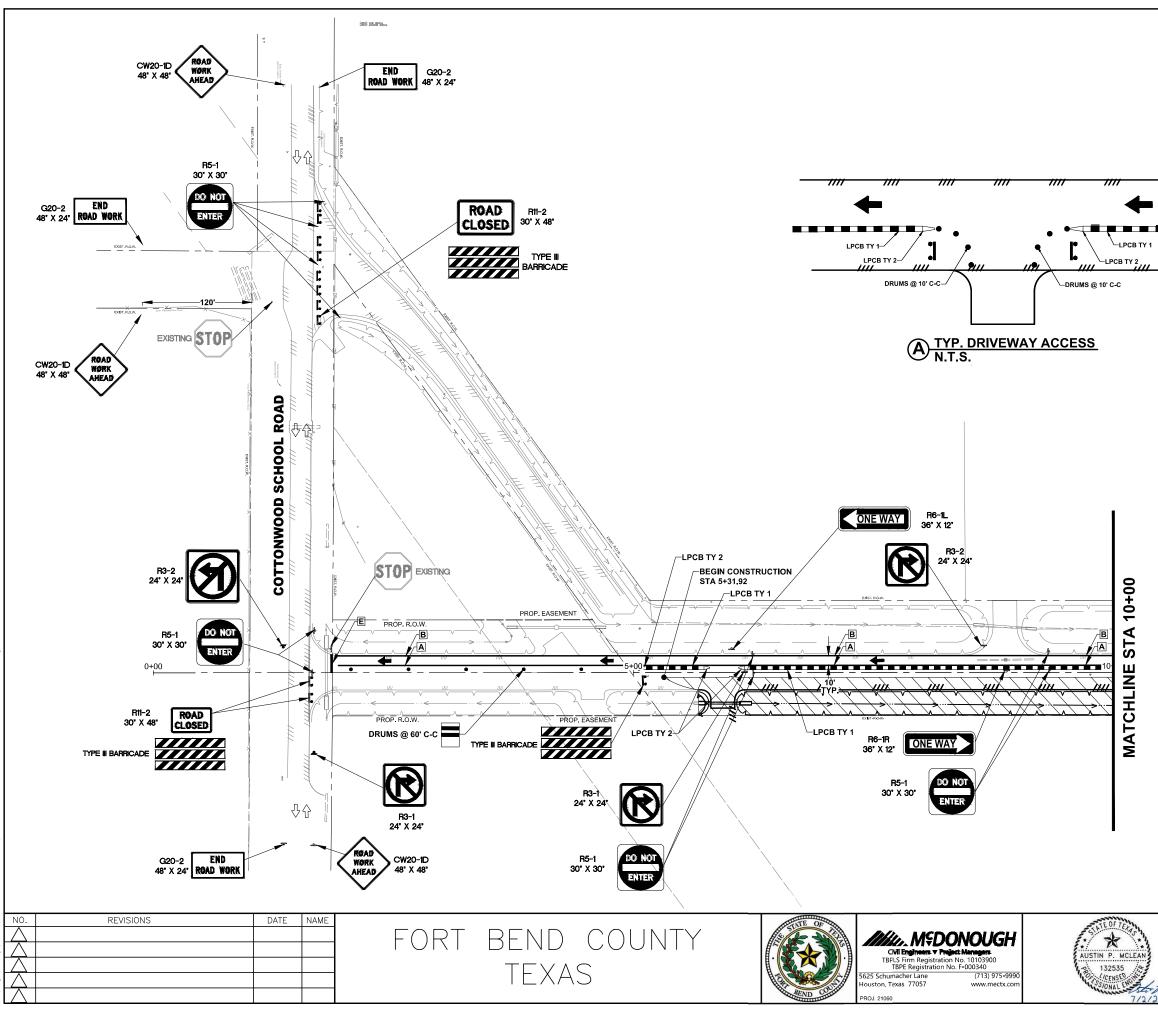
) .	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 1 STEP 1	
	SCALE: 1" = 40'	STA. 30+00 TO STA. 40+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	82 / 133





SCALE 1"=100' 100' 50' 0 100 LEGEND PROP. R.O.W. EXIST. R.O.W. PROP. TRAFFIC FLOW EXIST. TRAFFIC FLOW CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE CHANNELIZING DEVICE	
LEGEND PROP. R.O.W. EXIST. R.O.W. → PROP. TRAFFIC FLOW → EXIST. TRAFFIC FLOW CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 1 OCONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE OCHANNELIZING DEVICE	.
PROP. R.O.W. EXIST. R.O.W. PROP. TRAFFIC FLOW EXIST. TRAFFIC FLOW CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE C CHANNELIZING DEVICE	
PROP. R.O.W. EXIST. R.O.W. PROP. TRAFFIC FLOW EXIST. TRAFFIC FLOW CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE C CHANNELIZING DEVICE	
EXIST. R.O.W. PROP. TRAFFIC FLOW EXIST. TRAFFIC FLOW CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 TYPE III BARRICADE CHANNELIZING DEVICE	
EXIST. TRAFFIC FLOW CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE C CHANNELIZING DEVICE	
CONSTRUCTION PHASE 1 CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE C CHANNELIZING DEVICE	
CONSTRUCTION PHASE 2 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 C TYPE III BARRICADE CHANNELIZING DEVICE	
 PORT CTB LOW PROFILE (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 TYPE III BARRICADE CHANNELIZING DEVICE 	
 (LPCB) TY 1 PORT CTB LOW PROFILE (LPCB) TY 2 TYPE III BARRICADE CHANNELIZING DEVICE 	
 (LPCB) TY 2 TYPE III BARRICADE CHANNELIZING DEVICE 	
CHANNELIZING DEVICE	
ILWIP ASPRALI	
A 4" WHITE SOLID REMOVABLE	
B 4" YELLOW SOLID REMOVABLE	
C 4" WHITE SOLID NON-REMOVABLE	:
D 4" YELLOW SOLID NON-REMOVAB	LE
E 24" WHITE SOLID	
NOTES:	
PHASE 1	
1. SET LOW PROFILE CONCRETE BARRIER AND INSTALL TEMPORARY PAVEMENT MARKINGS, CONSTRUCTION SIGNS, AND BARRICADES AS SHOWN IN THE TRAFFIC CONTROL PLAN.	
2. CONSTRUCT PAVEMENT, STORM SEWER AND DITCHES FOR THIS PHASE.	
3. CONSTRUCT TEMPORARY ASPHALT PAVEMENT A THE LOCATION SHOWN.	чц
4. MAINTAIN ACCESS WITH CRUSHED LIMESTONE T ALL RESIDENTS AND BUSINESSES DURING THIS PHASE. SEE TYPICAL DRIVEWAY ACCESS DETAIL	
5. CONTRACTOR TO MAINTAIN DRAINAGE DURING A PHASES OF CONSTRUCTION.	
6. SEE SHEET 91 FOR EASTBOUND TRAFFIC DETOU	
LE: STELLA ROAD	R.

h .	PROJECT TITL	E: STELLA ROAD	
***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 1 STEP 2	
1 110	SCALE: 1" = 40'	STA. 0+00 TO STA. 10+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	84 / 133

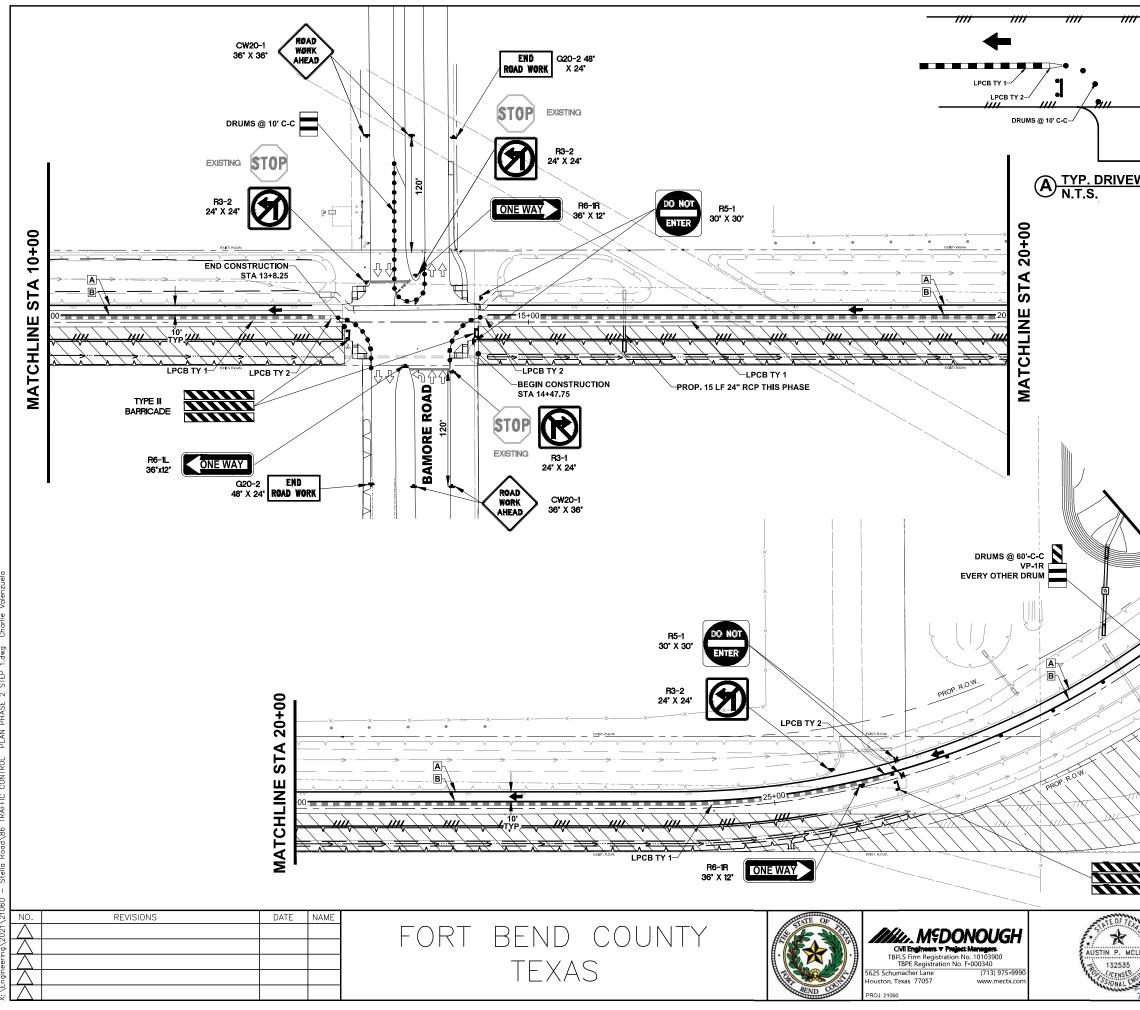


:\Engineering\2021\21060 - Stella Road\85 TRAFFIC CONTROL PLAN PHASE 2 STEP 1.dwg Charlie Val

	THE SECOND
	SCALE 1"=100'
100' 50'	0 100'
	EGEND
	PROP. R.O.W.
	EXIST. R.O.W.
→	PROP. TRAFFIC FLOW
	EXIST. TRAFFIC FLOW
	CONSTRUCTION PHASE 1
	CONSTRUCTION PHASE 2
	PORT CTB LOW PROFILE (LPCB) TY 1
	PORT CTB LOW PROFILE (LPCB) TY 2
Ľ	TYPE III BARRICADE
•	CHANNELIZING DEVICE
	TEMP ASPHALT
A	4" WHITE SOLID REMOVABLE
В	4" YELLOW SOLID REMOVABLE
С	4" WHITE SOLID NON-REMOVABLE
D	4" YELLOW SOLID NON-REMOVABLE
E	24" WHITE SOLID
NOTES:	
PHASE 2	
INSTALL T	PROFILE CONCRETE BARRIER AND EMPORARY PAVEMENT MARKINGS, CTION SIGNS, AND BARRICADES AS THE TRAFFIC CONTROL PLAN.
	CT PAVEMENT, STORM SEWER AND OR THIS PHASE.
	CT TEMPORARY ASPHALT PAVEMENT AT TION SHOWN.
ALL RESID	ACCESS WITH CRUSHED LIMESTONE TO SENTS AND BUSINESSES DURING THIS EE TYPICAL DRIVEWAY ACCESS DETAIL.
	TOR TO MAINTAIN DRAINAGE DURING ALL F CONSTRUCTION.

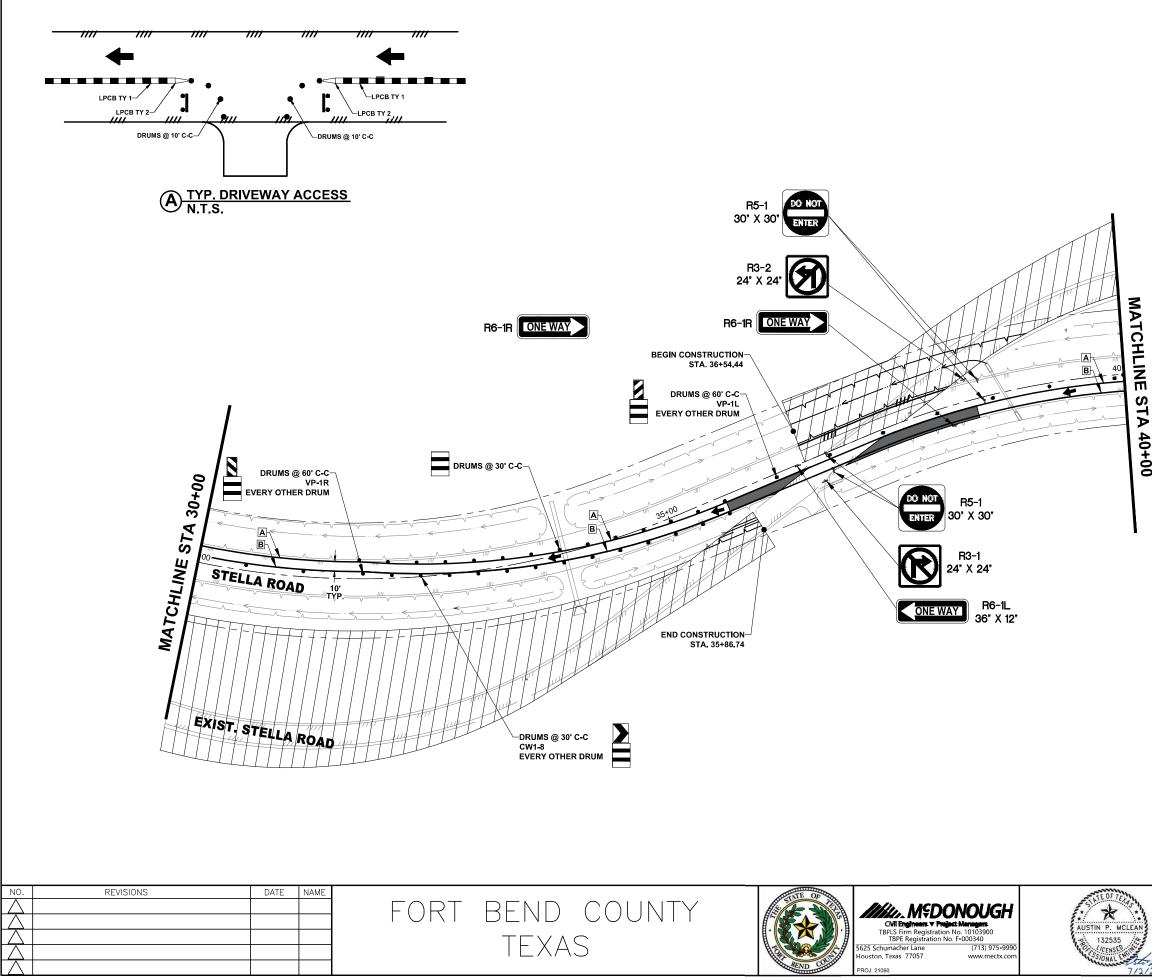
6. SEE SHEET 91 FOR EASTBOUND TRAFFIC DETOUR.

	PROJECT TITL	E: STELLA ROAD	
**** ****	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 2 STEP 1	
	SCALE: 1" = 40'	STA. 0+00 TO STA. 10+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	85 / 133



\Engineering\2021\21060 - Stella Road\86 TRAFFIC CONTROL PLAN PHASE 2 STEP 1.dwg Charlie Vc

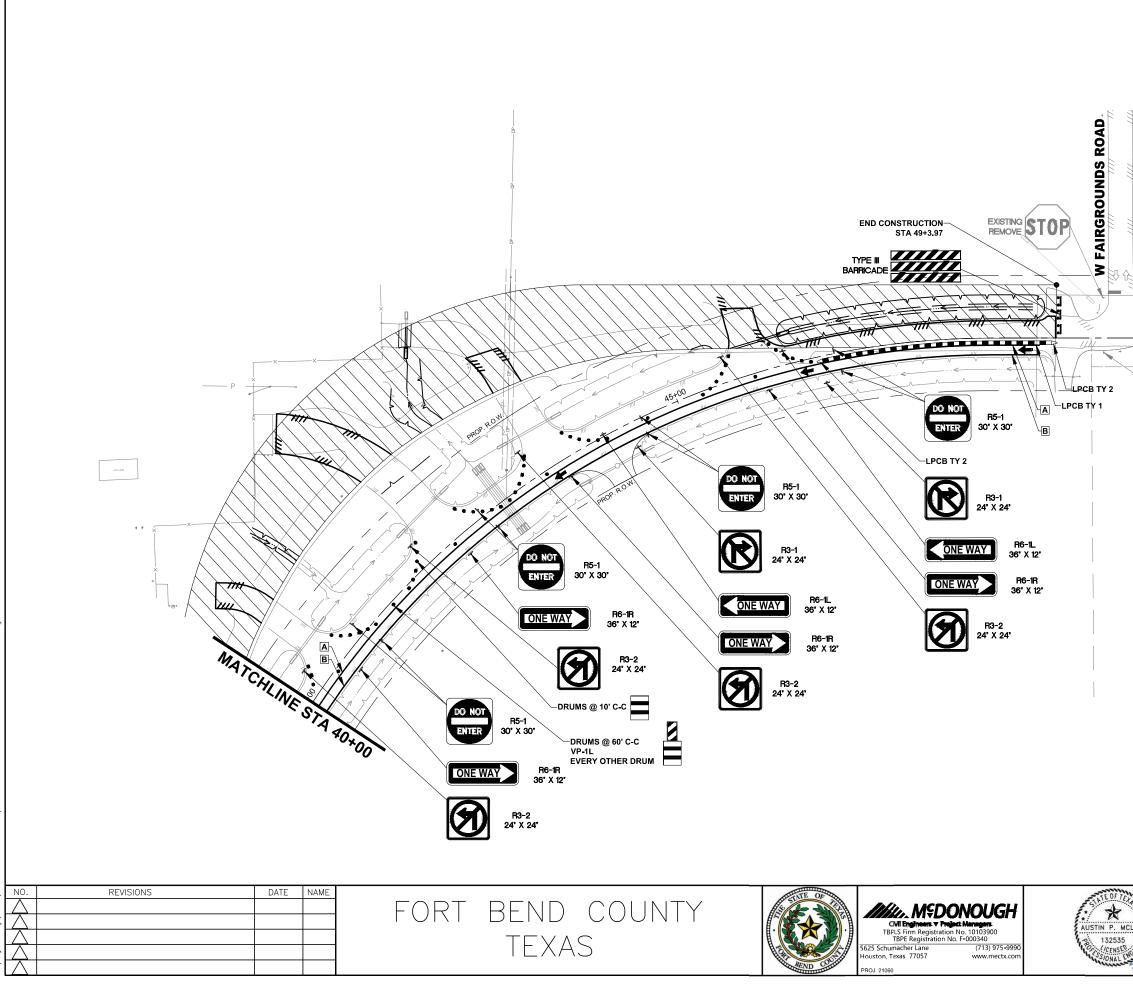
n m			— 、		
		4			
			·		
• •	<			H B	
٩	[PCB TY 1		A S	
		3 TY 2			
	DRUMS @ 10' C-C				
				SCALE 1"=100'	
		1	00' 50'	0	100'
WAY ACC	<u>ESS</u>				
			L	EGEND	
				PROP. R.O.W.	
				EXIST. R.O.W.	
			→	PROP. TRAFFIC FLOW	
			\leq	EXIST. TRAFFIC FLOW	
			~ ~		- /
			\mathbb{Z}	CONSTRUCTION PHAS	E 1
			$\overline{)}$	CONSTRUCTION PHAS	E 2
				PORT CTB LOW PROFI	LE
			В	(LPCB) TY 1 PORT CTB LOW PROFI	LE
				(LPCB) TY 2	
			.	TYPE III BARRICADE	
			•	CHANNELIZING DEVIC	E
				TEMP ASPHALT	
1			Α	4" WHITE SOLID REMO	VABLE
Tr.			В	4" YELLOW SOLID REM	IOVABLE
	KIN.		С	4" WHITE SOLID NON-F	REMOVABLE
	IIINE SI P		D	4" YELLOW SOLID NO	N-REMOVABLE
	Nr.	2	E	24" WHITE SOLID	
K/]	/,X	X	NOTES:		
		12	PHASE 2		
	\land				
			INSTALL MARKIN BARRIC	V PROFILE CONCRETE E . TEMPORARY PAVEMEN GS, CONSTRUCTION SIG ADES AS SHOWN IN THE	IT SNS, AND
	\searrow		2. CONSTR	DL PLAN. RUCT PAVEMENT, STORM	
	JAX'		3. CONSTR	CHES FOR THIS PHASE.	IALT
HAT	$// \rangle$	×			
			LIMESTO	IN ACCESS WITH CRUSH ONE TO ALL RESIDENTS SSES DURING THIS PHAS DRIVEWAY ACCESS DE	AND SE. SEE
			5. CONTRA	ACTOR TO MAINTAIN DR.	AINAGE
BARRIC	ADE			ET 91 FOR EASTBOUND	
24	PROJECT TITL	E:	STELLA ROA		
10 × 10	DRAWN BY: GB	COTTONWOOD	SCHOOL RD. TO	W. FAIRGROUNDS RD.	
CLEAN	CK'D BY: AM	SHEET DESCRIPTION		PHASE 2 STEP 1	1
No.	SCALE: 1" = 40'		A. 10+00 TO ST		SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:			86 / 133
11 x1 duay					



MATCHLINE STA 40+00

	100' 50'	SCALE 1"=100' 0 100' EGEND			
		PROP. R.O.W.			
		EXIST. R.O.W.			
	•	PROP. TRAFFIC FLOW			
	\leq	EAGT. TRAFFIC FLOW			
		CONSTRUCTION PHASE 1			
		CONSTRUCTION PHASE 2			
		PORT CTB LOW PROFILE (LPCB) TY 1			
		PORT CTB LOW PROFILE (LPCB) TY 2			
	Ľ	TYPE III BARRICADE			
	•	CHANNELIZING DEVICE			
		TEMP ASPHALT			
	А	4" WHITE SOLID REMOVABLE			
	В	4" YELLOW SOLID REMOVABLE			
	С	4" WHITE SOLID NON-REMOVABLE			
	D	4" YELLOW SOLID NON-REMOVABLE			
NOTI	<u>es:</u> E	24" WHITE SOLID			
PHAS	<u>SE 2</u>				
1.	SET LOW PROFILE CONC INSTALL TEMPORARY PA CONSTRUCTION SIGNS, SHOWN IN THE TRAFFIC	AVEMENT MARKINGS, AND BARRICADES AS			
2.	CONSTRUCT PAVEMENT, STORM SEWER AND DITCHES FOR THIS PHASE.				
3.	3. CONSTRUCT TEMPORARY ASPHALT PAVEMENT AT THE LOCATION SHOWN.				
4.	4. MAINTAIN ACCESS WITH CRUSHED LIMESTONE TO ALL RESIDENTS AND BUSINESSES DURING THIS PHASE. SEE TYPICAL DRIVEWAY ACCESS DETAIL.				
5.	CONTRACTOR TO MAINT PHASES OF CONSTRUCT	AIN DRAINAGE DURING ALL TON.			
6.	SEE SHEET 91 FOR EAST	BOUND TRAFFIC DETOUR.			

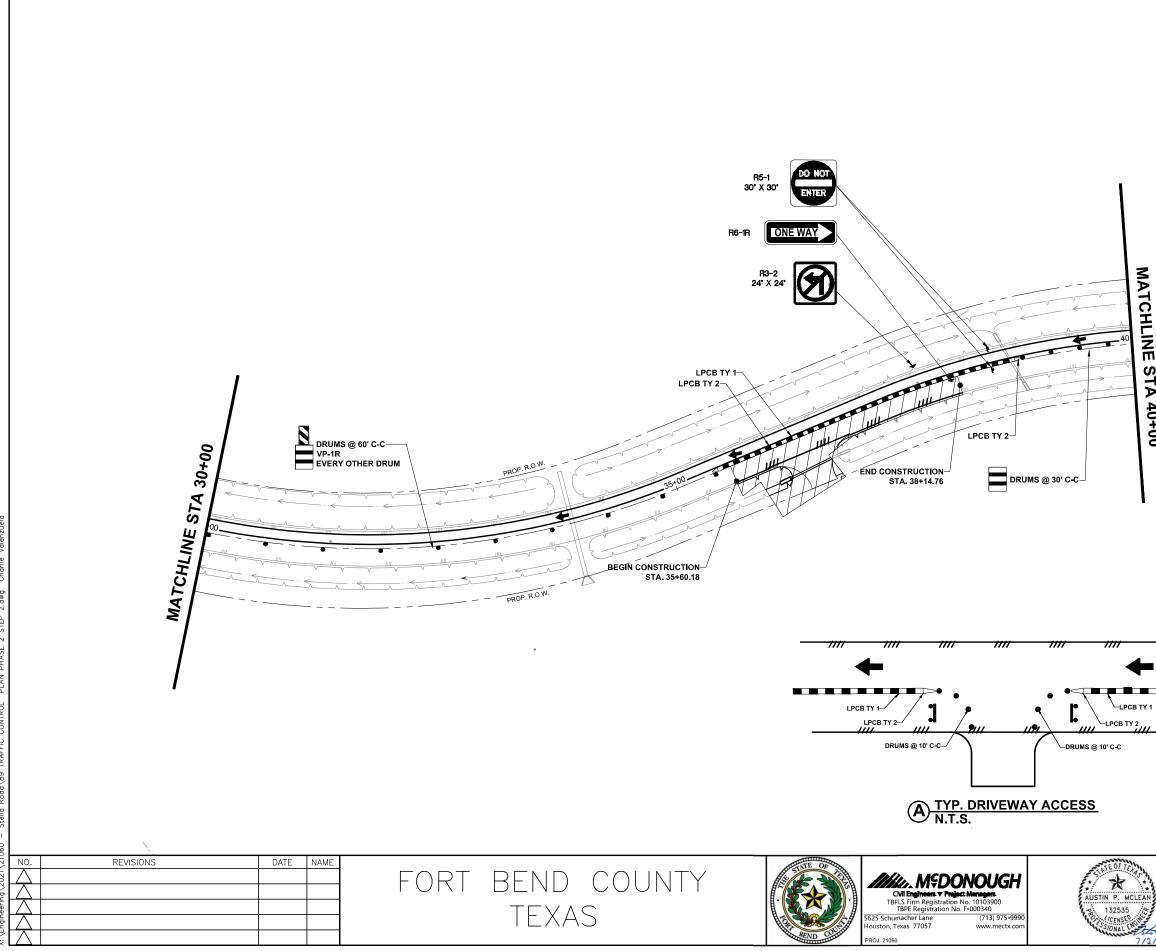
'n.	PROJECT TITLE: STELLA ROAD			
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.		
EAN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 2 STEP 1		
541 110	SCALE: 1" = 40'	STA. 30+00 TO STA. 40+00	SHEET NO:	
1/2/2024	DATE: 1/16/2023	APPROVED BY:	87 / 133	



			THE REAL PROPERTY OF THE PROPE
			SCALE 1"=100' 0' 0 100'
			LEGEND
	-		– PROP. R.O.W.
	-		– EXIST. R.O.W.
		→	PROP. TRAFFIC FLOW
			EXIST. TRAFFIC FLOW
STOP EXISTING REMOVE			CONSTRUCTION PHASE 1
50+00 <> +////			CONSTRUCTION PHASE 2
STELLA ROAD			PORT CTB LOW PROFILE (LPCB) TY 1
STOP EXISTING REMOVE			PORT CTB LOW PROFILE (LPCB) TY 2
TILMOYE		Ľ	TYPE III BARRICADE
		٠	CHANNELIZING DEVICE
			TEMP ASPHALT
		A	4" WHITE SOLID REMOVABLE
		В	4" YELLOW SOLID REMOVABLE
		С	4" WHITE SOLID NON-REMOVABLE
		D	4" YELLOW SOLID NON-REMOVABLE
		Ε	24" WHITE SOLID
	NOT	ES:	
	<u>PHA</u>	<u>SE 2</u>	
	1.	INSTALL TEMP	OFILE CONCRETE BARRIER AND PORARY PAVEMENT MARKINGS, ON SIGNS, AND BARRICADES AS E TRAFFIC CONTROL PLAN.
	2.	CONSTRUCT F	PAVEMENT, STORM SEWER AND THIS PHASE.
	3.	CONSTRUCT 1 THE LOCATIO	TEMPORARY ASPHALT PAVEMENT AT N SHOWN.
	4.	ALL RESIDEN	CESS WITH CRUSHED LIMESTONE TO TS AND BUSINESSES DURING THIS YPICAL DRIVEWAY ACCESS DETAIL.
	5.		TO MAINTAIN DRAINAGE DURING ALL ONSTRUCTION.

6. SEE SHEET 91 FOR EASTBOUND TRAFFIC DETOUR.

2	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 2 STEP 1	
CHART I TAR	SCALE: 1" = 40'	STA. 40+00 TO END	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	88 / 133

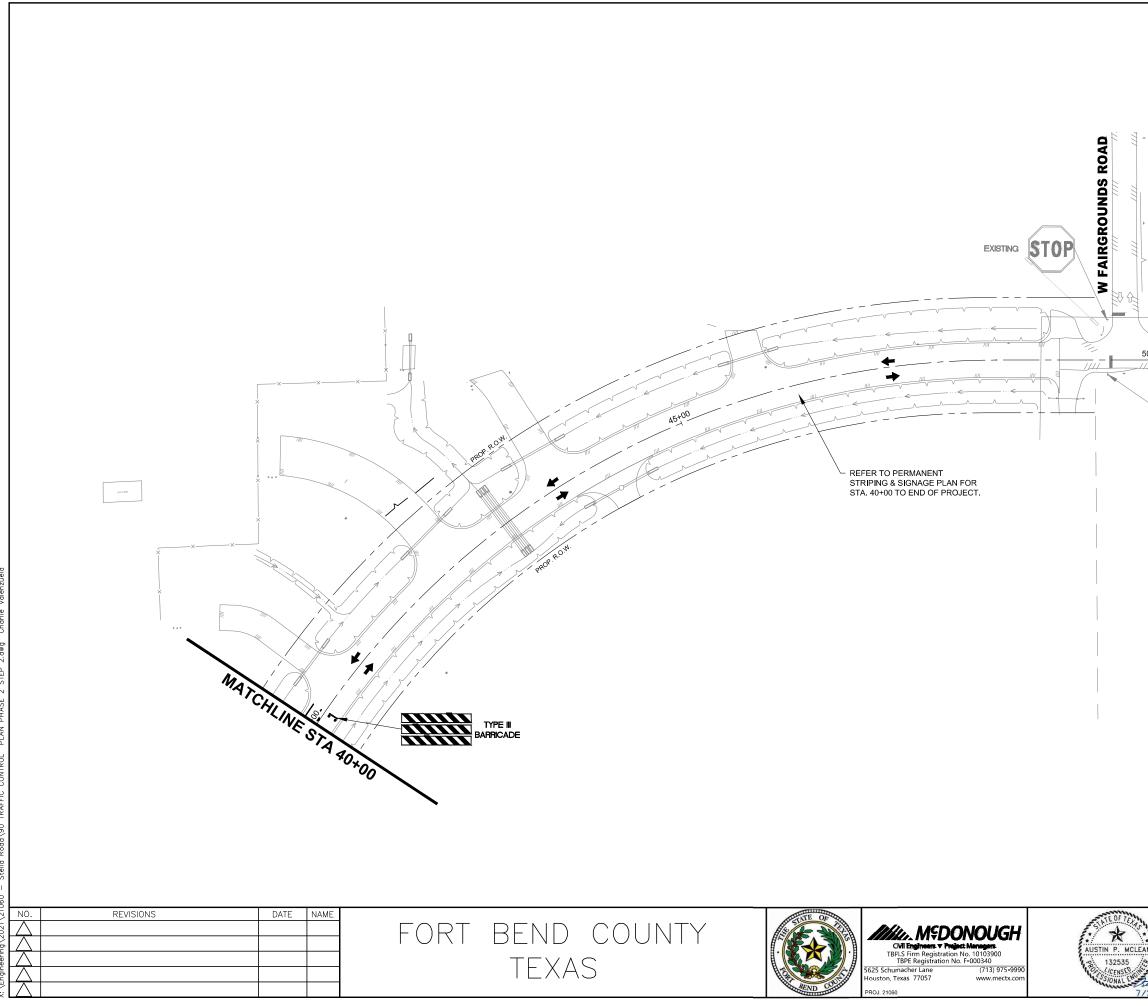


			_
			_
MATCHLINE STA 40+00			

	100' <u>50'</u>	SCALE 1"=100' 0 100'
	LI	EGEND
		PROP. R.O.W.
		EXIST. R.O.W.
	→	
	\leq	EXIST. TRAFFIC FLOW
		CONSTRUCTION PHASE 1
		CONSTRUCTION PHASE 2
		PORT CTB LOW PROFILE (LPCB) TY 1
	А	PORT CTB LOW PROFILE (LPCB) TY 2
	Ľ	TYPE III BARRICADE
	•	CHANNELIZING DEVICE
		TEMP ASPHALT
	А	4" WHITE SOLID REMOVABLE
	В	4" YELLOW SOLID REMOVABLE
	С	4" WHITE SOLID NON-REMOVABLE
	D	4" YELLOW SOLID NON-REMOVABLE
NOTE	E = <u>S:</u>	24" WHITE SOLID
PHAS	<u>SE 2</u>	
1.	SET LOW PROFILE CONCRE INSTALL TEMPORARY PAVE CONSTRUCTION SIGNS, AND SHOWN IN THE TRAFFIC CO	MENT MARKINGS,) BARRICADES AS
2.	CONSTRUCT PAVEMENT, ST DITCHES FOR THIS PHASE.	ORM SEWER AND
3.	CONSTRUCT TEMPORARY A THE LOCATION SHOWN.	SPHALT PAVEMENT AT
4.	MAINTAIN ACCESS WITH CR ALL RESIDENTS AND BUSIN PHASE. SEE TYPICAL DRIVE	ESSES DURING THIS

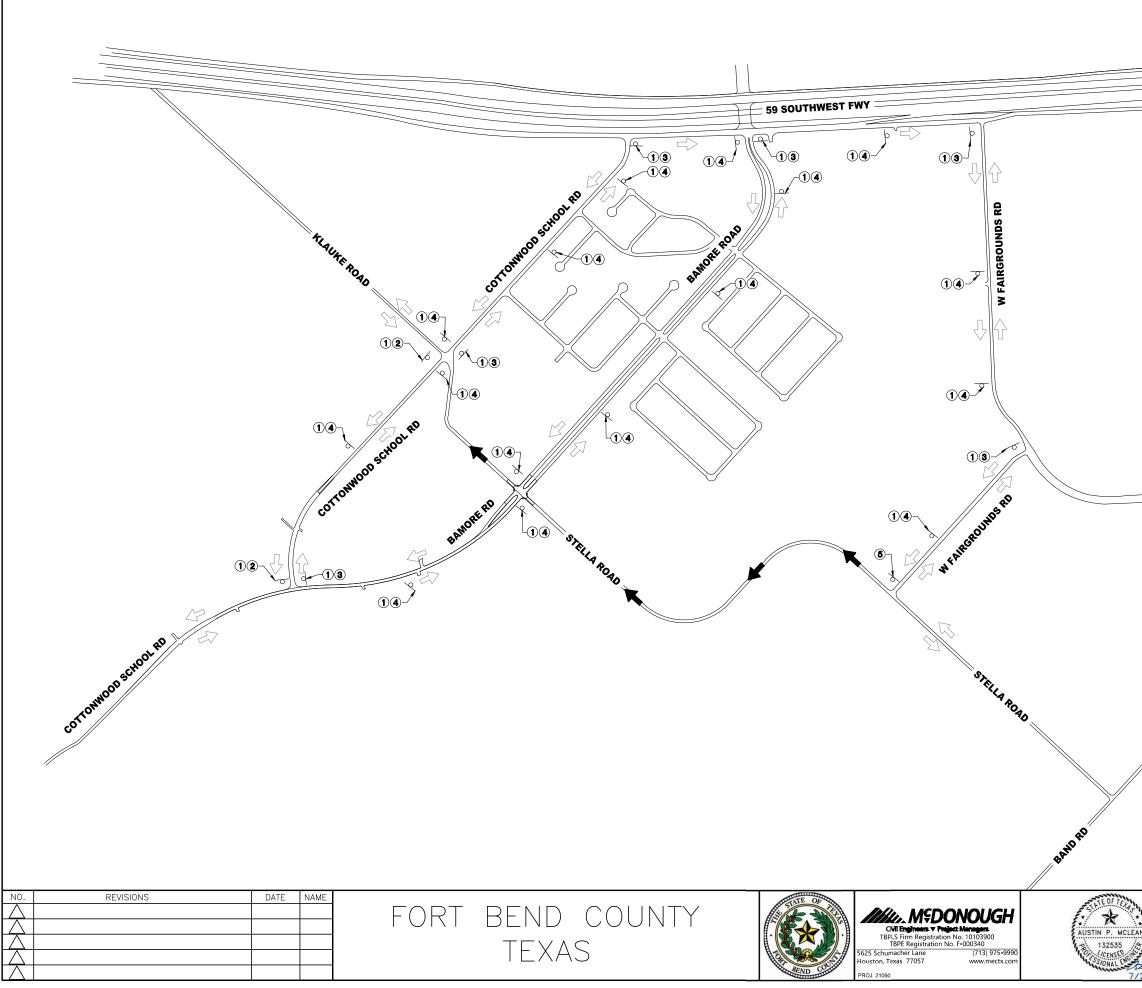
- 5. CONTRACTOR TO MAINTAIN DRAINAGE DURING ALL PHASES OF CONSTRUCTION.
- 6. SEE SHEET 91 FOR EASTBOUND TRAFFIC DETOUR.

1	PROJECT TITLE: STELLA ROAD		
×***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 2 STEP 2	
	SCALE: 1" = 40'	STA. 30+00 TO STA. 40+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	89 / 133

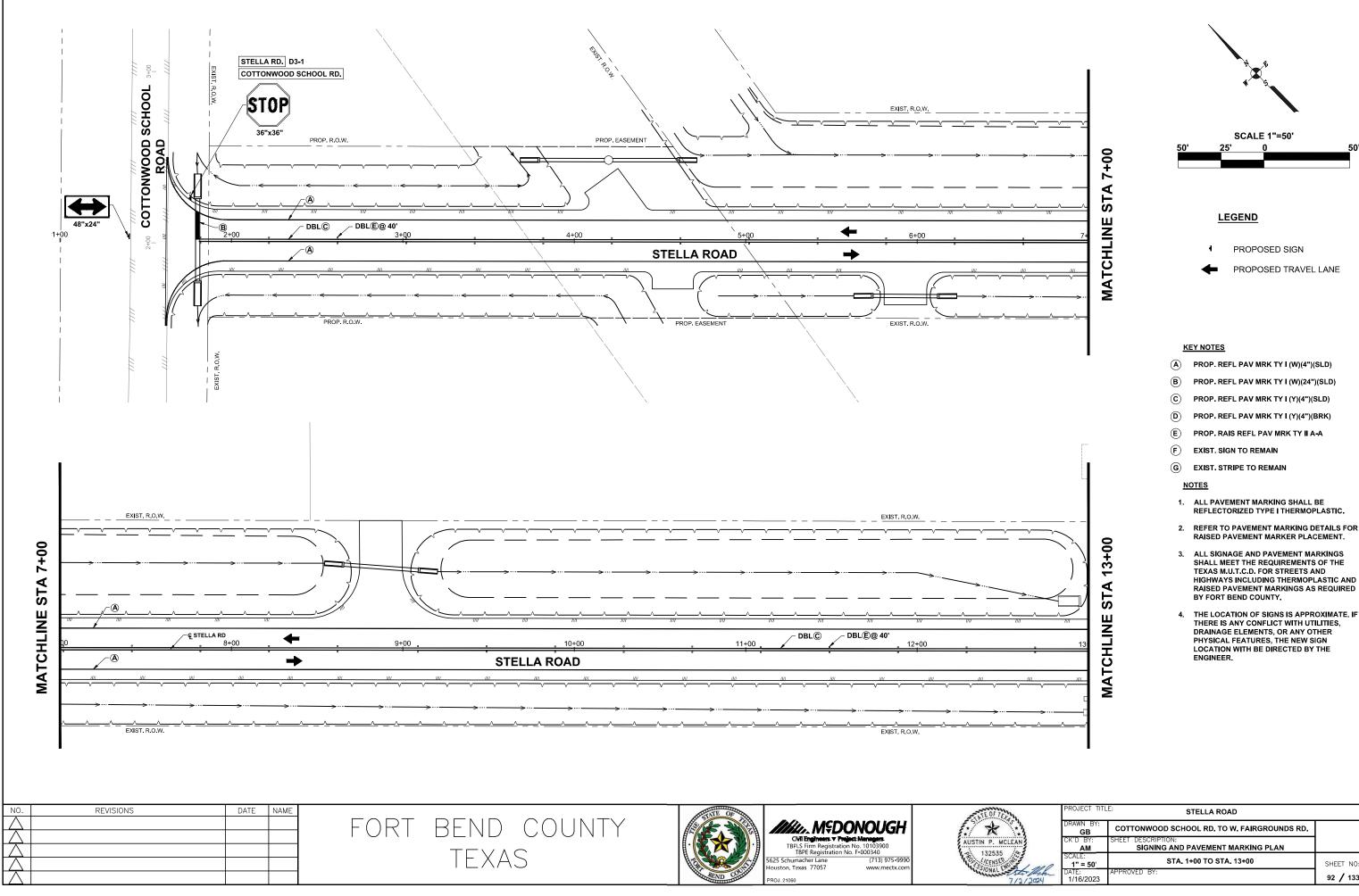


	×		100		SCALE 1"=100'	100'
				LI	EGEND	
	×				PROP. R.O.W.	
					EXIST. R.O.W.	
	×			_	PROP. TRAFFIC FLOW	
		EVICTINO			EXIST. TRAFFIC FLOW	
		EXISTING		\leq		
	STOP		Ē		CONSTRUCTION PHAS	E 1
50+00	[< •//// [テᠯ?]				CONSTRUCTION PHAS	E 2
STELL	A ROAD				PORT CTB LOW PROFI (LPCB) TY 1	
S1				\square	PORT CTB LOW PROFI (LPCB) TY 2	LE
	EXE	TING		Ľ	TYPE III BARRICADE	
				•	CHANNELIZING DEVICE	E
					TEMP ASPHALT	
				A	4" WHITE SOLID REMO	VABLE
				В	4" YELLOW SOLID REN	IOVABLE
				С	4" WHITE SOLID NON-F	
				D	4" YELLOW SOLID NON	I-REMOVABLE
		NOTE	<u>:S:</u>	E	24" WHITE SOLID	
		PHAS	<u>6E 2</u>			
		1.	INSTA CONS	LL TEMPORA	CONCRETE BARRIER AN RY PAVEMENT MARKING GNS, AND BARRICADES FFIC CONTROL PLAN.	SS,
		2.		TRUCT PAVE	MENT, STORM SEWER A	ND
		3.		TRUCT TEMPO	ORARY ASPHALT PAVEN DWN.	IENT AT
		4.	ALL R	RESIDENTS AN	WITH CRUSHED LIMEST D BUSINESSES DURING AL DRIVEWAY ACCESS [THIS
		5.			IAINTAIN DRAINAGE DU	RING
		6.	FROM SHEE	STA. 40+00 T	NT STRIPING AND SIGNA O END OF PROJECT. RE MANENT STRIPING AND	
			SEE S	HEET 91 FOR	EASTBOUND TRAFFIC D	ETOUR
	PROJECT TITL	E:		STELLA ROA	AD	
****	DRAWN BY: GB			HOOL RD. TO	W. FAIRGROUNDS RD.	
CLEAN	CK'D BY: AM	SHEET DESCRIF		NTROL PLAN	PHASE 2 STEP 2	

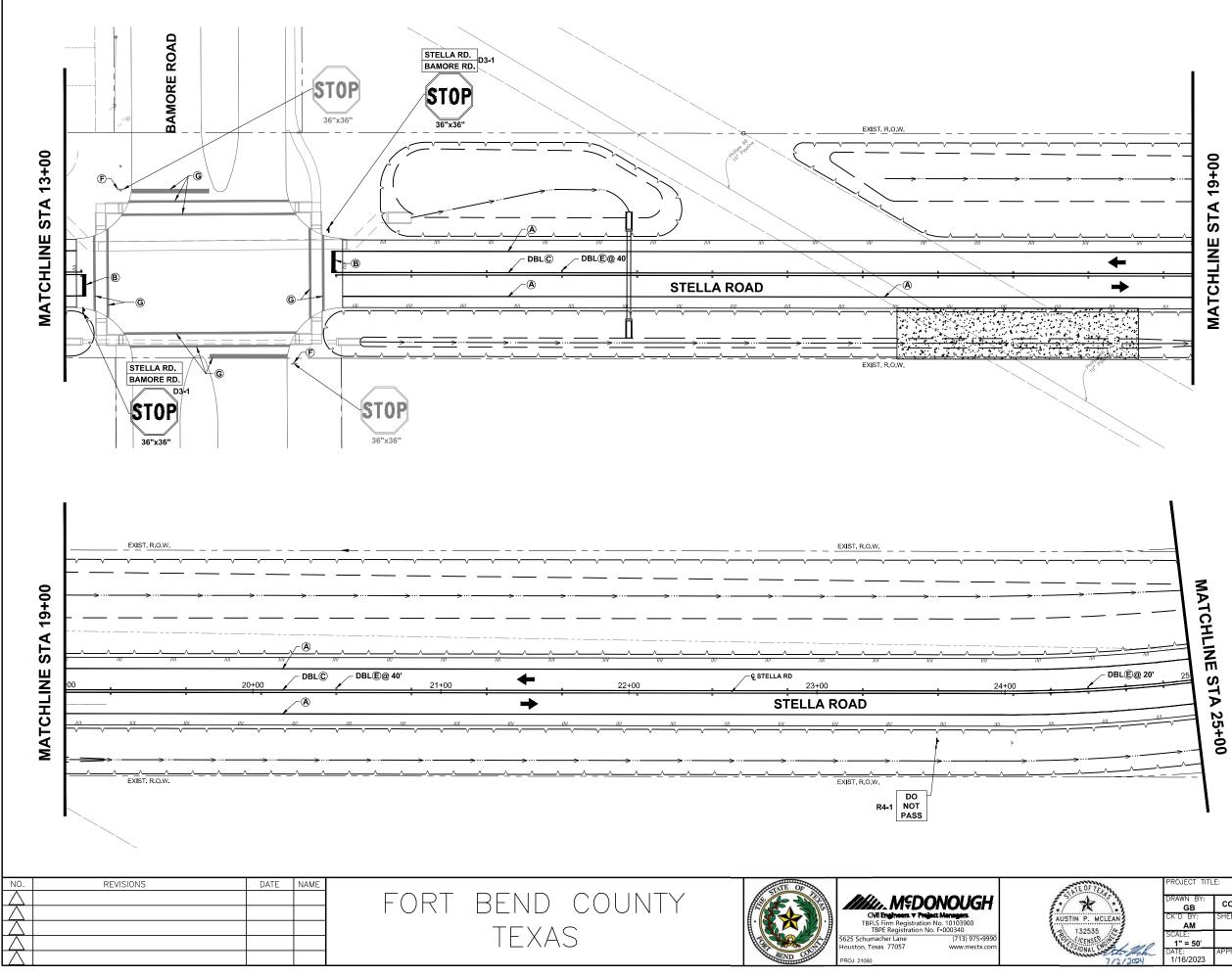
***	GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AN	CK'D BY: AM	SHEET DESCRIPTION: TRAFFIC CONTROL PLAN PHASE 2 STEP 2	
	SCALE: 1" = 40'	STA. 40+00 TO END	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	90 / 133
12/2024	DATE: 1/16/2023	APPROVED BY:	90 / 133



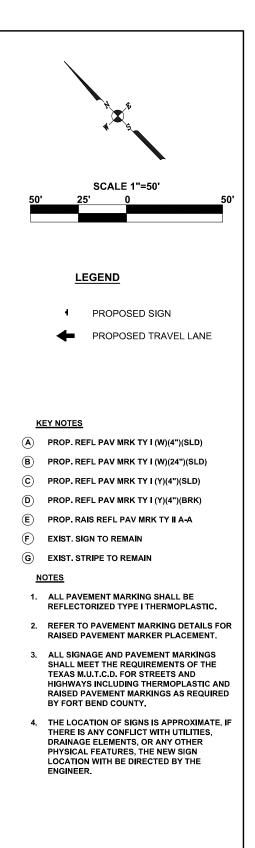
		H C S	
		N.T.S.	
		LEGEND	
Y 36 S		PROP. TRAFFIC FLC BARRELS OR DRUM EXIST. TRAFFIC FLC	IS
HIGHWAY 36 S		(1) EASTBOUND STELLA ROAD M4-12T	
		DETOUR M4-9, ALT-L 30' X 36'	
		(3) DETOUR M4-9, ALT-R 30' X 36'	
		(4) DETOUR M4-9, ALT-S 30" X 36"	
		(\$) END DETOUR M4-8a 24' X 18'	
M.	PROJECT TITL	E: STELLA ROAD	
15 may	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: STELLA ROAD	
CINE AND	SCALE: 1" = 40'	DETOUR PLAN	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	91 / 133



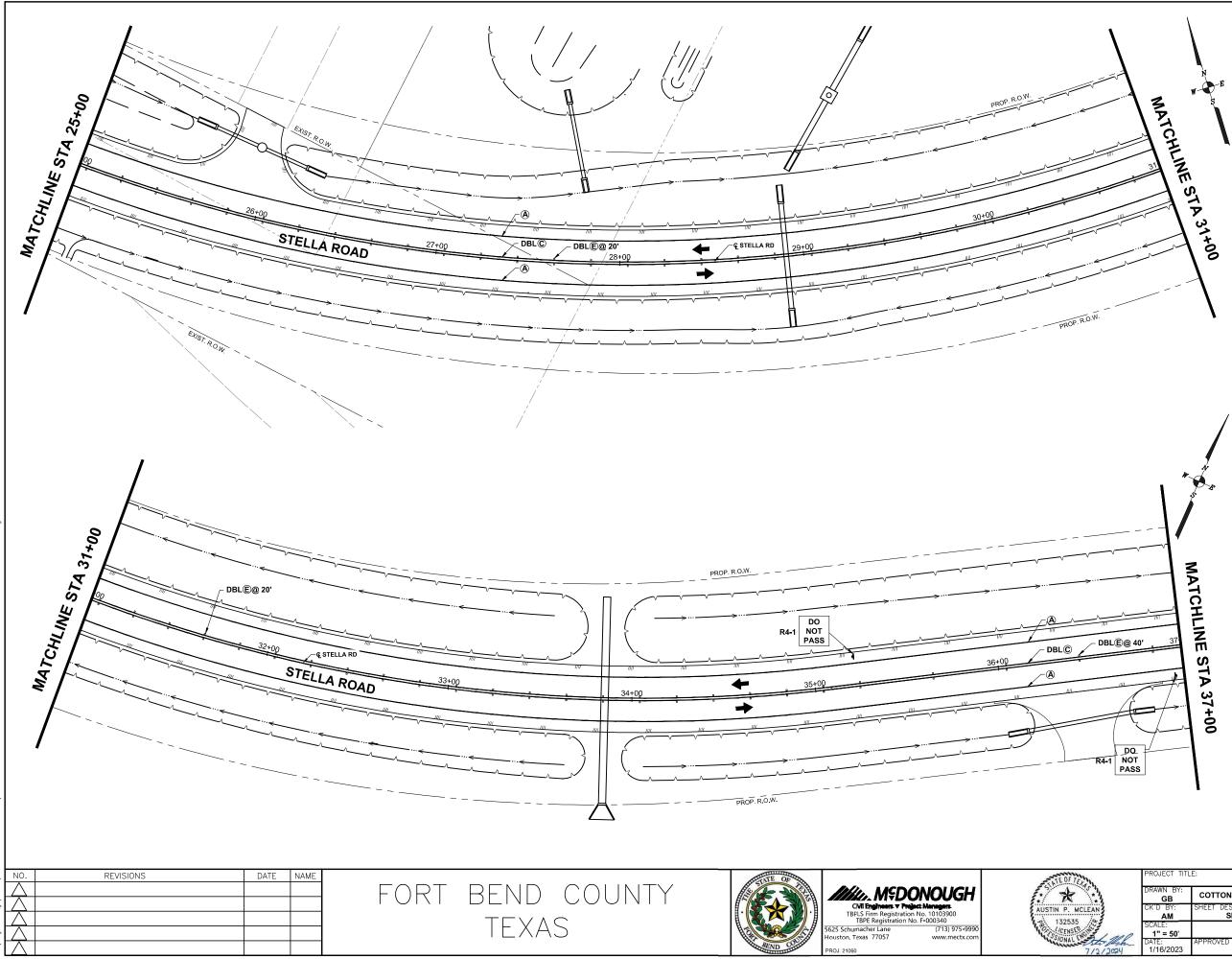
5	PROJECT IIIL	STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: SIGNING AND PAVEMENT MARKING PLAN	
Sal MA	SCALE: 1" = 50'	STA. 1+00 TO STA. 13+00	SHEET NO:
1/2/2024	DATE: 1/16/2023	APPROVED BY:	92 / 133

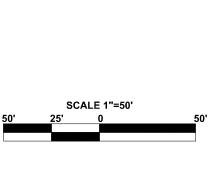


ingineering\2021\21060 - Stella Road\93 SIGNING AND PAVEMENT MARKING PLAN STA. 13+00 TO STA. 25+00.dwg Ct



PROJECT TITLE: STELLA ROAD			
3.0 1	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: SIGNING AND PAVEMENT MARKING PLAN	
CHARLE I MO	SCALE: 1" = 50'	STA. 13+00 TO STA. 25+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	93 / 133





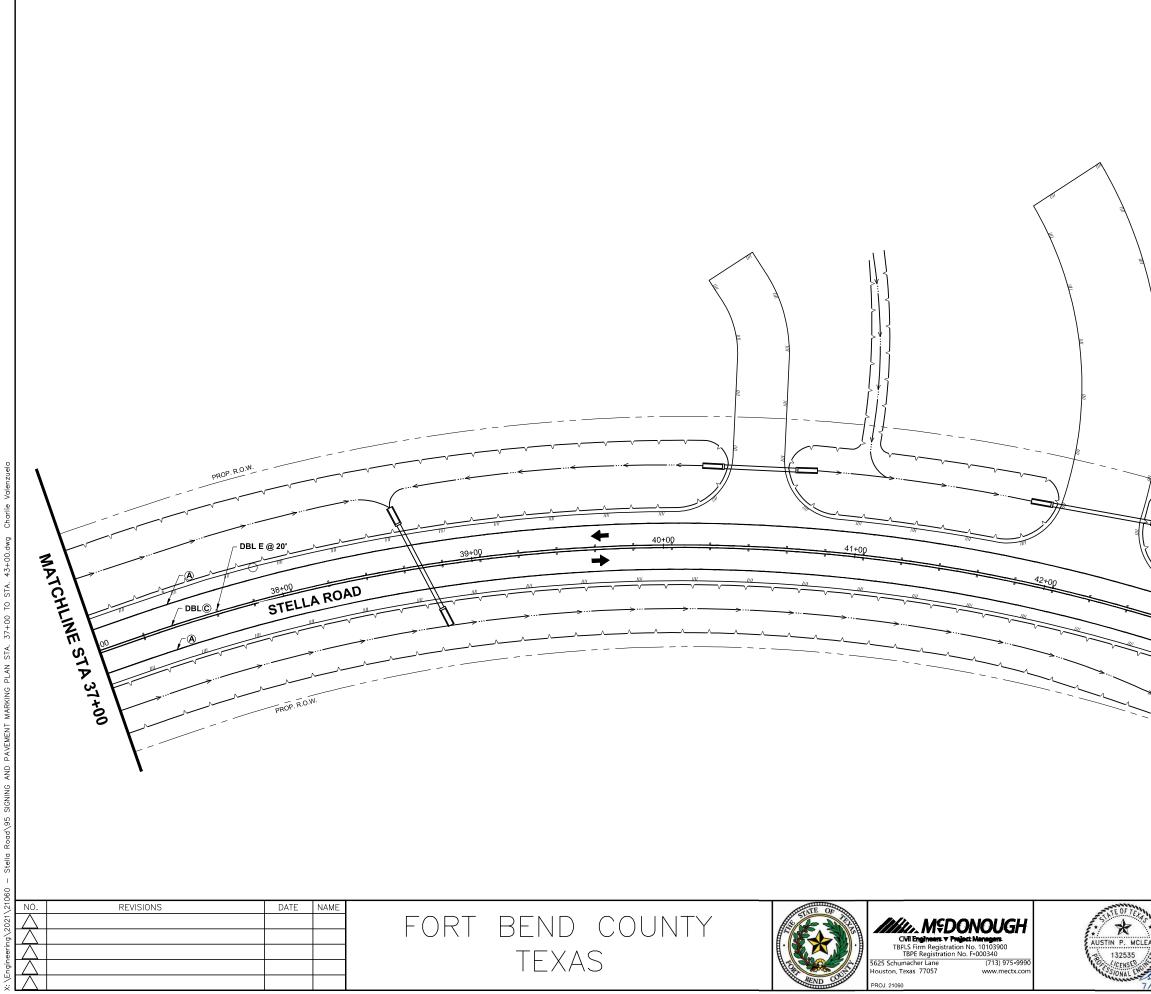


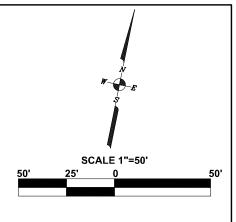
- PROPOSED TRAVEL LANE
- KEY NOTES
- (A) PROP. REFL PAV MRK TY I (W)(4")(SLD)
- B PROP. REFL PAV MRK TY I (W)(24")(SLD)
- C PROP. REFL PAV MRK TY I (Y)(4")(SLD)
- (D) PROP. REFL PAV MRK TY I (Y)(4")(BRK)
- (E) PROP. RAIS REFL PAV MRK TY II A-A
- (F) EXIST. SIGN TO REMAIN
- G EXIST. STRIPE TO REMAIN

<u>NOTES</u>

- 1. ALL PAVEMENT MARKING SHALL BE REFLECTORIZED TYPE I THERMOPLASTIC.
- 2. REFER TO PAVEMENT MARKING DETAILS FOR RAISED PAVEMENT MARKER PLACEMENT.
- 3. ALL SIGNAGE AND PAVEMENT MARKINGS SHALL MEET THE REQUIREMENTS OF THE TEXAS M.U.T.C.D. FOR STREETS AND HIGHWAYS INCLUDING THERMOPLASTIC AND RAISED PAVEMENT MARKINGS AS REQUIRED BY FORT BEND COUNTY.
- 4. THE LOCATION OF SIGNS IS APPROXIMATE. IF THERE IS ANY CONFLICT WITH UTILITIES, DRAINAGE ELEMENTS, OR ANY OTHER PHYSICAL FEATURES, THE NEW SIGN LOCATION WITH BE DIRECTED BY THE ENGINEER.

	PROJECT TITL	E: STELLA ROAD	
3	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: SIGNING AND PAVEMENT MARKING PLAN	
1 110	SCALE: 1" = 50'	STA. 25+00 TO STA. 37+00	SHEET NO
1 John	DATE: 1/16/2023	APPROVED BY:	94 / 133





- PROPOSED SIGN
- PROPOSED TRAVEL LANE

KEY NOTES

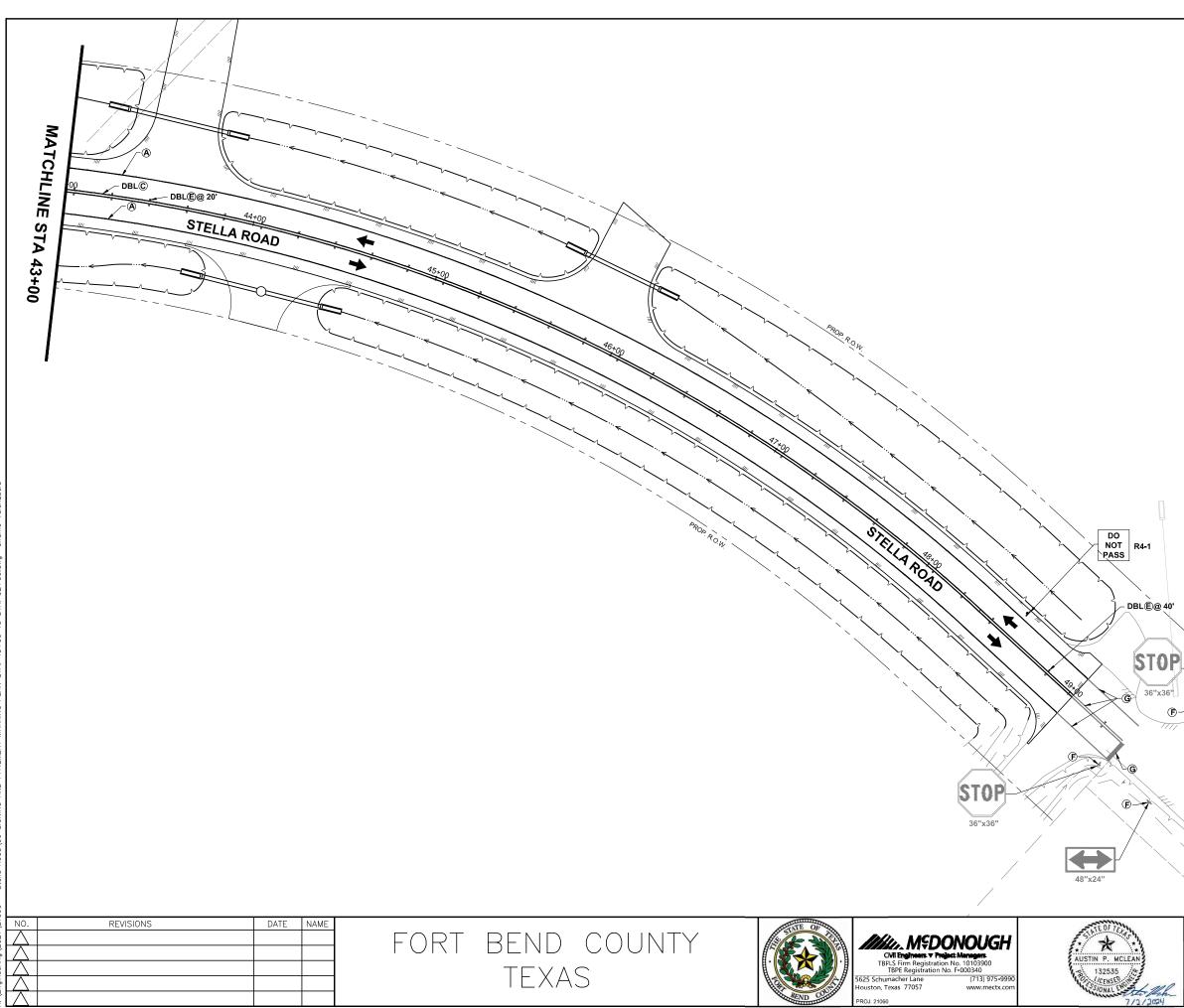
- A PROP. REFL PAV MRK TY I (W)(4")(SLD)
- B PROP. REFL PAV MRK TY I (W)(24")(SLD)
- C PROP. REFL PAV MRK TY I (Y)(4")(SLD)
- (D) PROP. REFL PAV MRK TY I (Y)(4")(BRK)
- (E) PROP. RAIS REFL PAV MRK TY II A-A
- (F) EXIST. SIGN TO REMAIN
- G EXIST. STRIPE TO REMAIN

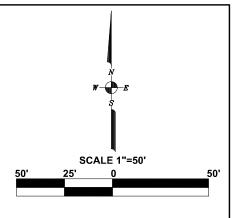
<u>NOTES</u>

MATCHLINE STA 43+00

- 1. ALL PAVEMENT MARKING SHALL BE REFLECTORIZED TYPE I THERMOPLASTIC.
- 2. REFER TO PAVEMENT MARKING DETAILS FOR RAISED PAVEMENT MARKER PLACEMENT.
- 3. ALL SIGNAGE AND PAVEMENT MARKINGS SHALL MEET THE REQUIREMENTS OF THE TEXAS M.U.T.C.D. FOR STREETS AND HIGHWAYS INCLUDING THERMOPLASTIC AND RAISED PAVEMENT MARKINGS AS REQUIRED BY FORT BEND COUNTY.
- 4. THE LOCATION OF SIGNS IS APPROXIMATE. IF THERE IS ANY CONFLICT WITH UTILITIES, DRAINAGE ELEMENTS, OR ANY OTHER PHYSICAL FEATURES, THE NEW SIGN LOCATION WITH BE DIRECTED BY THE ENGINEER.

	PROJECT TITL		
***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: SIGNING AND PAVEMENT MARKING PLAN	
in a second	SCALE: 1" = 50'	STA. 37+00 TO STA. 43+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	95 / 133





PROPOSED SIGN



KEY NOTES

- (A) PROP. REFL PAV MRK TY I (W)(4")(SLD)
- PROP. REFL PAV MRK TY I (W)(24")(SLD) B
- © PROP. REFL PAV MRK TY I (Y)(4")(SLD)
- **D** PROP. REFL PAV MRK TY I (Y)(4")(BRK)
- **(E**) PROP. RAIS REFL PAV MRK TY II A-A
- **(F**) EXIST. SIGN TO REMAIN
- G EXIST. STRIPE TO REMAIN

NOTES

36"x36"

Ē-

STOP

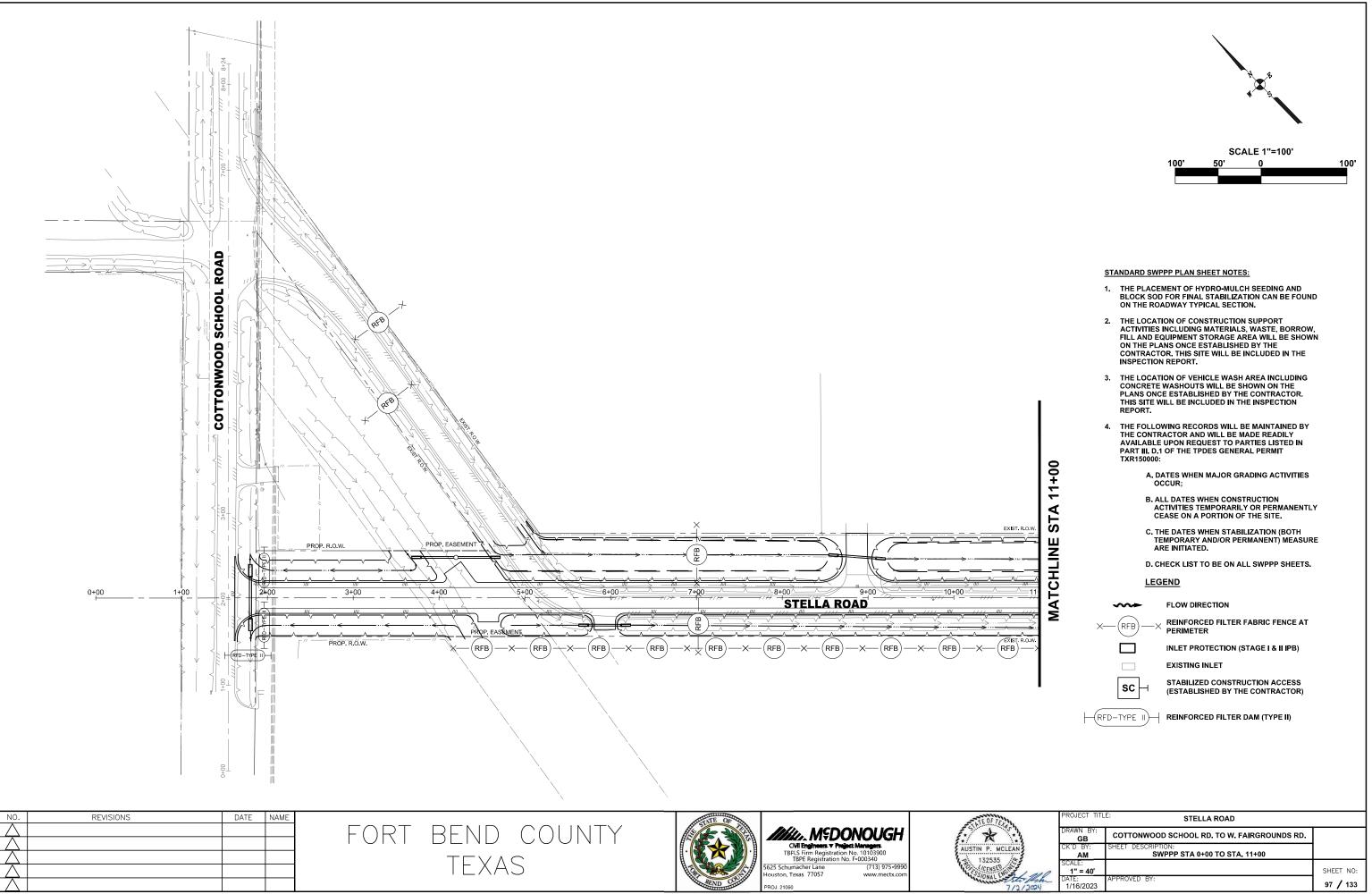
36"x36"

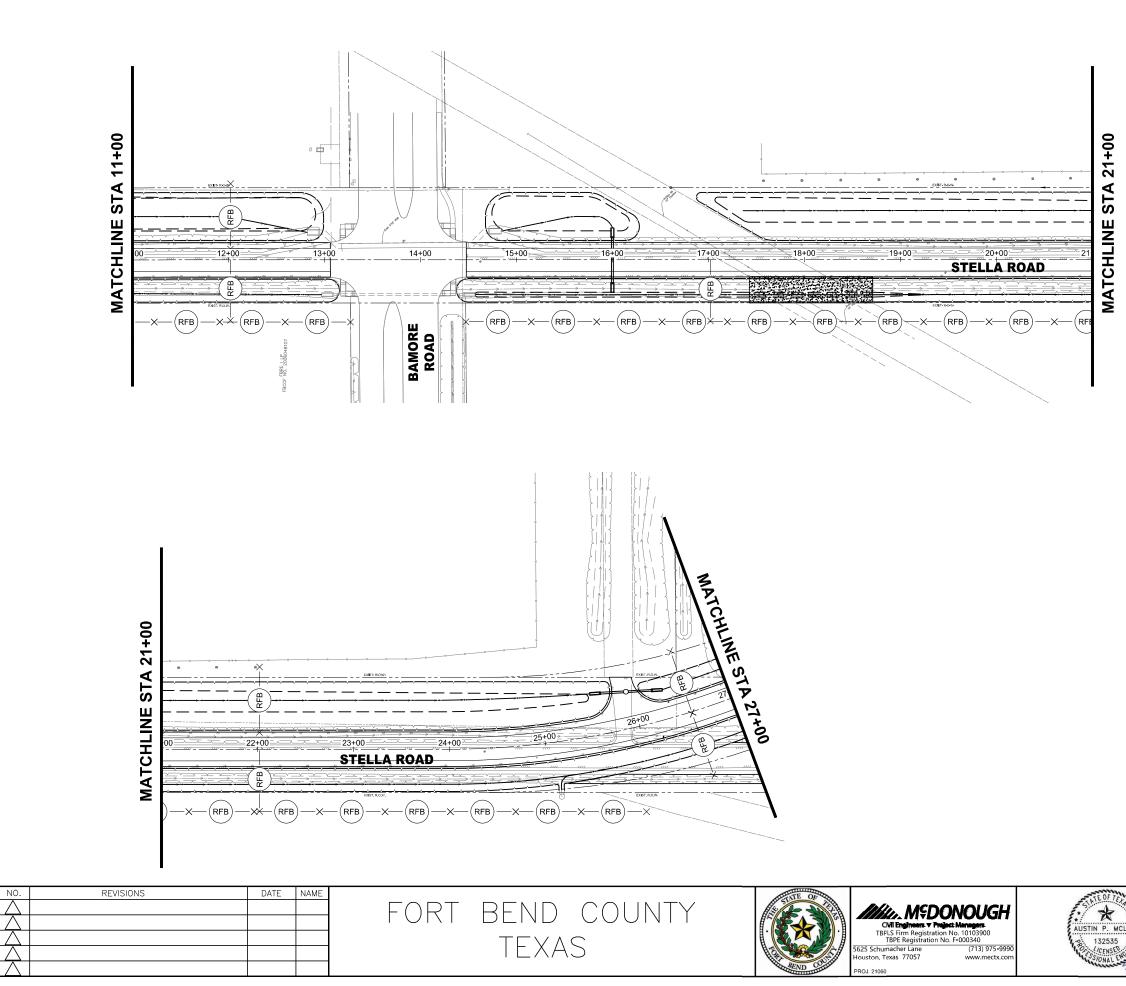
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\$0,×00

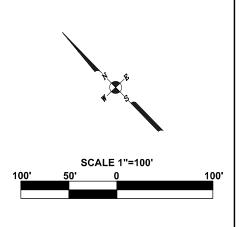
- 1. ALL PAVEMENT MARKING SHALL BE REFLECTORIZED TYPE I THERMOPLASTIC.
- 2. REFER TO PAVEMENT MARKING DETAILS FOR RAISED PAVEMENT MARKER PLACEMENT.
- ALL SIGNAGE AND PAVEMENT MARKINGS 3, WFAIRGROUNDS ROAD SHALL MEET THE REQUIREMENTS OF THE TEXAS M.U.T.C.D. FOR STREETS AND HIGHWAYS INCLUDING THERMOPLASTIC AND RAISED PAVEMENT MARKINGS AS REQUIRED BY FORT BEND COUNTY.
 - THE LOCATION OF SIGNS IS APPROXIMATE. IF 4. THERE IS ANY CONFLICT WITH UTILITIES, DRAINAGE ELEMENTS, OR ANY OTHER PHYSICAL FEATURES, THE NEW SIGN LOCATION WITH BE DIRECTED BY THE ENGINEER.

		C-7	
5.00 m	PROJECT TITL	SIELLA ROAD	
EAN	GB CK'D BY: AM	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD. SHEET DESCRIPTION: SIGNING AND PAVEMENT MARKING PLAN	
712/2024	SCALE: <u>1" = 50'</u> DATE: 1/16/2023	STA. 43+00 TO STA. 52+00 APPROVED BY:	SHEET NO: 96 / 133





(Engineering\2021\21060 - Stella Road\98 SWPPP STA 11+00 T0 STA. 27+00.dwg Charlie Vale

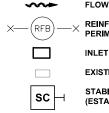


STANDARD SWPPP PLAN SHEET NOTES:

- 1. THE PLACEMENT OF HYDRO-MULCH SEEDING AND BLOCK SOD FOR FINAL STABILIZATION CAN BE FOUND ON THE ROADWAY TYPICAL SECTION.
- 2. THE LOCATION OF CONSTRUCTION SUPPORT ACTIVITIES INCLUDING MATERIALS, WASTE, BORROW, FILL AND EQUIPMENT STORAGE AREA WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 3. THE LOCATION OF VEHICLE WASH AREA INCLUDING CONCRETE WASHOUTS WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 4. THE FOLLOWING RECORDS WILL BE MAINTAINED BY THE CONTRACTOR AND WILL BE MADE READILY AVAILABLE UPON REQUEST TO PARTIES LISTED IN PART III. D.1 OF THE TPDES GENERAL PERMIT TXR150000:
 - A. DATES WHEN MAJOR GRADING ACTIVITIES OCCUR;
 - B. ALL DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE.
 - C. THE DATES WHEN STABILIZATION (BOTH TEMPORARY AND/OR PERMANENT) MEASURE ARE INITIATED.
 - D. CHECK LIST TO BE ON ALL SWPPP SHEETS.

LEGEND

FLOW DIRECTION



REINFORCED FILTER FABRIC FENCE AT

INLET PROTECTION (STAGE I & II IPB)

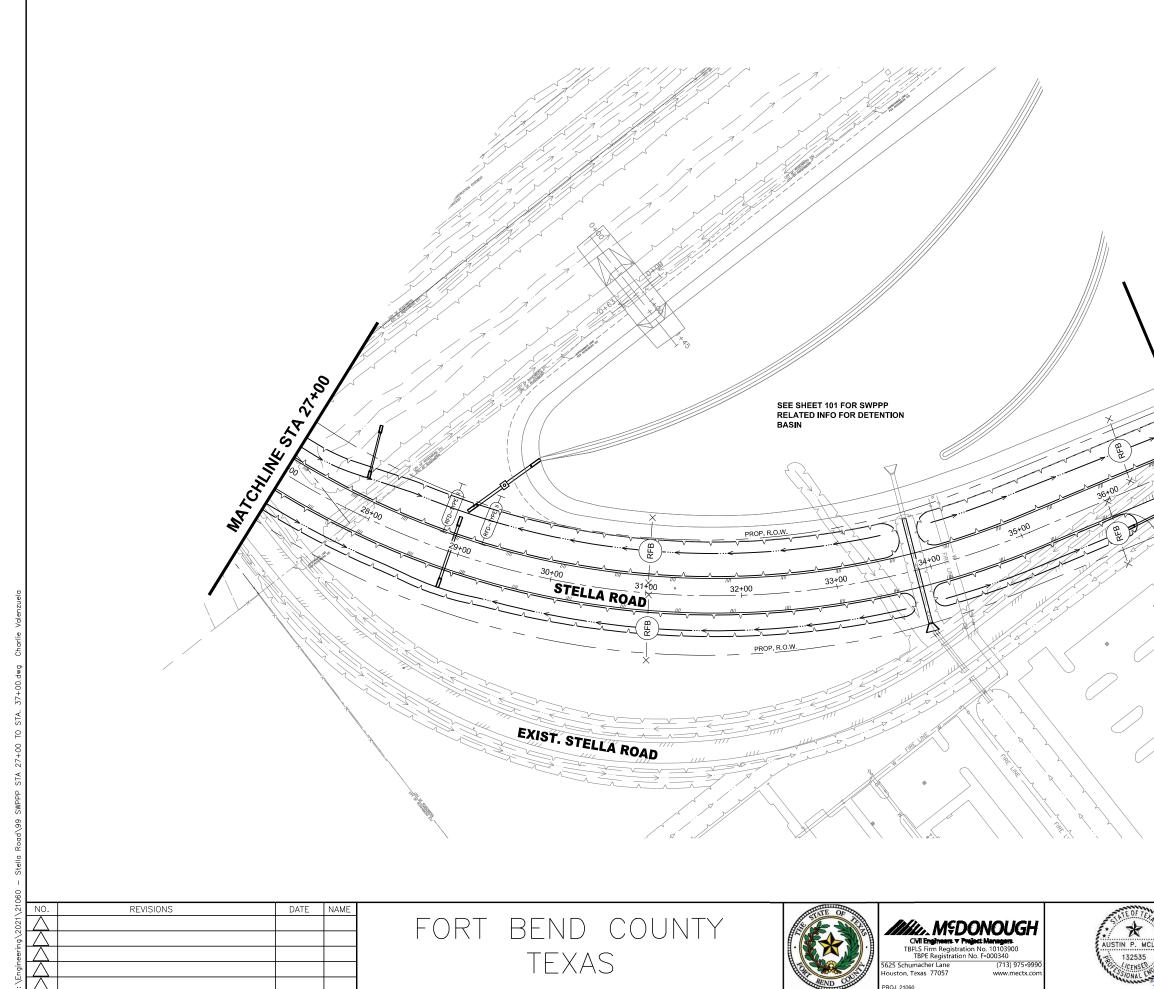
EXISTING INLET

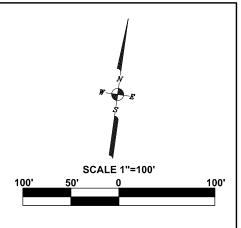
STABILIZED CONSTRUCTION ACCESS (ESTABLISHED BY THE CONTRACTOR)

RFD-TYPE II)

REINFORCED FILTER DAM (TYPE II)

11	PROJECT TITLE: STELLA ROAD				
* **	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.			
EAN	CK'D BY: AM	SHEET DESCRIPTION: SWPPP STA 11+00 TO STA. 27+00			
	SCALE: 1" = 40'		SHEET NO:		
1/2/2024	DATE: 1/16/2023	APPROVED BY:	98 / 133		





STANDARD SWPPP PLAN SHEET NOTES:

- 1. THE PLACEMENT OF HYDRO-MULCH SEEDING AND BLOCK SOD FOR FINAL STABILIZATION CAN BE FOUND ON THE ROADWAY TYPICAL SECTION.
- 2. THE LOCATION OF CONSTRUCTION SUPPORT ACTIVITIES INCLUDING MATERIALS, WASTE, BORROW, FILL AND EQUIPMENT STORAGE AREA WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR, THIS SITE WILL BE INCLUDED IN THE INSPECTION DEPODIT INSPECTION REPORT.
- 3. THE LOCATION OF VEHICLE WASH AREA INCLUDING CONCRETE WASHOUTS WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 4. THE FOLLOWING RECORDS WILL BE MAINTAINED BY THE CONTRACTOR AND WILL BE MADE READILY AVAILABLE UPON REQUEST TO PARTIES LISTED IN PART III. D.1 OF THE TPDES GENERAL PERMIT TXR150000
 - A. DATES WHEN MAJOR GRADING ACTIVITIES OCCUR;
 - B. ALL DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE.
 - C. THE DATES WHEN STABILIZATION (BOTH TEMPORARY AND/OR PERMANENT) MEASURE ARE INITIATED.
 - D. CHECK LIST TO BE ON ALL SWPPP SHEETS.

LEGEND

FLOW DIRECTION

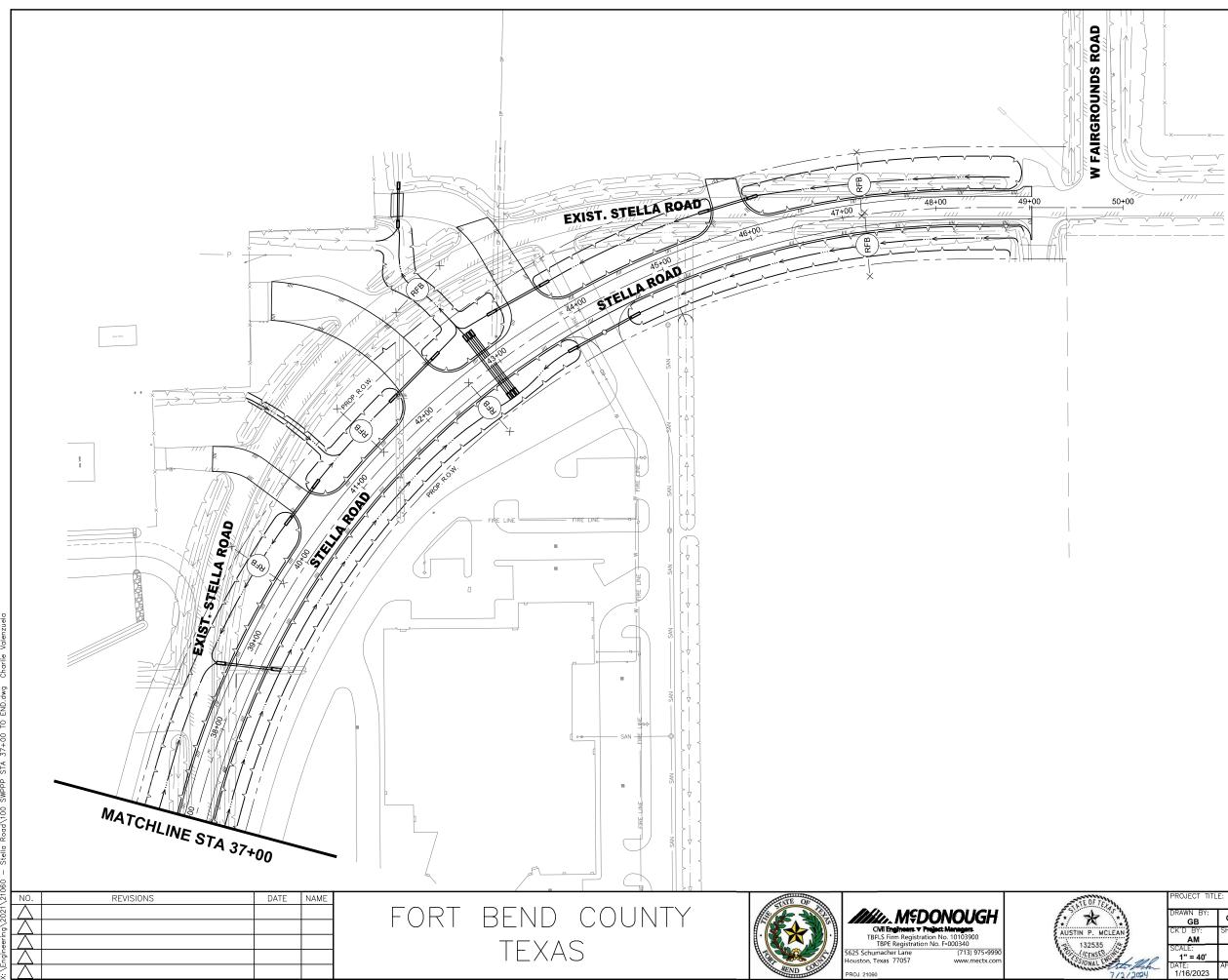
MATCHLINE

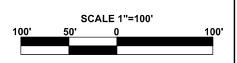
5TA 37+00

- INLET PROTECTION (STAGE I & II IPB)
- EXISTING INLET

STABILIZED CONSTRUCTION ACCESS (ESTABLISHED BY THE CONTRACTOR) sc

h	PROJECT TITI	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
N	CK'D BY: AM	SHEET DESCRIPTION: SWPPP STA 27+00 TO STA. 37+00	
1 110	SCALE: 1" = 40'		SHEET NO:
1/2024	DATE: 1/16/2023	APPROVED BY:	99 / 133



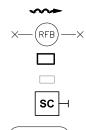


STANDARD SWPPP PLAN SHEET NOTES:

- 1. THE PLACEMENT OF HYDRO-MULCH SEEDING AND BLOCK SOD FOR FINAL STABILIZATION CAN BE FOUND ON THE ROADWAY TYPICAL SECTION.
- 2. THE LOCATION OF CONSTRUCTION SUPPORT ACTIVITIES INCLUDING MATERIALS, WASTE, BORROW, FILL AND EQUIPMENT STORAGE AREA WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 3. THE LOCATION OF VEHICLE WASH AREA INCLUDING CONCRETE WASHOUTS WILL BE SHOWN ON THE PLANS ONCE ESTABLISHED BY THE CONTRACTOR. THIS SITE WILL BE INCLUDED IN THE INSPECTION REPORT.
- 4. THE FOLLOWING RECORDS WILL BE MAINTAINED BY THE CONTRACTOR AND WILL BE MADE READILY AVAILABLE UPON REQUEST TO PARTIES LISTED IN PART III. D.1 OF THE TPDES GENERAL PERMIT TXR150000:
 - A. DATES WHEN MAJOR GRADING ACTIVITIES OCCUR;
 - B. ALL DATES WHEN CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE.
 - C. THE DATES WHEN STABILIZATION (BOTH TEMPORARY AND/OR PERMANENT) MEASURE ARE INITIATED.

D. CHECK LIST TO BE ON ALL SWPPP SHEETS.

LEGEND



FLOW DIRECTION

 \times REINFORCED FILTER FABRIC FENCE AT PERIMETER

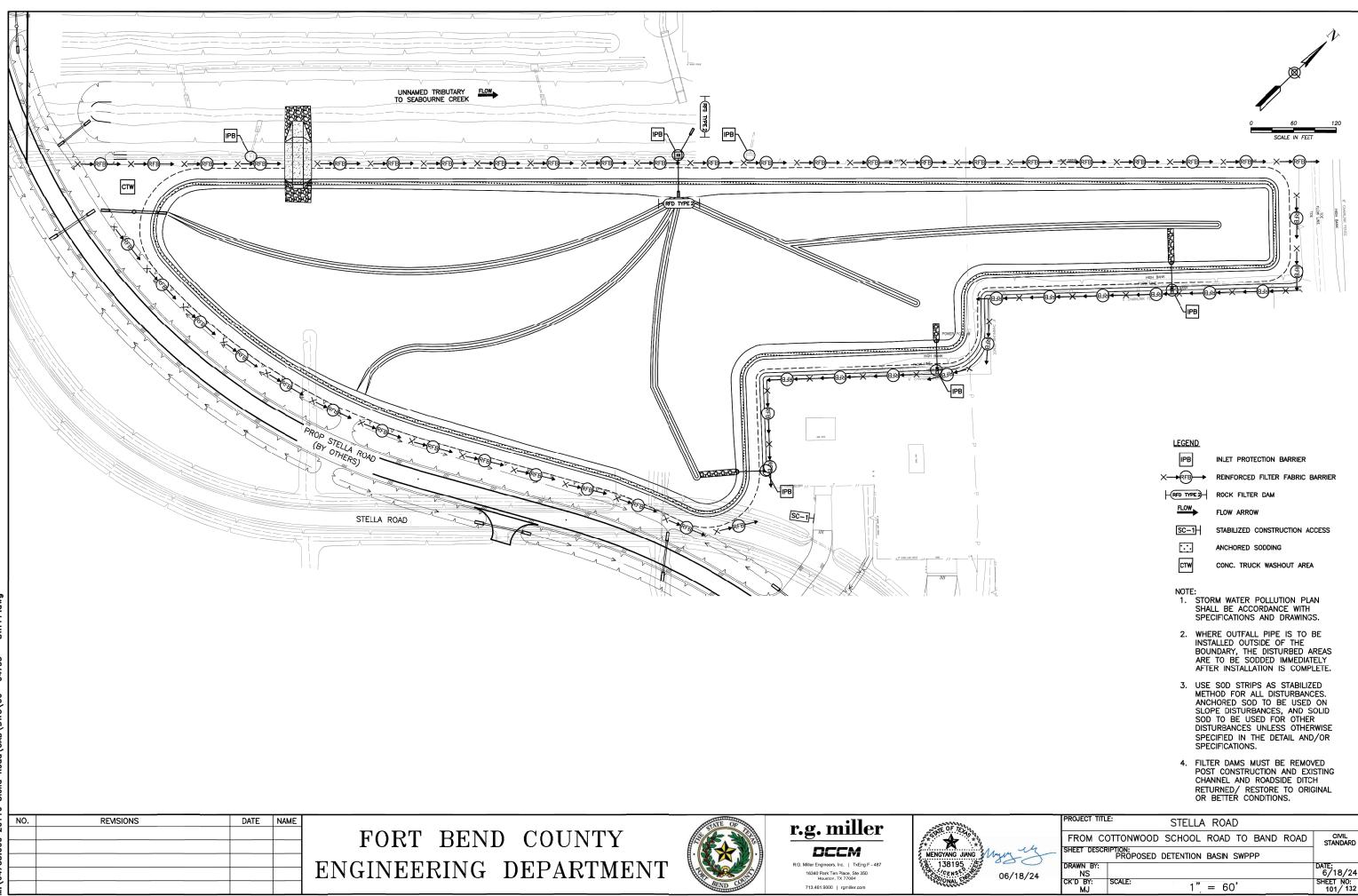
INLET PROTECTION (STAGE I & II IPB)

EXISTING INLET

STABILIZED CONSTRUCTION ACCESS (ESTABLISHED BY THE CONTRACTOR)

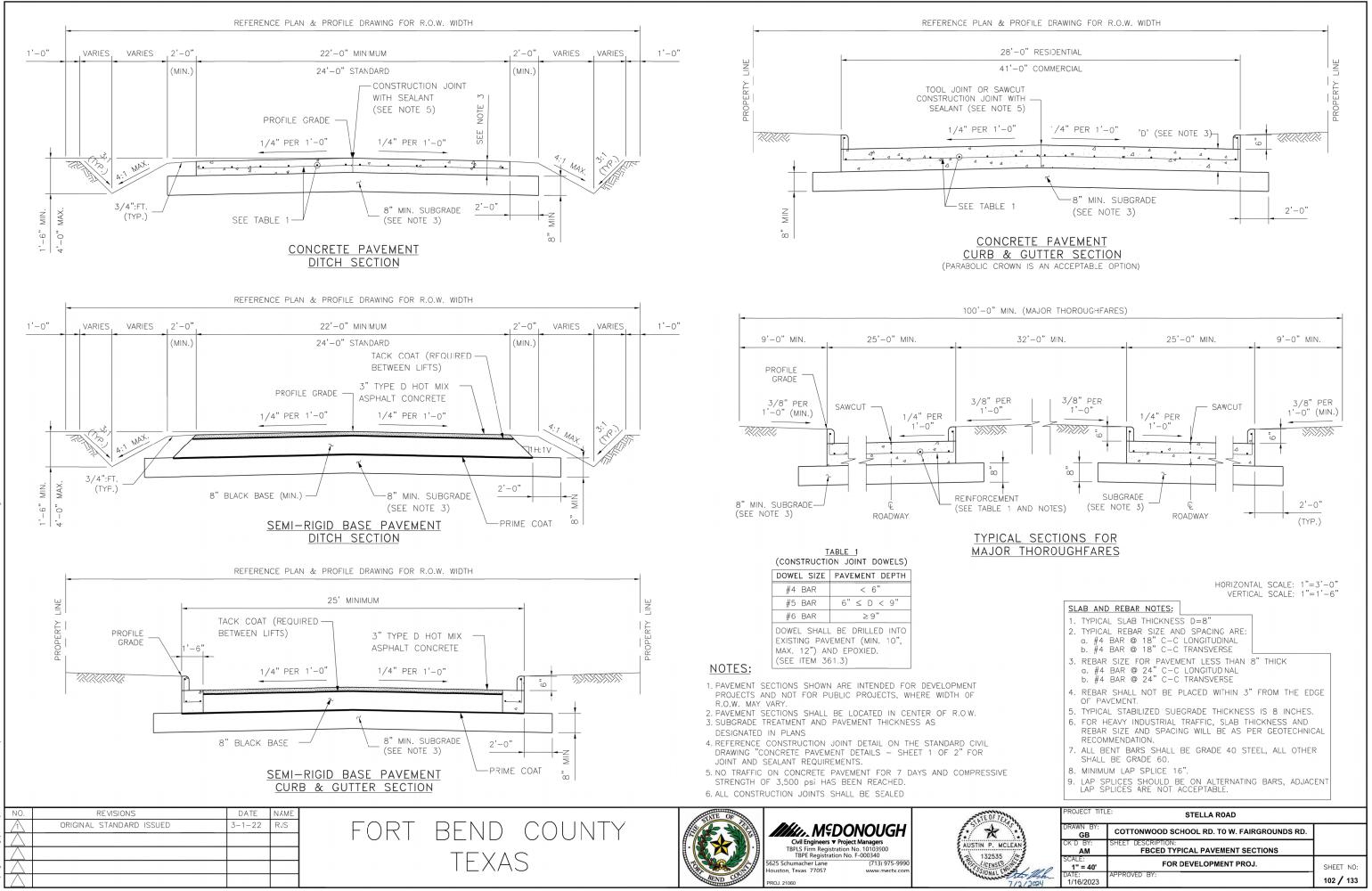
(RFD-TYPE II) REINFORCED FILTER DAM (TYPE II)

	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: SWPPP STA 37+00 TO END	
1 11	SCALE: 1" = 40'		SHEET NO
12024	DATE: 1/16/2023	APPROVED BY:	100 / 133

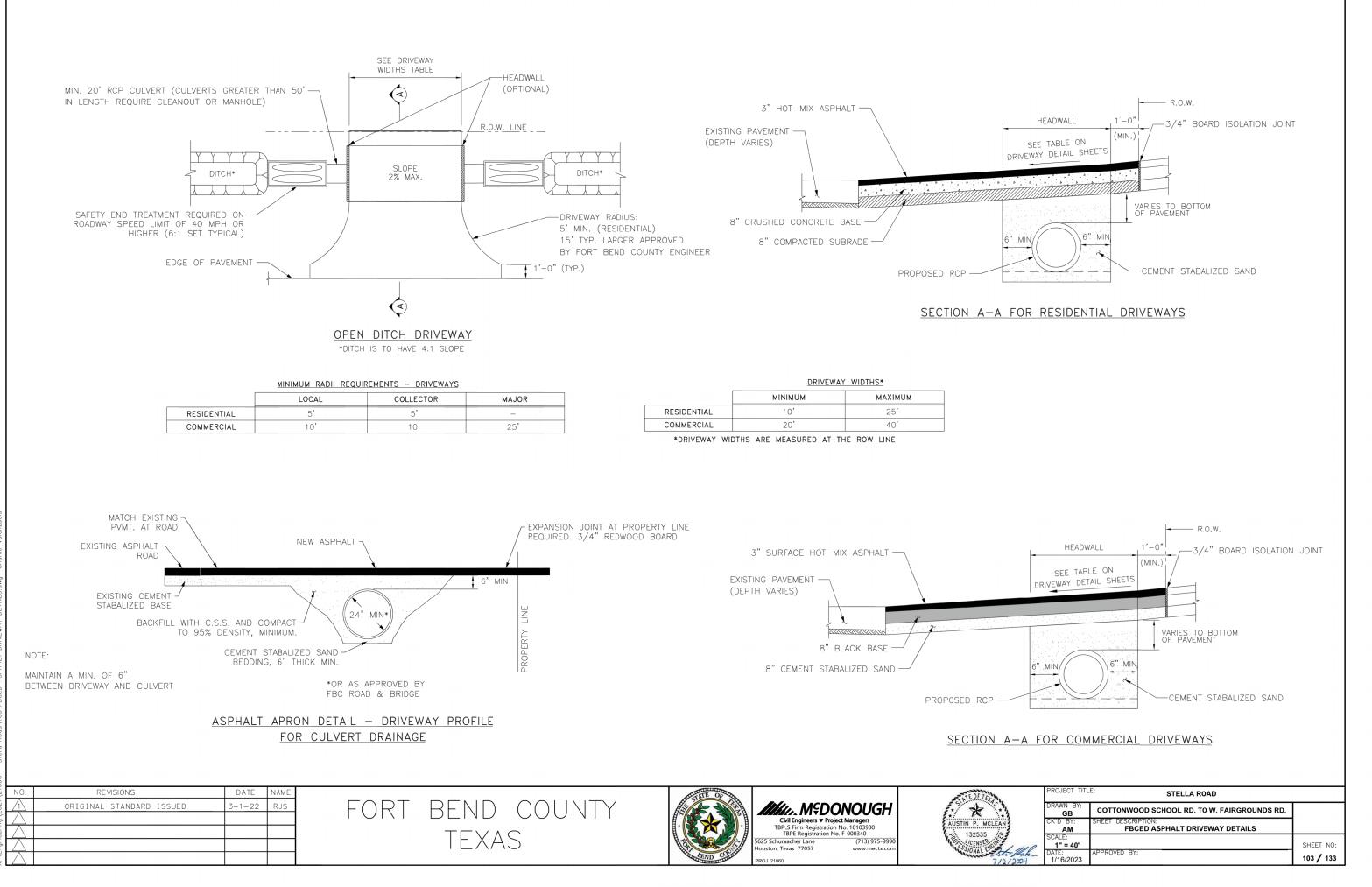


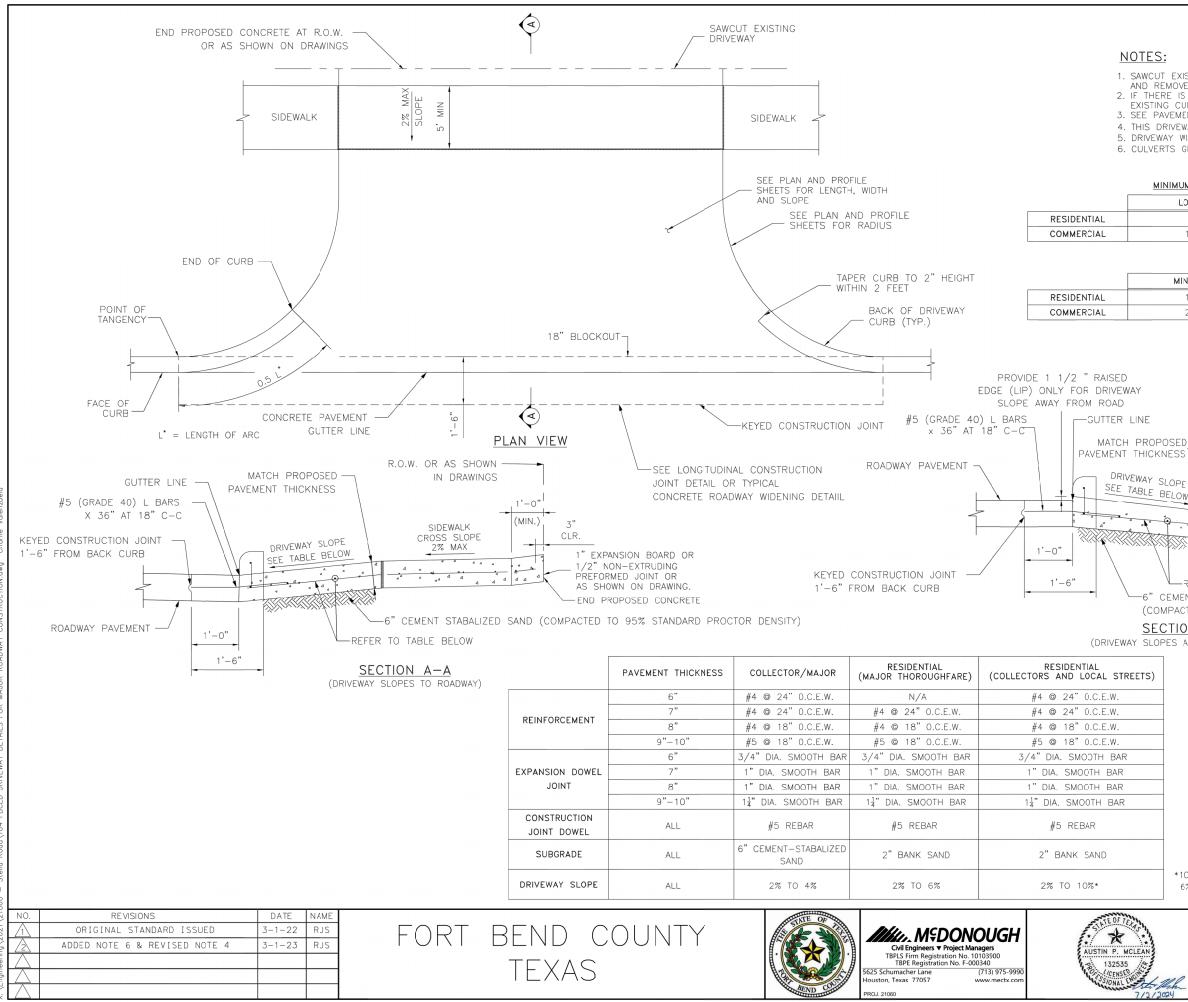
SWP - 1 04755 Road\CAD\DWG\06-Stella 20116 000. M:\04755.

	PROJECT TITLE: STELLA ROAD							
	FROM CO	OTTONWOOD	SCHOOL	ROAD	то	BAND	ROAD	CIVIL STANDARD
ying	SHEET DESCR	RIPTION: PROPOSED	DETENTION	BASIN	SWPI	PP		
6/18/24	DRAWN BY: NS							DATE: 6/18/24
,,	CK'D BY: MJ	SCALE:	1."	= 60)'			SHEET NO: 101/132



naineerina\2021\21060 - Stella Road\102 FBCED TYPICAL PAVEMENT SECTIONS FOR DEVELOPMENT P301.dwg Charlie Va



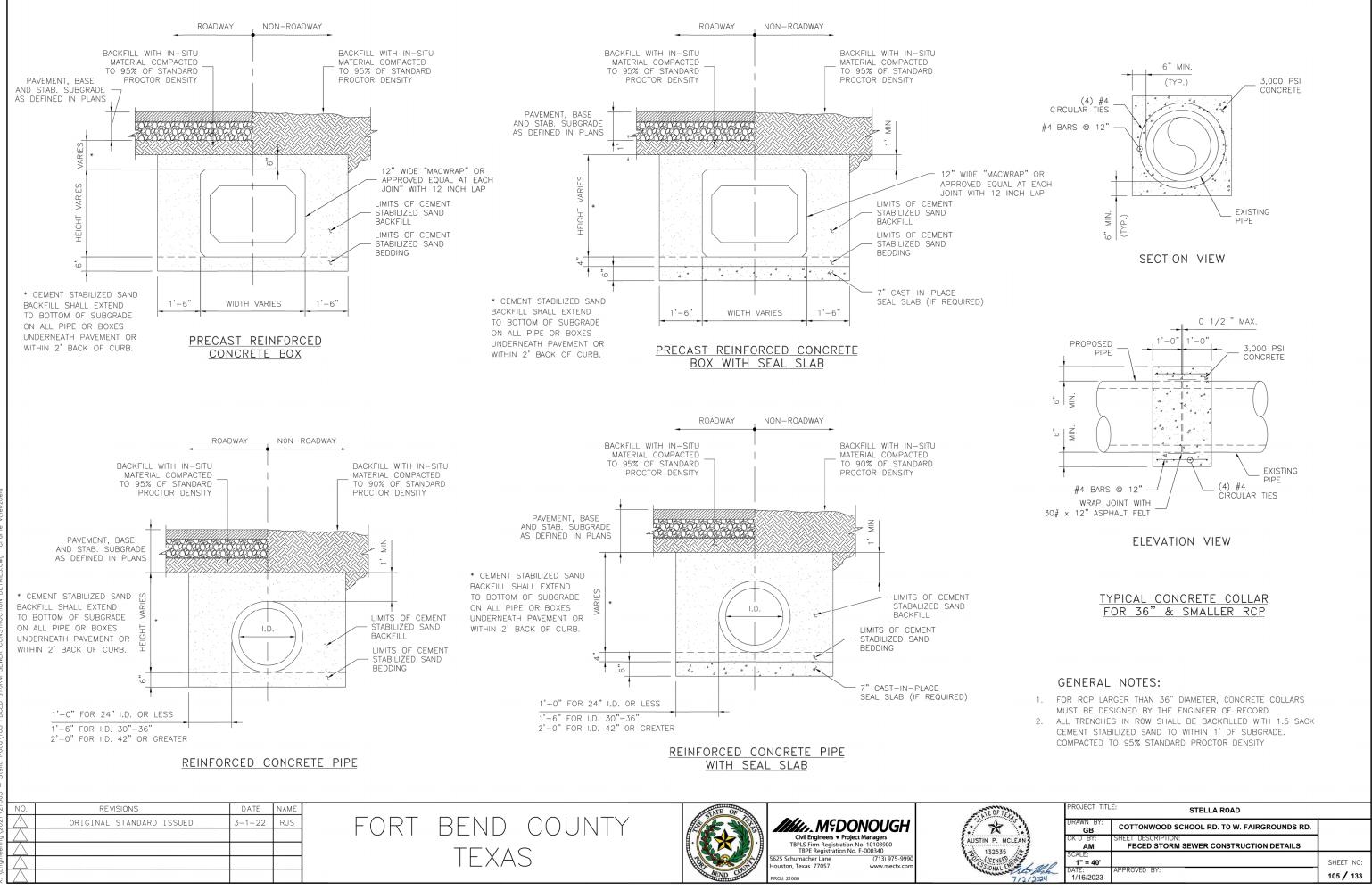


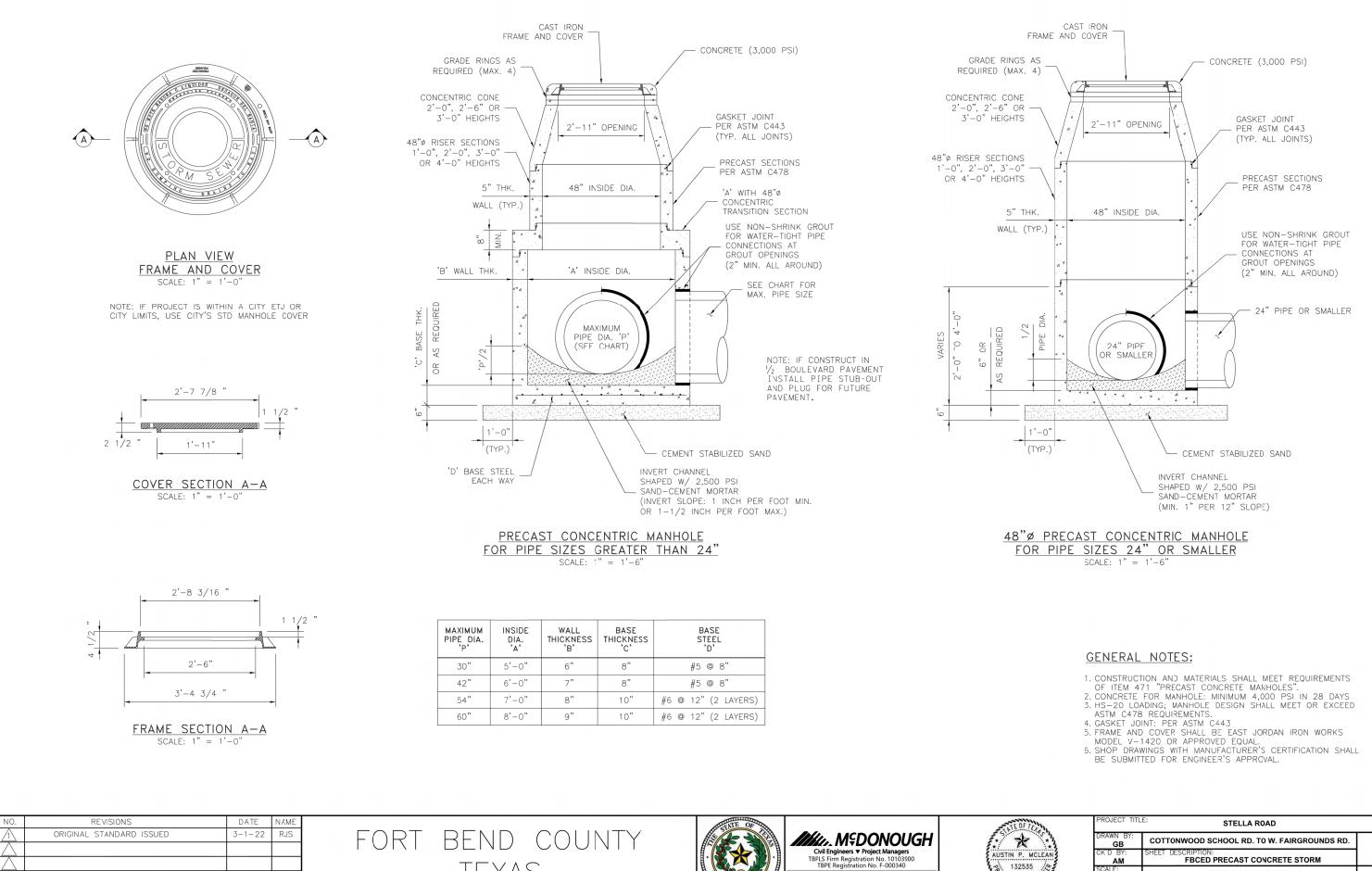
NOTES:

1. SAWCUT EXISTING DRIVEWAY AT R.O.W. LINE OR AS SHOWN ON DRAWING AND REMOVE EXISTING DRIVEWAY TO SAWCUT LINE. 2. IF THERE IS EXISTING CURB ON DRIVEWAY, CONNECT PROPOSED CURB TO EXISTING CURB; OTHERWISE TAPER CURB HEIGHT AS SHOWN. 3. SEE PAVEMENT DETAIL SHEET FOR CONCRETE CURB REINFORCEMENT. 4. THIS DRIVEWAY INSTALLATION IS GOVERNED BY HARRIS COUNTY ITEM 360 AND 433. 5. DRIVEWAY WIDTHS ARE MEASURED AT THE ROW LINE 6. CULVERTS GREATER THAN 50' IN LENGTH REQUIRE CLEANOUT OR MANHOLE

MINIMUM RADII REQUIREMENTS - DRIVEWAYS LOCAL COLLECTOR MAJOR 5' 5 _ 10' 10' 25' DRIVEWAY WIDTHS MINIMUM MAXIMUM 10' 25' 40' 20' R.O.W. OR AS SHOWN MATCH PROPOSED IN DRAWINGS 1'-0" DRIVEWAY SLOPE (MIN.) SEE TABLE BELOW SIDEWAI K CROSS SLOPE CLR 2% MAX 1" EXPANSION BOARD OR 1/2" NON-EXTRUDING PREFORMED JOINT OR a 4 4 4 4 AS SHOWN ON DRAWING. END PROPOSED -6" CEMENT STABALIZED SAND (COMPACTED TO 95% STANDARD PROCTOR DENSITY) SECTION A-A (DRIVEWAY SLOPES AWAY FROM ROADWAY) *10% ALLOWABLE ON PRIVATELY CONSTRUCTED PROJECTS 6% MAX ON PUBLIC PROJECTS

21	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
CLEAN	CK'D BY: AM	SHEET DESCRIPTION: FBCED DRIVEWAY DETAILS FOR MAJOR	
NGHUND I TOP	SCALE: 1" = 40'	ROADWAY CONSTRUCTION	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	104 / 133





TEXAS

(***) ***)	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
LEAN	CK'D BY: AM	SHEET DESCRIPTION: FBCED PRECAST CONCRETE STORM	
CINE AND	SCALE: 1" = 40'	SEWER MANHOLE DETAILS	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	106 / 133

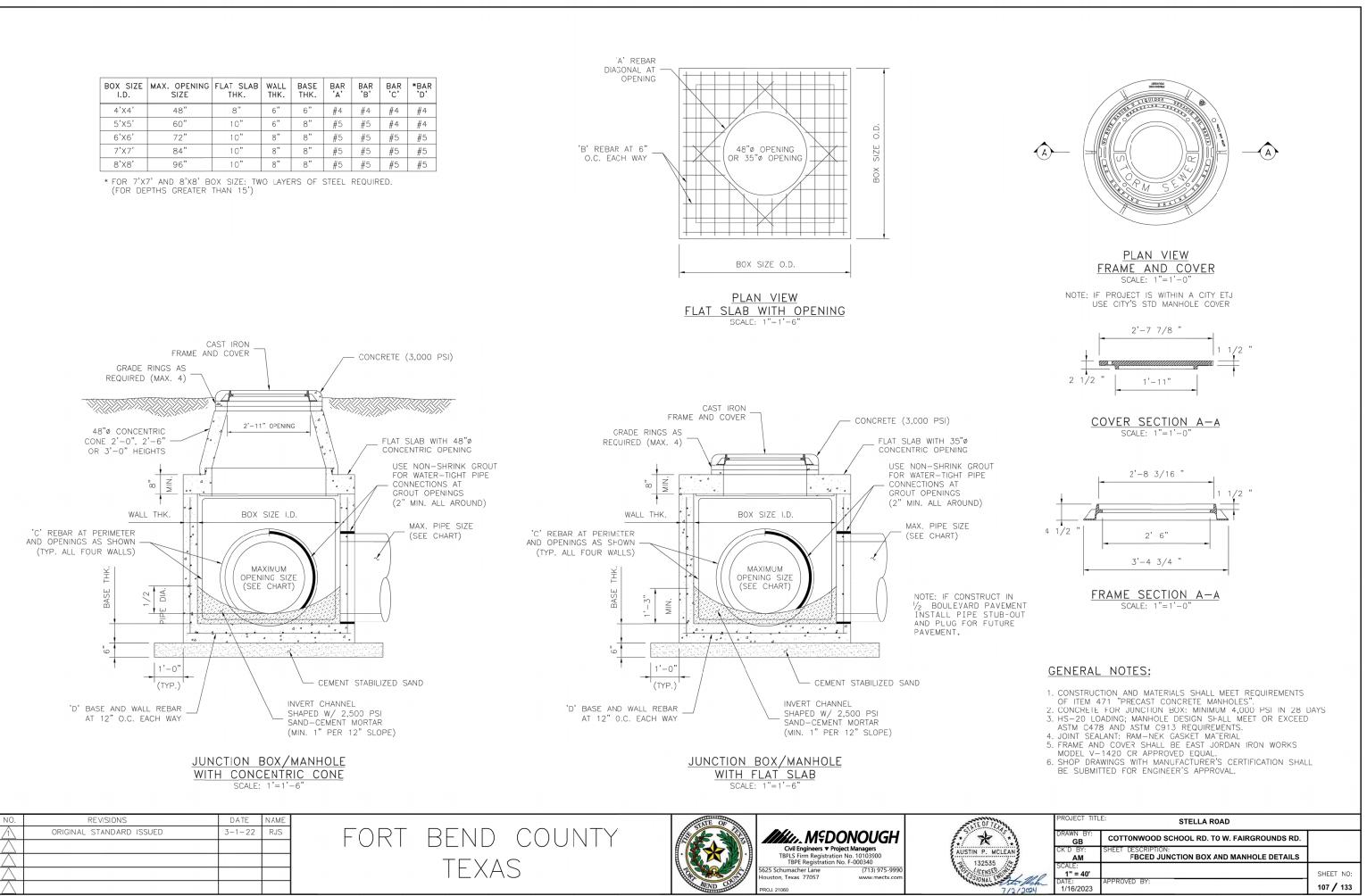
132555 132535

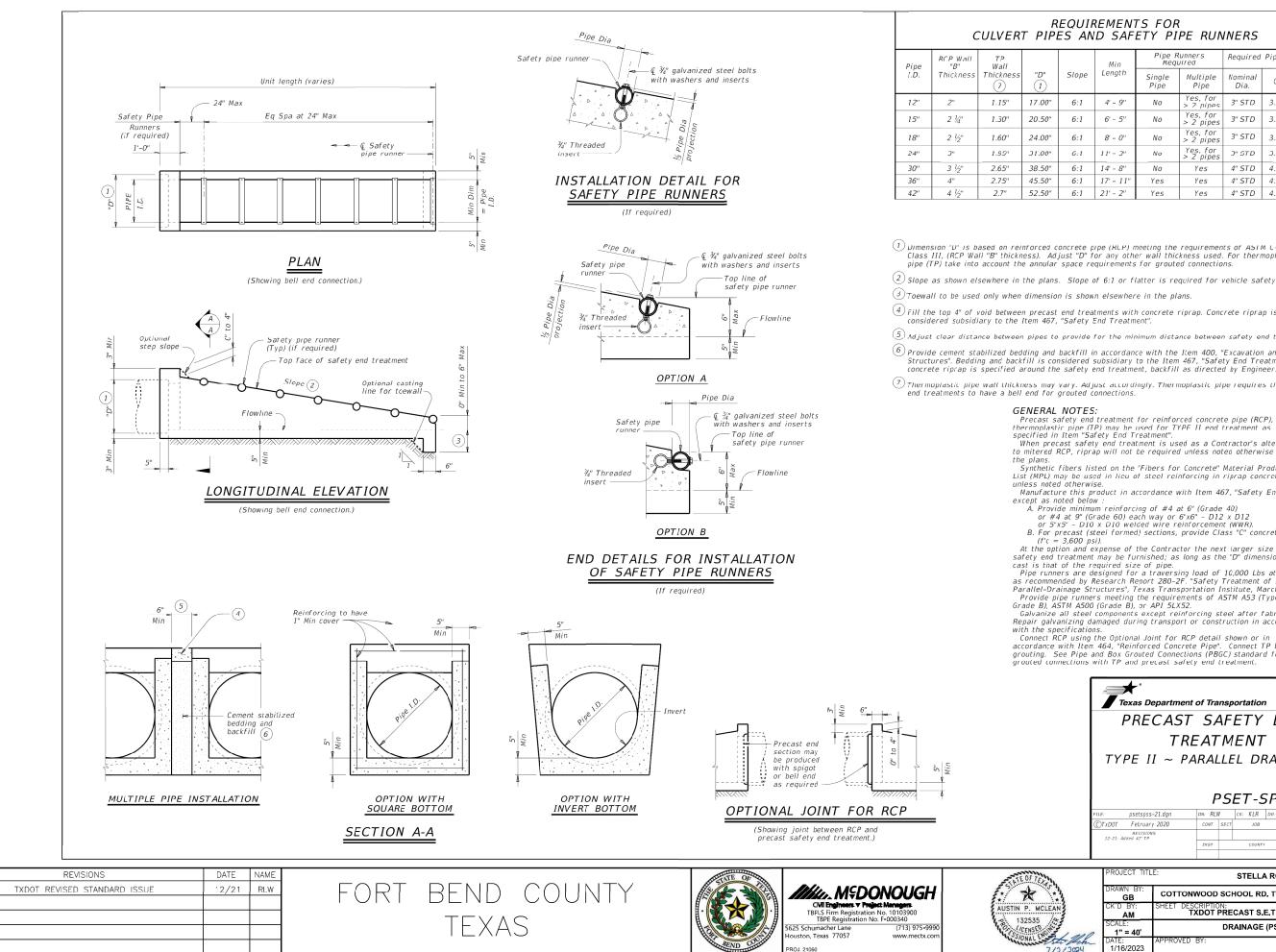
SIONA

625 Schumacher Lane

ton Texas 77057

(713) 975-9990





NO

REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS

		Min Length		lunners Jíred	Required	Pipe Run	ner Size
,"	Slope		Single Pipe	Multiple Pipe	Nominal Dia.	0.D.	I.D.
00''	6:1	4' - 9''	No	Yes, for > 7 pipes	3" STD	3.500"	3.068"
50"	6:1	6' - 5''	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
00''	6:1	8' - 0''	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
00''	G:1	11' - J"	No	Yes, for > 2 pipes	<i>כי ייכ</i>	3.500"	J.0G0''
50"	6:1	14' - 8''	No	Yes	4" STD	4.500"	4.026"
50"	6:1	17' - 11"	Yes	Yes	4" STD	4.500"	4.026"
50"	6:1	21' - 2"	Yes	Yes	4" STD	4.500"	4.026"

(1) Dimension "D" is based on reinforced concrete pipe (RCP) meeting the requirements of ASIM C-/6, Class III, (RCP Wall "B" thickness). Adjust "D" for any other wall thickness used. For thermoplastic pipe (TP) take into account the annular space requirements for grouted connections.

(4) Fill the top 4" of void between precast end treatments with concrete riprap. Concrete riprap is

 $^{(5)}$ Adjust clear distance between pipes to provide for the minimum distance between safety end treatments.

 $^{(6)}$ Provide cement stabilized bedding and backfill in accordance with the Item 400, "Excavation and Backfill for Structures". Bedding and backfill is considered subsidiary to the Item 467, "Safety End Treatment". When concrete riprap is specified around the safety end treatment, backfill as directed by Engineer.

 \bigcirc Thermoplastic pipe wall thickness may vary. Adjust accordingly. Thermoplastic pipe requires the safety end treatments to have a bell end for grouted connections.

GENERAL NOTES: Precast safety end treatment for reinforced concrete pipe (RCP), and thermoplastic pipe (TP) may be used for TYPE II end treatment as specified in Item "Safety End Treatment". When precast safety end treatment is used as a Contractor's alternate

to mitered RCP, riprap will not be required unless noted otherwise on

Synthetic fibers listed on the "Eibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Manufacture this product in accordance with Item 467, "Safety End Treatment" except as noted below : A. Provide minimum reinforcing of #4 at 6" (Grade 40)

or #4 at 9" (Grade 60) each way or 6"x6" - D12 x D12 or 5"x5" - D10 x D10 welded wire reinforcement (WWR).

B. For precast (steel formed) sections, provide Class "C" concrete (f'c = 3,600 psi).

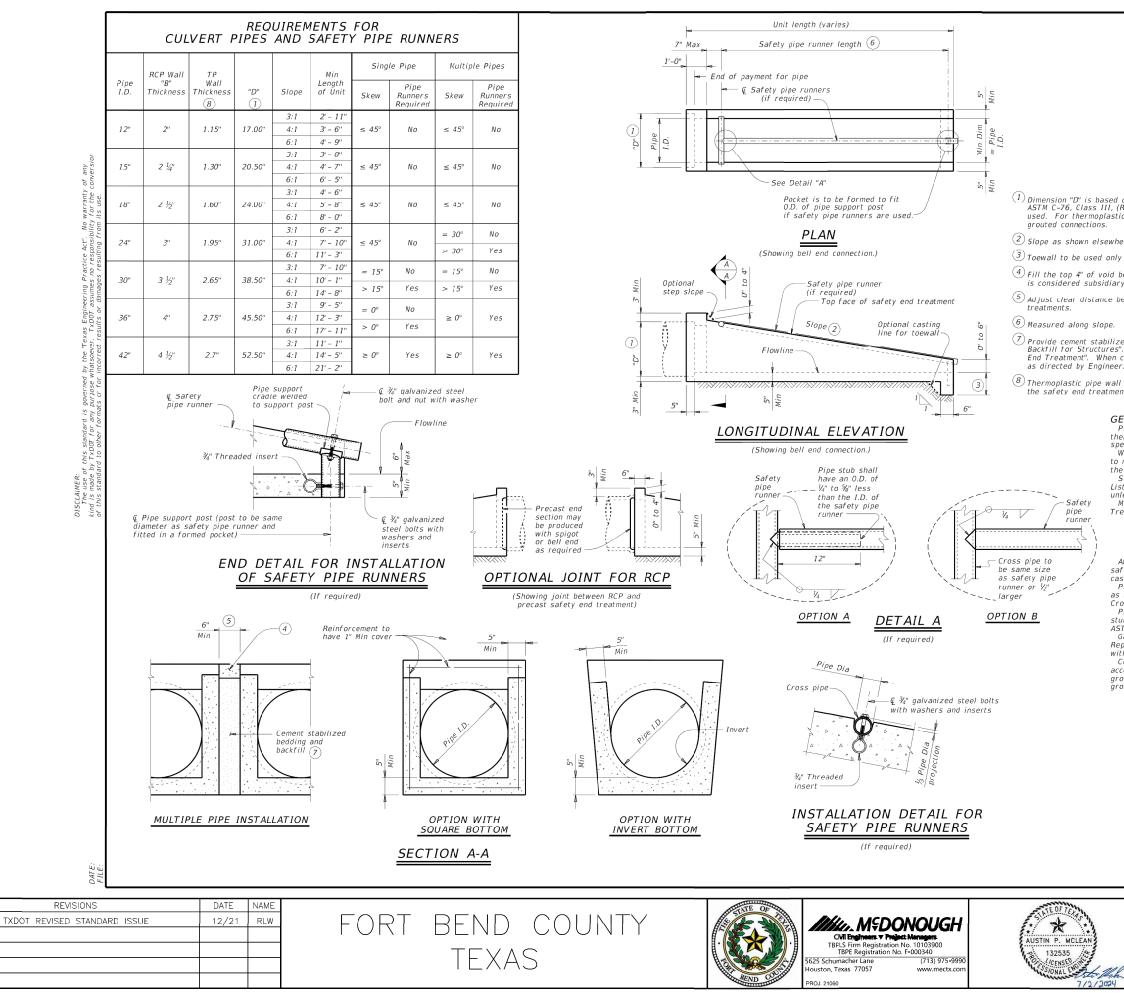
At the option and expense of the Contractor the next larger size of

At the option and expense of the Contractor the next larger size of safety end treatment may be furnished; as long as the "D" dimension cast is that of the required size of pipe. Pipe runners are designed for a traversing load of 10,000 Lbs at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981. Provide pipe runners meeting the requirements of ASTM A53 (Type E or S, Grade B), ASTM A500 (Grade B), or API 5LX52.

Galvanize all steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

Connect RCP using the Optional Joint for RCP detail shown or in accordance with Item 464, "Reinforced Concrete Pipe". Connect TP by grouting. See Pipe and Box Grouted Connections (PBGC) standard for grouted connections with TP and precast safety end treatment.





NO

SAFETY PIPE RUNNER DIMENSIONS

Max Safety	Required Pipe Runner Size					
Pipe Runner Length	Pipe Size	Pipe O.D.	Pipe I.D.			
11' - 2"	3" STD	3.500"	3.068"			
15' - 6"	3 ½" SID	4.000"	3.548''			
20' - 10''	4" STD	4.500"	4.026"			
35' - 4"	5" STD	5.563"	5.047"			

(1) Dimension "D" is based on reinforced concrete pipe (RCP) meeting the requirements of ASTM C-76, Class III, (RCP Wall "B" thickness). Adjust "D" for any other wall thickness for any other wall thickness for the second se used. For thermoplastic pipe (TP) take into account the annular space requirements for

2 Slope as shown elsewhere in plans. Slope of 3:1 or flatter is required for vehicle safety.

3 Toewall to be used only when dimension is shown elsewhere in the plans.

(4) Fill the top 4" of void between precast end treatments with concrete riprap. Concrete riprap is considered subsidiary to the Item 467, "Safety End Treatment".

 $^{(5)}$ Adjust clear distance between pipes to provide for the minimum distance between safety end

(7) Provide cement stabilized bedding and backfill in accordance with the Item 400, "Excavation and Backfill for Structures". Bedding and backfill is considered subsidiary to the Item 467, "Safety End Treatment", When concrete riprap is specified around the safety end treatment, backfill

 $^{(8)}$ Thermoplastic pipe wall thickness may vary. Adjust accordingly. Thermoplastic pipe requires the safety end treatments to have a bell end for grouted connections.

GENERAL NOTES:

Precast safety end treatment for reinforced concrete pipe (RCP), and thermoplastic pipe (TP) may be used for TYPE II end treatment as specified in Item "Safety End Treatment". When precast safety end treatment is used as a Contractor's alternate

to mitered RCP, riprap will not be required unless noted otherwise on the plans.

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Manufacture this product in accordance with Item 467, "Safety End Treatment" except as noted below : A. Provide minimum reinforcing of #4 at 6" (Grade 40)

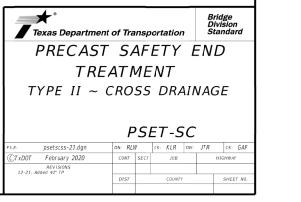
a. Provide minimi Protecting of the activity of t

(f'c = 3,600 psi).At the option and expense of the Contractor, the next larger size of

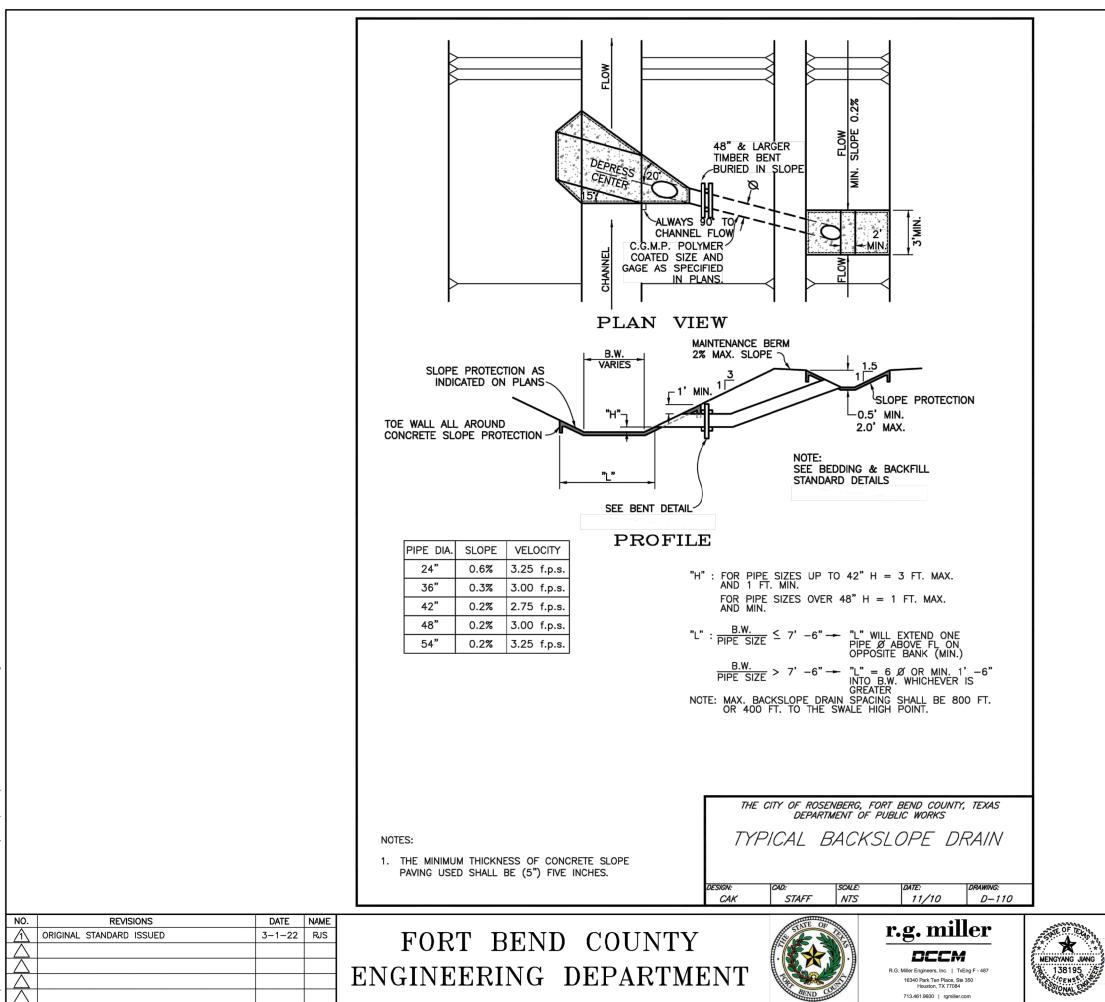
At the option and expense of the Contractor, the next larger size of safety end treatment may be furnished as long as the "D" dimension cast is that of the required size of pipe. Pipe runners are designed for a traversing load of 1,800 Lbs at yield as recommended by Research Report 280-1, "Safety Treatment of Roadside Cross-Drainage Structures", Texas Transportation Institute, March 1981. Provide safety pipe runners, cross pipes, pipe support posts, and pipe stubs meeting the requirements of ASTM AS3 (Type E or S, Grade B), ASTM AS00 (Grade B), or API 5LX52. Galvanize all steel components excent reinforcing steel after fabrication

Galvanize all steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

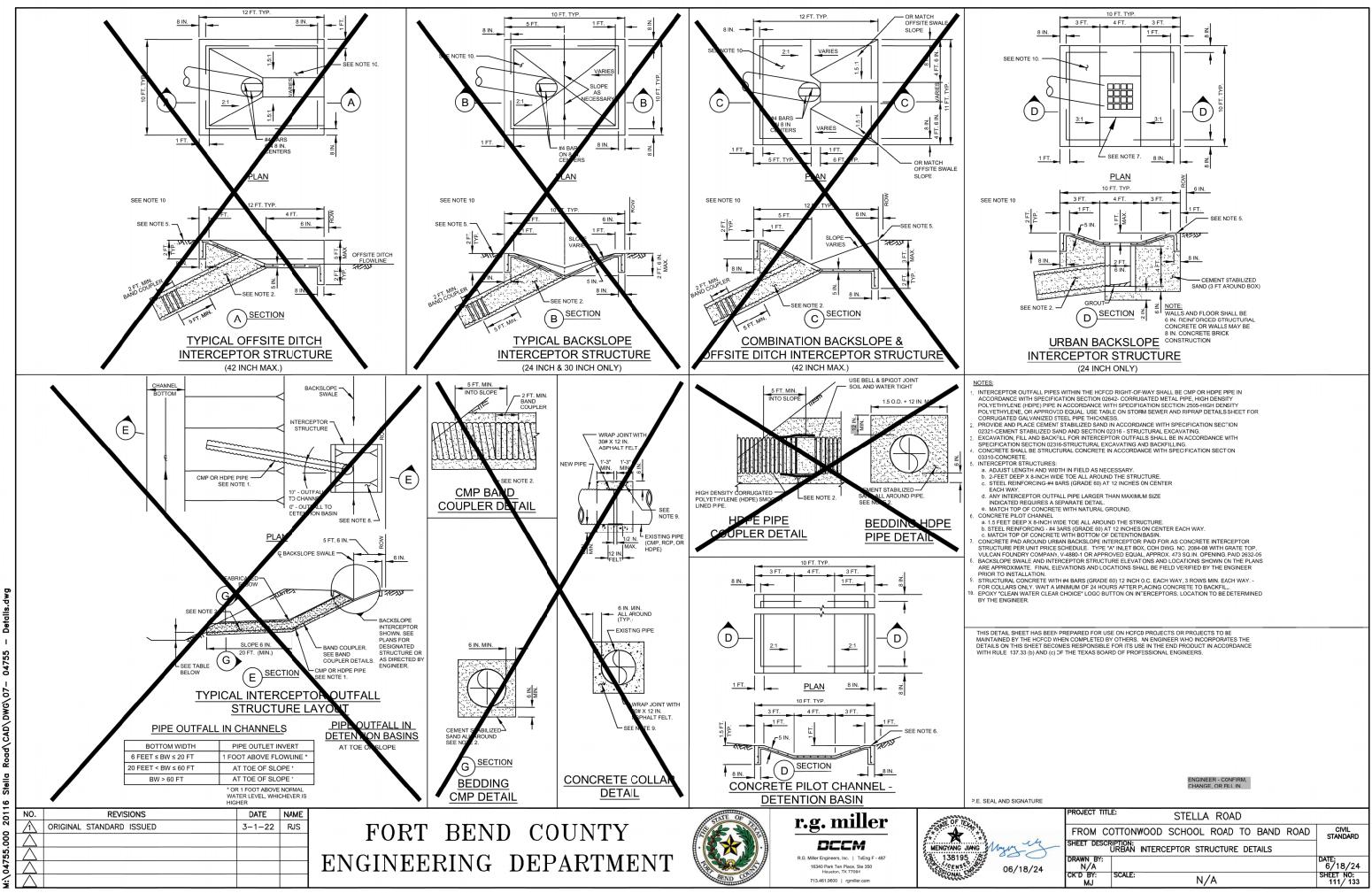
Connect RCP using the Optional Joint for RCP detail shown or in accordance with Item 464 "Reinforced Concrete Pipe". Connect TP by grouting. See Pipe and Box Grouted Connections (PBGC) standard for grouted connections with TP and precast safety end treatment.



	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
	CK'D BY: AM	SHEET DESCRIPTION: TXDOT PRECAST S.E.TTYII-CROSS	
0	SCALE: 1" = 40'	DRAINAGE (PSET-SC)	SHEET NO:
~	DATE: 1/16/2023	APPROVED BY:	109 / 133

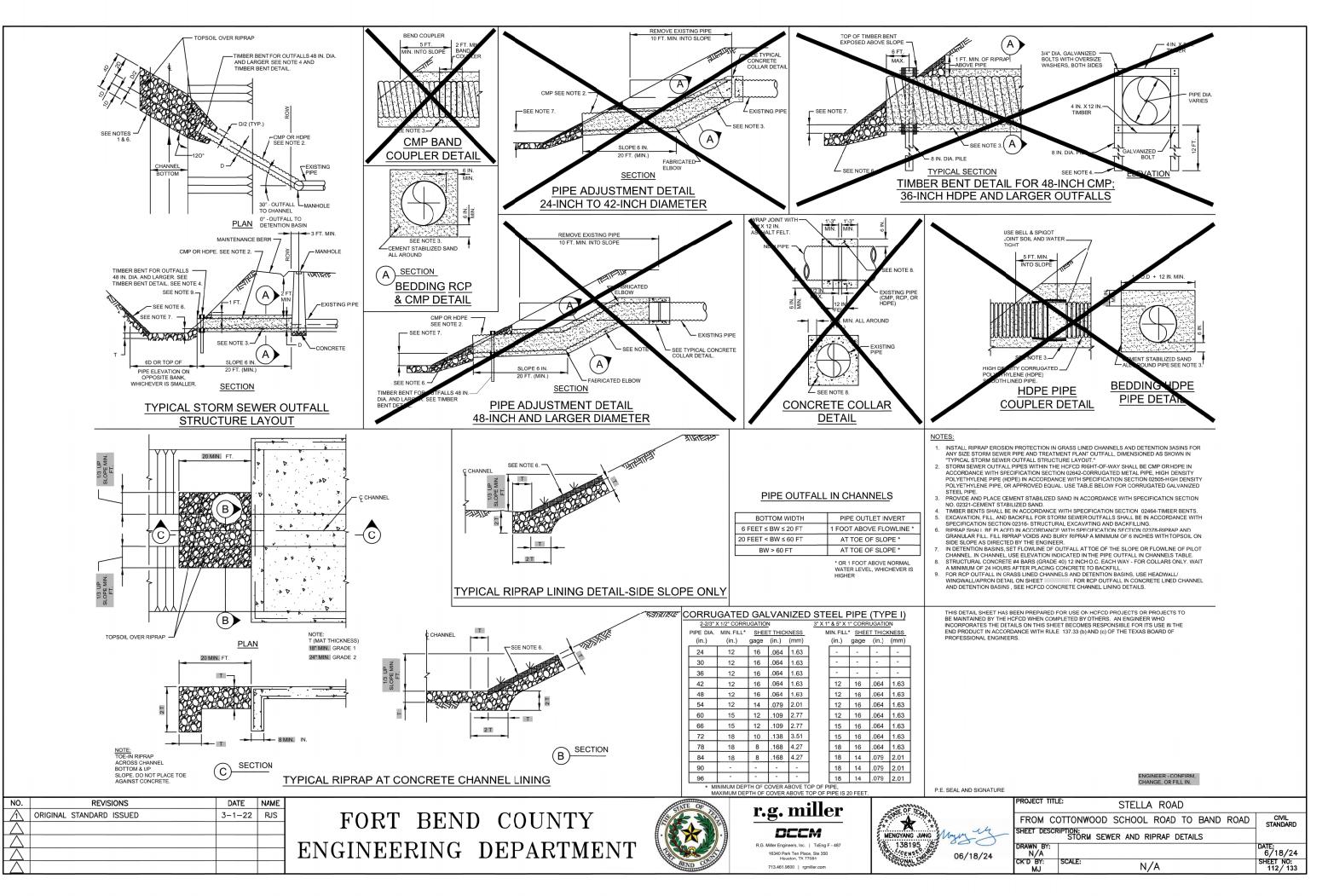


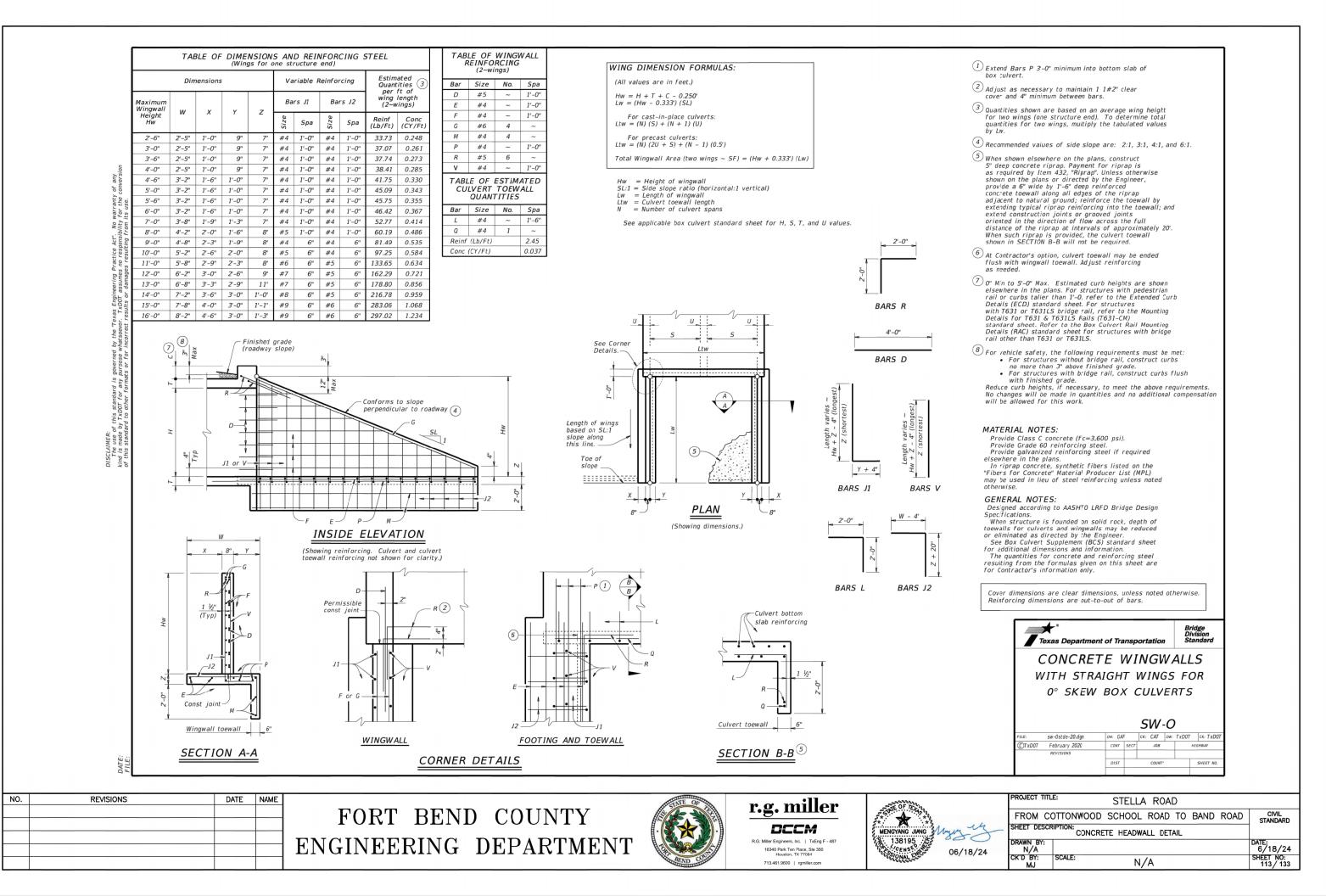
	PROJECT TIT	LE: STELLA ROAD	
		OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
my	SHEET DESC	RIPTION: INTERCEPTOR STRUCTURE DETAILS	
06/18/24	DRAWN BY: N/A		DATE: 6/18/24
,0,10,21	CK'D BY: MJ	SCALE: N/A	SHEET NO: 110/133



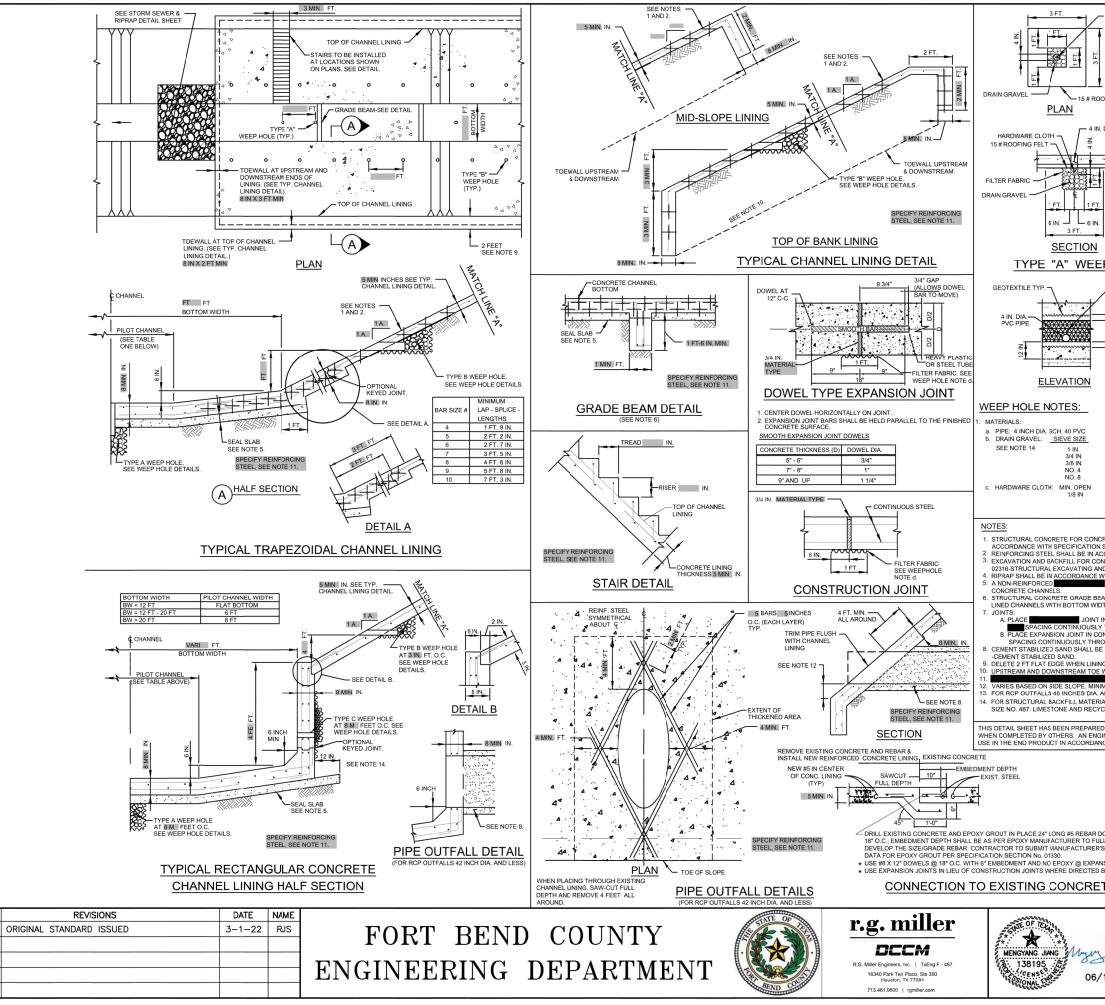
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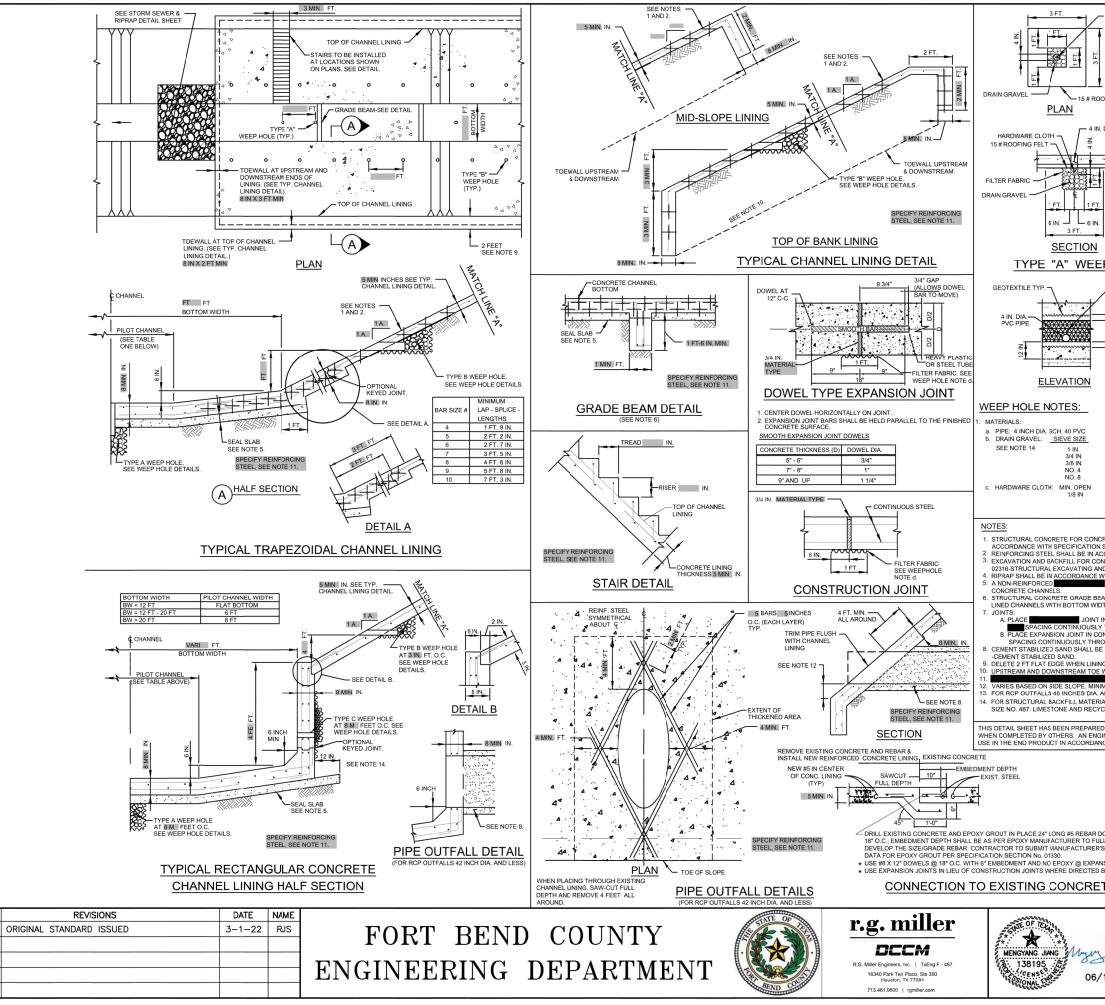
NO.

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- HARDWARE CLOTH		15 # ROOFING FELT	
T	HARDW. CLOTH	ARE 4 IN. FILTER FABRIC	
-	4 IN. DIA. PV TRIM PVC FL		
b	TRIM PVC FL TO FINISH CO	DNCRETE	
ł		DRAIN GRAVEL	
L OOFING FELT	15 # ROOFING	FELT J	
OUFING FELT		SECTION	
N. DIA. PVC			
CONC. CHA	NNEL 15	# ROOFING FELT - 4 IN. PVC PIPE WHARDWARE CLOTH	
TT T			
SEAL SLAB			
т.			
-		▼ I C DRAN GRAVEL	
N.		1 FT. 2 FT. 1 FT.	
l		SECTION	
- EP HOLE		TYPE "B" WEEP HOLE	
GEDCO	NUOUS DRAINAG	BOZ	
Z NON-W	VOVEN GEOTEXT		
z z			
20 IN			
	PPLY CONST. ADI		
	O FILTER FABRIC		
TYPE "	C" WEEF	P HOLE	
	d EILTE	R FABRIC:	
% FINER	NON-V	NOVEN GEOTEXTILE	
100 70 - 100	MIN. A MAX. J		
20 - 90 0 - 60 0	WEIG		
MAX. OPEN	3. TYPE "A"	P HOLE PIPE FLUSH WITH CONCRETE SURFACE. ' AND "C" WEEP HOLES FOR TYPICAL RECTANGULAR	
		TE CHANNEL LINING. TO BE STAGGERED.	
	IOLE DE		
N SECTION NO. (331	10-CONCRETE.	TRUCTURAL CONCRETE FOR SEAL SLAB SHALL BE IN	
ONCRETE CHANNEL		SECTION NO. 03310-CONCRETE. E IN ACCORDANCE WITH SPECIFICATION SECTION NO.	
		02378-RIPRAP AND GRANULAR FILL. AL SLAB IS REQUIRED BENEATH THE BOTTCM OF	
BEAMS SHALL BEINS			
IDTHS EQUAL TO OF			
T IN CONCRETE LOV LY THROUGH CHANN		COIDAL BOTTOM SECTIONS AND SLOPES AT	
	W, TRAPEZOIDAI	BOTTOM SECTIONS, AND SLOPES AT	
BE PROVIDED AND P	PLACED IN ACCOR	RDANCE WITH SPECIFICATION SECTION NO. 02321	
ING IS BELOW TOP O	OF BANK. SEE MIL	D-SLOPE LINING DETAIL. OM OF SLOPE AND 8 INCHES THICK.	
NIMUM 1 FOOT VERT			
RIAL PER SPECIFICA	TION SECTION 2	3WALL DETAIL ON SHEET COARSE AGGREGATE ASTM C33	
YCLED CONCRETE I	NOT ALLOWED.		
ED FOR USE ON HO	FC PROJECTS OF	R PROJECTS TO BE MAINTAINED BY THE HCFCD	
		F THE TEXAS BOARD OF PROFESSIONAL ENGINEERS.	
DOWELS @ ULLY			
R'S PRODUCT			
ANSION JOINTS. D BY ENGINEER.			
ETE		ENGINEER - CONFIRM,	
	P.E. SEA	L AND SIGNATURE CHANGE, OR FILL IN.	
	PROJECT TITL	e: STELLA ROAD	
	FROM CO	OTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL
un -	SHEET DESCR	RIPTION:	STANDARD
30	DRAWN BY:	CONCRETE CHANNEL LINING DETAILS	DATE:
/18/24	N/A	SCALE.	6/18/24
	CK'D BY: MJ	SCALE: N/A	SHEET NO: 114/133



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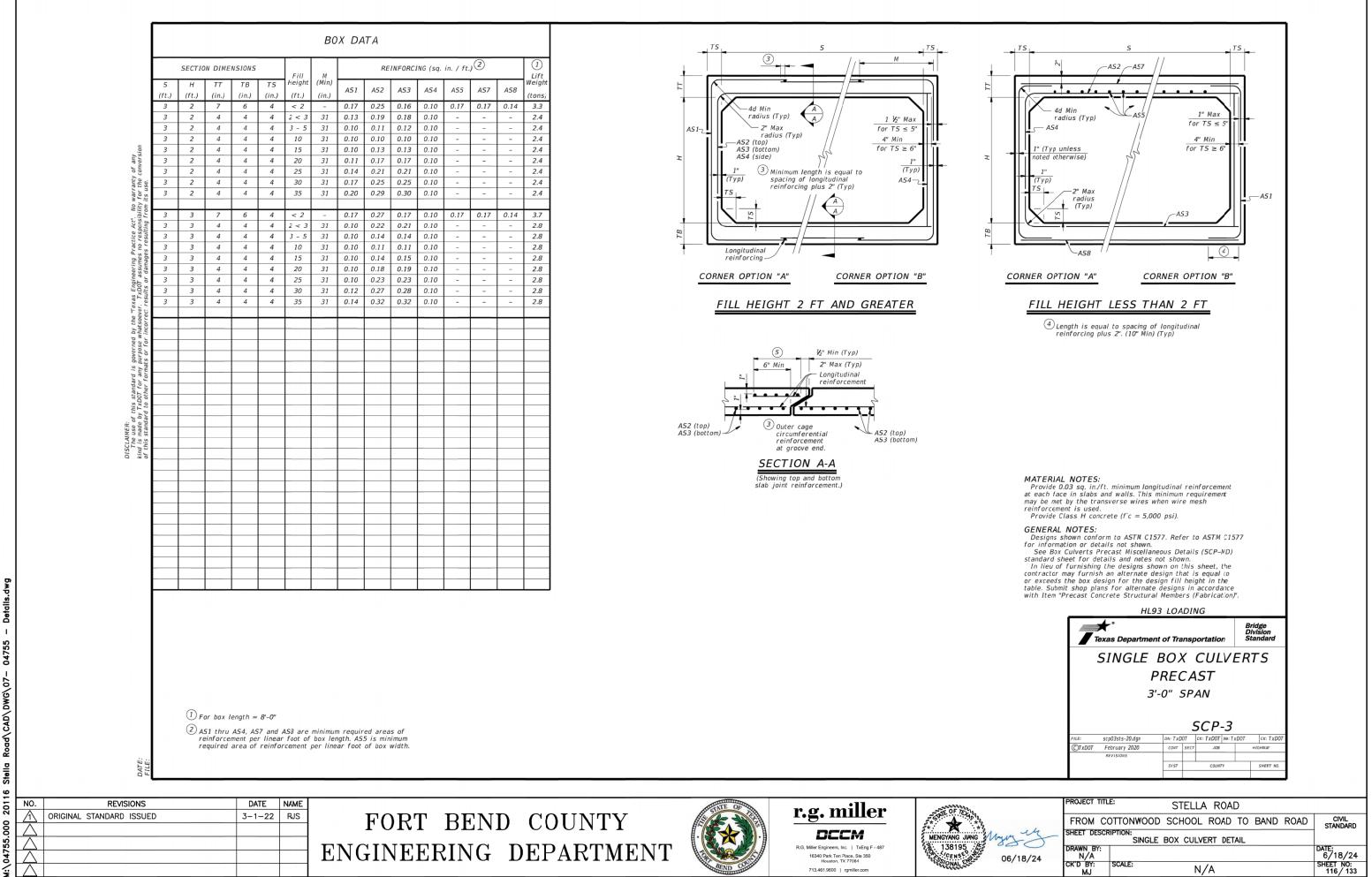
NO.

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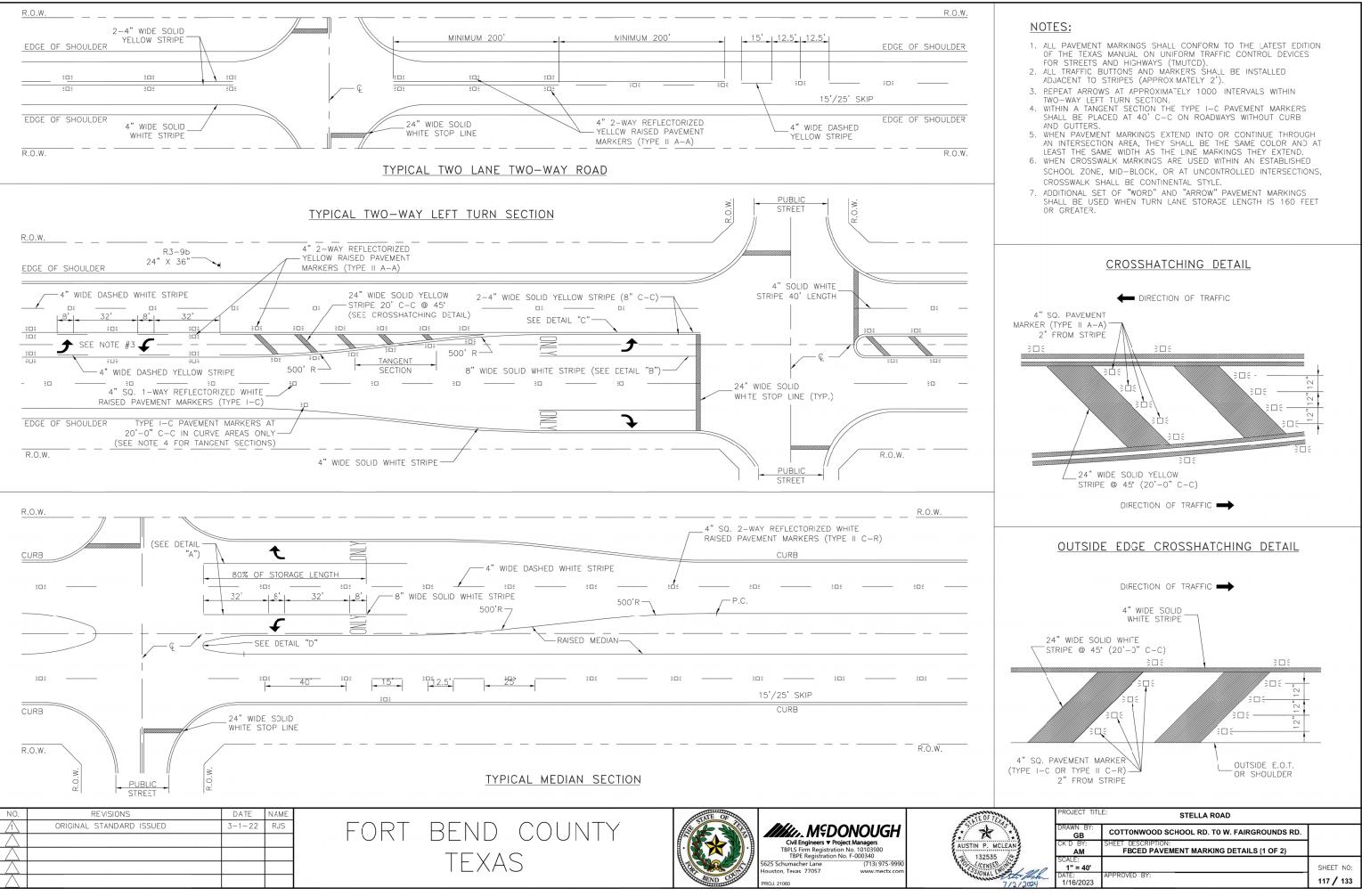
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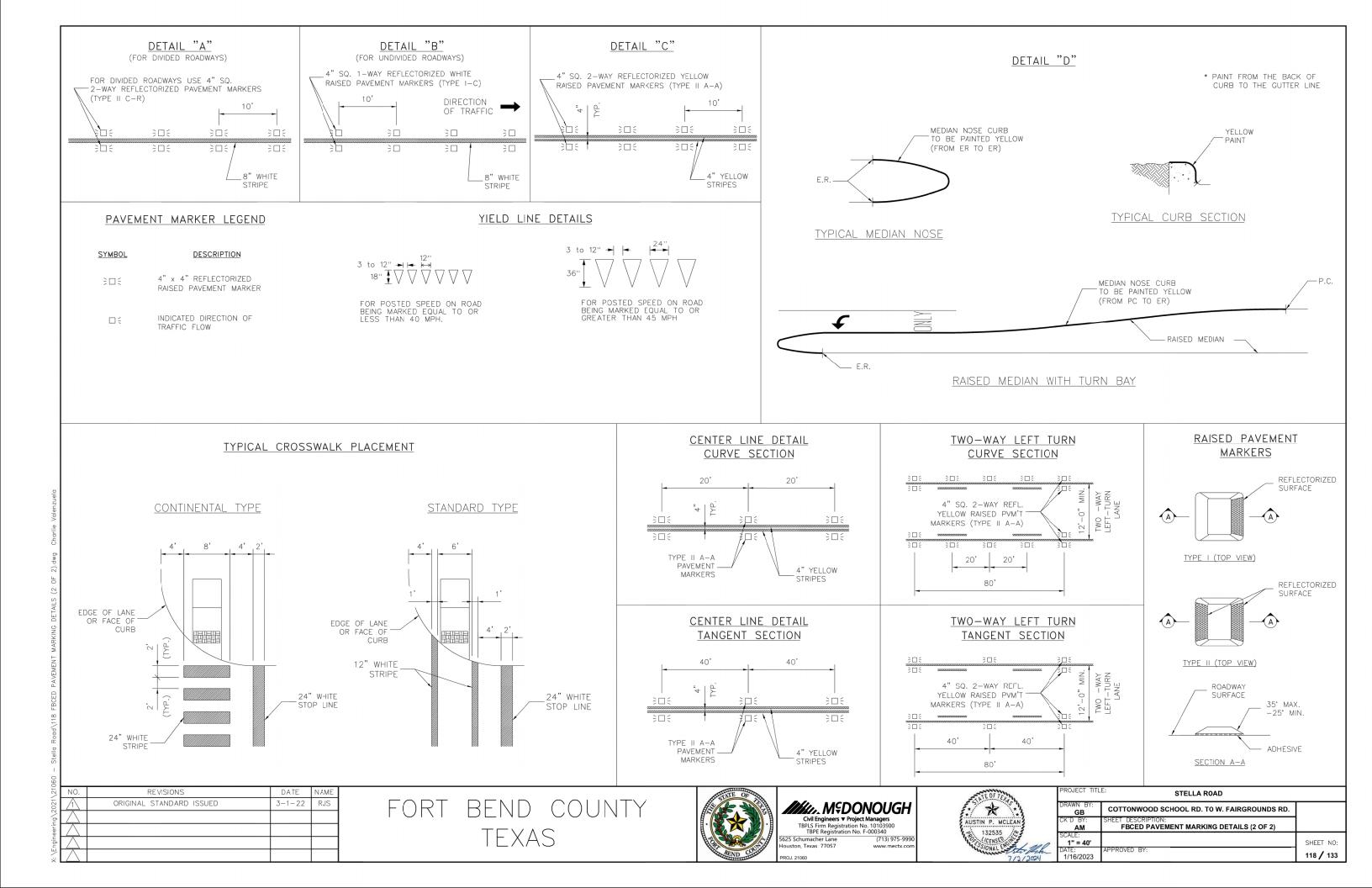
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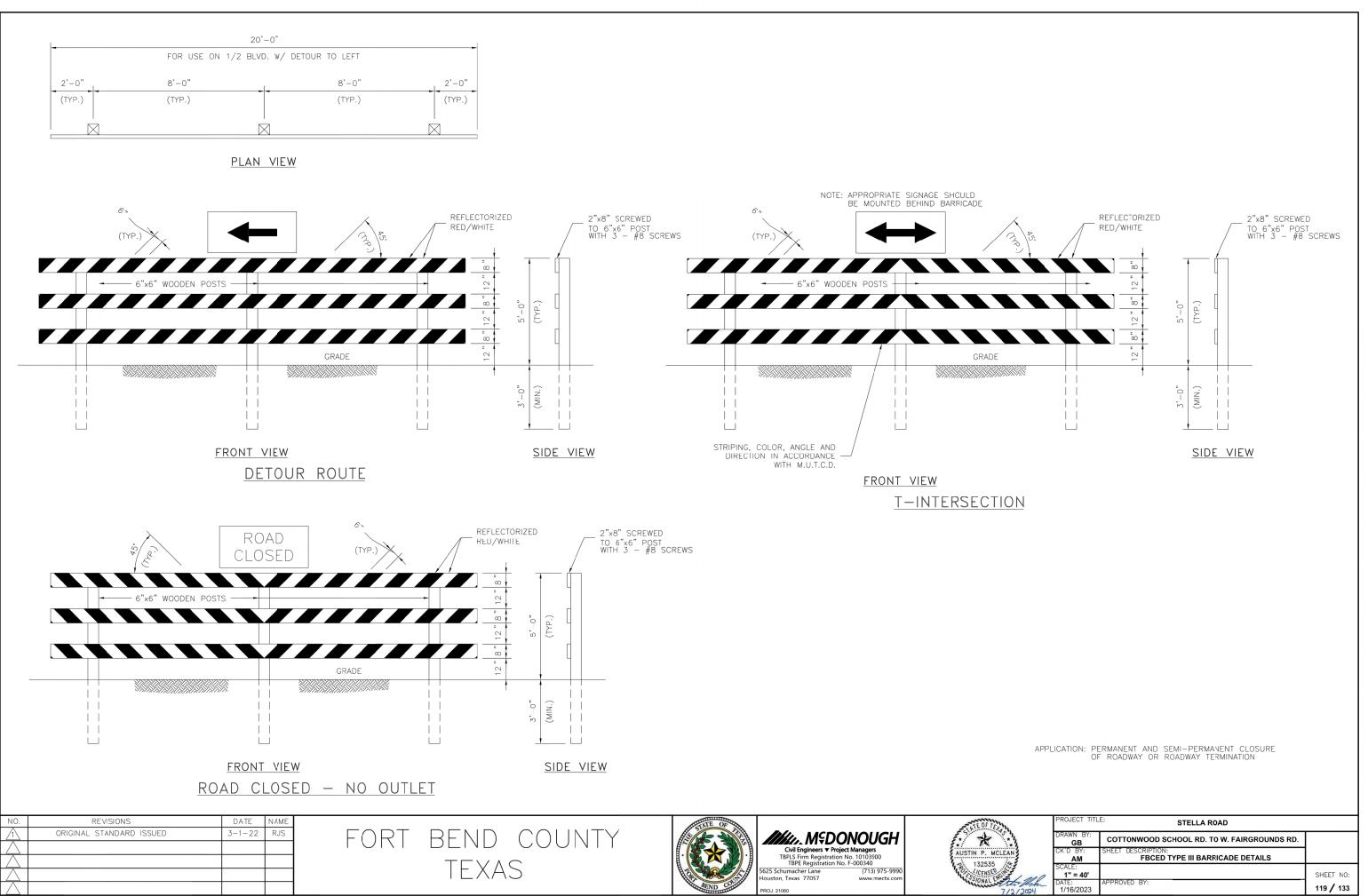
- HARDWARE CLOTH		
Ŧ	HARDWARE 4 IN. FILTER FABRIC	
-	4 IN. DIA. PVC TRIM PVC FLUSH	
5	TO FINISH CONCRETE	
	DRAIN GRAVEL	
OOFING FELT	15 # ROOFING FELT	
	SECTION	
N. DIA. PVC	- 4 IN. PVC PPE	
SEAL SLAB		
-1		
т.		
N.	1 FT. 2 FT. 1 FT.	
-		
<u> </u>	SECTION	
EP HOLE	TYPE "B" WEEP HOLE	
Z NON-W		
4		
12 IN 20 IN		
Z A		
тс	DILTER FABRIC TOP & SECTION	
	C" WEEP HOLE	
<u></u>		
% FINER	d. FILTER FABRIC: NON-WOVEN GEOTEXTILE	
100 70 - 100	MIN. AOS SIEVE NO. MAX. AOS SIEVE NO.	
20 - 90 0 - 60 0		
MAX. OPEN	 CUT WEEP HOLE PIPE FLUSH WITH CONCRETE SURFACE. TYPE "A" AND "C" WEEP HOLES FOR TYPICAL RECTANGULAR 	
	CONCRETE CHANNEL LINING. TO BE STAGGERED.	
	NING AND NONSTRUCTURAL CONCRETE FOR SEAL SLAB SHALL BE IN	
N SECTION NO. (331		
	LINING SHALL BE IN ACCORDANCE WITH SPECIFICATION SECTION NO.	
WITH SPECIFICATIO	DN SECTION NO. 02378-RIPRAP AND GRANULAR FILL. L CONCRETE SEAL SLAB IS REQUIRED BENEATH THE BOTTCM OF	
	TALLED TRANSVERSELY AT THE CENTERS ON CONCRETE	
	R GREATER THAN 20 FEET.	
LY THROUGH CHANN		
ROUGH CHANNEL LI	W, TRAPEZOIDAL BOTTOM SECTIONS, AND SLOPES AT CONTRACT AND SECTIONS AND SLOPES AT CONTRACT AND A SECTION SECTION NO. 02321	
	LACED IN ACCORDANCE WITH SPECIFICATION SECTION NO. 02321	
E WALL	AT BOTTOM OF SLOPE AND 8 INCHES THICK.	
	ICAL AT TOE OF SLOPE. HEADWALL/WINGWALL DETAIL ON SHEET	
	TION SECTION 2316, USE CONCRETE COARSE AGGREGATE ASTM C33	
	C PROJECTS OR PROJECTS TO BE MAINTAINED BY THE HCFCD	
IGINEER WHO INCOF	7.33 (b) AND (c) OF THE TEXAS BOARD OF PROFESSIONAL ENGINEERS.	
DOWELS @		
ULLY R'S PRODUCT		
ANSION JOINTS.		
D BY ENGINEER.		
ETE	ENGINEER -CONFIRM, P.E. SEAL AND SIGNATURE CHANGE, OR FILL IN.	
<u>ETE</u>	P.E. SEAL AND SIGNATURE CHANGE, OR FILL IN.	
<u>ETE</u>	P.E. SEAL AND SIGNATURE CHANGE, OR FILL IN. PROJECT TITLE: STELLA ROAD	01/11
<u>TE</u>	PROJECT TITLE: CHANGE, OR FILL IN. PROJECT TITLE: STELLA ROAD FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD	CIVIL STANDARD
TE	P.E. SEAL AND SIGNATURE CHANGE, OR FILL IN. PROJECT TITLE: STELLA ROAD FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD SHEET DESCRIPTION: CONCRETE CHANNEL LINING DETAILS	STANDARD
<u>TE</u> /18/24	P.E. SEAL AND SIGNATURE CHANGE, OR FILL IN. PROJECT TITLE: STELLA ROAD FROM COTTONWOOD SCHOOL ROAD TO BAND ROAD SHEFT DESCRIPTION:	

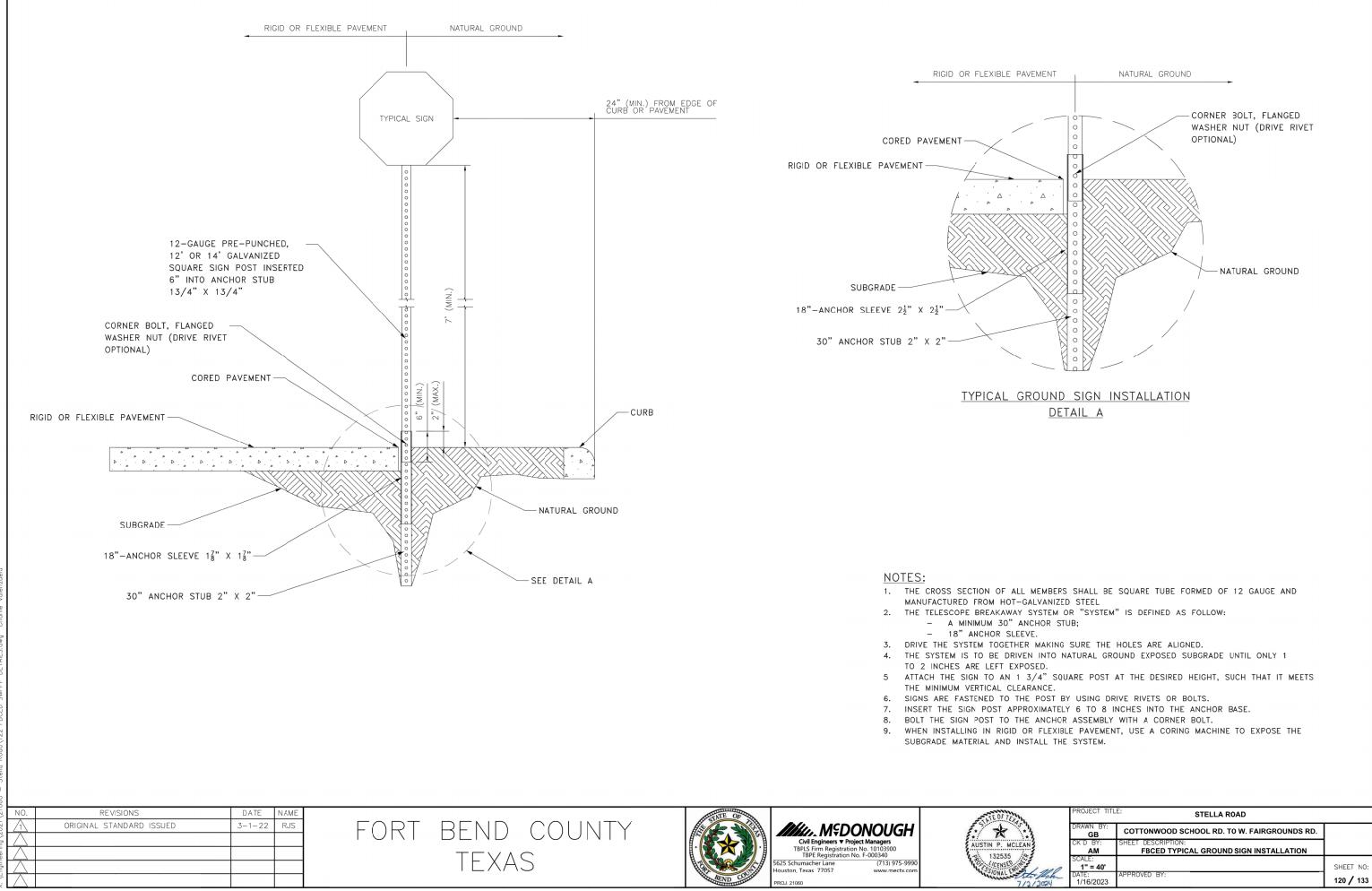


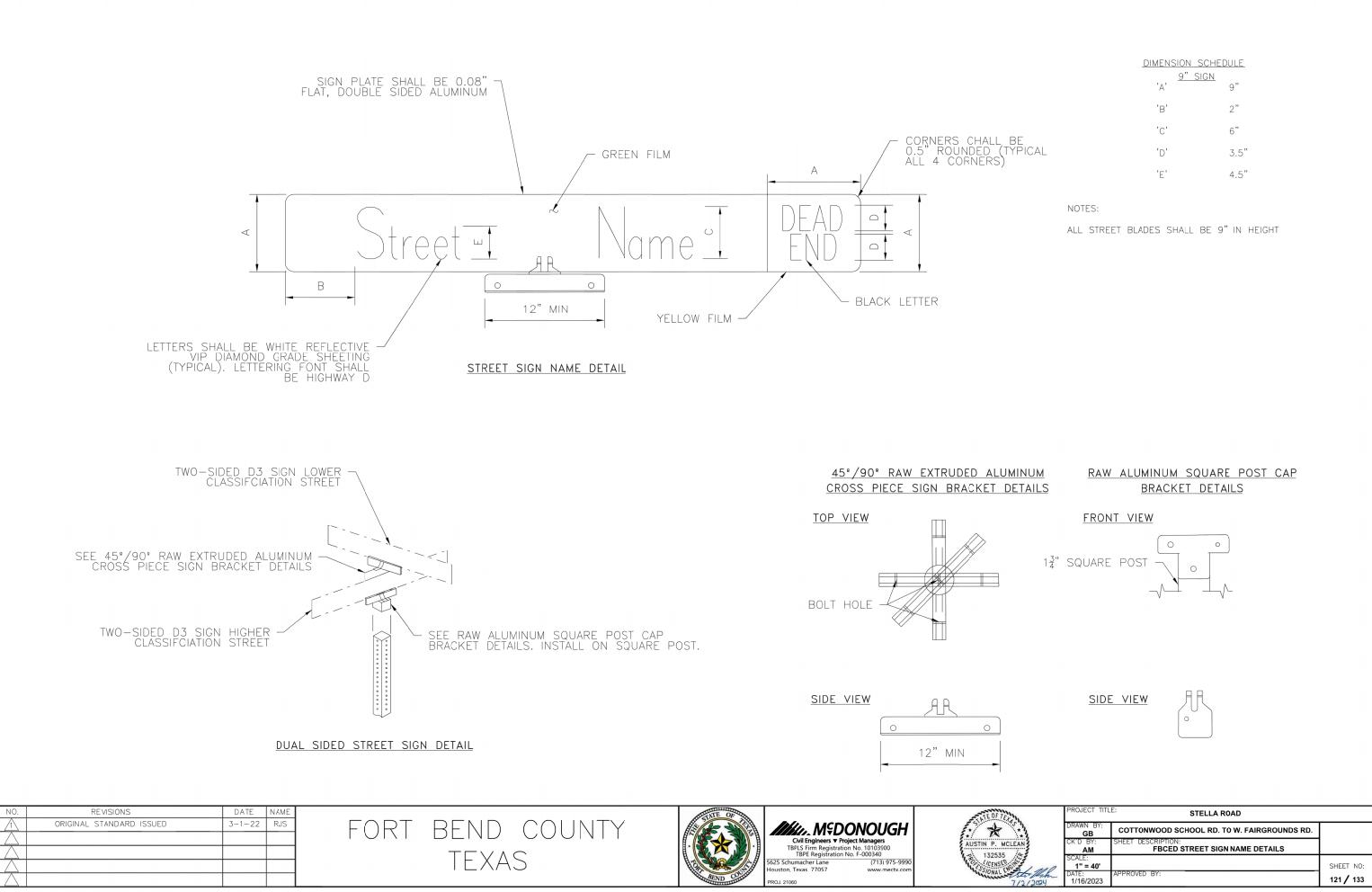
Details 1 04755 Road\CAD\DWG\07-Stella 9 201



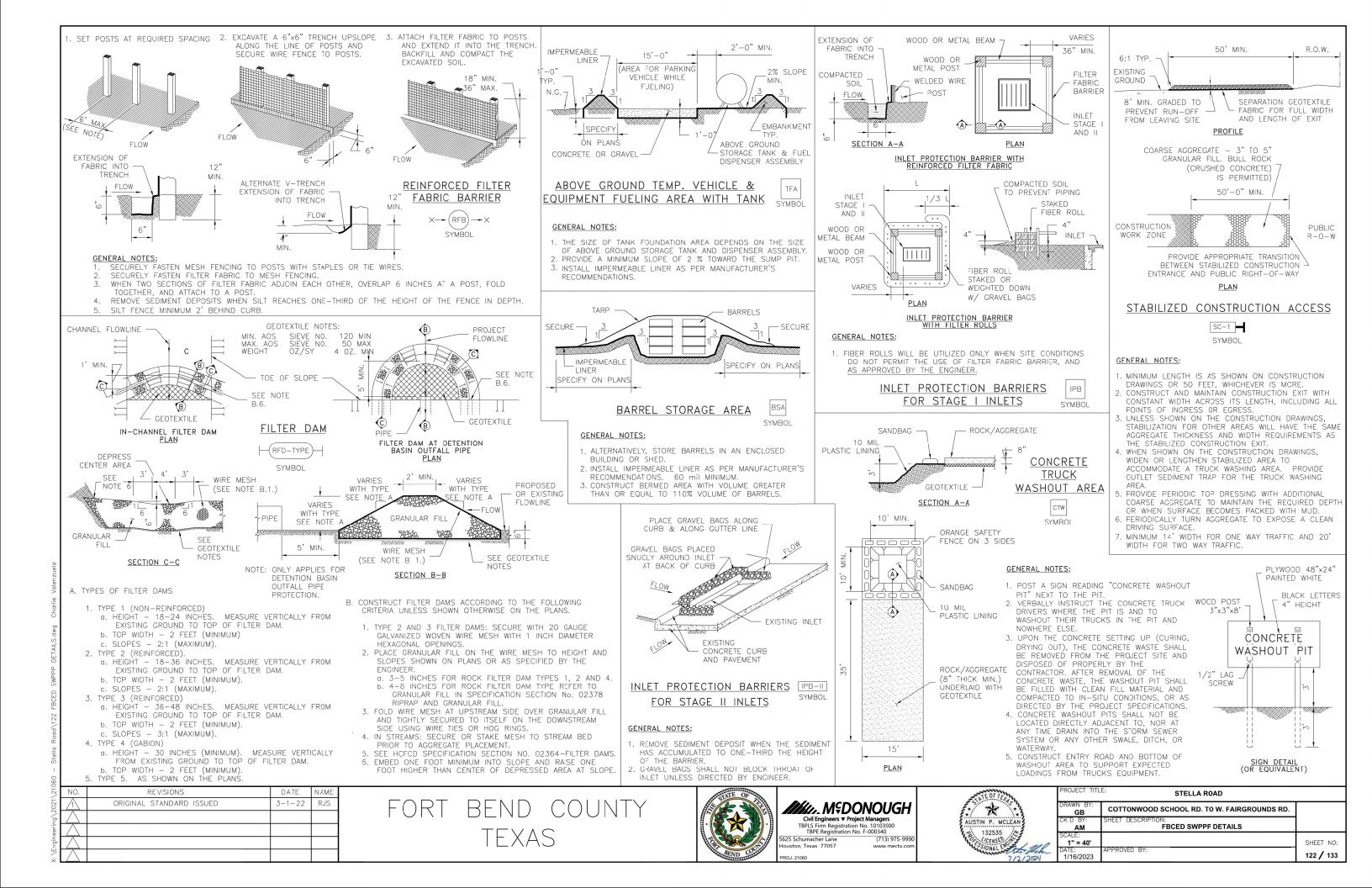


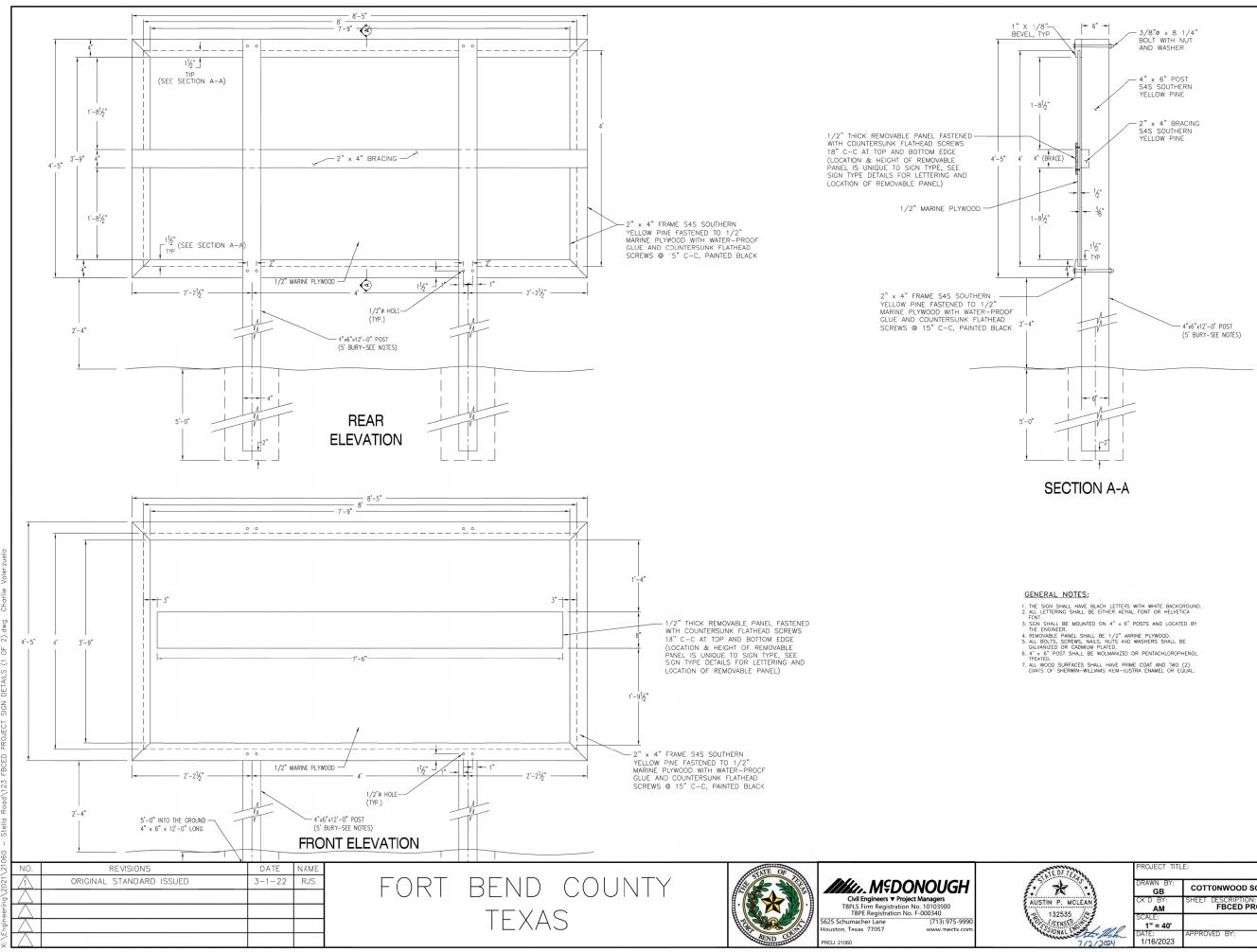




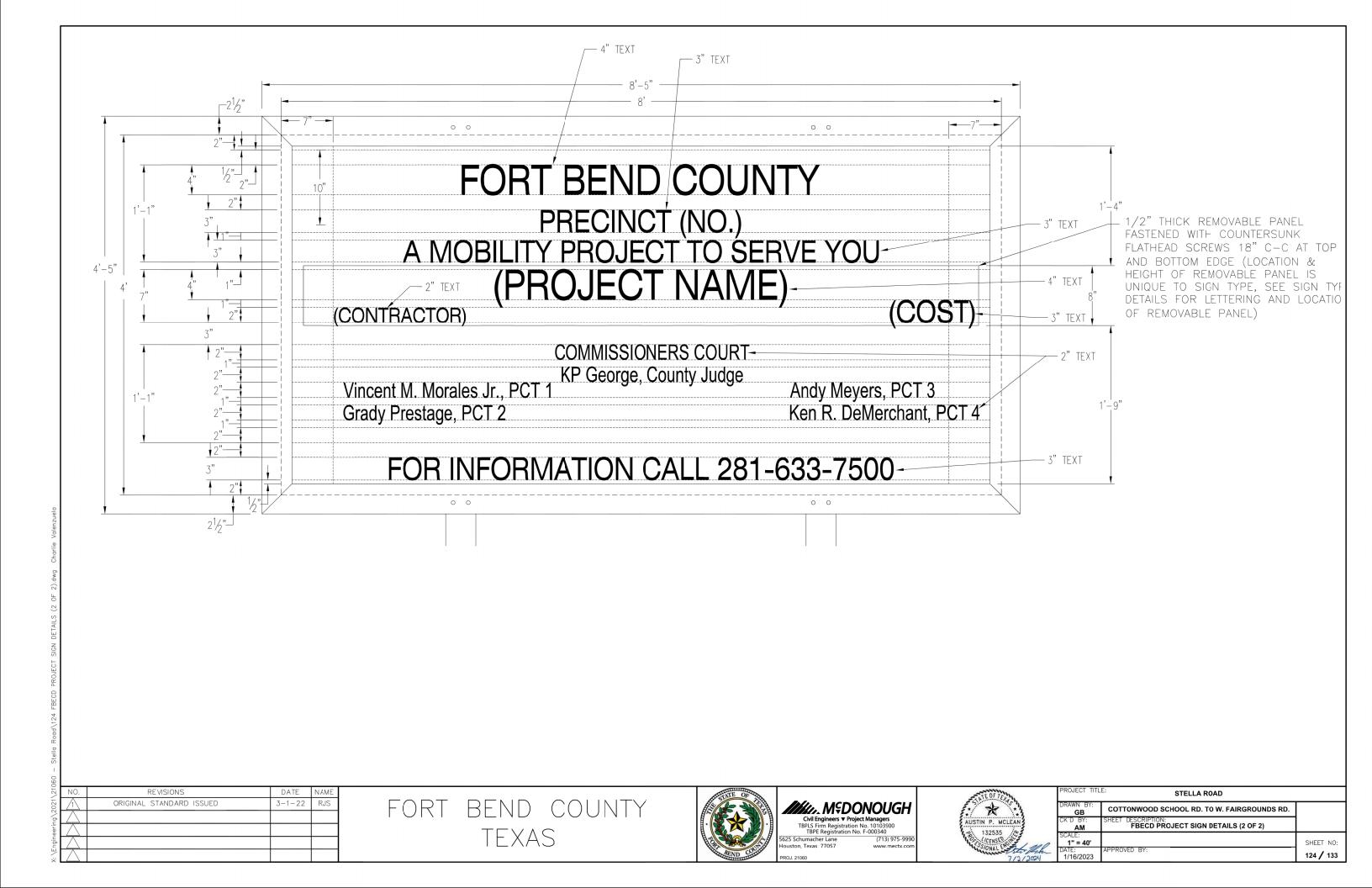


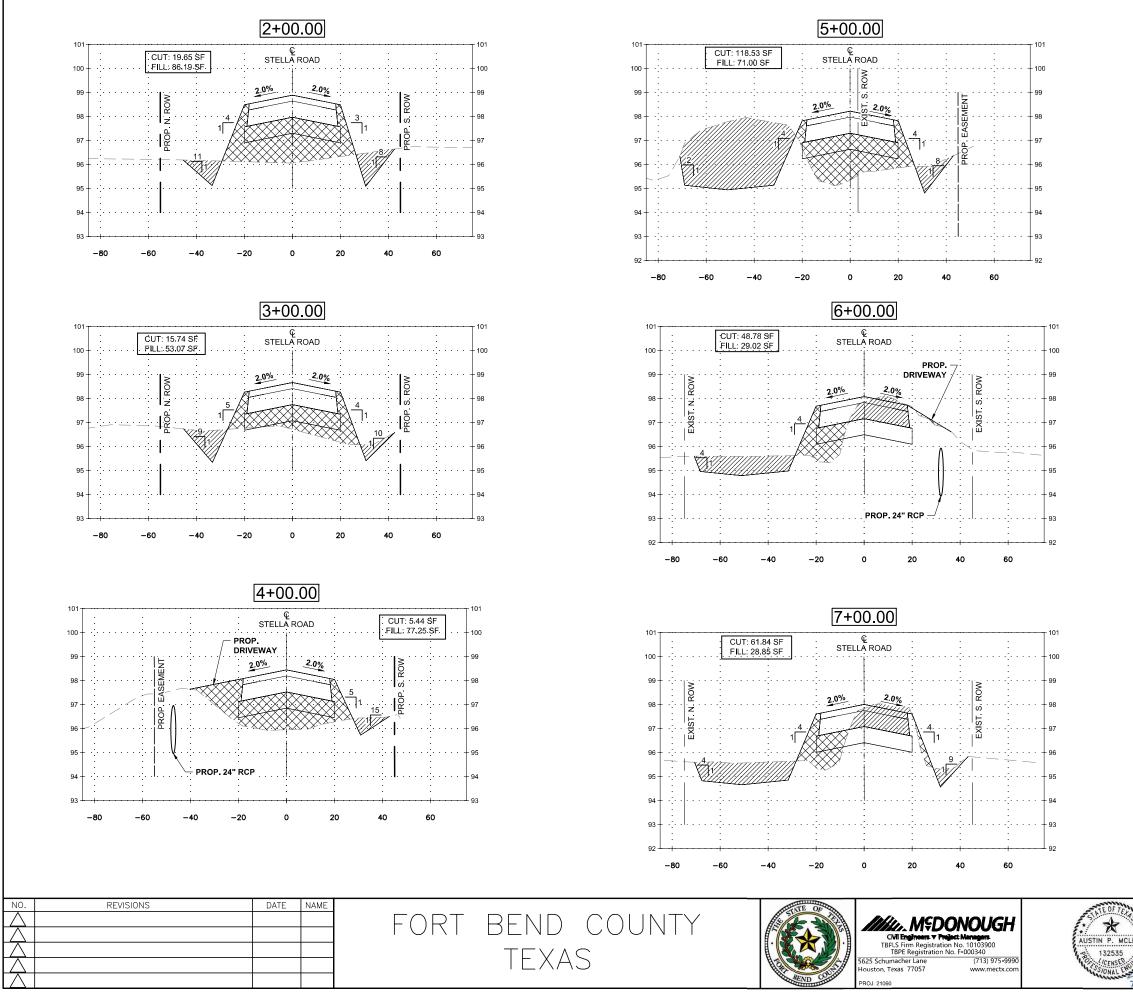
		DIMENSION SCHI	EDULE.
		'A'	9"
		'B'	2"
BE		'C'	6"
TYPICAL		'D'	3.5"
		Έ'	4.5"
	NOTES:		
	ALL STREET BL	ADES SHALL BE	9" IN HEIGHT

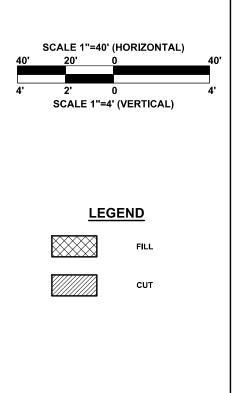




4,	PROJECT TITL	PROJECT TITLE: STELLA ROAD		
(***)	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.		
EAN	CK'D BY: AM	SHEET DESCRIPTION: FBCED PROJECT SIGN DETAILS (1 OF 2)		
	SCALE: 1" = 40'		SHEET NO:	
12/2024	DATE: 1/16/2023	APPROVED BY:	123 / 133	







NOTES:

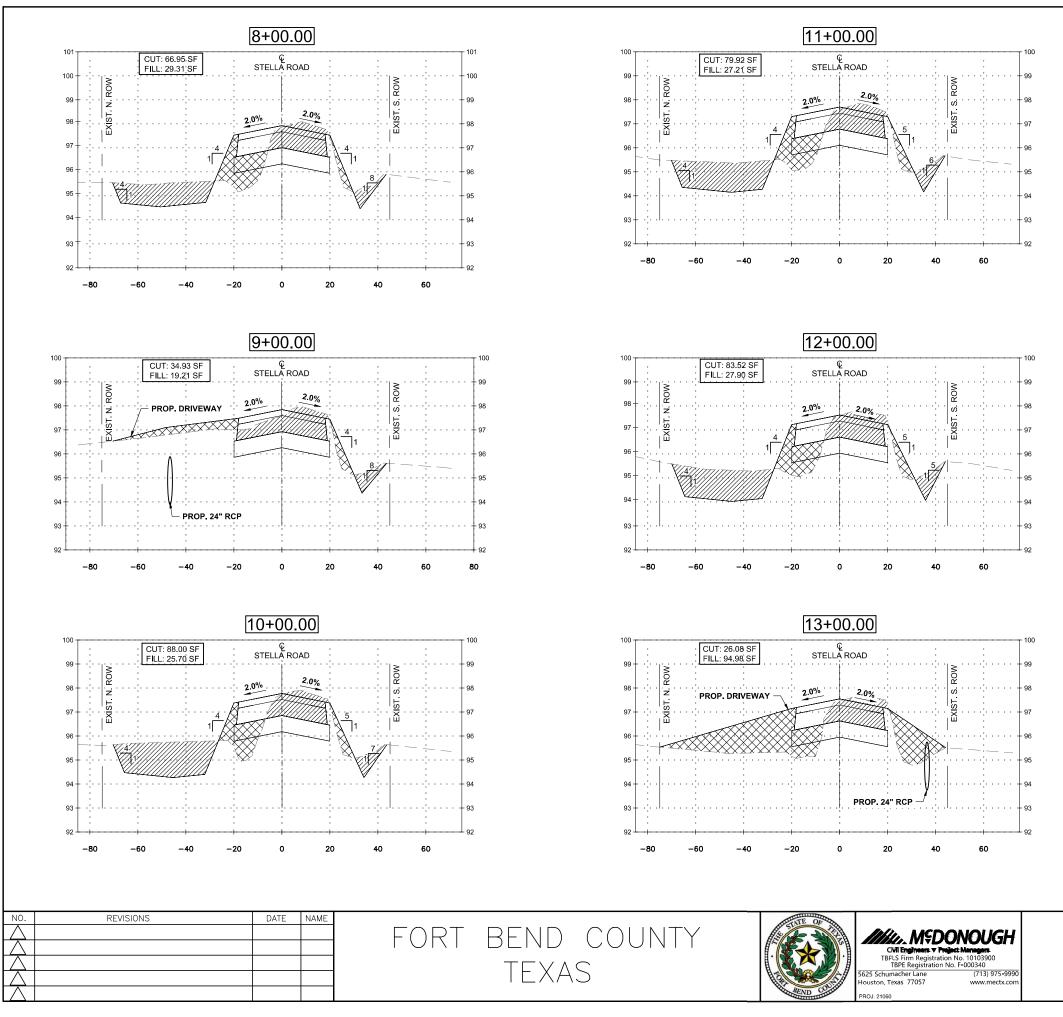
- 1) ALL EXISTING UTILITIES ARE SHOWN APPROXIMATELY IN BOTH VERTICAL AND HORIZONTAL LOCATION. CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL UTILITIES PRIOR TO CONSTRUCTION.
- 2) REGRADE FILLED DITCHES AND DEMOLISHED ROAD AREA TOWARDS STELLA ROAD PROP ROADSIDE SWALES.

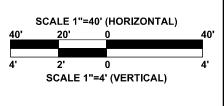
<i>۱</i> ۰۰,	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
1 10	SCALE: 1" = 20'	2+00 TO 7+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	125 / 133

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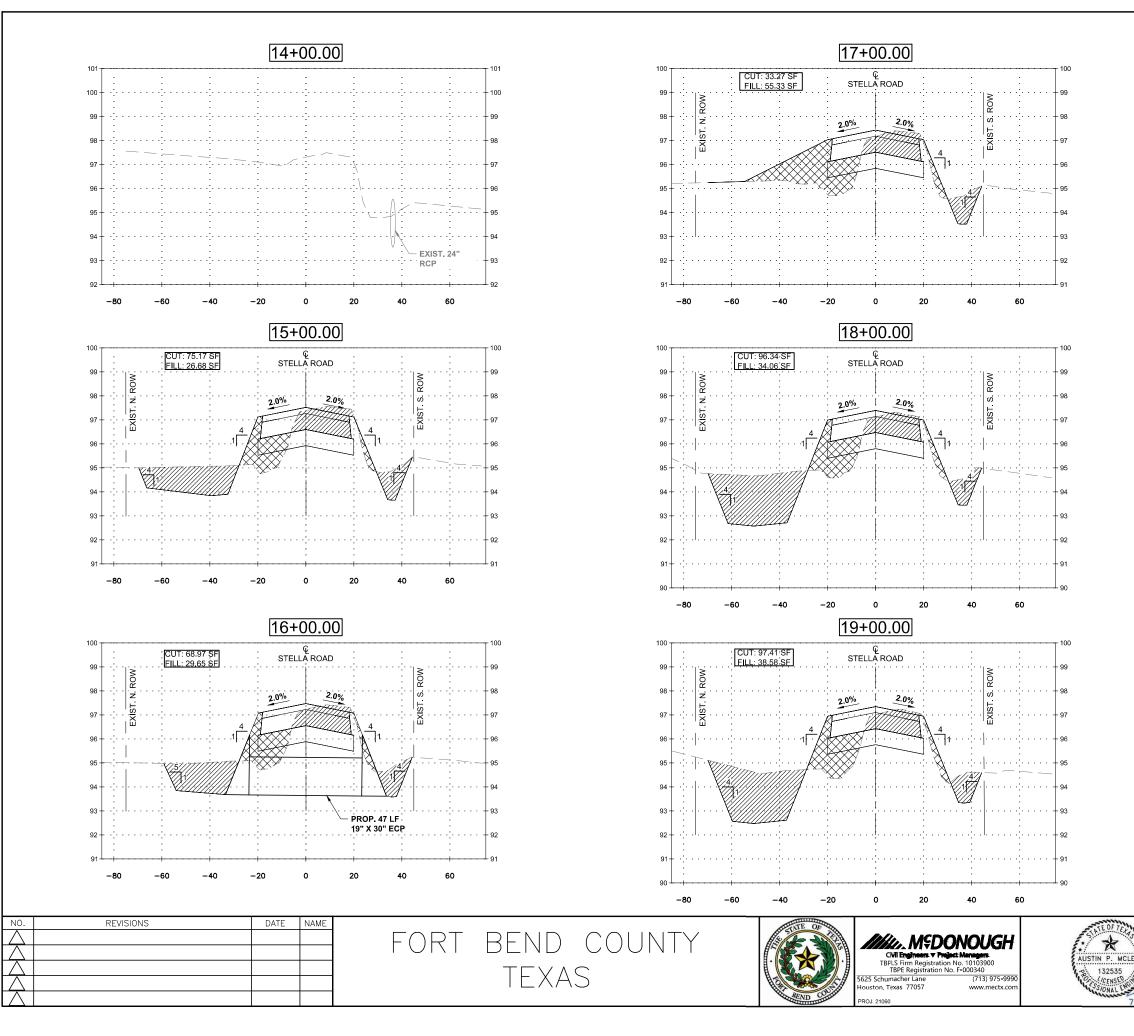


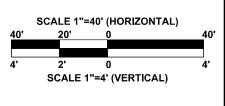
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TE OF TEX	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
AUSTIN P. MCLEAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
132535 (CENSED CH	SCALE: 1" = 40'	8+00 TO 13+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	126 / 133

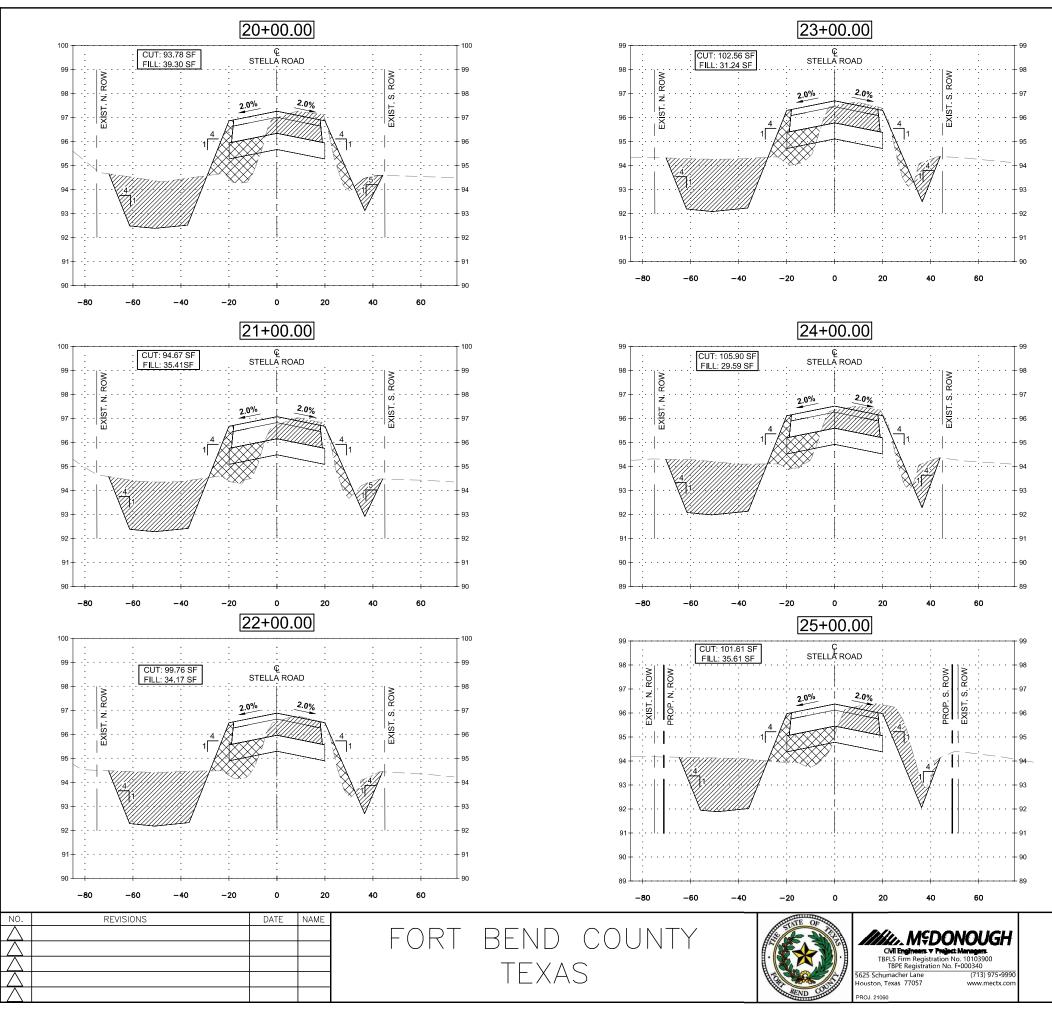




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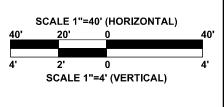


N .	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
Sal MA	SCALE: 1" = 40'	14+00 TO 19+00	SHEET NO:
1/2/2024	DATE: 1/16/2023	APPROVED BY:	127 / 133



Engineering\2021\21060 - Stella Road\128 CROSS SECTIONS 20+00 TO 25+00.dwg Charlie

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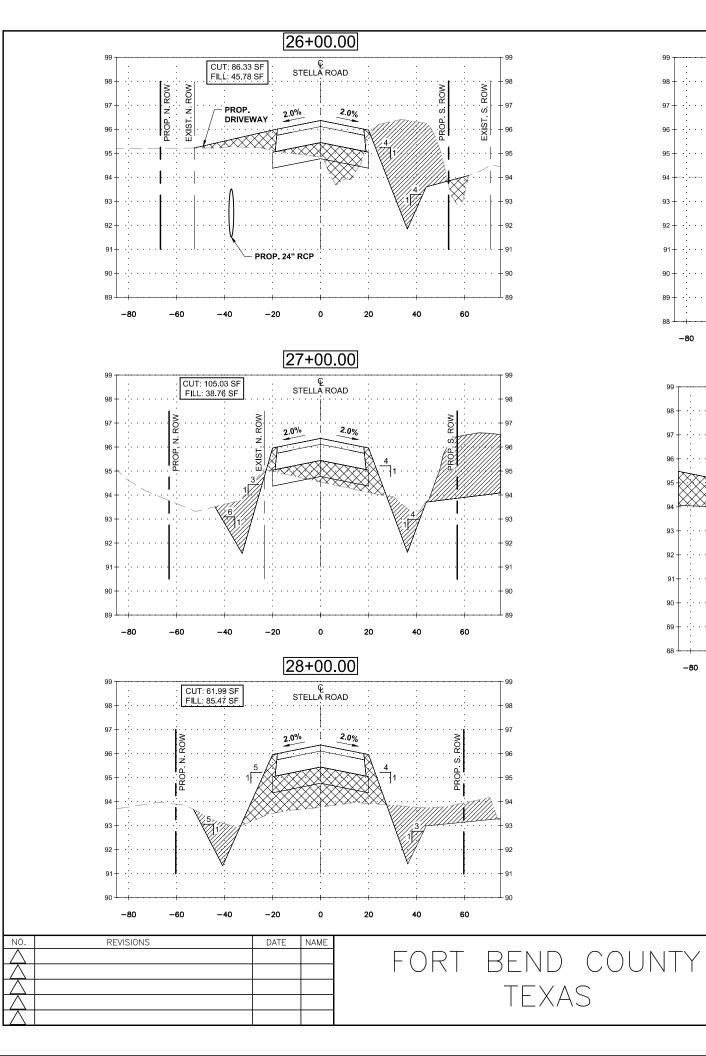


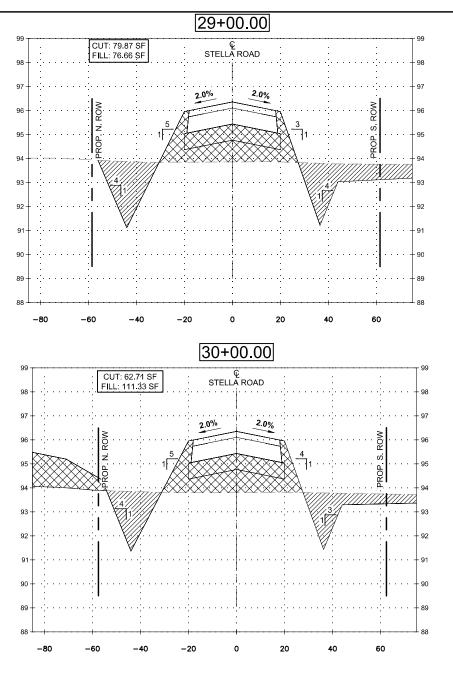
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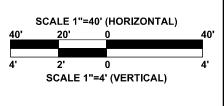
N N N N N N N N N N	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
1 10	SCALE: 1" = 40'	20+00 TO 25+00	SHEET NO:
1/2/2024	DATE: 1/16/2023	APPROVED BY:	128 / 133







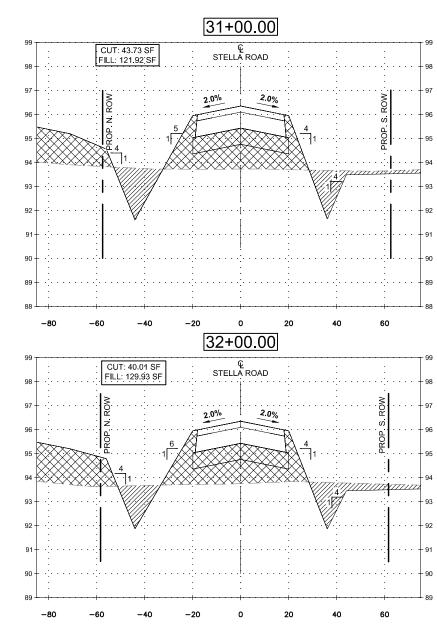


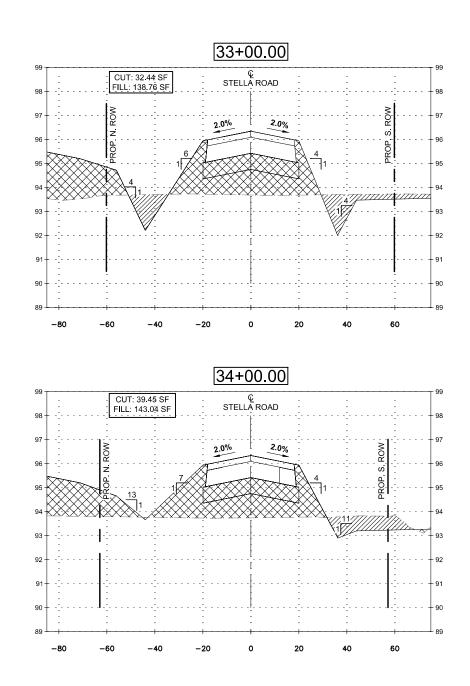


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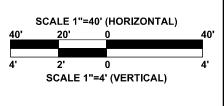


b).	PROJECT TITL	E: STELLA ROAD	
***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
1 10	SCALE: 1" = 40'	26+00 TO 31+00	SHEET NO:
1/2/2024	DATE: 1/16/2023	APPROVED BY:	129 / 133





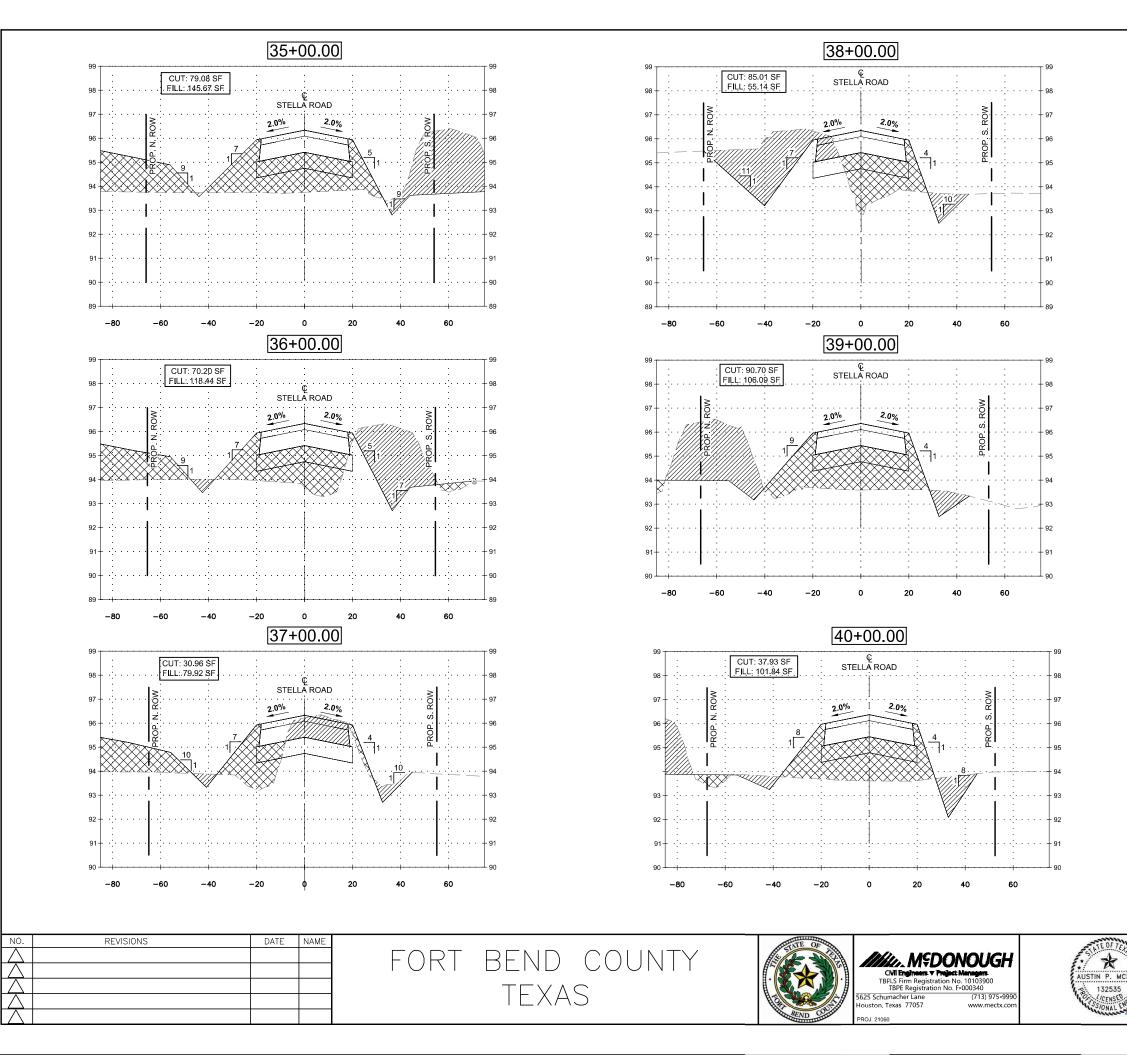


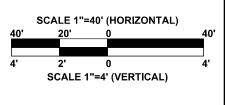


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·v.	PROJECT TITL	E: STELLA ROAD	
***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
	SCALE: 1" = 40'	32+00 TO 35+00	SHEET NO:
1/2/2024	DATE: 1/16/2023	APPROVED BY:	130 / 133



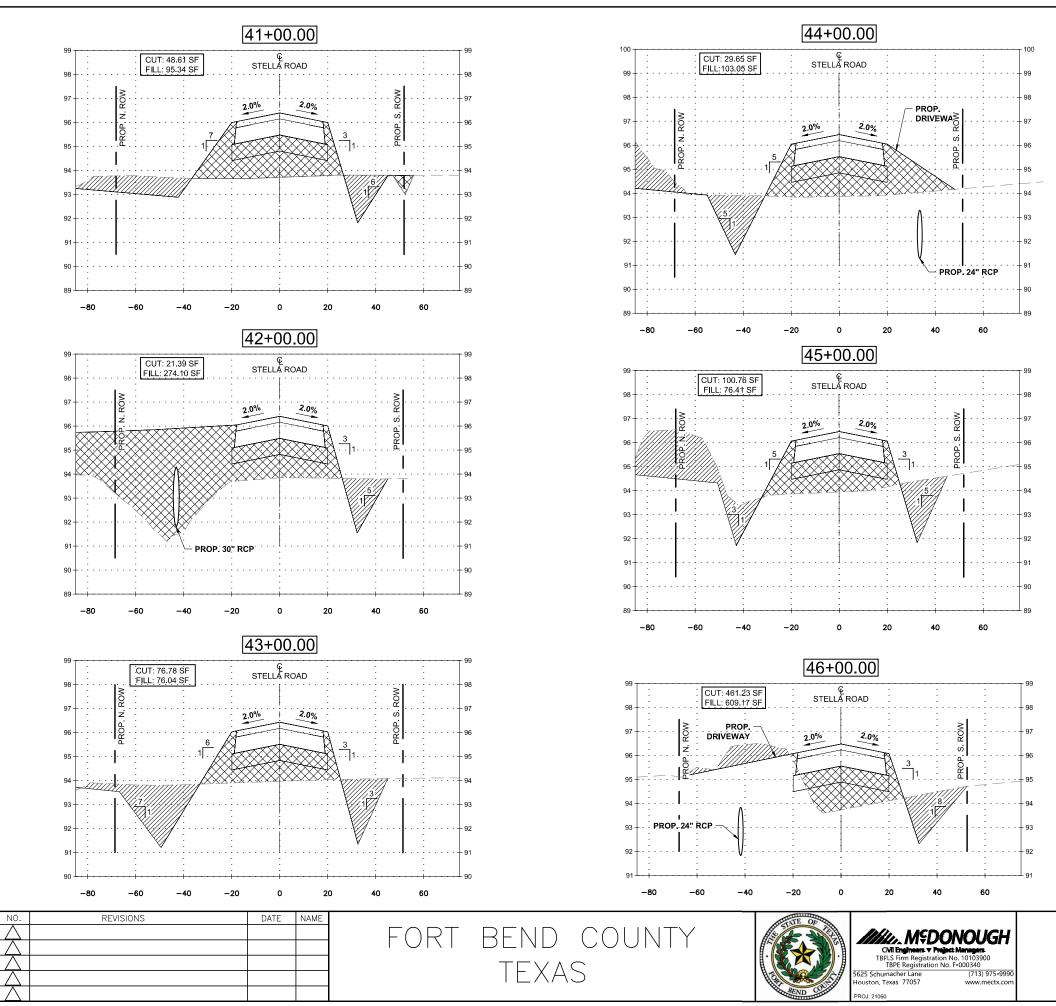


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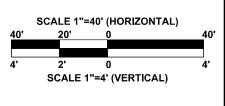
<i>۱</i> ۱.	PROJECT TITL	E: STELLA ROAD	
×***	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
	SCALE: 1" = 40'	36+00 TO 41+00	SHEET NO:
7/2/2024	DATE: 1/16/2023	APPROVED BY:	131 / 133



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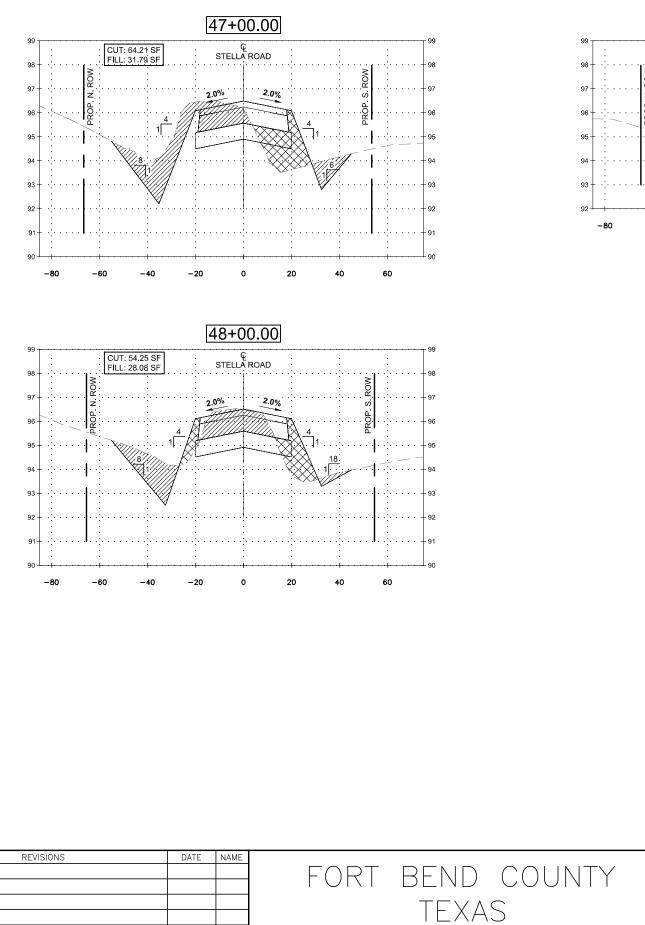


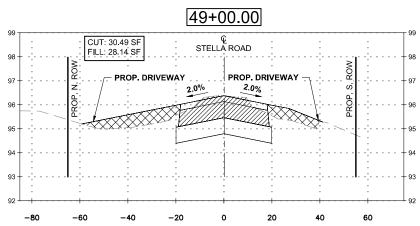
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۹.	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
1 10	SCALE: 1" = 40'	42+00 TO 47+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	132 / 133







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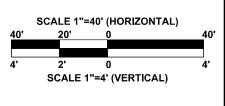
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Chil Engineers + Project Managers TBPLS Firm Registration No. 10103900 TBPE Registration No. F-000340 5 Schumacher Lane (713) 975-9990

5625 Schumacher Lane

iston, Texas 77057 01 21060





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١.	PROJECT TITL	E: STELLA ROAD	
	DRAWN BY: GB	COTTONWOOD SCHOOL RD. TO W. FAIRGROUNDS RD.	
EAN	CK'D BY: AM	SHEET DESCRIPTION: CROSS SECTIONS	
	SCALE: 1" = 40'	47+00 TO 49+00	SHEET NO:
12/2024	DATE: 1/16/2023	APPROVED BY:	133 / 133